

Illinois Power Generating Company 1500 Eastport Plaza Dr. Collinsville, IL 62234

October 25, 2021

Illinois Environmental Protection Agency
DWPC – Permits MC #15
Attn: Part 845 Coal Combustion Residual Rule Submittal
1021 North Grand Avenue East
P.O. Box 19276
Springfield, IL 62794-9276

Re: Coffeen Power Plant GMF Gypsum Stack Pond; IEPA ID # W1350150004-03

Dear Mr. LeCrone:

In accordance with 35 I.A.C. § 845.200, Illinois Power Generating Company (IPGC) is submitting an operating permit application for the Coffeen Power Plant GMF Gypsum Stack Pond (IEPA ID # W1350150004-03). One hardcopy and one digital copy are provided with this submittal.

The permit application was prepared in accordance with 35 I.A.C. § 845.230(d)(2) (Existing, Inactive and Inactive Closed CCR Surface Impoundment that have not completed an Agency approved closure before July 30, 2021). This submittal includes the completed permit forms as required by § 845.210.

Sincerely,

Cynthia Vodopivec

Cynthin E Way

SVP-Environmental Health and Safety

Enclosures

6555 SIERRA DRIVE IRVING, TEXAS 75039 **o** 214-812-4600 VISTRAENERGY.COM

Prepared for

Illinois Power Generating Company

1500 Eastport Plaza Drive Collinsville, Illinois 62234

INITIAL OPERATING PERMIT COFFEEN GMF GYPSUM STACK POND

Prepared by



425 South Woods Mill Road, Suite 300 St. Louis, MO 63017

October 25, 2021

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ii October 2021



1. INTRODUCTION

Illinois Power Generating Company operates the coal-fired Coffeen Power Plant located in Montgomery County, Illinois. The IEPA assigned identification number assigned to this impoundment is: W1350150004-03 for the GMF Gypsum Stack Pond. The National Inventory of Dams (NID) number assigned for the GMF Gypsum Stack Pond by the Illinois Department of Natural Resources (IDNR) is IL50579.

This initial operating permit application was developed in accordance with 35 Ill. Admin. Code 845, Standards for the Disposal of Coal Combustion Residuals in Surface Impoundments (Part 845).

This initial operating permit application is for the GMF Gypsum Stack Pond.

1.1. <u>Facility Information</u>

<u>Section 845.210(b)(1):</u> All permit applications must contain the name, address, email address and telephone number of the operator, or duly authorized agent, and the property owner to whom all inquiries and correspondence shall be addressed.

Facility: Coffeen GMF Gypsum Stack Pond

Coffeen Power Plant

134 Cips Lane Coffeen, IL 62017

Owner/Operator: Illinois Power Generating Company

1500 Eastport Plaza Drive Collinsville, Illinois 62234



1.2. Owner Signatures

<u>Section 845.210(b)(2):</u> All permit applications must be signed by the owner, operator or a duly authorized agent of the operator.

The owner of the Coffeen Power Plant is a corporation.

<u>Section 845.210(b)(3):</u> An application submitted by a corporation must be signed by a principal executive officer of at least the level of vice president, or his or her duly authorized representative, if that representative is responsible for the overall operation of the facility described in the application form..

The signature of Cynthia Vodopivec on behalf of Illinois Power Generating Company can be found in the permit applications located in Section 3.

1.3. Legal Description

<u>Section 845.210(c):</u> All permit applications must contain a legal description of the facility boundary and a description of the boundaries of all units included in the facility.

A legal description has been developed in compliance with Section 845.210(c) and is included in Attachment A.

1.4. Previous Assessments

<u>Section 845.210(d):</u> Previous Assessments, Investigations Plans, and Programs

Previous assessments were performed in accordance with 40 CFR § 257 and are referenced within the permit application and included in the appropriate Attachments.

<u>Section 845.210(d)(1):</u> The Agency may approve the use of any hydrogeologic site investigation or characterization, groundwater monitoring well or system, or groundwater monitoring plan, bearing the seal and signature of an Illinois Licensed Professional Geologist or Licensed Professional Engineer, completed before April 21, 2021 to satisfy the requirements of this Part.

A previous hydrogeologic site investigation or characterization, groundwater monitoring well or system, or groundwater monitoring plan have been completed with a seal from an Illinois Licensed Professional Geologist or Licensed Professional Engineer. However, field investigations have been completed that supplement that work that will be utilized in the following sections of this report.



<u>Section 845.210(d)(2):</u> For existing CCR surface impoundments, the owner or operator of the CCR surface impoundment may use a previously completed location restriction demonstration required by Section 845.300 (Placement Above the Uppermost Aquifer), Section 845.310 (Wetlands), Section 845.320 (Fault Areas), Section 845.330 (Seismic Impact Zones), and Section 845.340 (Unstable Areas) provided that the previously completed assessments meet the applicable requirements of those Sections.

Previous assessments are provided for Section 845.300 (Placement Above the Uppermost Aquifer), Section 845.310 (Wetlands), Section 845.320 (Fault Areas), Section 845.330 (Seismic Impact Zones), and Section 845.340 (Unstable Areas) in Attachment D.

<u>Section 845.210(d)(3):</u> For existing CCR surface impoundments, the owner or operator of the CCR surface impoundment may use a previously completed assessment to serve as the initial assessment required by Section 845.440 (Hazard Potential Classification Assessment), Section 845.450 (Structural Stability Assessment) and Section 845.460 (Safety Factor Assessment) provided that the previously completed assessment: A) Was not completed more than five years ago; and B) Meets the applicable requirements of those Sections.

Previous assessments are provided for Section 845.440 (Hazard Potential Classification Assessment), Section 845.450 (Structural Stability Assessment) and Section 845.460 (Safety Factor Assessment) in Attachments O, P, and Q respectively. The addendum and certification for the Hazard Potential Classification Assessment, Structural Stability Assessment and Safety Factor Assessment are located in Attachment U.

<u>Section 845.210(d)(4):</u> For inactive closed CCR surface impoundments, the owner or operator of the CCR surface impoundment may use a post-closure care plan previously approved by the Agency.

No post-closure care plan was previously approved by the Agency.



2. OPERATING PERMIT

2.1. Initial Operating Permit

<u>Section 845.230(d):</u> Initial Operating Permit for Existing, Inactive and Inactive Closed CCR Surface Impoundments

The Coffeen GMF Gypsum Stack Pond as defined by IEPA is an existing CCR surface impoundment that has not completed post-closure care. Per Part 845, Dynegy is submitting an initial operating permit application to IEPA by October 31, 2021. The following sections contain information or references to documents required for the Operating Permit application (Section 845.230).

2.2. History of Construction

Section 845.230(d)(2)(A): The history of construction specified in Section 845.220(a)(1);

The history of construction prepared in 2016 pursuant to 40 CFR § 257.73(c) is provided in Attachment B. An amendment to the history of construction has been prepared in compliance with Section 845.220(a)(1) and is provided in Attachment U.

2.3. Chemical Constituents

<u>Section 845.230(d)(2)(B):</u> An analysis of the chemical constituents found within the CCR to be placed in the CCR surface impoundment;

An analysis of the chemical constituents found within the CCR placed within the Coffeen GMF Gypsum Stack Pond is provided in Attachment C.

<u>Section 845.230(d)(2)(C):</u> An analysis of the chemical constituents of all waste streams, chemical additives and sorbent materials entering or contained in the CCR surface impoundment;

An analysis of the chemical constituents of all waste streams, chemical additives and sorbent materials entering or contained within the Coffeen GMF Gypsum Stack Pond is provided in Attachment C.

2.4. Location Standards Demonstration

<u>Section 845.230(d)(2)(D):</u> A demonstration that the CCR surface impoundment, as built, meets, or an explanation of how the CCR surface impoundments fails to meet, the location standards in the following Sections:



The Coffeen GMF Gypsum Stack Pond location standards as specified in Section 845.230(d)(2)(D) are described in the following sections.

<u>Section 845.230(d)(2)(D)(i):</u> Placement Above the Uppermost Aquifer;

The previous upper aquifer demonstration was certified by a qualified professional engineer stating that the demonstration meets the requirements of 40 C.F.R. § 257.60. The requirements described in 40 C.F.R. § 257.60 are identical to the requirements contained in Section 845.300. Pursuant to Section 845.210(d)(2), a certification is not required for this demonstration. The previously completed upper aquifer demonstration is included in Attachment D.

Section 845.230(d)(2)(D)(ii): Wetlands;

The previous wetlands demonstration was certified by a qualified professional engineer stating that the demonstration meets the requirements of 40 C.F.R. § 257.61. The requirements described in 40 C.F.R. § 257.61 are identical to the requirements contained in Section 845.310. Pursuant to Section 845.210(d)(2), a certification is not required for this demonstration. The previously completed wetlands demonstration is included in Attachment D.

Section 845.230(d)(2)(D)(iii): Fault Areas;

The previous fault area demonstration was certified by a qualified professional engineer stating that the demonstration meets the requirements of 40 C.F.R. § 257.62. The requirements described in 40 C.F.R. § 257.62 are identical to the requirements contained in Section 845.320. Pursuant to Section 845.210(d)(2), a certification is not required for this demonstration. The previously completed fault area demonstration is included in Attachment D.

Section 845.230(d)(2)(D)(iv): Seismic Impact Zone; and

The previous seismic impact zone demonstration was certified by a qualified professional engineer stating that the demonstration meets the requirements of 40 C.F.R. § 257.63. The requirements described in 40 C.F.R. § 257.63 are identical to the requirements contained in Section 845.330. Pursuant to Section 845.210(d)(2), a certification is not required for this demonstration. The previously completed seismic impact zone demonstration is included in Attachment D.

<u>Section 845.230(d)(2)(D)(v):</u> Unstable Areas and Floodplains;

The previous unstable area demonstration was certified by a qualified professional engineer stating that the demonstration meets the requirements of 40 C.F.R. § 257.64. The requirements described in 40 C.F.R. § 257.64 are identical to the requirements contained in Section 845.340. Pursuant to



Section 845.210(d)(2), a certification is not required for the unstable area demonstration. The previously completed unstable area demonstration is included in Attachment D.

The boundaries of the impoundment were determined by a survey conducted by a professional surveyor licensed in the State of Illinois. The boundaries of the GMF Gypsum Stack Pond were compared to the existing FEMA floodplain map, and it was determined that the GMF Gypsum Stack Pond is not located within the floodplain. A certification attesting to this is provided in Attachment D.

2.5. Permanent Markers

<u>Section 845.230(d)(2)(E):</u> Evidence of permanent markers required by Section 845.130 have been installed:

Evidence of permanent markers at the Coffeen GMF Gypsum Stack Pond as required by Section 845.130 is provided in Attachment E.

2.6. Slope Maintenance

<u>Section 845.230(d)(2)(F):</u> Documentation that the CCR surface impoundment, if not incised, will be operated and maintained with one of the forms of slope protection specified in Section 845.430;

The Coffeen GMF Gypsum Stack Pond is not incised. Documentation of slope protection as required by Section 845.430 is provided in Attachment J.

2.7. Initial Emergency Action Plan

<u>Section 845.230(d)(2)(G):</u> Initial Emergency Action Plan and accompanying certification (see Section 845.520(e));

The initial emergency action plan and certification has been completed as specified by Section 845.520(e) and is provided in Attachment F.

2.8. Fugitive Dust Control Plan

<u>Section 845.230(d)(2)(H):</u> Fugitive dust control plan and accompanying certification (see Section 845.500(b)(7));

The fugitive dust control plan and certification has been completed as specified by Section 845.500(b)(7) and is provided in Attachment G.



2.9. Groundwater Monitoring

Section 845.230(d)(2)(I): Groundwater monitoring information:

The groundwater monitoring information for the Coffeen GMF Gypsum Stack Pond is described in the following sections.

<u>Section 845.230(d)(2)(I)(i):</u> Hydrogeologic site characterization (see Section 845.620);

Hydrogeologic site characterization for the Coffeen GMF Gypsum Stack Pond is provided in Attachment H.

<u>Section 845.230(d)(2)(I)(ii):</u> Design and construction plans of a groundwater monitoring system (see Section 845.630);

Design and construction plans of a groundwater monitoring system are provided in Attachment I.

<u>Section 845.230(d)(2)(I)(iii):</u> A groundwater sampling and analysis program that includes selection of the statistical procedures to be used for evaluating groundwater monitoring data (see Section 845.640); and

A groundwater sampling and analysis program that meets the requirements of Section 845.640 is provided in Attachment I.

<u>Section 845.230(d)(2)(I)(iv):</u> Proposed groundwater monitoring program that includes a minimum of eight independent samples for each background and downgradient well (see Section 845.650(b));

A proposed groundwater monitoring program that meets the requirements of Section 845.650(b) is provided in Attachment I.

2.10. Initial Post-Closure Care Plan

<u>Section 845.230(d)(2)(K):</u> Initial written post-closure care plan, if applicable (see Section 845.780(d));

The Coffeen GMF Gypsum Stack Pond closure will be completed by capping the CCR in place. The initial post closure care plan was developed in accordance with Section 845.780 and is provided in Attachment K.



2.11. Liner Certification

<u>Section 845.230(d)(2)(L):</u> The certification required by Section 845.400(h), or a statement that the CCR surface impoundment does not have a liner that meets the requirements of Section 845.400(b) or (c);

A liner certification has been completed for the Coffeen GMF Gypsum Stack Pond as specified by Section 845.400(h) that states the surface impoundment does have a composite liner that meets the requirements of Section 845.400(b) is provided in Attachment L.

2.12. History of Groundwater Exceedances

<u>Section 845.230(d)(2)(M):</u> History of known exceedances of the groundwater protection standards in Section 845.600, and any corrective action taken to remediate the groundwater;

A history of known exceedances and any corrective action taken is provided in Attachment M.

2.13. Financial Assurance Requirements

<u>Section 845.230(d)(2)(N):</u> A certification that the owner or operator meets the financial assurance requirements of Subpart I;

A certification meeting the requirement of Section 845.230(d)(2)(N) stating that the Owner meets the financial assurance requirements of *Subpart I* is provided in Attachment N.

2.14. Hazard Potential Classification

<u>Section 845.230(d)(2)(O):</u> Hazard potential classification assessment and accompanying certification (see Section 845.440(a)(2));

The previous Hazard Potential Classification Assessment completed in compliance with 40 CFR §257.73(a) is provided in Attachment O. The addendum to the Hazard Potential Classification Assessment and certification as required by Section 845.440(a) is provided in Attachment U.

2.15. Structural Stability Assessment

<u>Section 845.230(d)(2)(P):</u> Structural stability assessment and accompanying certification (see Section 845.450(c));



The previous Structural Stability Assessment completed in compliance with 40 CFR §257.73(d) is provided in Attachment P. The addendum to the Structural Stability Assessment and certification as required by Section 845.450(c) is provided in Attachment U.

2.16. Safety Factor Assessment

<u>Section 845.230(d)(2)(Q):</u> Safety factor assessment and accompanying certification (see Section 845.460(b));

The previous Safety Factor Assessment completed in compliance with 40 CFR §257.73(e) is provided in Attachment Q. The addendum to the Safety Factor Assessment and certification as required by Section 845.460(b) is provided in Attachment U.

2.17. Inflow Design Flood Control System Plan

<u>Section 845.230(d)(2)(R):</u> Inflow design flood control system plan and accompanying certification (see Section 845.510(c)(3));

The previous Inflow Design Flood Control System Plan Assessment completed in compliance with 40 CFR §257.82 is provided in Attachment R. The addendum to the Inflow Design Flood Control Plan Assessmentas required by Section 845.510(c)(3) is provided in Attachment U.

2.18. Safety and Health Plan

Section 845.230(d)(2)(S): Safety and health plan (see Section 845.530); and

The safety and health plan in accordance with Section 845.530 is included in Attachment S.

2.19. Proposed Closure Priority Categorization

<u>Section 845.230(d)(2)(T):</u> For CCR surface impoundments required to close under 845.700, the proposed closure priority categorization required by Section 845.700(g).

A CCR Surface Impoundment Category Designation and Justification letter was submitted to IEPA on May 19, 2021. The Coffeen GMF Gypsum Stack Pond was designated as Category 5 Existing CCR surface impoundment with exceedances of the groundwater protection standards in Section 845.600. This letter is provided in Attachment T.



3. PERMIT APPLICATION

All permit applications must be made on the forms prescribed by the Agency and must be mailed or delivered to the address designated by the Agency on the forms. The permit applications (CCR-1 and CCR-2E) are provided below.

Form CCR 1



Illinois Environmental Protection Agency CCR Surface Impoundment Permit Application Form CCR 1 – General Provisions

Bu	reau of	Water ID Number:	For IE	PA Use Only	
CC	R Perm	it Number:			
- -	ailitu Na				
ra	cility Na	me:			
S	ECTION	1: FACILITY, OPERATOR, AND C	WNER INFORMATION (35 III. Adn	n. Code 845.210(b))	
	1.1	Facility Name			
	1.2	Illinois EPA CCR Permit Number (if ap	oplicable)		
	1.3	Facility Contact Information			
ation		Name (first and last)	Title	Phone Number	
Facility, Operator, and Owner Information		Email address			
wner	1.4	Facility Mailing Address			
, and 0		Street or P.O. box			
perator		City or town	State	Zip Code	
ty, o	1.5	Facility Location			
Facili		Street, route number, or other specific	dentifier		
		County name	County code (if known)		
		City or town	State	Zip Code	
	1.6	Name of Owner/Operator			

ıfο	1.7	Owner/Operator Contact Information				
)wner li		Name (first and last)	Title		Phone Number	
r, and C		Email address				
erato	1.8	Owner/Operator Mailing Ad	ddress			
Facility, Operator, and Owner Info		Street or P.O. box				
Faci		City or town		State	Zip Code	
		SECTION 2: LEGA	L DESCRIPTION (35	III. Adm. Code	845.210(c))	
tion	2.1	Legal Description of the fac	ility boundary			
Legal Description						
SE	CTION 3	B: PUBLICLY ACCESSIBI	LE INTERNET SITE I	REQUIREMENTS	6 (35 III. Adm. Code 845.810)	
	3.1	Web Address(es) to publicly	y accessible internet sit	e(s) (CCR website)		
nternet Site						
_	3.2	Is/are the website(s) titled "	Illinois CCR Rule Comp	oliance Data and In	formation"	
		Yes	No			
		SECTION	4: IMPOUNDMENT	IDENTIFICATIO	N	
ation	4.1	List all the impoundment ide indicate that you have attack			neck the corresponding box to dment.	
Impoundment Identification				Attache	ed written description	
				Attache	ed written description	
				Attache	ed written description	
punc					ed written description	
lmpc					ed written description	
				Attache	ed written description	

		1				
			Attached wri	tten desc	ription	
			Attached wri	tten desc	ription	
			Attached wri	tten desc	ription	
			Attached wri	tten desc	ription	
		SECTION 5: CHECKLIST AND CERTIFICATION	STATEM	ENT		
	5.1	In Column 1 below, mark the sections of Form 1 that you have application. For each section, specify in Column 2 any attachr				n your
		Column 1			Column 2	
ınt		Section 1: Facility, Operator, and Owner Information		w/attacl	nments	
teme		Section 2: Legal Description		w/attacl	nments	
Sta		Section 3: Publicly Accessible Internet Site Requirement		w/attachments		
ation		Section 4: Impoundment Identification		w/attacl	nments	
Checklist and Certification Statement	5.2	Certification Statement				
		I certify under penalty of law that this document and all attachmor supervision in accordance with a system designed to assure and evaluate the information submitted. Based on my inquiry system, or those persons directly responsible for gathering the to the best of my knowledge and belief, true, accurate, and consignificant penalties for submitting false information, including for knowing violations.	that qualifient of the person information inplete. I am	ed person n or perso , the infor n aware th	nel properly ons who manamation submation are	gather age the itted is,
		Name (print or type first and last name) of Owner/Operator			Official Title	e
		Signature Cyrthin E Wdy			Date Signe	d

Form CCR 2E

Illinois Environmental Protection Agency



CCR Surface Impoundment Permit Application Form CCR 2E – Initial Operating Permit for Existing or Inactive CCR Surface Impoundments That Have Not Completed an Agency-approved Closure Before July 30, 2021

	Agonoy approvod Glocal	bololo daly do, zdz i
Bureau of Water ID Number:		For IEPA Use Only
CCR Perr	mit Number:	
Facility N	ame:	

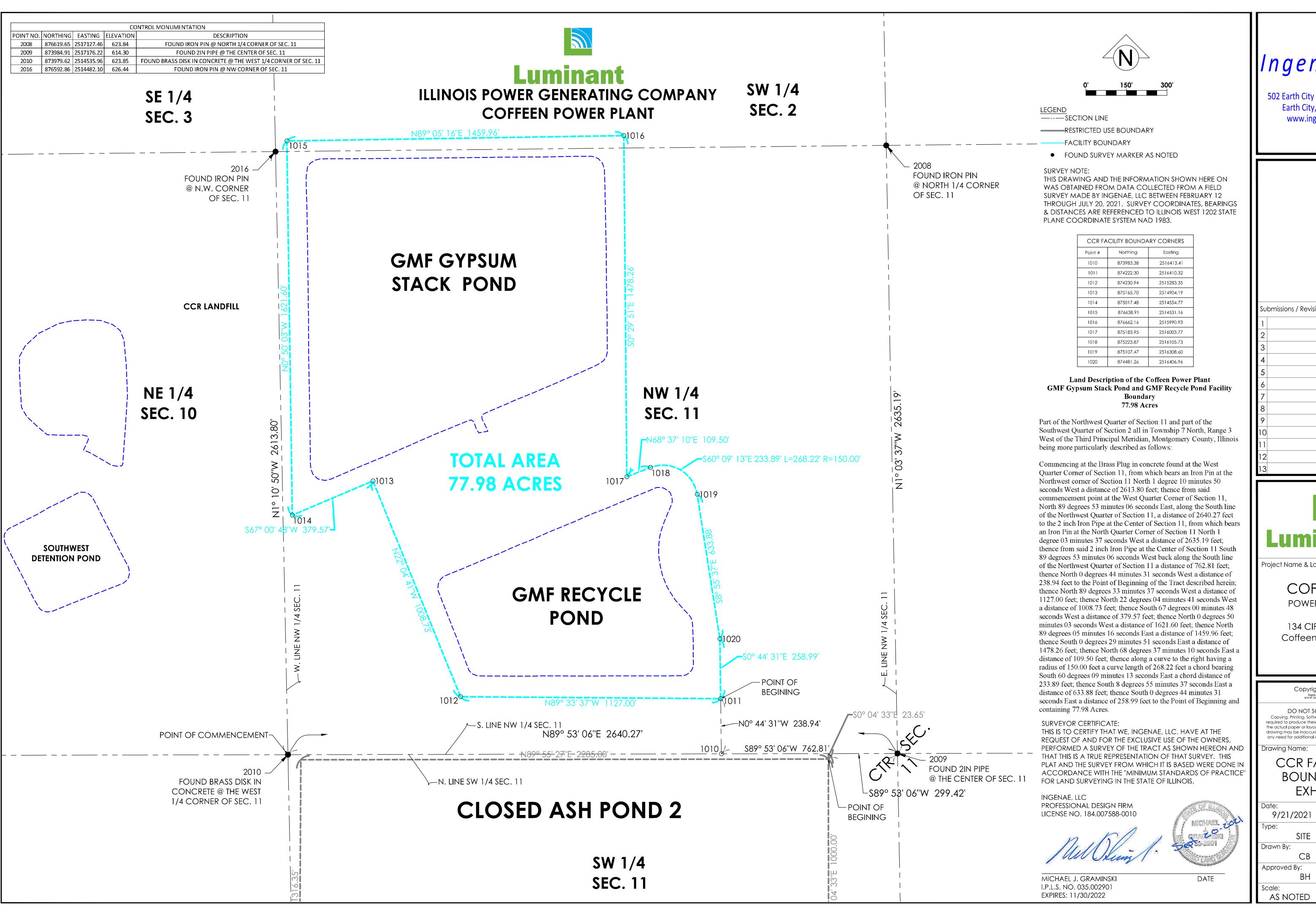
SEC	ΓΙΟΝ 1:	CONSTRUCTION HISTORY (35 III. Adm. Code 845.220 AND 35 III. Adm. Code 845.230)
	1.1	CCR surface impoundment name.
	1.2	Identification number of the CCR surface impoundment (if one has been assigned by the Agency).
istory	1.3	Description of the boundaries of the CCR surface impoundment (35 III. Adm. Code 845.210(c)).
Construction History	1.4	State the purpose for which the CCR surface impoundment is being used.
O	1.5	How long has the CCR surface impoundment been in operation?
	1.6	List the types of CCR that have been placed in the CCR surface impoundment.

	1.7 List name of the watershed within which the CCR surface impoundment is located.			
	1.8	Size in acres of the watershed within which the CCR surface impoundment is located.		
	1.9	Check the corresponding box to indicate that you have attached the following:		
		Description of the physical and engineering properties of the foundation and abutment materials on which the CCR surface impoundment is constructed.		
		Description of the type, size, range, and physical and engineering properties of the materials used in constructing each zone or stage of the CCR surface impoundment.		
(pen		Describe the method of site preparation and construction of each zone of the CCR surface impoundment.		
Construction History (Continued)		A listing of the approximate dates of construction of each successive stage of construction of the CCR surface impoundment.		
ory (Drawing satisfying the requirements of 35 III. Adm. Code 845.220(a)(1)(F).		
Hist		Description of the type, purpose, and location of existing instrumentation.		
tion		Area capacity curves for the CCR Impoundment.		
onstruc		Description of each spillway and diversion design features and capacities and provide the calculations used in their determination.		
ၓ		Construction specifications and provisions for surveillance, maintenance, and repair of the CCR surface impoundment.		
	1.10.1	Is there any record or knowledge of structural instability of the CCR surface impoundment?		
		Yes No		
	1.10.2	If you answered yes to Item 1.10.1, provide detailed explanation of the structural instability.		
	SECTIO	ON 2: ANALYSIS OF CHEMICAL CONSTITUENTS (35 III. Adm. Code 845.230(d)(2)(B))		
ts	2.1	Check the corresponding boxes to indicate you have attached the following:		
Constituents		An analysis of the chemical constituents found within the CCR to be placed in the CCR surface impoundment.		
Con		An analysis of the chemical constituents of all waste streams, chemical additives and sorbent materials entering or contained in the CCR surface impoundment.		

	SECTIO	ON 3: DEMONSTRATIONS AND CERTIFICATIONS	TIONS (35 III. Adm. Code 8	345.230(d)(2)(D))			
	3.1	Indicate whether you have attached a demonstrative meets, or an explanation of how the CCR surface the following sections:					
Demonstrations		35 III. Adm. Code 845.300 (Placement Above the Uppermost Aquifer)	Demonstration	Explanation			
stra		35 III. Adm. Code 845.310 (Wetlands)	Demonstration	Explanation			
mon		35 III. Adm. Code 845.320 (Fault Areas)	Demonstration	Explanation			
De		35 III. Adm. Code 845.330 (Seismic Impact Zones)	Demonstration	Explanation			
		35 III. Adm. Code 845.340 (Unstable Areas and Floodplains)	Demonstration	Explanation			
		SECTION 4: ATTA	CHMENTS				
	4.1	Check the corresponding boxes to indicate that y	ou have attached the following	g:			
		Evidence that the permanent markers re installed.	quired by 35 III. Adm. Code 84	5.130 have been			
		Documentation that the CCR surface imperintained with one of the forms of slope					
		Initial Emergency Action Plan and accon 845.520(e).	panying certification required	by 35 III. Adm. Code			
ents		Fugitive dust control plan and accompan 845.500(b)(7).	ying certification required by 3	5 III. Adm. Code			
hm		Preliminary written closure plan as specified in 35 III. Adm. Code 845.720(a).					
Attachments		Initial written post-closure care plan as s	pecified in 35 III. Adm. Code 84	45.780(d), if applicable.			
,		A certification as specified in 35 III. Adm. impoundment does not have a liner than 845.400(b) or (c).					
		History of known exceedances of the gro 845.600, and any corrective action taken					
		Safety and health plan, as required by 35	5 III. Adm. Code 845.530.				
		For CCR surface impoundments require proposed closure priority categorization					
		SECTION 5: GROUNDWAT	ER MONITORING				
Groundwater	5.1	Check the corresponding boxes to indicate you hinformation:	ave attached the following gro	undwater monitoring			
nnd		A hydrogeologic site characterization me	eting the requirements of 35 II	I. Adm. Code 845.620.			
Gro		Design and construction plans of a groun of 35 III. Adm. Code 845.630.	ndwater monitoring system me	eting the requirements			

		A groundwater sampling and analysis program that includes section of the statistical procedures to be used for evaluating groundwater monitoring data, required by 35 III. Adm. Code 845.640.
		Proposed groundwater monitoring program that includes a minimum of eight independent samples for each background and downgradient well, required by 35 III. Adm. Code 845.650(b).
		SECTION 6: CERTIFICATIONS
	6.1	Check the corresponding boxes to indicate you have attached the following certifications:
v		A certification that the owner or operator meets the financial assurance requirements of Subpart I, as required by 35 III. Adm. Code 845.230(d)(2)(N).
Certifications		Hazard potential classification assessment and accompanying certifications required by 35 III. Adm. Code 845.440(a)(2).
Certifi		Structural stability assessment and accompanying certification, required by 35 III. Adm. Code 845.450(c).
		Safety factor assessment and accompanying certification, as required by 35 III. Adm. Code 845.460(b).
		Inflow design flood control system plan and accompanying certification, as required by 35 III. Adm. Code 845.510(c)(3).

ATTACHMENT A





502 Earth City Plaza, Suite 120 Earth City, MO 63045 www.ingenae.com

Submissions / Revisions	: Date:
1	
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Project Name & Location:

COFFEEN POWER PLANT

134 CIPS Lane Coffeen, IL 62017

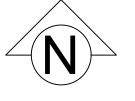
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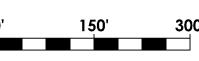
auired to produce these prints can stretch or shrink the actual paper or layout. Therefore, scaling of this drawing may be inaccurate. Contact IngenAE with

CCR FACILITY BOUNDARY **EXHIBIT**

Date: 9/21/2021	Project No.
Туре:	Drawing No.
SITE	
Drawn By:	4
СВ	
Approved By:	
ВН	
Scale:	
AS NOTED	







----SECTION LINE

RESTRICTED USE BOUNDARY

FACILITY BOUNDARY

FOUND SURVEY MARKER AS NOTED

SURVEY NOTE:

THIS DRAWING AND THE INFORMATION SHOWN HERE ON WAS OBTAINED FROM DATA COLLECTED FROM A FIELD SURVEY MADE BY INGENAE, LLC BETWEEN FEBRUARY 12 THROUGH JULY 20, 2021. SURVEY COORDINATES, BEARINGS & DISTANCES ARE REFERENCED TO ILLINOIS WEST 1202 STATE PLANE COORDINATE SYSTEM NAD 1983.

CCR FACILITY BOUNDARY CORNERS				
Point #	Northing	Easting		
1010	873983.38	2516413.41		
1011	874222.30	2516410.32		
1012	874230.94	2515283.35		
1013	875165.70	2514904.19		
1014	875017.48	2514554.77		
1015	876638.91	2514531.16		
1016	876662.16	2515990.93		
1017	875183.95	2516003.77		
1018	875223.87	2516105.73		
1019	875107.47	2516308.60		
1020	874481.26	2516406.96		



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bmissions / Revisions:	Date:



Project Name & Location:

COFFEEN POWER PLANT

134 CIPS Lane Coffeen, IL 62017

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Drawing Name: CCR FACILITY BOUNDARY **EXHIBIT**

Project No.
Drawing No.
')

ATTACHMENT B



October 2016

Illinois Power Generating Company 134 CIPS Lane Coffeen, IL 62017

RE: History of Construction

USEPA Final CCR Rule, 40 CFR § 257.73(c)

Coffeen Power Station

Coffeen, Illinois

On behalf of Illinois Power Generating Company, AECOM has prepared the following history of construction for Ash Pond No. 1, Ash Pond No. 2, the GMF Pond, and the GMF Recycle Pond at the Coffeen Power Station in accordance with 40 CFR § 257.73(c).

BACKGROUND

40 CFR § 257.73(c)(1) requires the owner or operator of an existing coal combustion residual (CCR) surface impoundment that either (1) has a height of five feet or more and a storage volume of 20 acre-feet or more, or (2) has a height of 20 feet or more to compile a history of construction by October 17, 2016 that contains, to the extent feasible, the information specified in 40 CFR § 257.73(c)(1)(i)–(xii).

The history of construction presented herein was compiled based on existing documentation, to the extent that it is reasonably and readily available (see 80 Fed. Reg. 21302, 21380 [April 17, 2015]), and AECOM's site experience. AECOM's document review included record drawings, geotechnical investigations, construction specifications, etc. for Ash Pond No. 1, Ash Pond No. 2, the GMF Pond, and the GMF Recycle Pond at the Coffeen Power Station.



HISTORY OF CONSTRUCTION

§ 257.73(c)(1)(i): The name and address of the person(s) owning or operating the CCR unit; the name associated with the CCR unit; and the identification number of the CCR unit if one has been assigned by the state.

Owner: Illinois Power Generating Company

Address: 1500 Eastport Plaza Drive

Collinsville, IL 62234

CCR Units: Ash Pond No. 1

Ash Pond No. 2

GMF Pond, IDNR Dam ID No. IL50579

GMF Recycle Pond, IDNR Dam ID No. IL50578

Ash Pond No. 1 and Ash Pond No. 2 do not have a state assigned identification number.

§ 257.73(c)(1)(ii): The location of the CCR unit identified on the most recent USGS $7^{1}/_{2}$ or 15 minute topographic quadrangle map or a topographic map of equivalent scale if a USGS map is not available.

The locations of Ash Pond No. 1, Ash Pond No. 2, the GMF Pond, and the GMF Recycle Pond have been identified on an USGS 7-1/2 minute topographic quadrangle map in **Appendix A**.

§ 257.73(c)(1)(iii): A statement of the purpose for which the CCR unit is being used.

The following captures the purposes of the CCR units:

- Ash Pond No. 1 is being used to store and dispose of bottom ash and other-non-CCR waste and to clarify recycled process water for plant operations. Ash Pond No. 2 (inactive) was used to store and dispose of bottom ash and fly ash.
- The GMF Pond is being used to store and dispose of gypsum and to clarify recycled process water for plant operations.
- The GMF Recycle Pond was used to store and dispose of gypsum from the plant's scrubber operations prior to the in-service date of the GMF Pond in 2010. The GMF Recycle Pond currently only receives and stores clear process water from the GMF Pond.

Notice of intent to close Ash Pond No. 2 was provided in November, 2015.1

¹ This history of construction report was prepared on a facility-wide basis for CCR surface impoundments at the Coffeen Power Station. The inclusion of Ash Pond No. 2 in this history of construction report does not concede and should not be construed to concede that Ash Pond No. 2 is subject to the Design Criteria or all Operating Criteria in the CCR Rule.



§ 257.73(c)(1)(iv): The name and size in acres of the watershed where the CCR unit is located.

Ash Pond No. 1, Ash Pond No. 2, the GMF Pond, and the GMF Recycle Pond are located in the Coffeen Lake Watershed with a 12-digit Hydrologic Unit Code (HUC) of 071402030304 and a drainage area of 11,695 acres (USGS, 2016).

§ 257.73(c)(1)(v): A description of the physical and engineering properties of the foundation and abutment materials on which the CCR unit is constructed.

The foundation and abutment materials of Ash Pond No. 1, Ash Pond No. 2, the GMF Pond, and the GMF Recycle Pond consist of native fine-grained soils of wind-blown origin (loess), with some coarse-grained layers, underlain by glacial till. The physical properties of the finegrained soils are described as low- to medium-plasticity silty clay, sandy lean clay, or lean clay with sand, often with trace amounts of gravel; or high plasticity fat clay, often with trace amounts of sand. The clay soils vary from soft to very stiff, moist to wet, and brown to gray. The physical properties of the coarse-grained soils are described as clayey sand, silty sand, or fine to coarse sand, with trace amounts of gravel. The sand is wet and varies from loose to dense and brown to gray. A thin layer of native silty or sandy lean clay is located immediately above the glacial till deposits. The clay is very soft to medium stiff, low to medium plasticity, wet, and orange brown to gray. The physical properties of the glacial till are described as lean clay, or silty to sandy lean clay, with trace amounts of fine gravel, hard, low plasticity, moist to wet, and brown to gray. An available summary of the engineering property typical ranges of the foundation and abutment materials is presented in Table 1 below. The engineering properties are based on previous geotechnical explorations and laboratory testing.

Ash Pond No. 1 and Ash Pond No. 2 are enclosed impoundments with embankments and do not have abutments. The GMF Pond and GMF Recycle Pond were constructed as incised impoundments enclosed by embankments.

Table 1. Summary of Foundation and Abutment Material Engineering Properties

Material	Unit Weight	Effective (drained) Shear Strength Parameters		Total (undrained) Shear Strength Parameters	Post-Earthquake Shear Strength
	(pcf)	Cohesion, c' (psf)	Friction Angle, ¢' (deg)	S _u /p'	S _{ur} /p'
Foundation Clay (Under Embankment)	125	0	32	$S_u/p' = 0.39-0.45$, Min. $S_u = 700 \text{ psf}$	Peak Undrained
Foundation Clay (Free Field)	125	0	30	$S_u/p' = 0.24-0.28$, Min. $S_u = 450 \text{ psf}$	Peak Undrained
Soft Foundation Clay	125	0	30	$S_u/p' = 0.22-0.28$, Min. $S_u = 275 \text{ psf}$	$S_u/p' = 0.13-0.16$, Min. $S_{ur} = 200 \text{ psf}$
Glacial Till	135	0	40	$S_u/p' = 0.45-0.64$, Min. $S_u = 700 \text{ psf}$	Peak Undrained



§ 257.73(c)(1)(vi): A statement of the type, size, range, and physical and engineering properties of the materials used in constructing each zone or stage of the CCR unit; the method of site preparation and construction of each zone of the CCR unit; and the approximate dates of construction of each successive stage of construction of the CCR unit.

Physical properties for the embankment construction materials for Ash Pond No. 1, Ash Pond No. 2, GMF Pond, and GMF Recycle Pond are described as silty clay, sandy lean clay, or lean clay with sand, with trace amounts of fine gravel. The fill is soft to very stiff in consistency, low to medium plasticity, moist to wet, and brown to gray. Trace amounts of organic material and ash are present. The embankment fill generally appears to be well-compacted. An available summary of the engineering properties of the embankment construction materials is presented in **Table 2** below. The engineering properties are based on previous geotechnical explorations and laboratory testing.

Table 2. Summary of Construction Material Engineering Properties for Embankments

Material	Unit Weight		Peak Drained near Strength	Peak Undrained Shear Strength	Post- Earthquake Shear Strength
Waterial	(pcf)	Cohesion, c' (psf)	Friction Angle, f' (deg)	S _u /p'	S _{ur} /p'
Embankment Fill	135	0	31	$S_u/p' = 0.60$, Min. $S_u = 450 \text{ psf}$	Peak Undrained

The GMF Pond and GMF Recycle Pond contain liner systems. The liner system within the GMF Pond consists of a 60-mil textured high density polyethylene (HDPE) geomembrane underlain by a 3-foot thick layer of compacted clay. A typical cross section profile of the GMF Pond liner system is shown on drawing C-10206 (sh. 9) presented in **Appendix B**. An available summary of the engineering properties of the GMF Pond liner construction materials from Hanson (2008) is presented in **Table 3** below. The liner system within the GMF Recycle Pond consists of a 60-mil textured HDPE geomembrane underlain by smooth-drum rolled native soil. A typical cross section profile of the GMF Recycle Pond liner system is shown on drawing C-10206 (sh. 20) presented in **Appendix B**.

Table 3. Summary of Construction Material Engineering Properties for Liner

Material	Unit Weight	Effective (drained) Shear Strength Parameters		Total (undrained) Shear Strength Parameters	
	(pcf)	c' (psf)	Ф' (°)	c (psf)	Ф (°)
Clay Liner	121.2	0	28.3	1950	0

The method of site preparation and construction of Ash Pond No. 1 and Ash Pond No. 2 is not reasonably and readily available. Site preparation and construction of the GMF Pond and GMF Recycle Pond were completed in accordance with the applicable construction specification (see § 257.73(c)(1)(xi) below).



The approximate dates of construction of each successive stage of construction of Ash Pond No. 1, Ash Pond No. 2, the GMF Pond, and the GMF Recycle Pond are provided in **Table 4** below.

Table 4. Approximate dates of construction of each successive stage of construction.

Date	Event		
1964	Construction of Ash Pond No. 1		
1971	Construction of Ash Pond No. 2		
1978-1979	Installation of internal embankment and new recycle intake structure in Ash Pond No. 1 and abandonment of existing outfall structure		
1984-1985	Closure of Ash Pond No. 2 by installing a clay cover		
2000	Installation of a sheet pile wall to facilitate construction of drainage flume along the northeast corner of the Ash Pond No. 1		
2009	Installation of well dewatering system in Ash Pond No. 2		
2008-2010	2008-2010 Construction of the GMF Pond and the GMF Recycle Pond		

§ 257.73(c)(1)(vii): At a scale that details engineering structures and appurtenances relevant to the design, construction, operation, and maintenance of the CCR unit, detailed dimensional drawings of the CCR unit, including a plan view and cross sections of the length and width of the CCR unit, showing all zones, foundation improvements, drainage provisions, spillways, diversion ditches, outlets, instrument locations, and slope protection, in addition to the normal operating pool surface elevation and the maximum pool surface elevation following peak discharge from the inflow design flood, the expected maximum depth of CCR within the CCR surface impoundment, and any identifiable natural or manmade features that could adversely affect operation of the CCR unit due to malfunction or mis-operation.

Drawings that contain items pertaining to the requested information for Ash Pond No. 1, Ash Pond No. 2, the GMF Pond, and the GMF Recycle Pond are listed in **Table 5** below. Items marked as "Not Available" are items not found during a review of the reasonably and readily available record documentation.



Table 5. List of drawings containing items pertaining to the information requested in

§ 257.73(c)(1)(vii).

§ 257.73(c)(1)(VII).				
	Ash Pond No. 1	Ash Pond No. 2	GMF Pond	GMF Recycle Pond
Dimensional plan view (all zones)	B-35, S-44, S-45	B-560, A1000 (sh. 1)	C-10206 (sh. 4, 9, 10)	C-10206 (sh. 4, 19)
Dimensional cross sections	B-35, S-47 to S-50	B-561	C-10206 (sh. 9)	C-10206 (sh. 20)
Foundation Improvements	Not Applicable	Not Applicable	C-10206 (sh. 10)	C-10206 (sh. 20)
Drainage Provisions	Not Applicable	A1000 (sh. 4)	C-10206 (sh. 15, 16, 20)	C-10206 (sh. 21)
Spillways and Outlets	S-8, S-49	W1008 (sh. 2)	C-10206 (sh. 20)	C-10206 (sh. 22)
Diversion Ditches	Not Applicable	A1000 (sh. 1)	Not Applicable	Not Applicable
Instrument Locations	Plate 2, Figure 2A	Figure 2B	Figure 2B Not Applicable	
Slope Protection	S-49	B-561	C-10206 (sh. 9)	C-10206 (sh. 20)
Normal Operating Pool Elevation	S-8, S-49	Not Applicable	C-10201-25	Not Available
Maximum Pool Elevation	S-8	Not Applicable	C-10201-25	Not Available
Approximate Maximum Depth of CCR in 2016	15 feet	28 feet	16 feet	12 feet

All drawings referenced in **Table 5** above can be found in **Appendix B** and **Appendix C**.

Based on the review of the drawings listed above, no natural or manmade features that could adversely affect operation of these CCR units due to malfunction or mis-operation were identified.



§ 257.73(c)(1)(viii): A description of the type, purpose, and location of existing instrumentation.

Existing instrumentation at Ash Pond No. 1 and Ash Pond No. 2 include vibrating-wire and open-standpipe piezometers. The purpose of the piezometers is to measure the phreatic surface within and around the impoundments. Two (2) open-standpipe piezometers (AP-P1 and AP-P2) were installed at Ash Pond No. 2 in 2009 and the locations are presented on Figure 2A in **Appendix C**. Two (2) open-standpipe piezometers (B-2 and B-4) were installed at Ash Pond No. 1 in 2010 and the locations are presented on Plate 2 in **Appendix C**. Twelve (12) open-standpipe and vibrating-wire piezometers were installed at Ash Pond No. 1 and Ash Pond No. 2 in 2015 and the locations are presented on Figure 2A in **Appendix C**.

The GMF Pond does not contain existing instrumentation. Existing instrumentation at the GMF Recycle Pond consists of one (1) ultrasonic level transmitter. The purpose of the ultrasonic level transmitter is to measure the water level within the GMF Recycle Pond. The location of the ultrasonic level transmitter is shown on drawing C-10206 (sh. 19) presented in **Appendix B**.

§ 257.73(c)(1)(ix): Area-capacity curves for the CCR unit.

Area-capacity curves for Ash Pond No. 2 and the GMF Recycle Pond are not reasonably and readily available. The area-capacity curves for Ash Pond No. 1 and the GMF Pond are presented in **Figures 1** and **2**, respectively, below. "Area-capacity curves", as defined by 40 CFR § 257.53, "means graphic curves which readily show the reservoir water surface area, in acres, at different elevations from the bottom of the reservoir to the maximum water surface, and the capacity or volume, in acre-feet, of the water contained in the reservoir at various elevations."

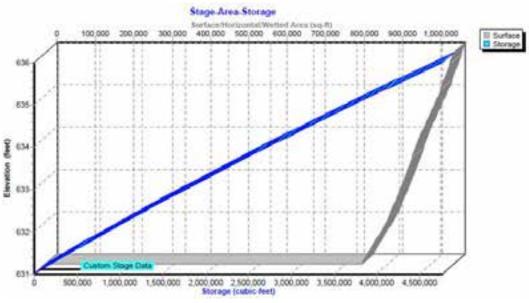


Figure 1. Area-capacity curve for Ash Pond No. 1



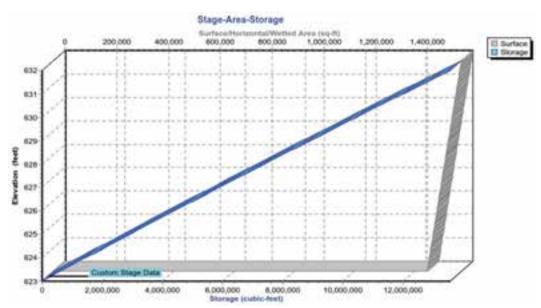


Figure 2. Area-capacity curve for GMF Pond

The area-capacity curves shown were taken from the pond modeling analysis. Actual pond capacity is limited to the approximate berm elevation listed in **Table 6** below. Any information above berm elevation should be disregarded.

§ 257.73(c)(1)(x): A description of each spillway and diversion design features and capacities and calculations used in their determination.

Ash Pond No. 1 contains a concrete intake structure that drains into a 48-inch diameter (dia.) steel pipe. The steel pipe leads to the recycle pump house. In 2016, the discharge capacity of Ash Pond No. 1 was evaluated using HydroCAD 10 software modeling a 1,000-year, 24-hour rainfall event. The results of the HydroCAD 10 analysis are presented below in **Table 6**.

Ash Pond No. 2 was closed in 1984-1985 by installing a clay cover. Non-contact stormwater is collected in ditches along the clay cover and drain off the pond cover via concrete-lined ditch outlets. CCR-contact stormwater collected within the pond is pumped into the GMF Pond via the well dewatering system at the discretion of plant personnel. The capacity of the diversion ditches and well pumps during a model rainfall event has not been evaluated.

The GMF Pond contains a 14-inch high-density polyethylene (HDPE) pipe culvert for normal flow and a weir-like spillway for high water flow. The GMF Pond also contains a 10-inch dia. HDPE siphon pipe used for dewatering. In 2016, the discharge capacity of the GMF Pond was evaluated using HydroCAD 10 software modeling a 1,000-year, 24-hour rainfall event. The results of the HydroCAD 10 analysis are presented below in **Table 6**.

The GMF Recycle Pond contains a decant structure that drains into two (2) 18-inch dia. HDPE pipes that lead to a pump house. The capacity of the decant structure during a model rainfall event has not been evaluated.



Table 6. Results of HydroCAD 10 analyses

	Ash Pond No. 1	GMF Pond
Approximate Minimum Berm Elevation ¹ (ft)	635.0	631.0
Approximate Emergency Spillway Elevation ¹ (ft)	Not Applicable	624.0
Starting Pool Elevation ¹ (ft)	631.0	621.2
Peak Elevation ¹ (ft)	632.0	623.8
Time to Peak (hr)	24.4	24.1
Surface Area (ac)	20.4	33.4
Storage ² (ac-ft)	19.5	88.3

Note:

- 1. Elevations are based on NAVD88 datum
- 2. Storage given is from Starting Pool Elevation to Peak Elevation.

§ 257.73(c)(1)(xi): The construction specifications and provisions for surveillance, maintenance, and repair of the CCR unit.

The construction specifications for Ash Pond No. 1 and Ash Pond No. 2 are not reasonably and readily available. The construction specifications for the GMF Pond and the GMF Recycle Pond are located in *Project Specifications, Gypsum Stack and Recycle Pond Construction* presented in **Appendix D**.

The provisions for surveillance, maintenance, and repair of Ash Pond No. 1 are located in *Operation & Maintenance Manual for #1 Ash Pond* presented in **Appendix E**. The provisions for surveillance, maintenance, and repair of Ash Pond No. 2 are not reasonably and readily available. The provisions for surveillance, maintenance, and repair of the GMF Pond and the GMF Recycle Pond are located in *Operation and Maintenance Manual, Gypsum Management Facility* presented in **Appendix F**.

The operations and maintenance plans for the CCR units identified in this report are currently being revised by Illinois Power Generating Company.

§ 257.73(c)(1)(xii): Any record or knowledge of structural instability of the CCR unit.

In March, 2009, shallow sloughing was observed along the eastern embankment of Ash Pond No. 2. The sloughing was inspected by Hanson Professional Services Inc. A dewatering



system was installed in Ash Pond No. 2 to lower the phreatic surface within the pond. In December, 2015, additional sloughing was observed on the embankment of Ash Pond No. 2 and on the embankment of Ash Pond No. 1. The sloughing was believed to be caused by recent heavy rains and was repaired. Photos of the 2015 sloughing repair are presented in Appendix G.

There is no record or knowledge of structural instability at the GMF Pond and the GMF Recycle Pond at Coffeen Power Station.

LIMITATIONS

The signature of AECOM's authorized representative on this document represents that to the best of AECOM's knowledge, information and belief in the exercise of its professional judgment, it is AECOM's professional opinion that the aforementioned information is accurate as of the date of such signature. Any recommendation, opinion or decisions by AECOM are made on the basis of AECOM's experience, qualifications and professional judgment and are not to be construed as warranties or guaranties. In addition, opinions relating to environmental, geologic, and geotechnical conditions or other estimates are based on available data and that actual conditions may vary from those encountered at the times and locations where data are obtained, despite the use of due care.

Sincerely,

Claudia Prado

Program Manager

Victor Modeer, P.E., D.GE Senior Project Manager



REFERENCES

Hanson Professional Services Inc. (2008), Support Document for IDNR/OWR Permit Application, Coffeen Power Generating Station Gypsum Management Facility Montgomery County, Illinois

United States Environmental Protection Agency (USEPA). (2015). *Hazardous and Solid Waste Management System; Disposal of Coal Combustion Residuals From Electric Utilities; Final Rule.* 40 CFR Parts 257 and 261, 80 Fed. Reg. 21302, 21380 April 17, 2015.

United States Geological Survey (USGS). (2016). The National Map Viewer. http://viewer.nationalmap.gov/viewer/. USGS data first accessed in March of 2016.

APPENDICES

Appendix A: History of Construction Vicinity Map Appendix B: Coffeen Power Station Drawings

Appendix C: Coffeen Power Station Boring and Piezometer Locations

Appendix D: Project Specifications, Gypsum Stack and Recycle Pond Construction (Hanson 2008)

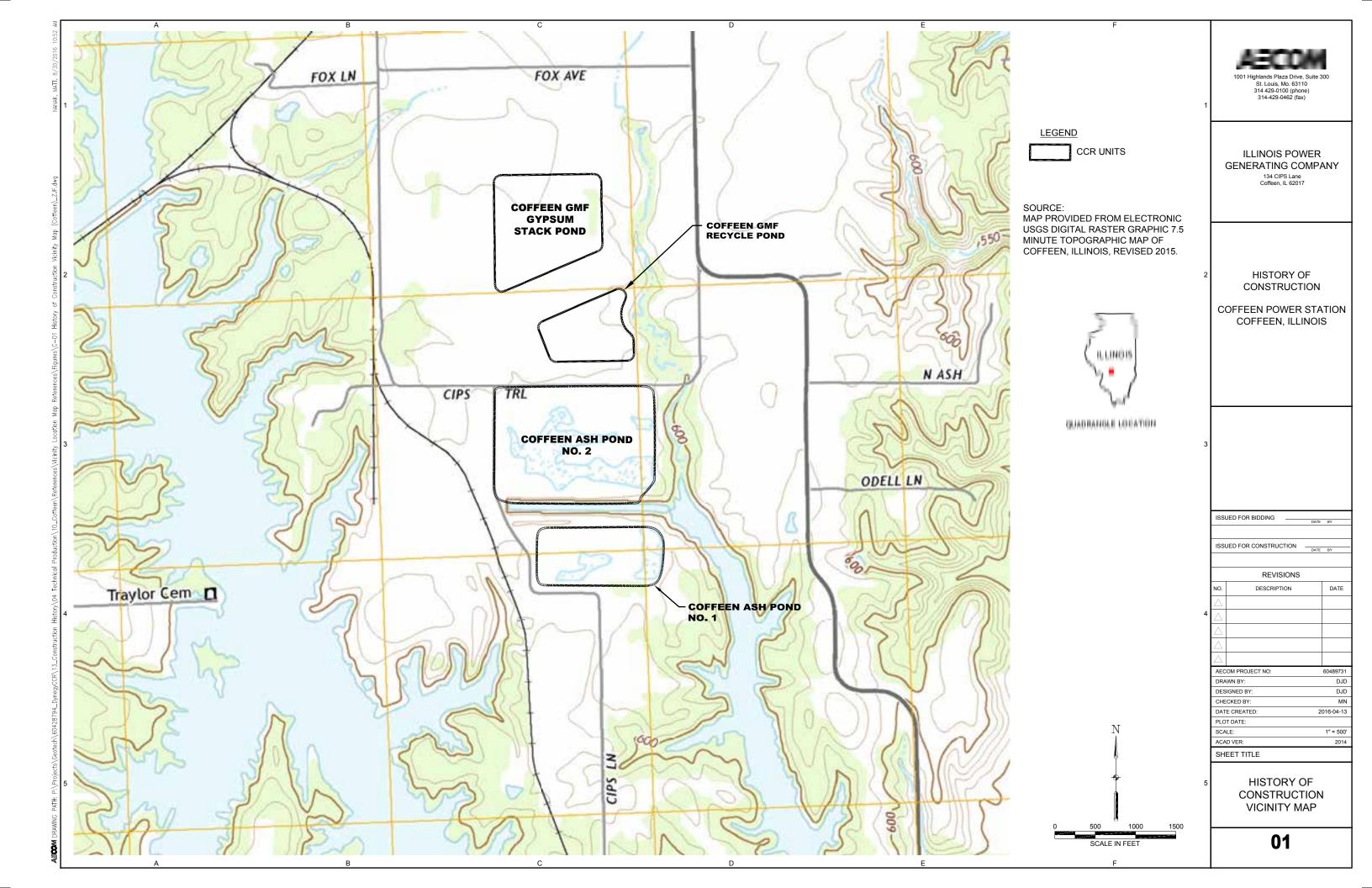
Appendix E: Operation & Maintenance Manual for #1 Ash Pond

Appendix F: Operation and Maintenance Manual, Gypsum Management Facility Operation (2015)

Appendix G: Photos of 2015 Sloughing Repair



Appendix A: History of Construction Vicinity Map





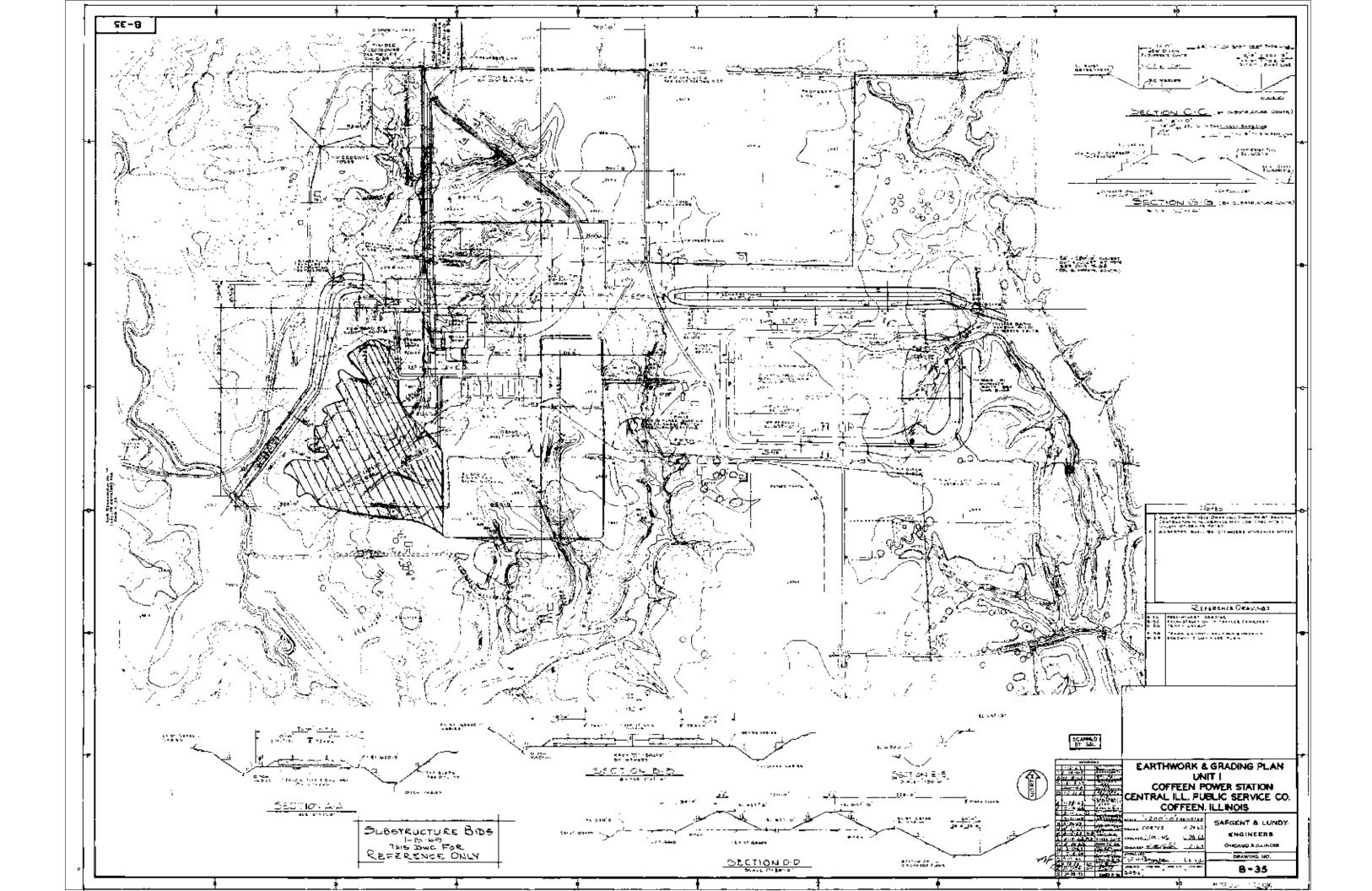
Appendix B: Coffeen Power Station Drawings

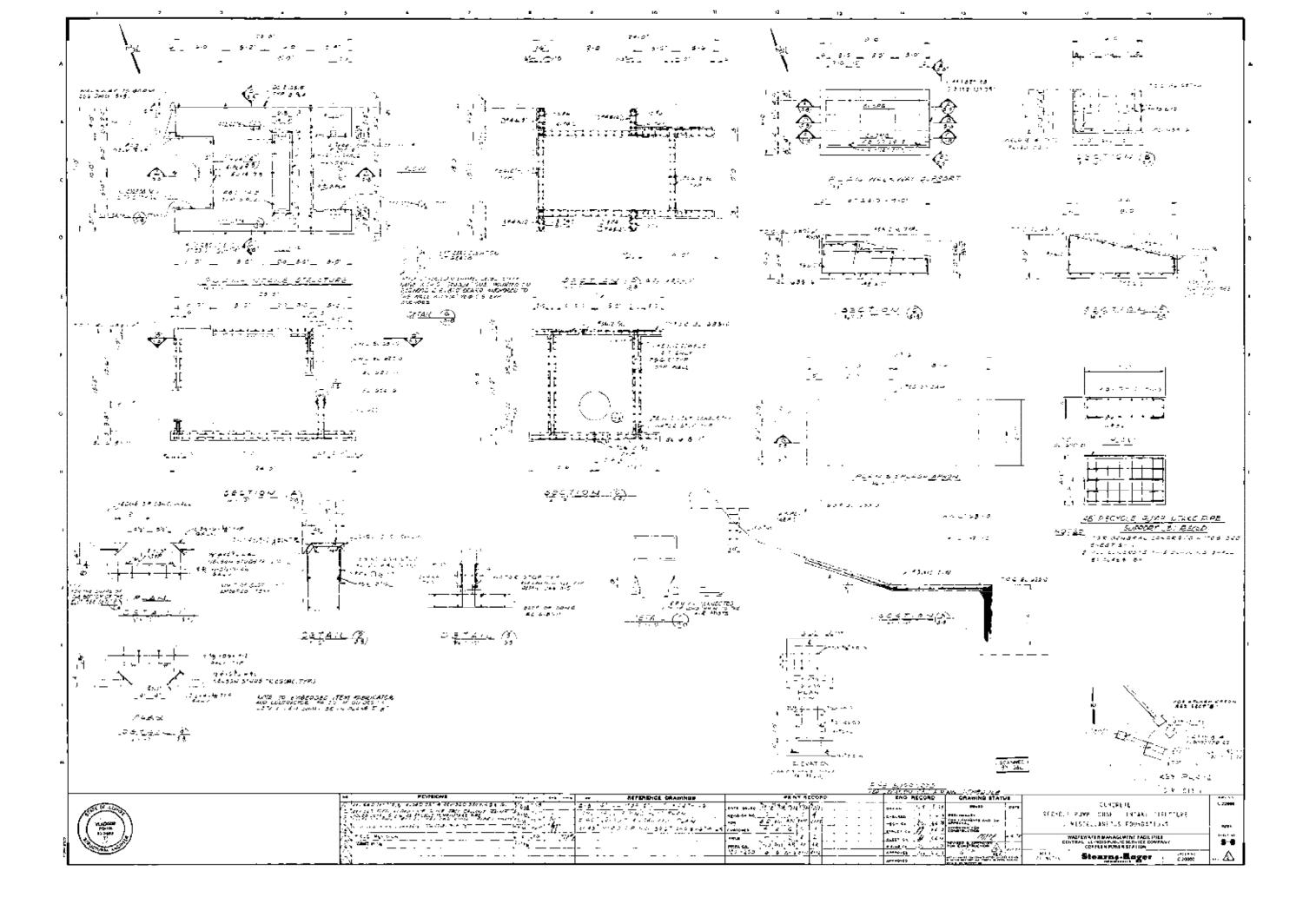
- 1. "Earthwork & Grading Plan", Drawing No. B-35, Revision S, 8 September, 1995, Sargent & Lundy Engineers.
- 2. "Concrete Recycle Pump House Intake Structure", Drawing No. S-8, Revision 6, 23 February, 1996, Stearns-Roger Incorporated.
- 3. "Civil Layout & Grading Plan Sheet 4", Drawing No. S-44, Revision 6, 23 February, 1996, Stearns-Roger Incorporated.
- 4. "Civil Layout & Grading Plan Sheet 5", Drawing No. S-45, Revision 9, 23 February, 1996, Stearns-Roger Incorporated.
- 5. "Civil Miscellaneous Sections and Details, Sheet 2", Drawing No. S-47, Revision 2, 23 February, 1996, Stearns-Roger Incorporated.
- 6. "Civil Ash Pond No 1 Sections and Details", Drawing No. S-48, Revision 2, 23 February, 1996, Stearns-Roger Incorporated.
- 7. "Civil Miscellaneous Sections and Details, Sheet 4", Drawing No. S-49, Revision 4, 23 February, 1996, Stearns-Roger Incorporated.
- 8. "Civil Miscellaneous Sections and Details", Drawing No. S-50, Revision 4, 23 February, 1996, Stearns-Roger Incorporated.
- 9. "Ash Storage Area, Plan", Drawing No. B-560, Revision A, 9 February, 1971, Sargent & Lundy Engineers.
- 10. "Ash Storage Area, Sections & Details", Drawing No. B-561, Revision A, 9 February, 1971, Sargent & Lundy Engineers.
- 11. "Overall Site Plan, Dewatering System, Ash Pond #2", Drawing No. A1000 (sh. 1), Revision A, 12 October, 2009, Ameren Energy Resources Generating.
- 12. "Site Details, Dewatering System, Ash Pond #2", Drawing No. A1000 (sh. 4), Revision A, 12 October, 2009, Ameren Energy Resources Generating.
- 13. "Proposed Site Plan, CCB Management Facility", Drawing No. C-10206 (sh. 4), Revision 0, 5 January, 2011, Ameren Energy Generating.
- 14. "Groundwater Monitoring & Boring Plan, CCB Management Facility", Drawing No. C-10206 (sh. 5), Revision 0, 5 January, 2011, Ameren Energy Generating.
- 15. "Anchor Trench and Liner System, CCB Management Facility", Drawing No. C-10206 (sh. 9), Revision 0, 5 January, 2011, Ameren Energy Generating.
- 16. "Cell G1-Foundation Grade & Control Data, CCB Management Facility", Drawing No. C-10206 (sh. 10), Revision 0, 5 January, 2011, Ameren Energy Generating.
- 17. "Cell G1-Process Water Recovery System, CCB Management Facility", Drawing No. C-10206 (sh. 15), Revision 0, 5 January, 2011, Ameren Energy Generating.
- 18. "Cell G1-PWRS Drain Details, CCB Management Facility", Drawing No. C-10206 (sh. 16), Revision 0, 5 January, 2011, Ameren Energy Generating.
- 19. "Recycle Pond Plan & Control Data, CCB Management Facility", Drawing No. C-10206 (sh. 19), Revision 0, 5 January, 2011, Ameren Energy Generating.
- 20. "Recycle Pond Process Water Transfer Channel Details, CCB Management Facility", Drawing No. C-10206 (sh. 20), Revision 0, 5 January, 2011, Ameren Energy Generating.

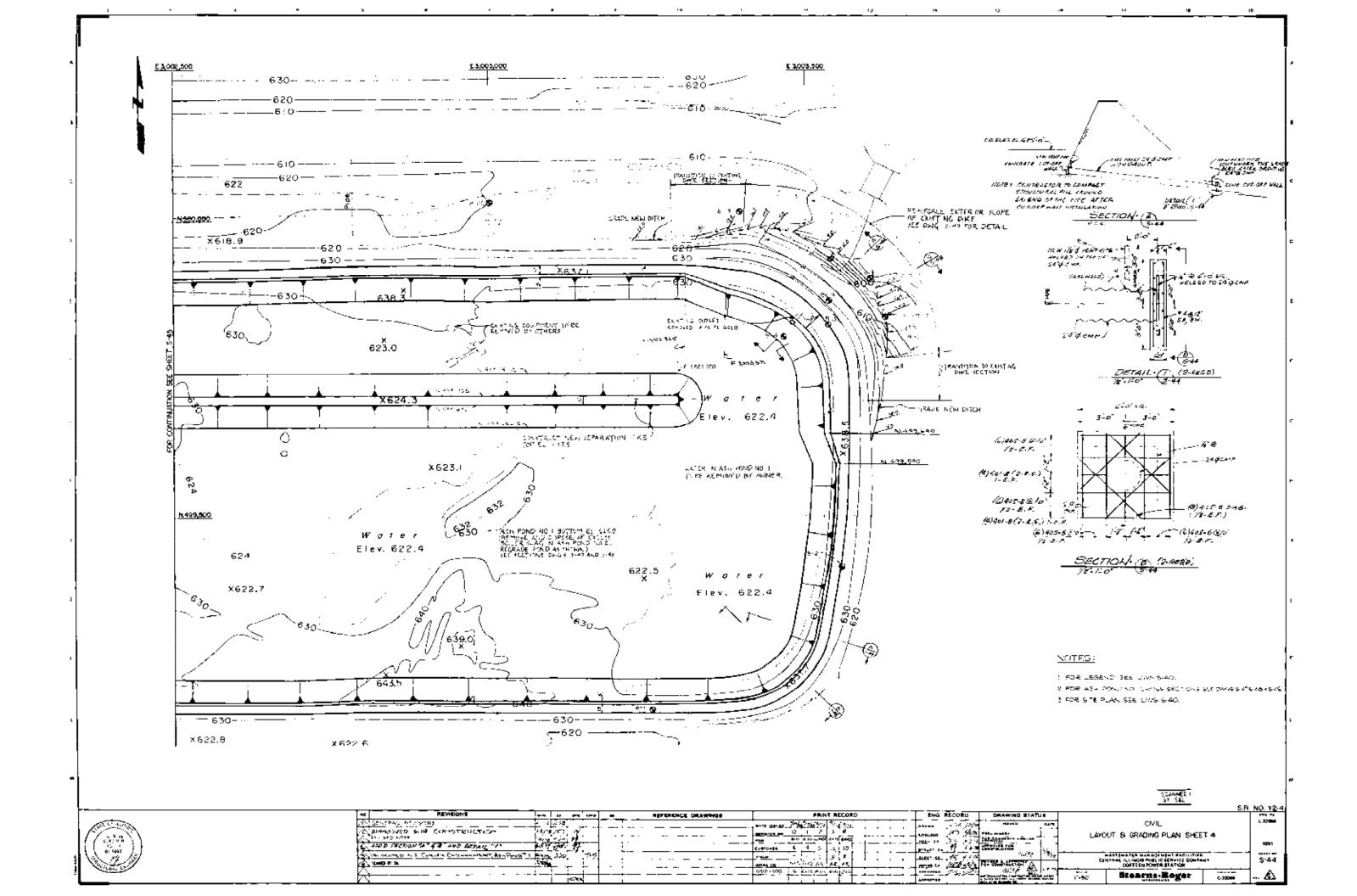


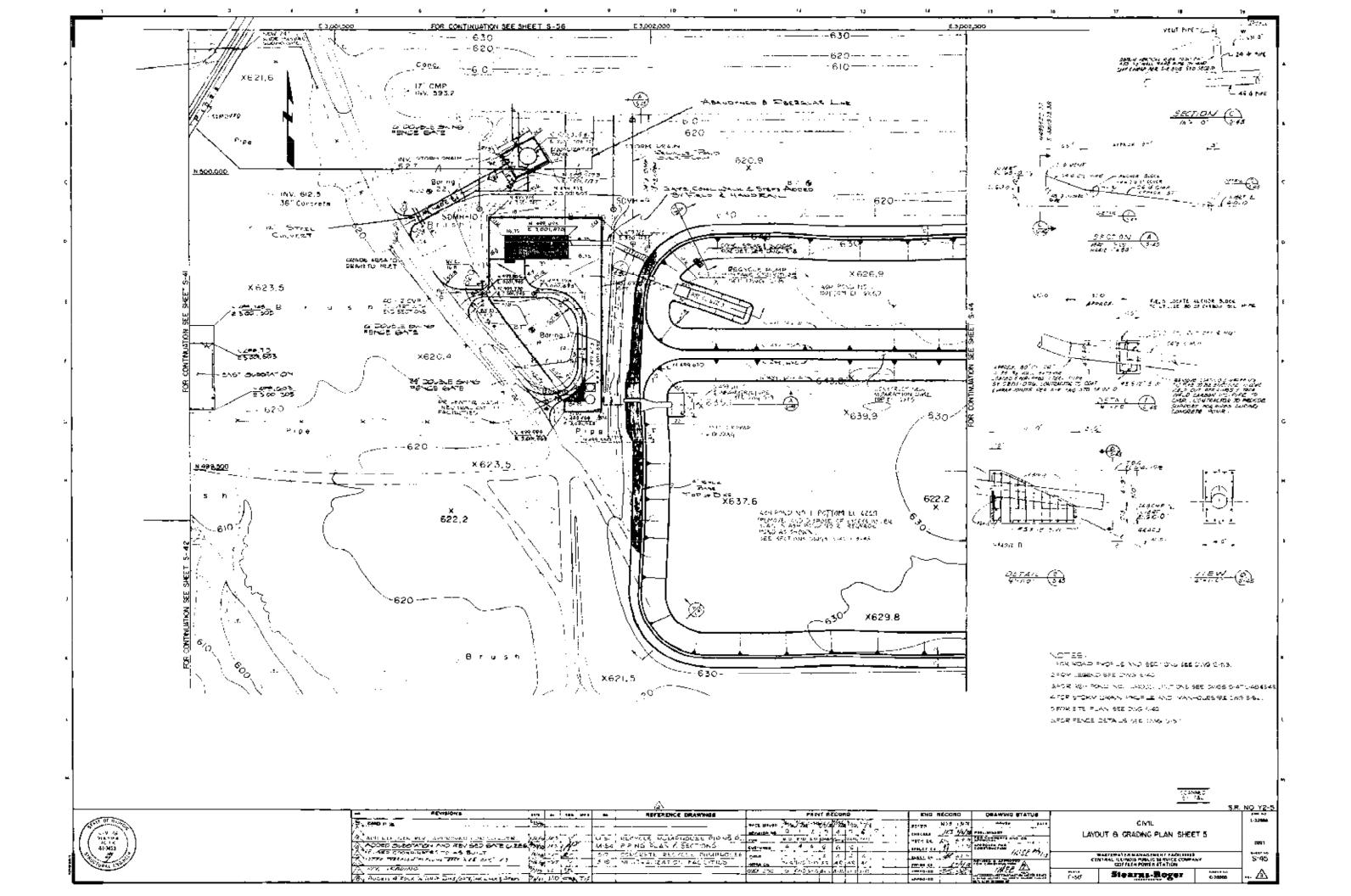
Appendix B: Coffeen Power Station Drawings (continued)

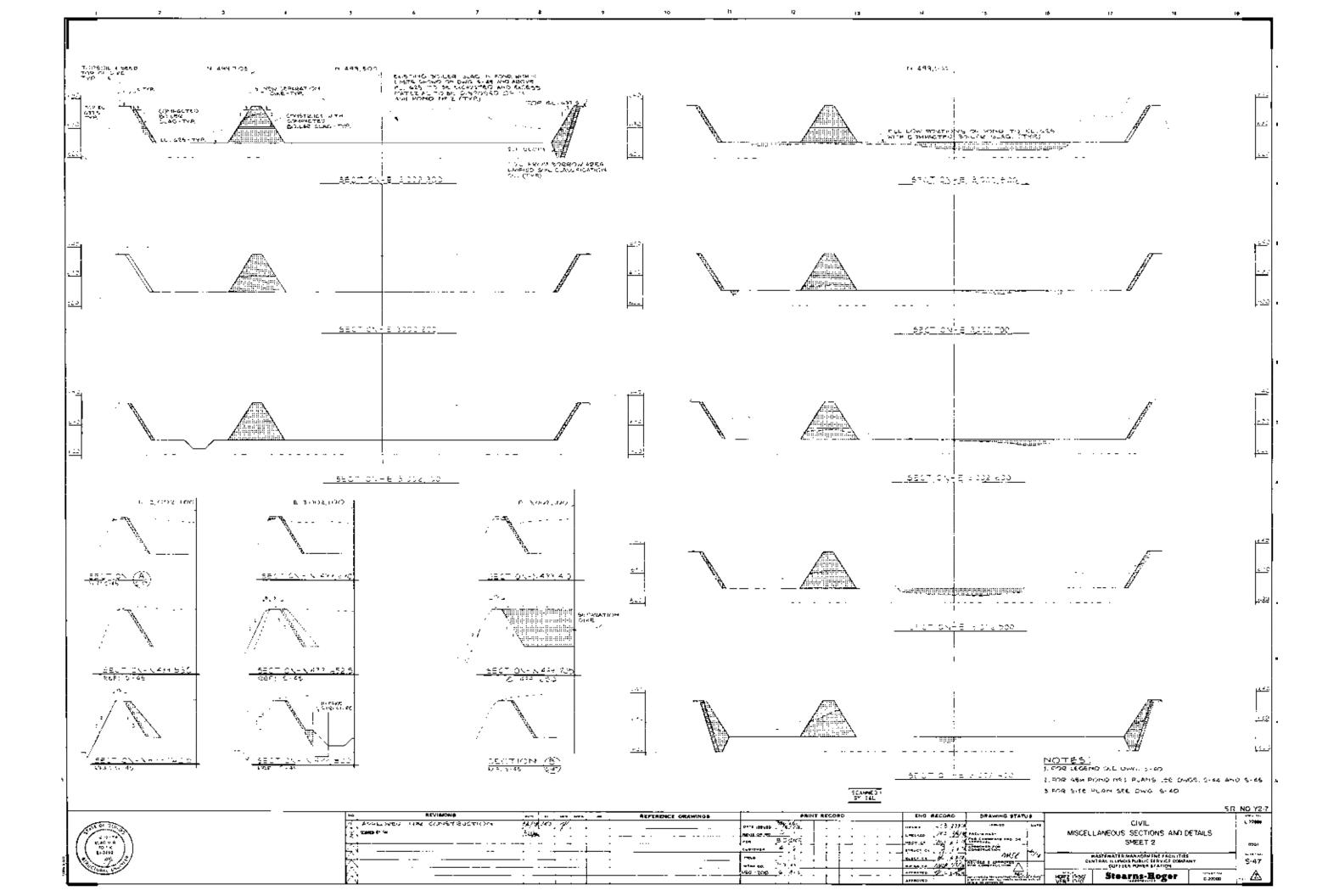
- 21. "Recycle Pond–Process Water Decant Sections & Details, CCB Management Facility", Drawing No. C-10206 (sh. 21), Revision 0, 5 January, 2011, Ameren Energy Generating.
- 22. "Recycle Pond Emergency Spillway Sections & Details, CCB Management Facility", Drawing No. C-10206 (sh. 22), Revision 0, 5 January, 2011, Ameren Energy Generating.
- 23. "Ash Pond #2, Drainage Modifications", Drawing No. W1008 (sh. 2), Ameren Energy Generating.

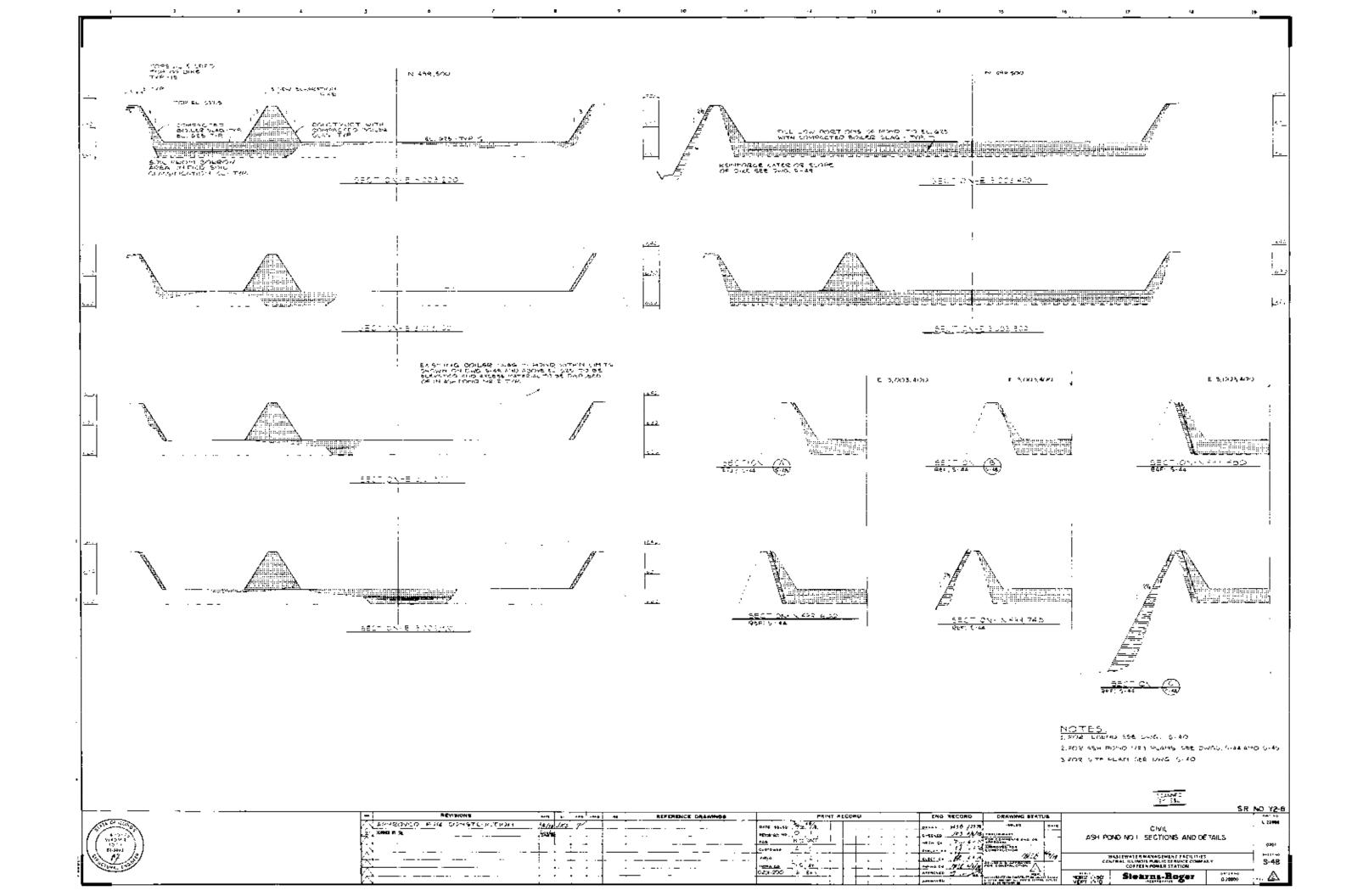


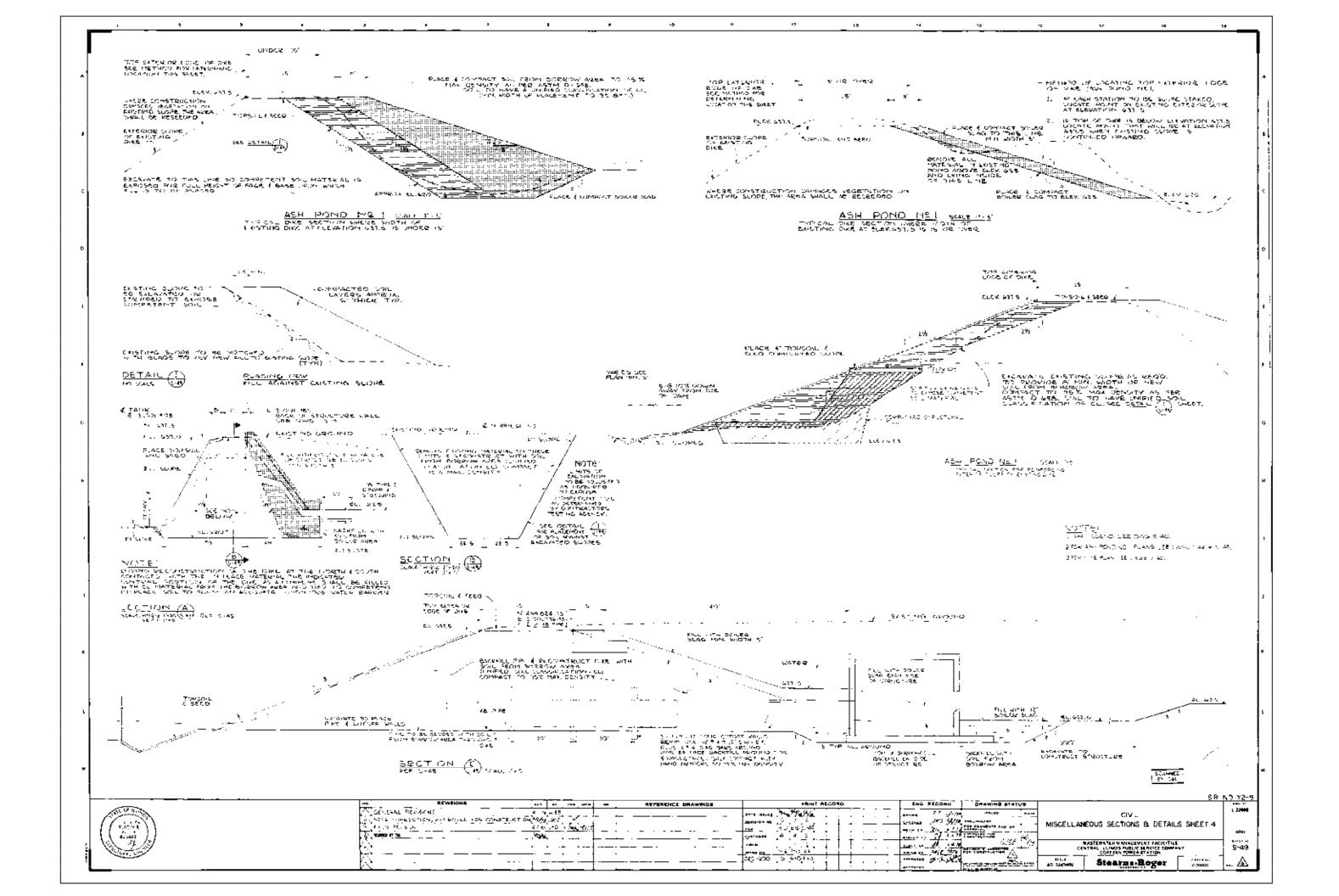


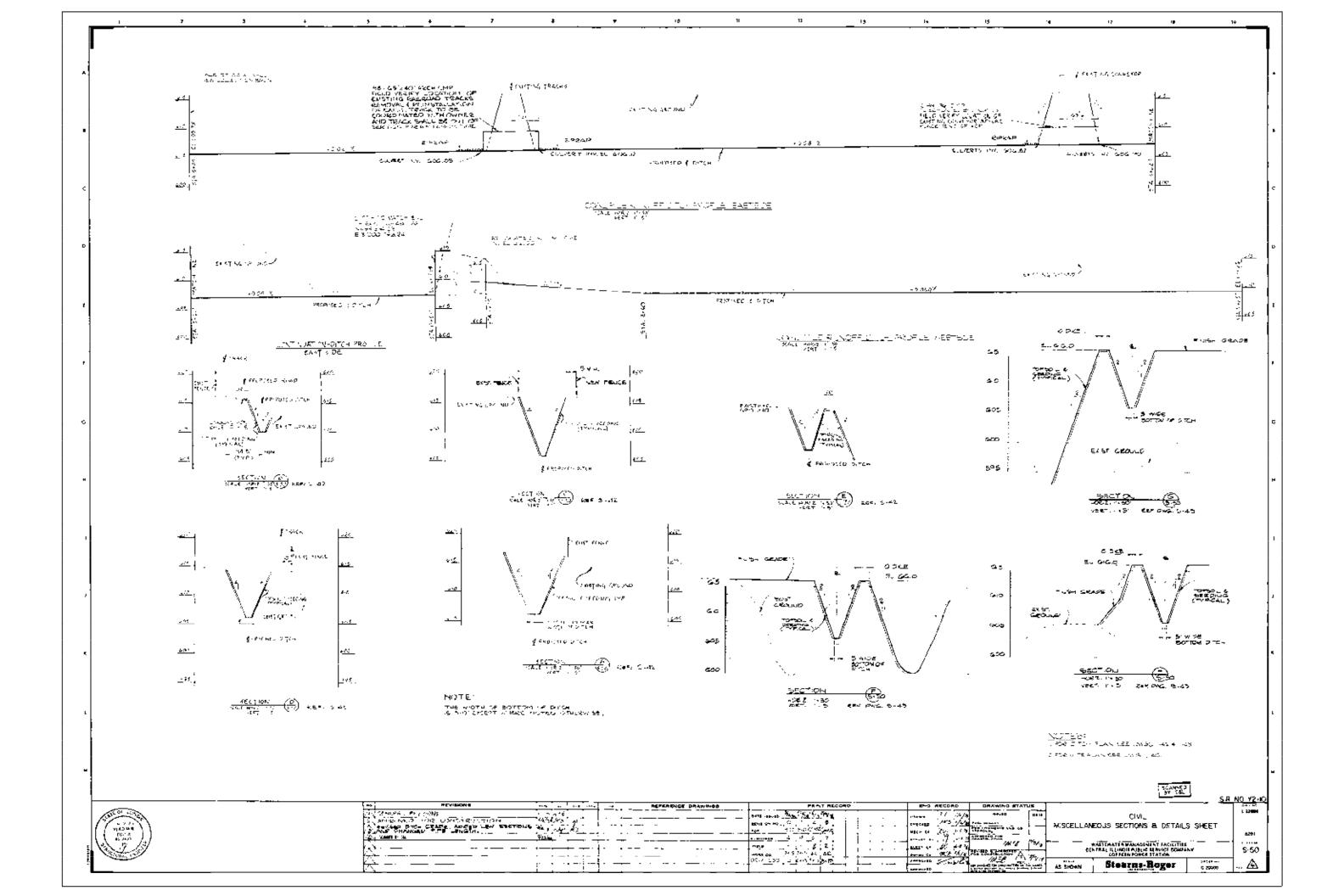


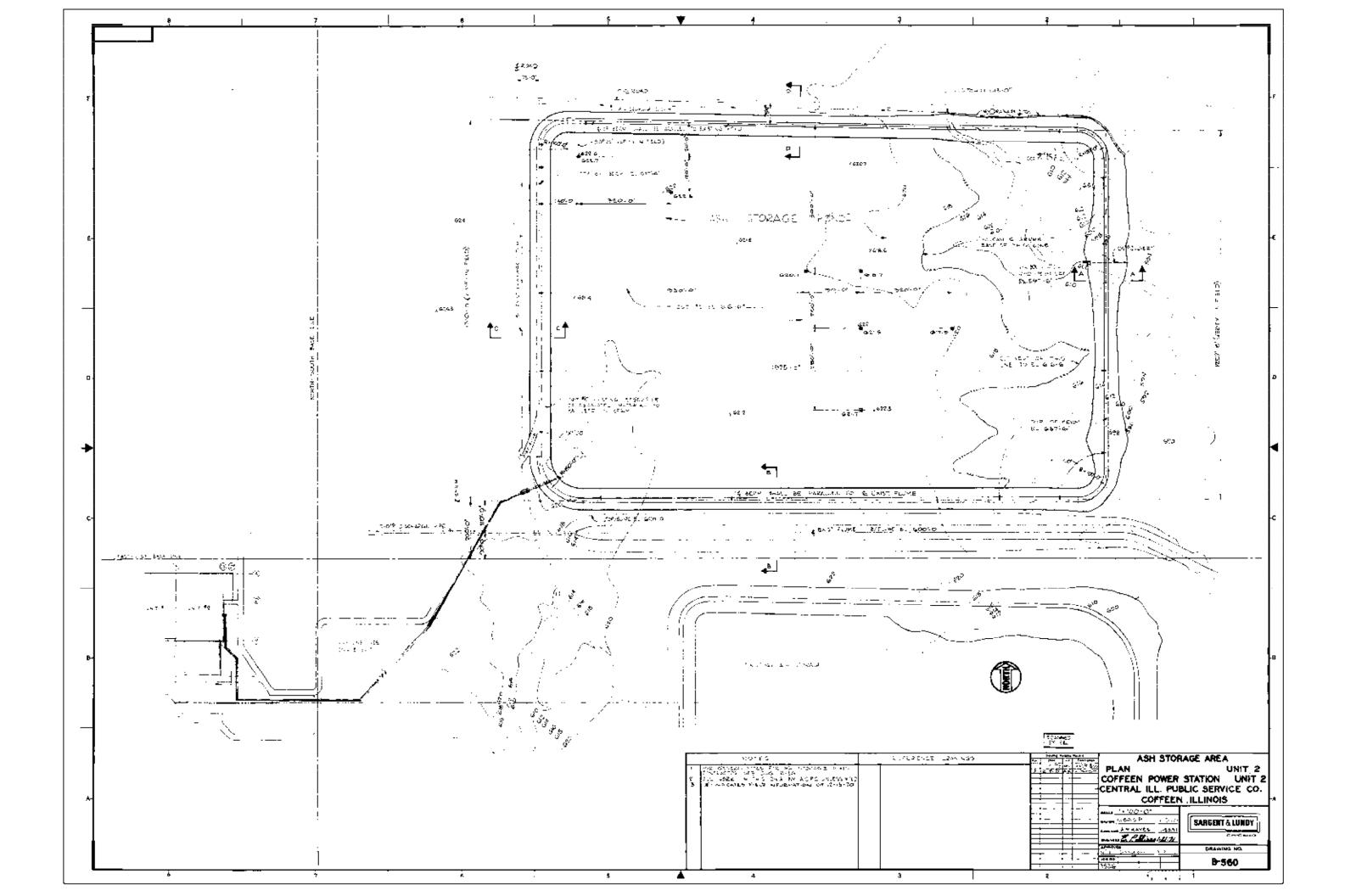


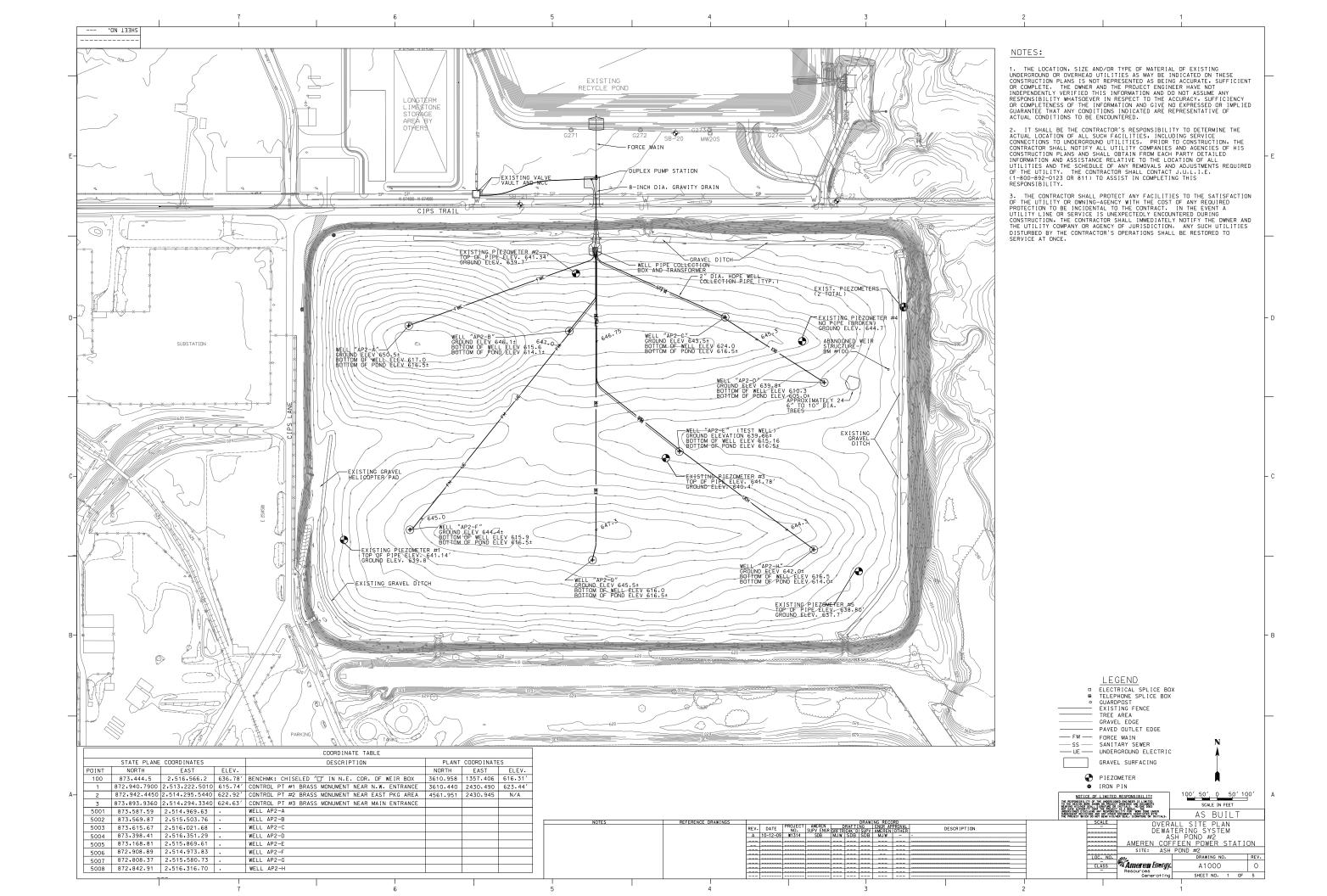


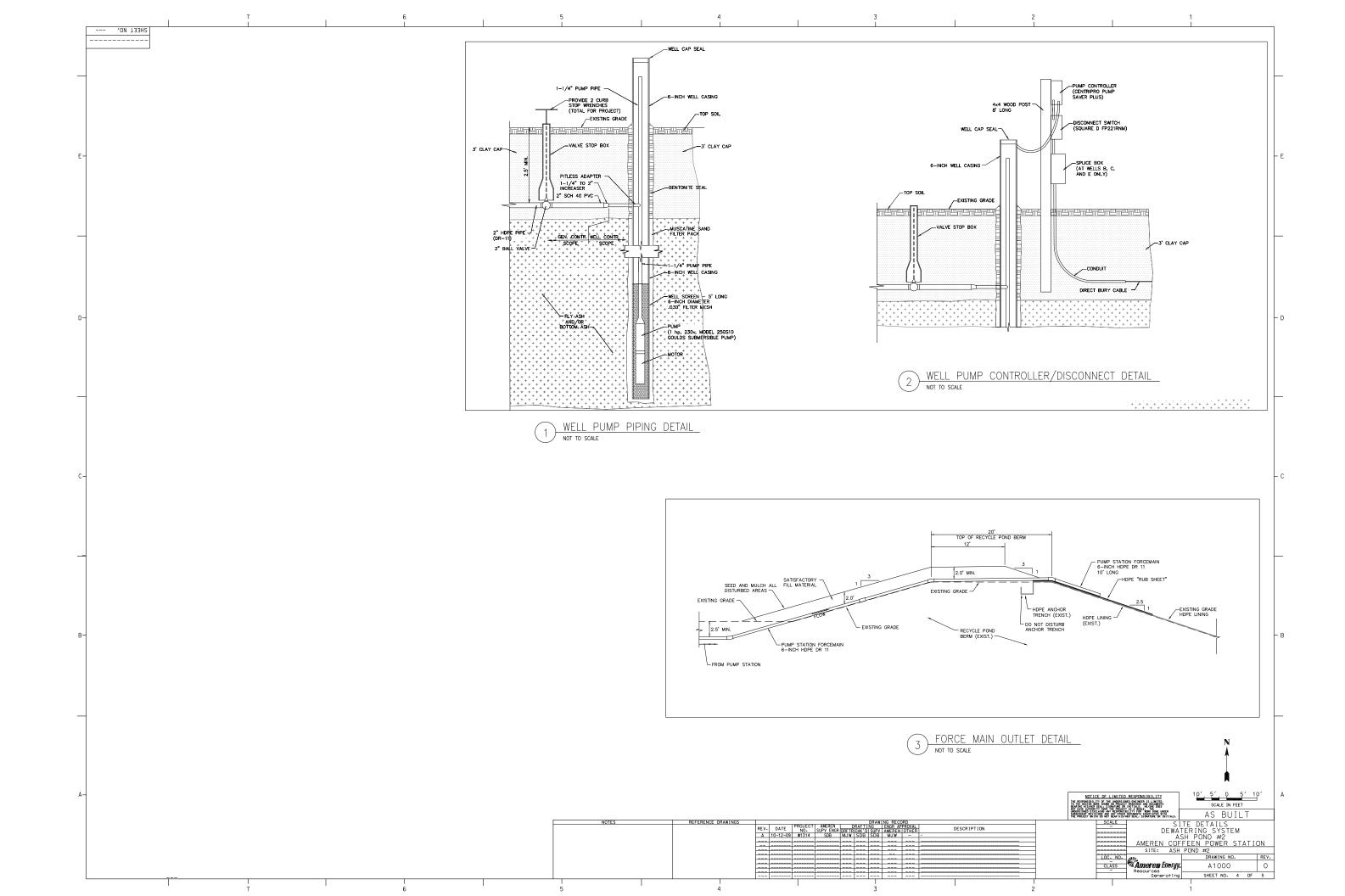


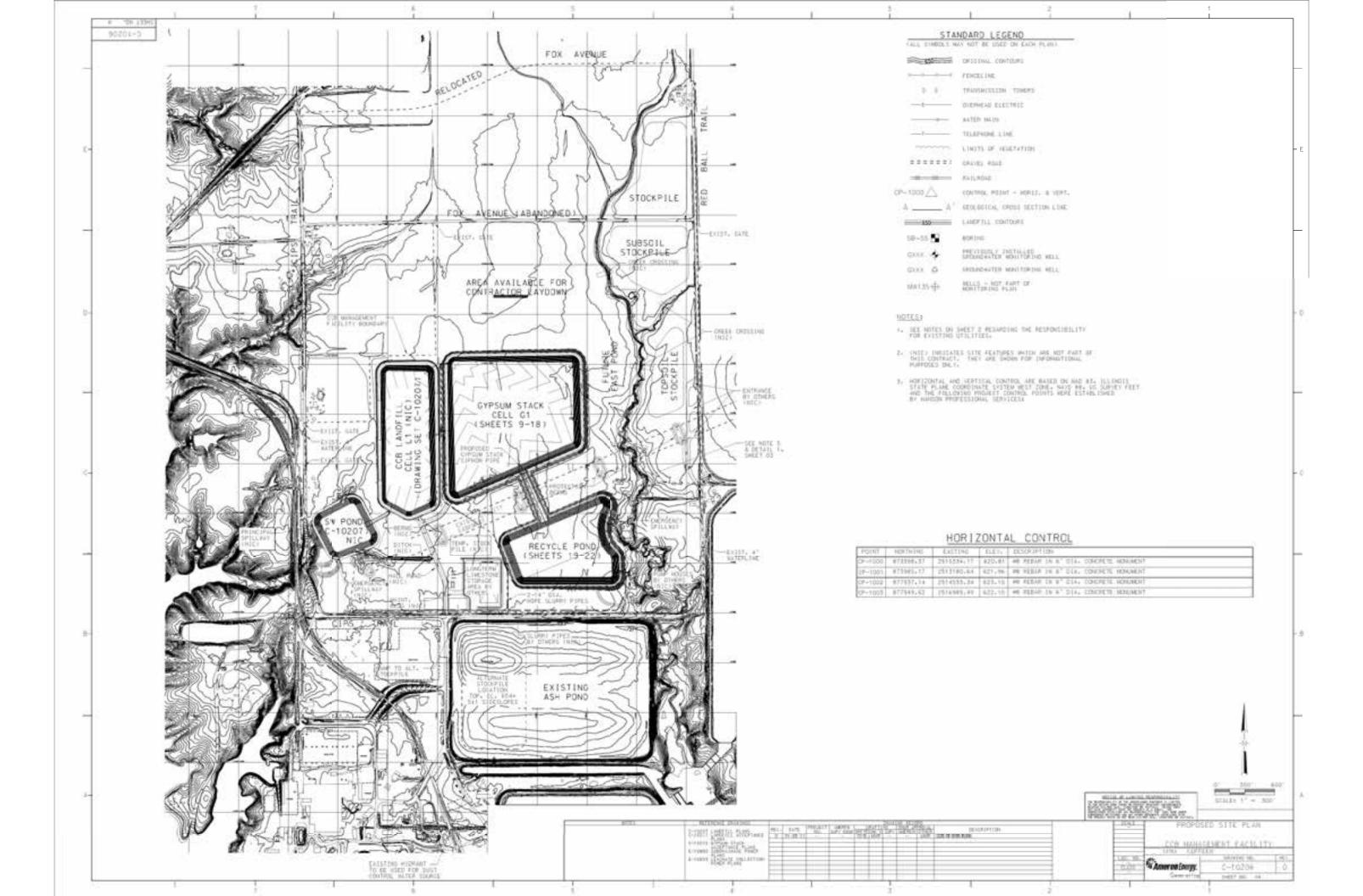


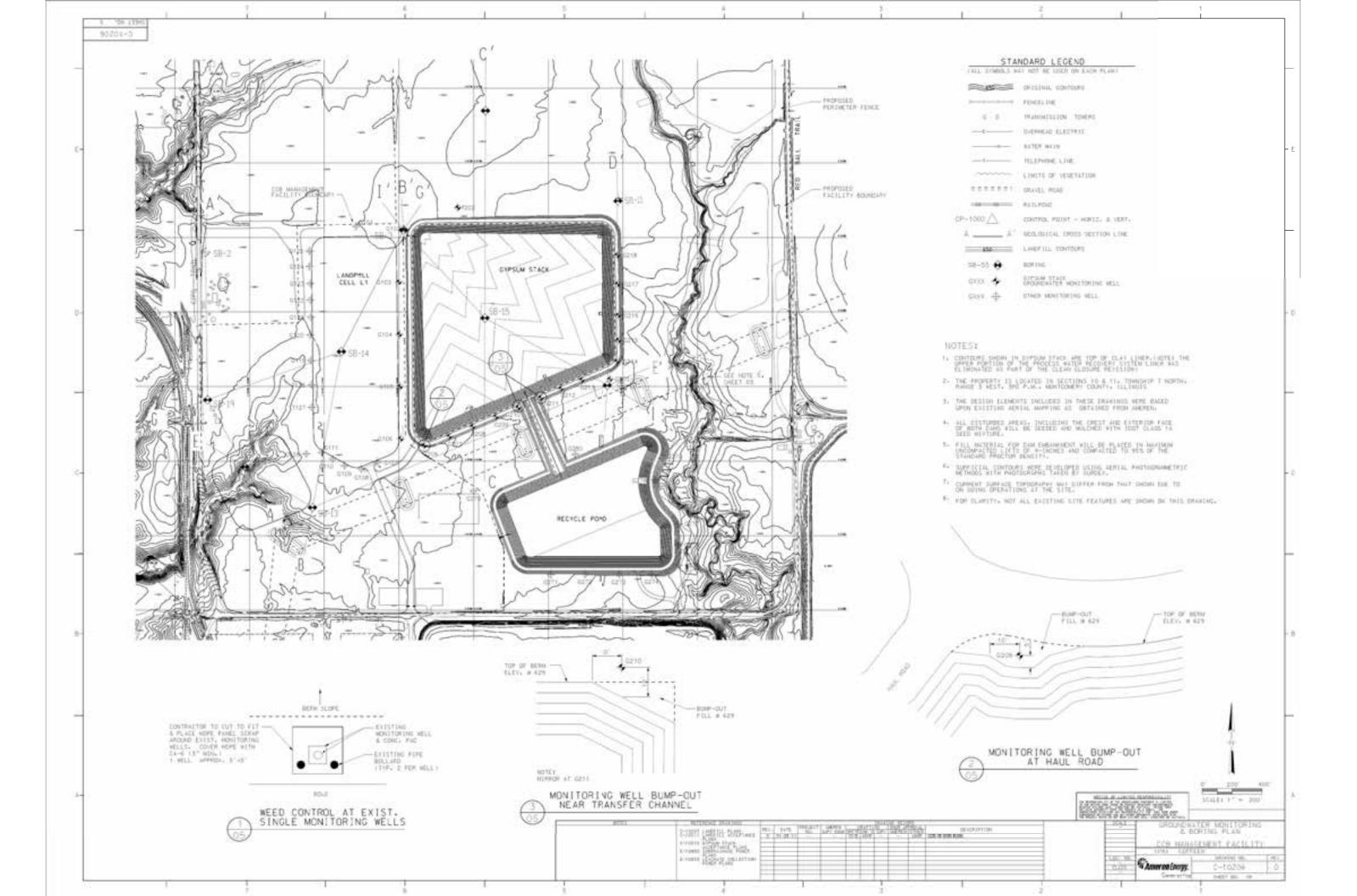


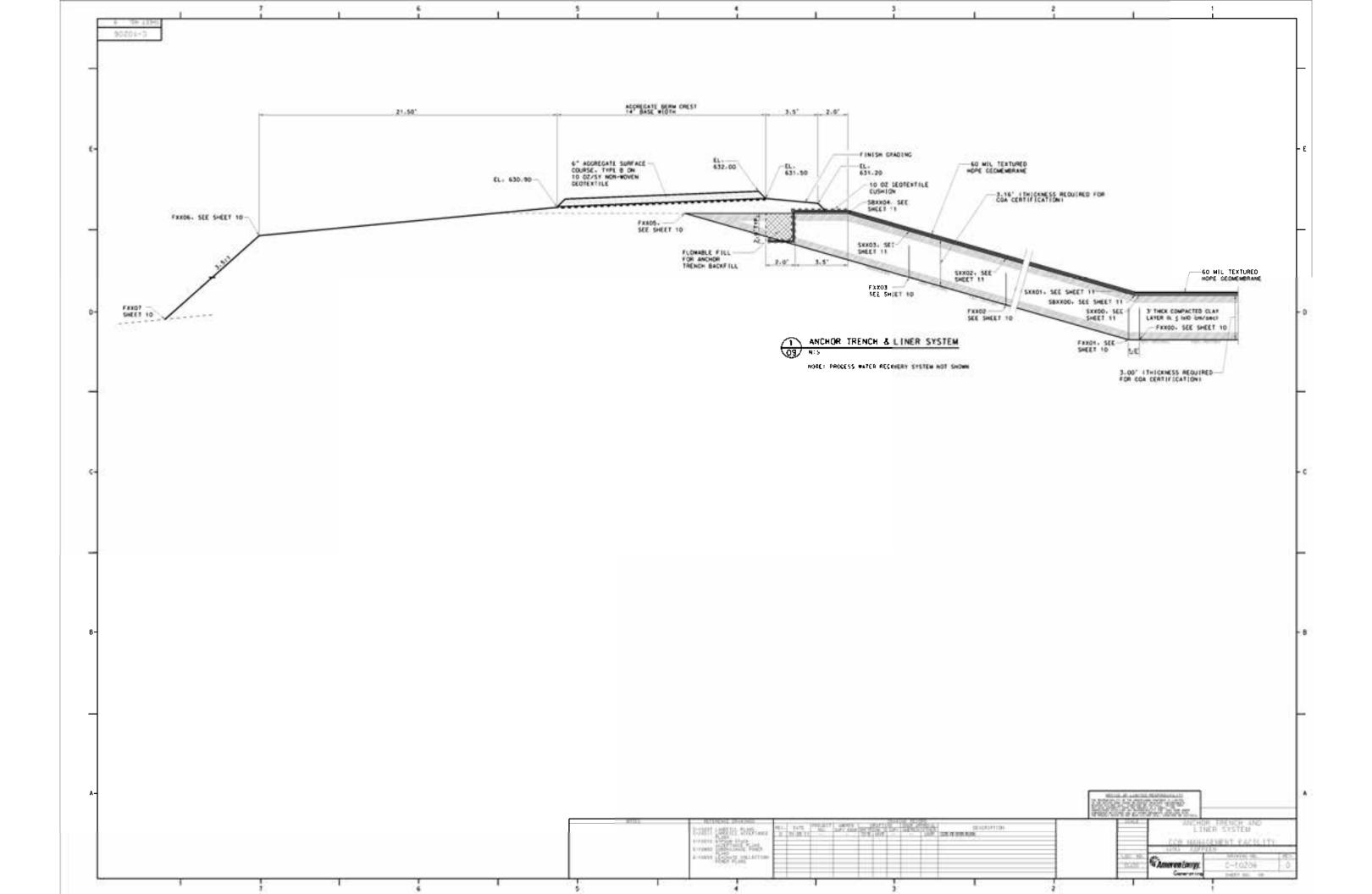


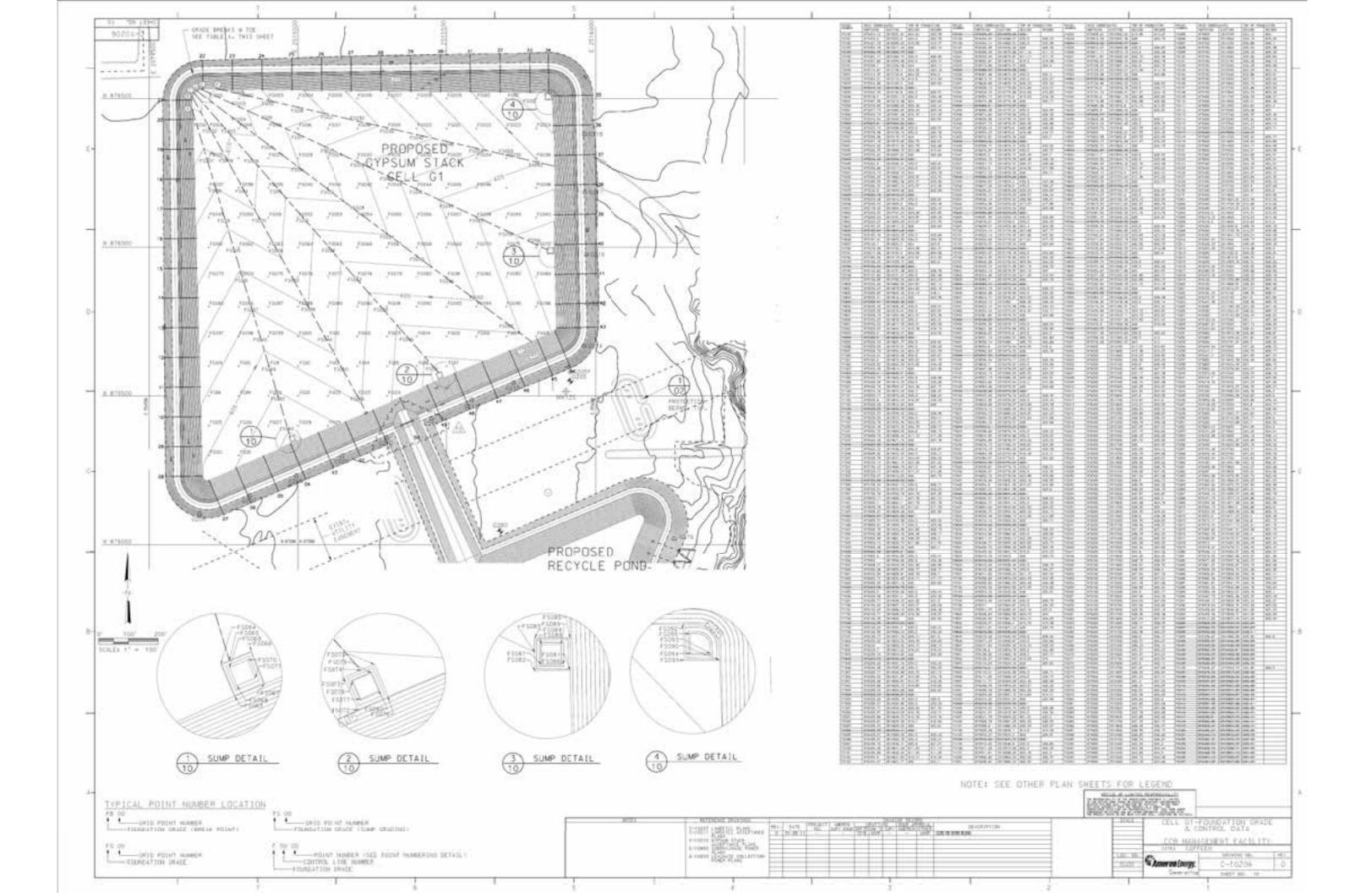


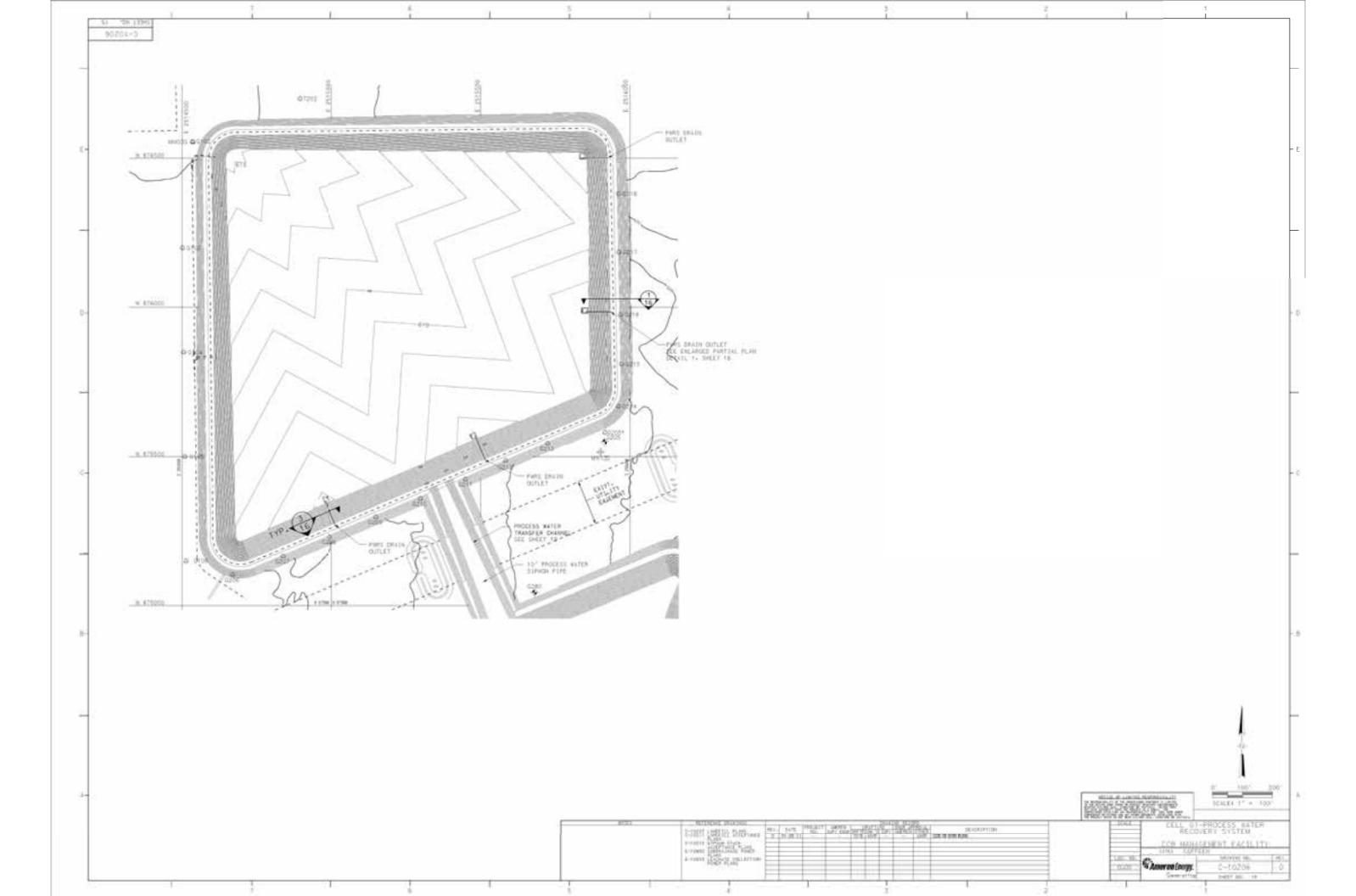


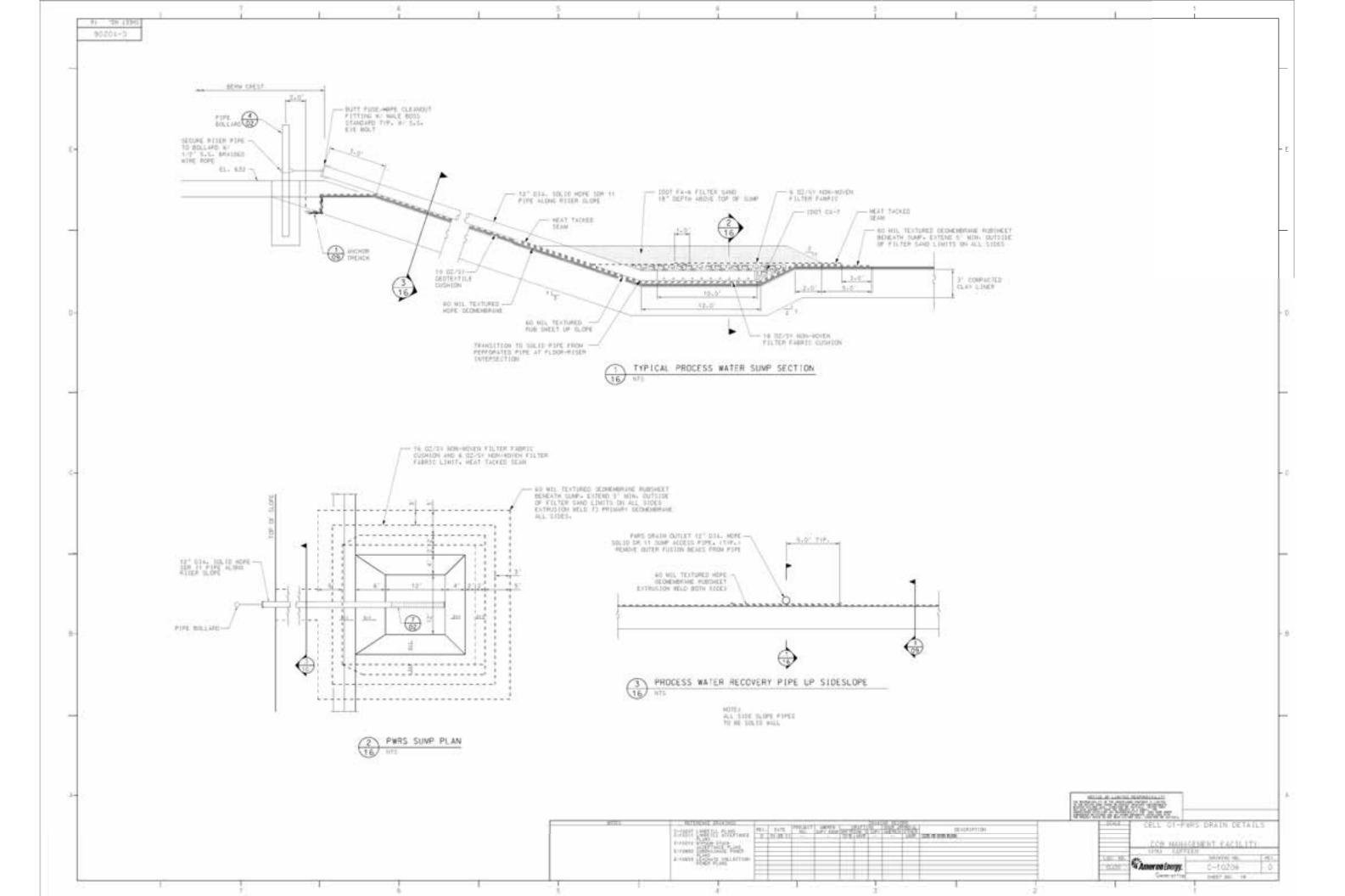


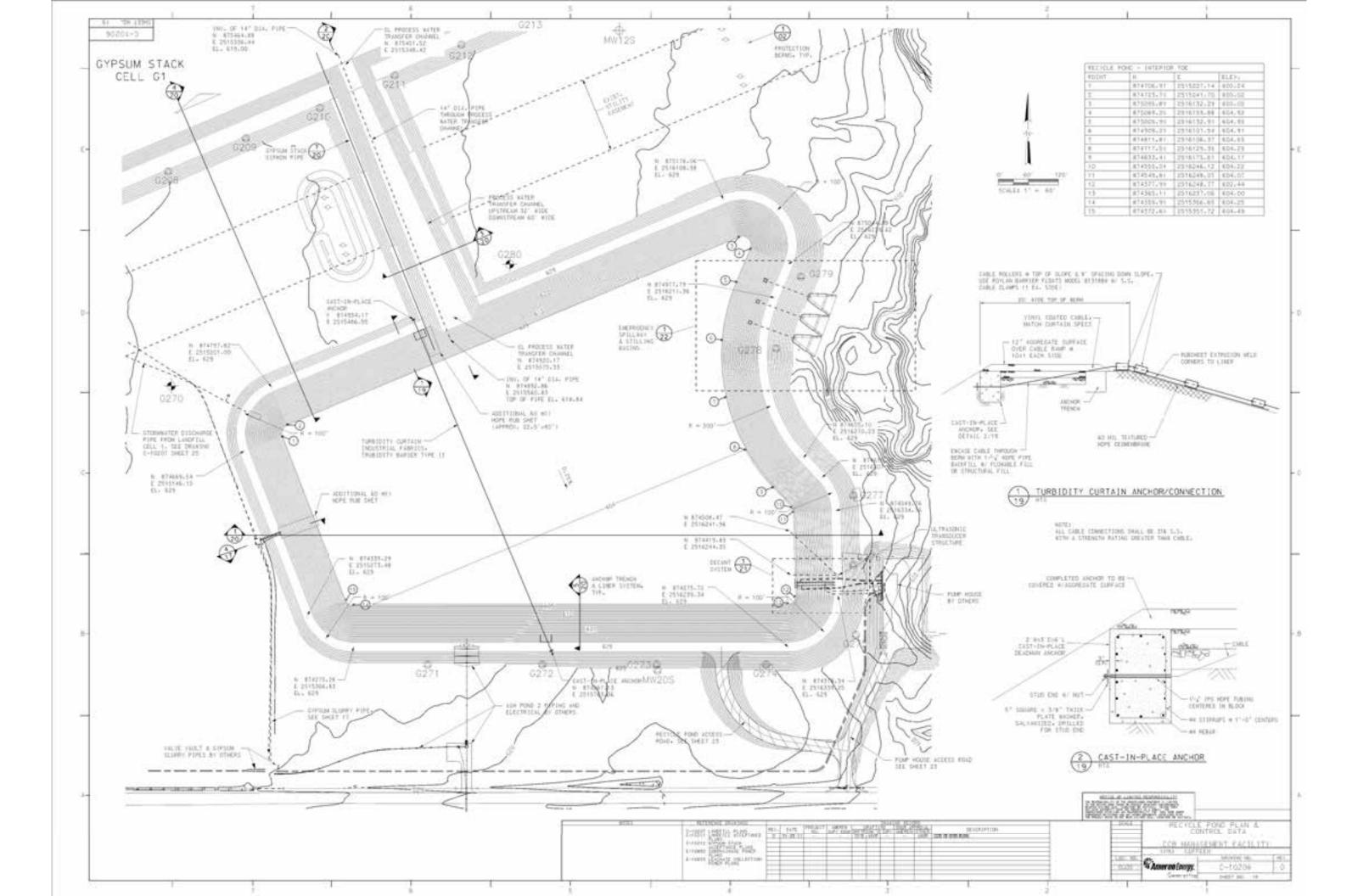


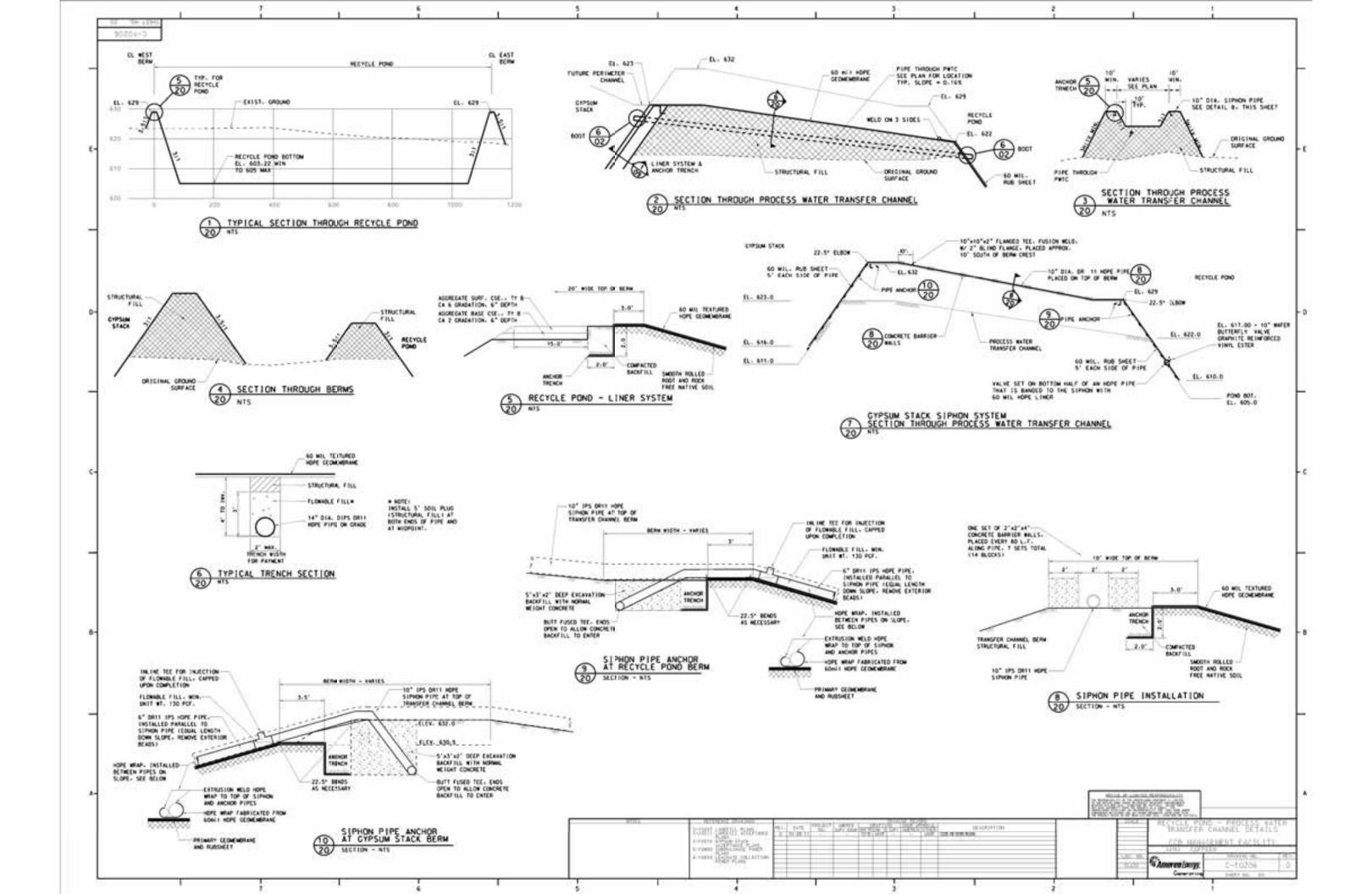


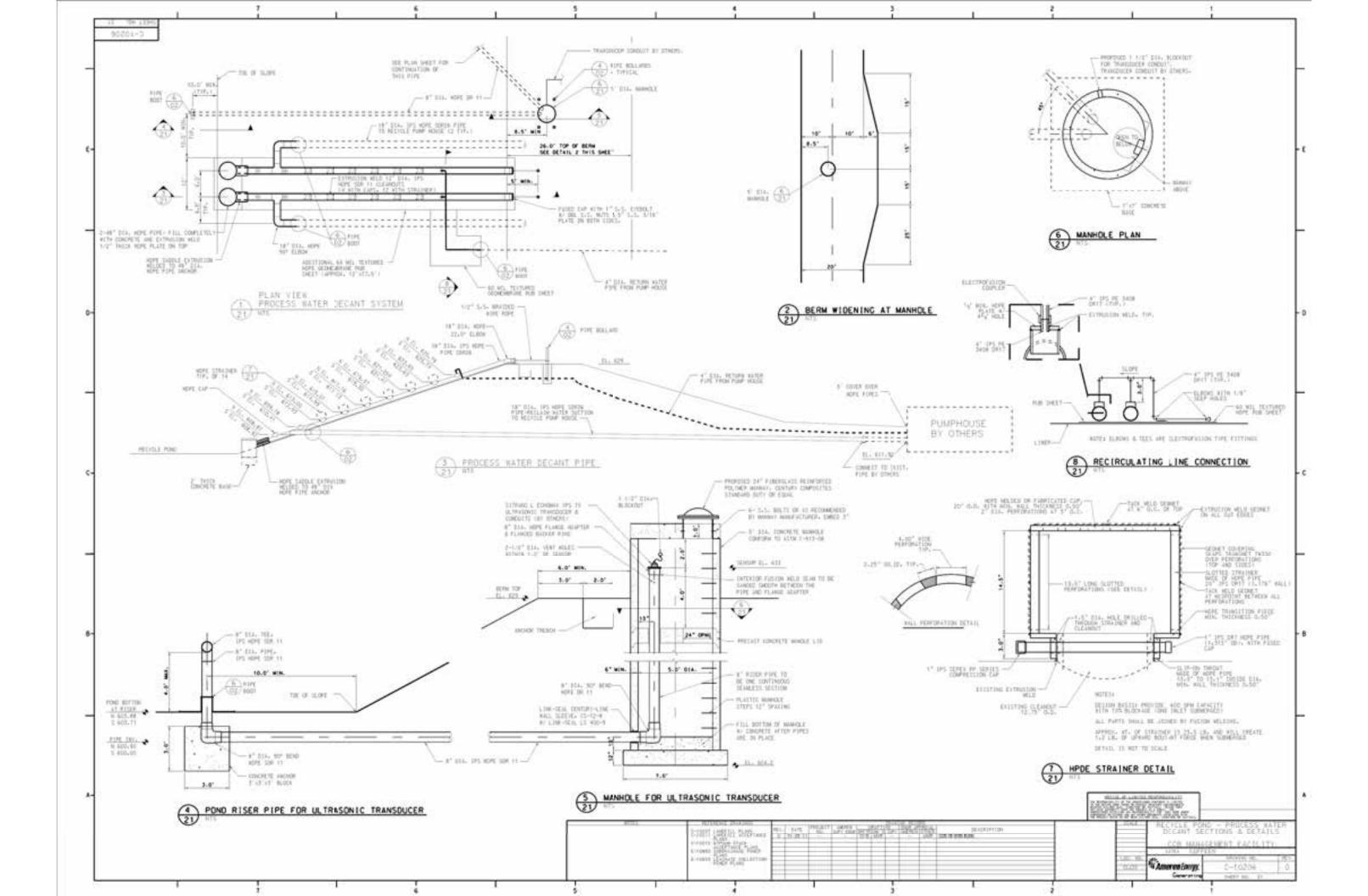


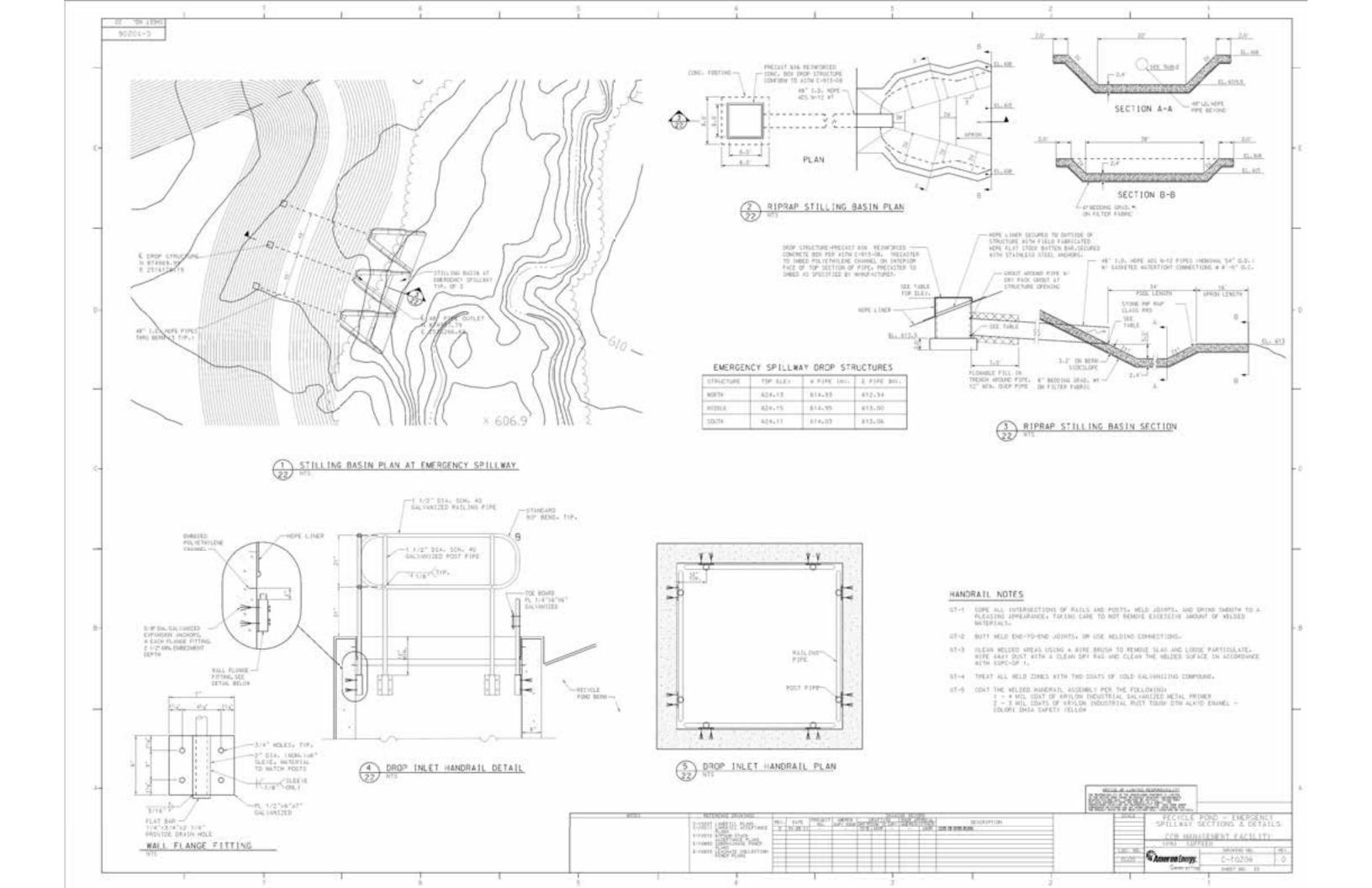


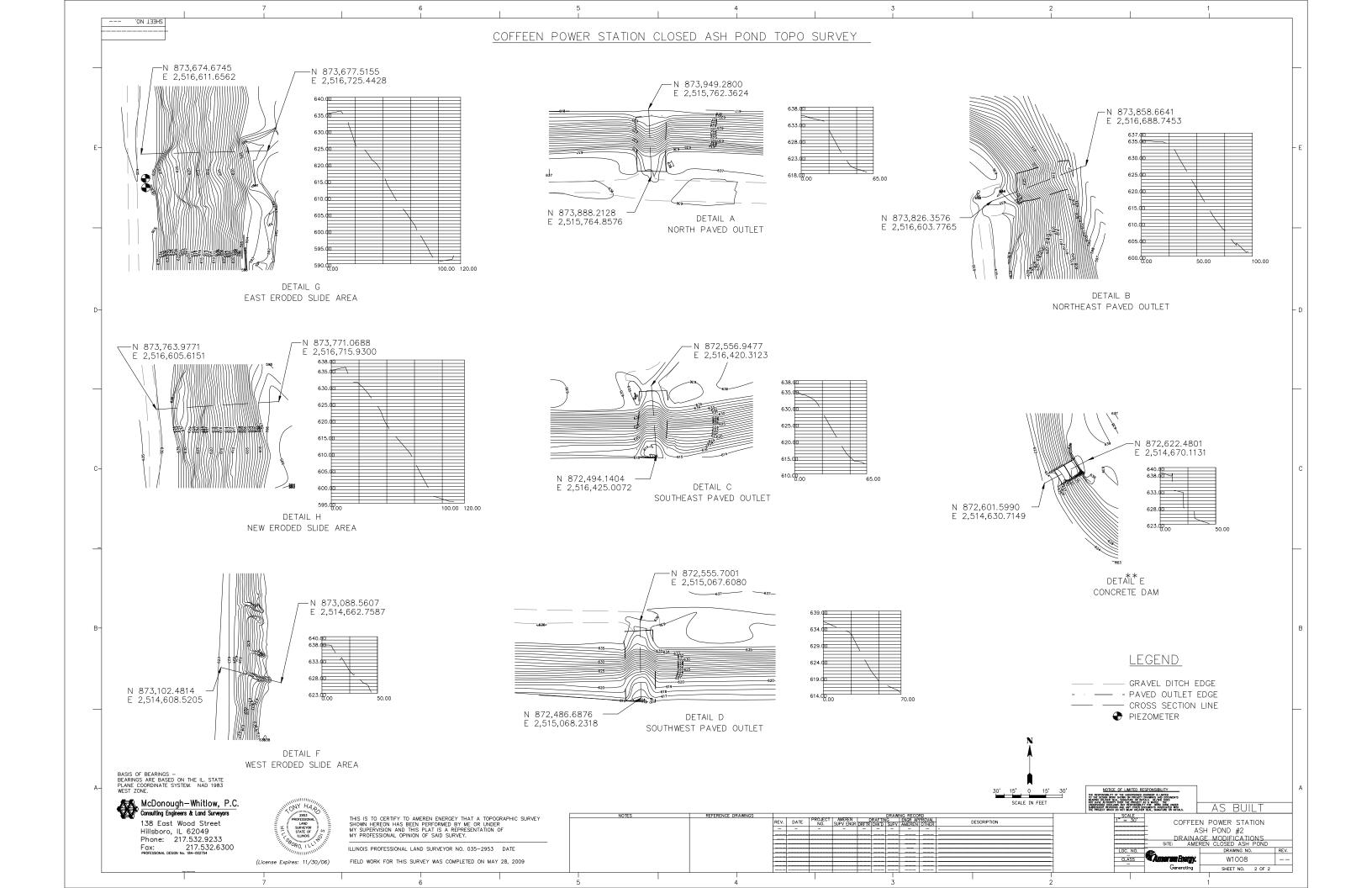






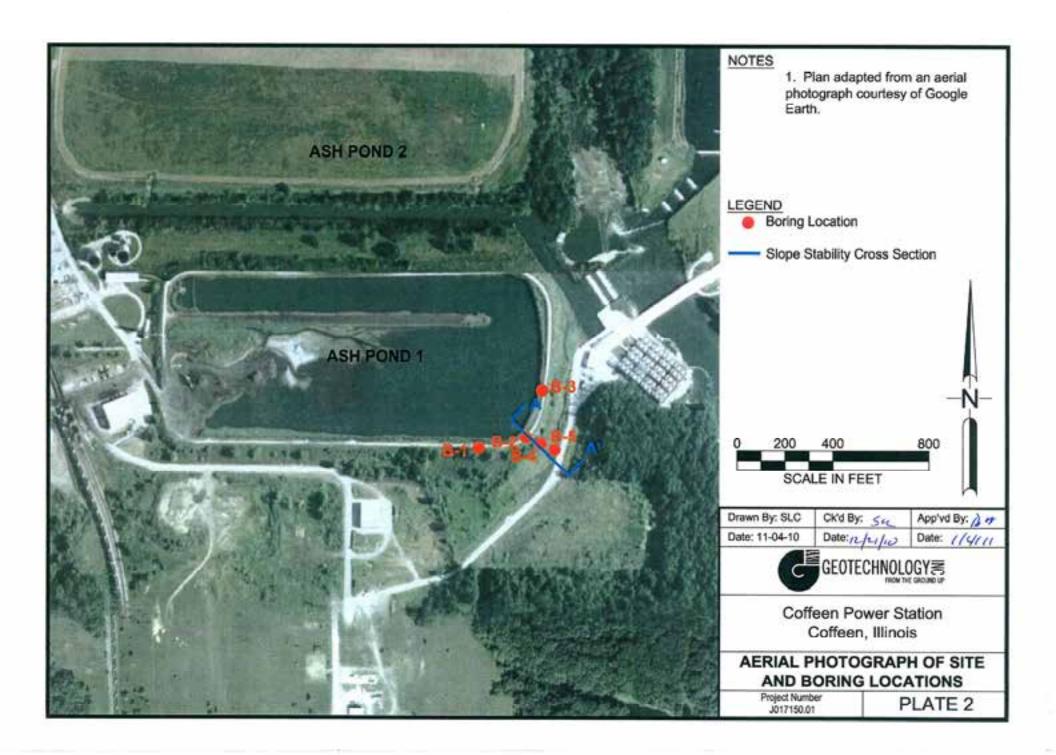


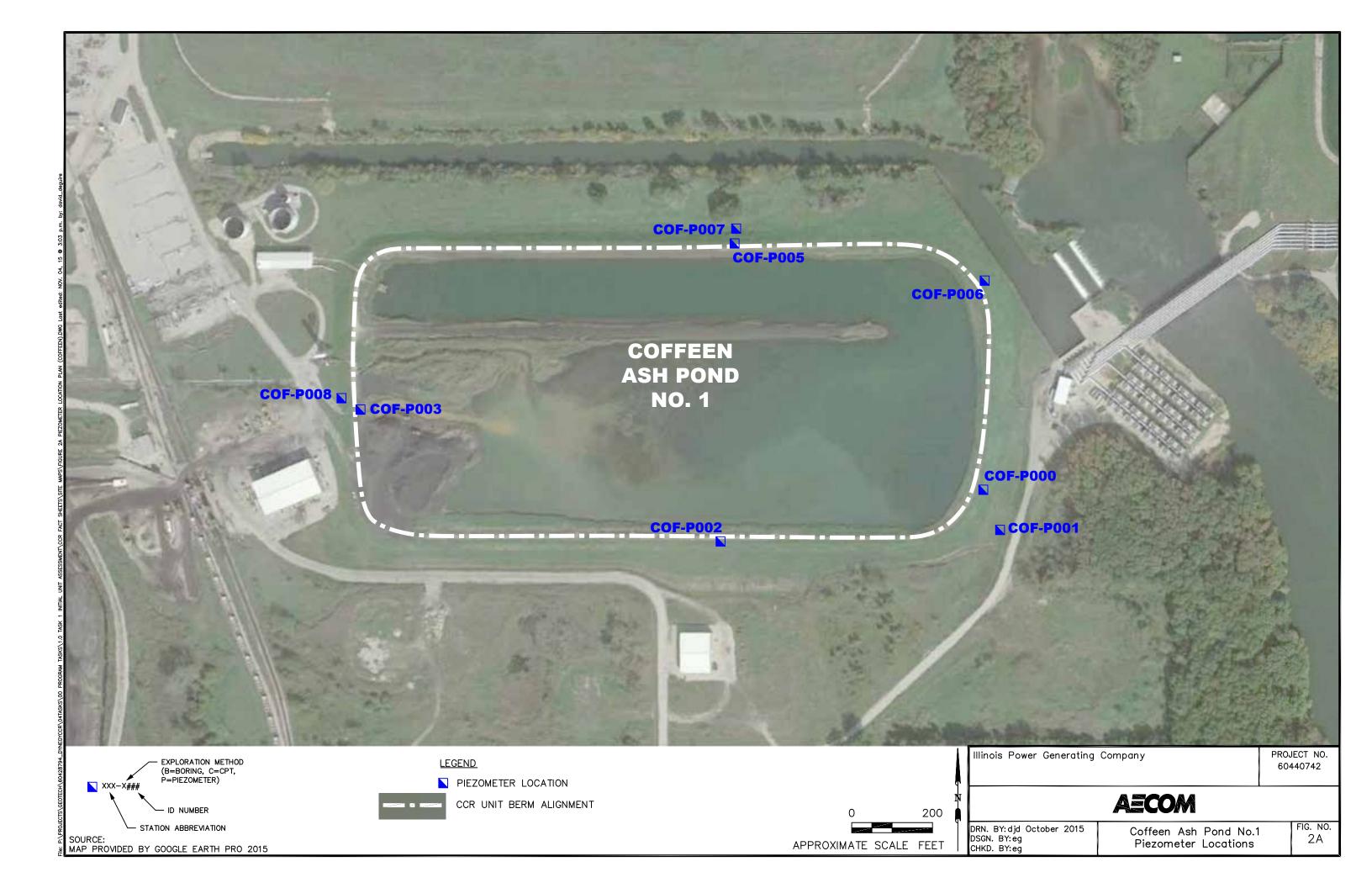


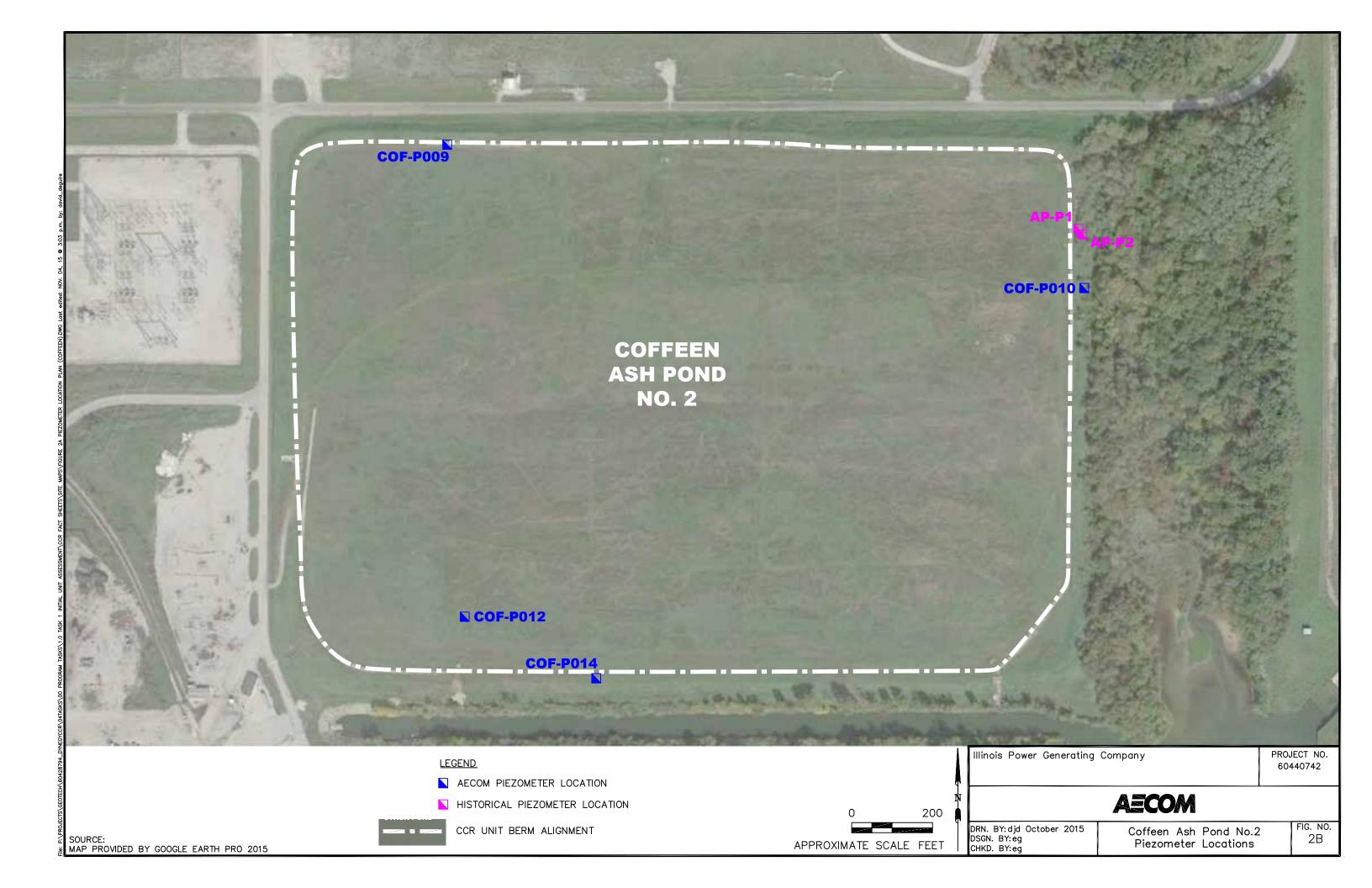




Appendix C: Coffeen Power Station Boring and Piezometer Locations









Appendix D: Project Specifications, Gypsum Stack and Recycle Pond Construction (Hanson 2008)

PROJECT SPECIFICATIONS GYPSUM STACK AND RECYCLE POND CONSTRUCTION GYPSUM MANAGEMENT FACILITY COFFEEN POWER STATION MONTGOMERY COUNTY, ILLINOIS

Prepared For:

AMEREN ENERGY GENERATING COMPANY

Prepared By:

HANSON PROFESSIONAL SERVICES INC. 1525 South Sixth Street Springfield, Illinois 62703

GYPSUM STACK AND RECYCLE POND CONSTRUCTION GYPSUM MANAGEMENT FACILITY COFFEEN POWER STATION MONTGOMERY COUNTY, ILLINOIS

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DIVISION1-GENERAL REQUIREMENTS

Section 01356 - Storm Water Pollution Prevention Measures

PART 1. GENERAL

1.01 DESCRIPTION

A. This section pertains to the construction and maintenance of temporary erosion control systems to control erosion and sediment damage to adjacent properties and water resources, and the removal of erosion control devices when they are no longer required.

1.02 RELATED SECTIONS

The following sections contain items which are related to the work in this section:

02936 - Topsoil, Seeding, and Mulching.

1.03 REFERENCES

The following reference, or cited portions thereof, governs the work:

Illinois Department of Transportation (IDOT): Standard Specifications for Road and Bridge Construction, adopted January 1, 2007.

1.04 SUBMITTALS

- Submittals shall follow the provisions of Section 01010.
- B. Preconstruction Submittals: A storm water best management practices (BMP) plan shall be submitted that includes the following items:
 - Site drawing showing anticipated locations of structural erosion controls, areas of disturbed soils, and drainage patterns;
 - Inspection and record-keeping procedures; and
 - Maintenance procedures for erosion controls.

PART 2. PRODUCTS

2.01 EROSION CONTROL SYSTEMS

Materials for erosion control systems shall be in accordance with Article 280.02 of the IDOT Standard Specifications.

PART 3. EXECUTION

3.01 EXAMINATION

The site shall be examined to determine the extent of work required.

3.02 PRECONSTRUCTION JOBSITE INSPECTION

- A. The person who shall be at the jobsite during construction and who shall be responsible for insuring that erosion control work is completed in a timely manner shall be identified at the preconstruction meeting.
- B. A jobsite inspection shall be conducted with the Owner's Representative to review and designate the locations and types of erosion protection to be placed. The inspection shall be scheduled at the preconstruction conference and carried out on the job site before beginning any work that will disturb existing drainage or potentially create erodible conditions.

3.03 CONSTRUCTION

- A. Temporary erosion control systems shall be constructed in accordance with IDOT Standard 280001 and Article 280.04 of the Standard Specifications and as directed by the Owner's Representative. Erosion control devices shall be in place and approved by the Owner's Representative prior to beginning other work.
- B. Incorporate permanent erosion control features into the project at the earliest practicable time to minimize the need for temporary erosion controls.

3.04 MAINTENANCE

- A. Temporary erosion control systems shall be maintained in accordance with Article 280.05 of the Standard Specifications, except that measurement and payment provisions shall not apply.
- B. Temporary erosion control systems for unprotected disturbed areas shall be cleaned of trapped sediment and repaired immediately prior to project close out.
- Temporary seeding shall be applied to all disturbed areas except the gypsum stack excavation and the future fill and topsoil stockpiles.

3.05 REMOVAL AND DISPOSAL

When the Owner's Representative deems that temporary erosion control systems are no longer needed, they shall be removed and properly disposed, and silt deposits shall be removed or regarded as directed by the Owner's Representative, and the area seeded. Non-biodegradable temporary erosion control materials shall be disposed of off site. Biodegradable erosion control devices may be disposed of in spoil areas designated by the Owner's Representative. All laws and regulations in disposing of the materials shall be obeyed.

END OF SECTION 01356

1:05jobs/05s3004A\Gypsum Stacking\IDNR Dum Safety Permit Application\Specs\S01356_Storm Water Pollution Prevention Measures.doc

PART 1. GENERAL

1.01 DESCRIPTION

This section pertains to stripping of topsoil and vegetation from areas of the site that are to be excavated.

1.02 RELATED SECTIONS

No related sections.

PART 2. PRODUCTS

No products used.

PART 3. EXECUTION

3.01 EXAMINATION

The Contractor shall examine the site to determine the extent of work required.

3.02 SITE PREPARATION - STRIPPING

- A. All vegetation and topsoil encountered within the Gypsum Stack grading limits shall be stripped. Topsoil shall be kept clean and free of all foreign material, and stored in separate stockpiles from vegetation and common excavations. Stockpiles shall be located as indicated on the drawings or as directed by the Owner's Representative.
- Payment for stripping shall be based upon removal of 24 inches of topsoil in areas that require stripping.

3.03 DISPOSAL

All materials resulting from site preparation operations shall be stockpiled in the designated spoil area. Contractor shall obey all laws and regulations when disposing of the materials.

END OF SECTION 02100

1:\03jobs\03s5010\Gypsum Stack\Specs\Gypsum Mangement Facility\Pre-Final Specs\S02100_Site Preparation.doc

PART 1. GENERAL

1.01 DESCRIPTION

This section pertains to excavation, fill, and backfill required for foundation preparation, construction of low-permeability soil layer, anchor trench construction, miscellaneous site grading and berm construction.

1.02 RELATED SECTIONS

- A. The following sections contain items which are related to the work in this section:
 - 01356 Storm Water Pollution Prevention Measures
 - 2. 02100 Site Preparation
 - 02373 Geotextiles
 - 4. 02936 Topsoil, Seeding, and Mulching

1.03 REFERENCES

The following references, or cited portions thereof, govern the work:

- Illinois Department of Transportation (IDOT): Standard Specifications for Road and Bridge Construction, adopted January 1, 2007
- Department of Sustainable Natural Resources, Soil Survey Standard Test Method, Unified Soil Classification System: Field Method (USCS).

1.04 MEASUREMENT AND PAYMENT

- A. The Contractor shall be responsible for estimating the extent of excavation and fill required to complete the work, including, but not limited to, excavation to required elevations; loading, transporting, placing, and compacting low permeability soil; excavation and backfill of anchor trench; and miscellaneous site grading and berm construction. The Contractor shall include the dollar amount associated with all earthwork in his Lump Sum Bid amount.
- B. Removal and replacement of unsuitable foundation material and subgrade stabilization measures directed by the Owner's Representative will be paid for on a time and material basis.

1.06 COORDINATION

Existing utilities or other plant facilities shall not be interrupted, except when permitted in writing by the Owner's Representative and then only after acceptable

temporary services have been provided. A minimum 48-hour notice shall be provided prior to proceeding with an approved temporary interruption.

1.08 SUBMITTALS

Materials Handling Plan.

A materials handling plan shall be submitted for construction and protection of the low permeability soil liner. The plan shall describe the following:

- Processing and placement of the low permeability soil type, model number, weight, and critical dimensions of equipment to be used for soil processing, compaction, scarification, and smooth rolling;
- Method of protecting low permeability soil from changes in moisture content and freezing after placement.

B. Construction Access Ramp Layout.

Layout drawings shall be submitted showing alignment, profile, and typical section of the construction access ramps from the haul road into the bottom of the Gypsum Management Facility excavation. The minimum width of the ramp shall be 50 ft., and the longitudinal grade shall not exceed 8 percent.

PART 2. PRODUCTS

2.01 MATERIALS

A. Earth Fill Material

Earth Fill Material shall consist of a mixture of clay, silt, sand, and gravel-sized particles obtained from previously constructed subsoil stockpiles. These materials can be used separately or mixed as required for best results. When placed, Embankment Material shall have a USCS classification of SM, ML, or CL and shall be uniform. This material shall be free of ice, snow, organic matter, rubbish, and debris. Coarse-grained particles shall be well dispersed to prevent the development of segregated pockets or zones with insufficient fine material to fill the interstices.

B. Soil Liners

The Soil Liner for the Gypsum Management Facility is considered a Clay Liner, and shall be soil classification CL, CL-ML, or CH. The material shall be free of roots, debris, organic or frozen material, and shall have a maximum clod size no greater than the length of the compactor foot for the compaction equipment proposed by the Contractor. When compacted, the material shall have a hydraulic conductivity of less than 1 x 10⁻⁴ cm/sec.

C. Soil Stabilizers and Moisture Conditioning Agents

Additives to accelerate drying or to improve stability and workability of soil shall not be permitted unless approved in writing by the Owner's Representative.

2.02 EQUIPMENT

A. Compaction Equipment

Tamping foot rollers

Compaction equipment shall consist of tamping foot rollers which have a minimum weight of 40,000 pounds. At least one tamping foot shall be provided for each 110 square in. of drum surface. The length of each tamping foot, measured from the outside surface of the drum, shall be at least 1 in. longer than the loose lift thickness.

Steel-Wheeled Rollers

Equipment used to produce a smooth compacted surface shall be a smooth, non-vibratory steel wheeled roller weighing not less than 1,000 lb. per lineal ft.

B. Scarification Equipment

Discs, rotor tillers, or other equipment used to scarify the surface shall be capable of uniformly disturbing the upper 6 in. of surface to provide good bonding between lifts.

C. Mixing and Spreading Equipment

Discs, harrows, and motor graders or other similar equipment shall be available at the site for use in spreading, mixing, and drying Compacted Subsoil Stockpile Material.

PART 3. EXECUTION

3.01 PREPARATION

A. Control of Work

Benchmarks, monuments, and other reference points shall be maintained throughout the work area.

B. Utility Location

Before starting excavation, the location and extent of underground utilities in the work area shall be established.

3.02 EXCAVATION

A. General

Excavation consists of removal and redistribution of material encountered when establishing required grade and subgrade elevations. The Contractor shall be responsible for dewatering, protection, shoring, and disposal of excavated materials as necessary to complete the excavation.

B. Procedures

Excavation may be accomplished by any method and by use of any equipment that is suitable to the work, except that blasting will not be permitted. Based on previous construction experience at the site, it is recommended that excavation to the foundation grade be completed as far in advance of low permeability soil placement as possible to allow the foundation surface to dry and form a "crust" capable of sustaining compactive effort.

C. Overexcavation

All excavation shall be performed to the lines and grades indicated on the plans. Any overexcavation or excess excavation not requested by the Owner's Representative shall be at the expense of the Contractor.

D. Disposal of Excavated Materials

Contractor shall utilize excavated material as stockpile materials for future use as specified in paragraph E.

E. Stockpile Requirements

- Excavated clay and silty clay materials are to be stockpiled in the shortterm subsoil stockpile area.
- Excess excavated materials are to be stockpile in the areas designated on the drawings.
- Materials not suitable for use as fill or backfill shall be disposed of onsite in the locations specified by the Owner's Representative.
- Spread fill material for use by others, topsoil, and low permeability soil are to be stockpiled in layers not to exceed 1 ft loose thickness.
- Tops of stockpiles are to be graded to ensure positive drainage. Side slopes for stockpiles shall be no steeper than 3H:1V.
- Perimeter ditches are to be excavated to intercept runoff flowing toward stockpile areas and to route it to outlet locations approved by the Owner's Representative.

3.03 SUBGRADE PREPARATION

- A. Areas to receive fill shall be proof rolled under the observation of the Owner's Representative. Soft, loose, weak, or wet materials shall be removed and replaced with compacted fill or stabilized with geotechnical fabric or geogrid as directed by the Owner's Representative. Joints, fractures, and moisture seeps shall be repaired, and local sand deposits, if present at foundation grade, shall be removed and backfilled with compacted fill material as directed by the Owner's Representative.
- B. The Owner's Representative may recommend additional drying time for soft, wet subgrade that has not been exposed long enough to permit "crust" formation. If approved by the Owner's Representative, the Contractor may install, at his own expense, geotechnical fabric or geogrid to stabilize the wet subgrade and expedite construction.
- C. No fill shall be placed until the subgrade has been examined and approved.

3.04 GENERAL FILL

A. Placement

- Unless otherwise indicated on the plans, all fill shall be composed of Earth Fill Material.
- 2. Fill materials used in embankment construction shall normally be placed in lanes parallel to the embankment axis and shall be placed in conformance with the lines, grades, and slopes as indicated on the plans. Placement of fill materials in lanes which are not parallel to the embankment will be allowed only where working room is too restricted for normal placement as determined by the Owner's Representative.
- 3. Fill shall be spread in approximately flat layers in such a manner as to obtain lifts of relatively uniform thickness without spaces between successively deposited loads. Segregation shall be prevented during placing and spreading. Hauling equipment shall be routed across the fill in such a way as to promote uniform compaction and to prevent the formation of ruts.
- 4. The maximum compacted thickness of each lift shall not exceed 8 in. where heavy compaction equipment will be used. The maximum compacted thickness shall not exceed 3 in. where power tampers or similar smaller equipment will be used. It may be necessary to reduce the thickness of lifts in order to obtain the required minimum density.
- Where compacted earth fill is to be placed against existing slopes, each lift shall be keyed into existing slope by removing existing slope material in steps as each new lift is placed.
- 6. The surface of the fill shall be kept reasonably smooth. The fill surface shall be sloped transverse to the axis of the embankments to allow drainage. If the compacted surface is, in the opinion of the Owner's Representative, too smooth or too dry to bond properly with the

succeeding lift, it shall be roughened by scarifying, light discing, or other acceptable means, and it shall be sprinkled before the succeeding lift is placed thereon. If the surface becomes rutted or uneven subsequent to compaction, it shall be flattened and leveled before placing the next lift. This extra work shall be at the Contractor's expense.

- 7. Fill operations shall be suspended during periods of extended wet weather. Upon resuming operations, all fill materials that are excessively wet or soft shall be reprocessed in place or removed and stockpiled for reprocessing. The removal of soft material shall be carried to such depth as is necessary to expose firm materials. Fill shall not be placed on frozen surfaces.
- 8. When filling operations at any section will be suspended for any period in excess of 12 hours or in wet weather, the surface of the fill shall be rolled smooth to seal it against excessive absorption of moisture and to facilitate runoff. Prior to resuming fill placement and compaction, the fill surface shall be scarified and/or disced and moisture conditioned as required.
- The Contractor will receive no additional compensation for any removal, reprocessing, stockpiling, recompaction, wasting, or similar operation related to suspensions or conditions due to weather or other causes unless caused by the Owner.
- Earth fill access ramps shall not be constructed within the limits of the compacted embankments without prior approval. When such ramps are approved, they shall be constructed of low permeability soil (in-board of the perimeter berm) or compacted fill (out-board of the perimeter berm).

B. Compaction -

- Fill materials shall be compacted to a dry density equal to or greater than the following:
 - The Gypsum Management Facility: 95 percent of the maximum dry density obtained from the Standard Proctor Test, ASTM D698.

In order to insure uniform coverage and to facilitate construction inspection and control, the compaction of each layer shall proceed in a systematic, orderly, and continuous manner. Rolling shall be parallel to the embankment axis, except where there is insufficient working room for such operations.

- The moisture content of all earth fill materials shall be as uniform as
 practicable throughout each lift. Fill shall be compacted at a moisture
 content that is no more than 2 percent below and no more than 2 percent
 above optimum moisture content.
- Moisture conditioning of fill materials shall be performed by discing, harrowing, plowing, blading, or other suitable means prior to excavation. Moisture conditioning where the fill is placed shall be limited to minor adjustments prior to compaction. Addition of moisture shall be by using a

- pressure spray bar mounted in front of or to one side of a water tanker so that water will not collect in the tracks of the truck.
- 4. Compaction of fill materials shall not commence if the moisture content is not within the specified limits. Any materials that are placed but not compacted prior to drying out or becoming too wet shall be removed and replaced or reprocessed at the Contractor's expense.
- No admixtures as drying agents or to improve the workability of the soil will be allowed.

3.05 SOIL LINERS

A. Sources

The Soil Liners for the Gypsum Management Facility shall be constructed from Soil Liner Material as described in paragraph 2.01(B) above.

B. Test Liner

A compacted low permeability soil test liner of the actual full scale liner shall be constructed in accordance with the following requirements:

- Test liner will be constructed from the same soil material sources, to the same design specifications, and with similar equipment and procedures as are proposed for the full scale liner.
- Test liner will be at least four times the width of the widest piece of equipment to be used.
- Test liner will be no less than 100 ft long to allow equipment to reach normal operating speed before reaching a central 40-ft test area.
- Test liner will be constructed with maximum 8-in. compacted lifts for a total liner thickness of 3 ft.
- Test liner will be tested by the Owner's Testing Consultant as described below for each of the following physical properties:
 - a. Multiple two-stage Boutwell permeameter tests will be used on the test liner to determine the hydraulic conductivity. The two-stage field hydraulic conductivity test is a falling head infiltration test conducted in a cased borehole, typically 4 in. in diameter. The test is cited in the U.S. EPA Technical Guidance Document: Quality Assurance and Quality Control for Waste Containment Facilities, September 1993 (EPA/600/R-93/182).
 - Undisturbed samples (Shelby tubes) will be tested in the laboratory for hydraulic conductivity to determine if there is a statistical correlation to the field testing results.
 - c. Other engineering parameters including, but not limited to, particle size analysis, liquid limits, plasticity, water content, and in-place density that are needed to evaluate the full scale liner will be determined.

 Additional test fills will be constructed for each new soil type or for each change in equipment or procedures.

C. Full Scale Liner Construction:

- Full scale liner construction shall not be commenced until the results of the in-place compaction testing and Boutwell permeameter tests on the test liner confirm that the construction procedures and specified compaction requirements produce a in-situ hydraulic conductivities as specified in Section 2.01(B) above.
- 2. The liner shall be constructed according to the placement and compaction requirements for general fill, except the material shall be compacted to a density of no less than 95 percent of maximum dry density at a moisture content between 100 percent and 105 percent of optimum. The same compaction procedures, such as number of passes, speed, and compaction equipment used on construction of the test liner shall be used. Grade stakes shall not be driven into the clay liner.
- The completed liner shall be smooth rolled to limit moisture loss and promote run-off of surface water. Moisture content shall be maintained within the specified range and erosion or other damage that occurs in the soil liner shall be repaired as directed by the Owner's Representative until the geosynthetic liner is placed.
- Repair of any rutting or other damage caused by the installation of the geosynthetic liner will be paid for on a time and material basis.
- 5. Voids created in the clay barrier layer during construction (including, but not limited to, penetrations for test samples, and other penetrations necessary for construction) shall be repaired by removing material that does not meet the requirements for low permeability soil, placing low permeability soil backfill, granular or pelletized bentonite, or a mixture of bentonite and low permeability soil in lifts no thicker than 2 in. and tamping each lift with a steel rod. Each lift shall be tamped a minimum of 25 times altering the location of the rod within the void for each blow. Other ruts and depressions in the surface of the lifts shall be scarified, filled, and then compacted to grade.

3.06 CUSHION DIRT

Cushion Dirt to be placed beneath the upper High Density Polyethylene (HDPE) Geomembrane is to be placed to the specifications for General Fill in Section 3.04 above, except fill materials for Cushion Dirt shall be compacted to a dry density equal to or greater than 90 percent of the maximum dry density obtained from the Standard Proctor Test, ASTM D698.

3.07 ANCHOR TRENCH CONSTRUCTION

A. Gypsum Management Facility

- A ledge at the bottom of the anchor trench elevation shall be excavated.
 Low permeability soil shall be placed and compacted on the ledge as shown on the anchor trench details in the plans.
- The anchor trench shall be excavated to the depth and width shown on the anchor trench details. The front edge of the trench shall be rounded to eliminate any sharp corners that could cause excessive stress to the geosynthetic liners. Loose soil shall be removed or compacted into the floor of the trench.
- 3. Subsequent to Geosynthetic Clay Liner (GCL), Bottom HDPE Geomembrane and Geotextile Cushion installation, it shall be verified that the liners cover the entire trench floor, but do not extend up the back of the trench wall. After the liner installation in the trench has been inspected and approved by the Owner's Representative, the trench shall be backfilled with 1 ft. of low permeability soil. The backfill shall be deposited and compacted according to the requirements for general fill in such a manner as to prevent damage to the GCL and liner materials.
- 4. Subsequent to installation of separation geotextile on top of drainage layer, it shall be verified that the fabric extends across the top of the initial 1 ft layer of trench backfill, but does not extend up the back of the trench wall. After the fabric installation in the trench has been inspected and approved by the Owner's Representative, the trench shall be backfilled with 1 ft of low permeability soil. The backfill shall be deposited and compacted according to the requirements for general fill in such a manner as to prevent damage to the geotextile fabric.
- 5. Subsequent to installation of the upper HDPE Geomembrane, verify that the liner extends across the top of the initial 1 ft layer of trench backfill, but does not extend up the back of the trench wall. After the liner installation in the trench has been inspected and approved by the Owner's Representative, backfill the remainder of the trench to the top of the low permeability soil layer. Deposit and compact the backfill according to the requirements for general fill in such a manner as to prevent damage to the HDPE Geomembrane.

3.08 TESTING

A. Construction Quality Assurance (CQA) compaction and permeability tests will be made by the Owner's Testing Consultant during the progress of the work as indicated in Appendix 2. The Contractor shall cooperate with the Testing Consultant and allow such tests to be performed. B. If tests indicate that an area of fill or low permeability soil liner does not meet the specified requirements, additional tests shall be performed to determine the extent of non-compliance. The Contractor shall moisture condition and recompact that area until a passing test result is obtained.

3.08 FINISH GRADING

All excavated and filled areas shall be fine graded and leveled to provide a smooth finish free of debris, foreign matter, objectionable stones, clods, lumps, pockets, or high spots, properly drained and true to indicated elevations. Finish grading shall be only near completion of work or when requested. Any portions of the berm damaged by construction shall be restored. The berm ditch shall be finished to design grade, and the ditch side slopes shaped and trimmed to provide a uniform ditch cross section.

3.09 CONSTRUCTION TOLERANCES

- A. The foundation grade and finished earthwork grades shall be no more than 0.4 ft below and not above plan grade.
- B. The minimum thickness of low permeability soil layer shall be 3 ft.

END OF SECTION 02200

1:05jobs/05s3004A)Gypsum Stacking/IDNR Dam Safety Permit Application/Specs/S02200 Earthwork.doc

PART 1. GENERAL

1.01 DESCRIPTION

This section pertains to the placement of riprap for erosion control.

1.02 RELATED SECTIONS

The following section contains items which are related to the work in this section:

02200 - Earthwork

1.03 REFERENCES

Specified references or cited portions thereof, current at date of bidding documents unless otherwise specified, govern the work.

A. Illinois Department of Transportation (IDOT): Standard Specifications for Road and Bridge Construction, adopted January 1, 2007.

1.04 SUBMITTALS

Product Data: Provide quarry name and material type prior to delivery.

PART 2. PRODUCTS

2.01 MATERIALS

- A. Stone Riprap and Bedding materials according to Article 1005.01 of the Illinois Standard Specifications for Road and Bridge Construction.
- B. Filter Fabric material for Stone Riprap according to Article 1080.03, with an AOS (Apparent Opening Size) as indicated on the plans.
- Supplier shall be listed on the current IDOT Approved Aggregate Source List.
- D. Gradation as indicated in the drawings. Quality shall be Class A.

PART 3. EXECUTION

3.01 CONSTRUCTION REQUIREMENTS

- A. Stone Riprap and Bedding shall be installed in accordance with Section 281 of the Illinois Standard Specifications for Road and Bridge Construction for the placement of Stone Riprap. Measurement and payment provisions of Section 281 shall not apply.
- Filter Fabric for Stone Riprap shall be installed in accordance with Section 282 of the Illinois Standard Specifications for Road and Bridge Construction.
- C. The Owner's Representative shall be allowed to visually inspect Riprap for compliance with specifications prior to placement.

END OF SECTION 02275

DIVISION 2 – SITEWORK Section 02315 – Granular Materials

PART 1. GENERAL

1.01 DESCRIPTION

A. Gypsum Management Facility

This section pertains to the following:

- Furnishing and placing granular drainage materials for the drainage layer and leachate collection system.
- Furnishing and placing coarse aggregate for encasement of the ring drain collection piping.
- Furnishing and installing materials for roadbed construction related to the Gypsum Management Facility access roads and the McKinley Road relocation.
- 4. Recycle Pond Drain.

1.02 RELATED SECTIONS

The following sections contain items which are related to the work in this section:

- 02300 Earthwork
- 02373 Geotextiles
- 02640 HDPE Piping

1.03 REFERENCES

The following references, or cited portions thereof, govern the work:

- Illinois Department of Transportation (IDOT): Standard Specifications for Road and Bridge Construction, adopted January 1, 2007.
- American Society for Testing and Materials (ASTM):
 - ASTM D 75 (2003) Practice for Sampling Aggregates.
 - ASTM D 422 (1963; R 2002) Test Method for Particle-Size Analysis of Soils.
 - ASTM D 2434 (1968, R 2000) Test Method for Permeability of Granular Soils (Constant Head).
 - d. ASTM D 3042 (2003) Test Method for Insoluble Residue in Carbonate Aggregates.
 - e. ASTM C 1260 (2005) Standard Test Method for Potential Alkali Reactivity of Aggregates (Mortar-Bar Method).

 American Geological Institute (AGI). Geoscience Handbook AGI Data Sheets, 4th Edition.

1.04 MEASUREMENT AND PAYMENT

The Contractor shall be responsible for estimating the extent of granular materials required to complete the work including, but not limited to, construction of drainage layer, encasement of leachate collection piping, and road-bed construction. The Contractor shall include the dollar amount associated with furnishing and placing all granular materials in his Lump Sum Bid amount.

1.05 COORDINATION

A. The geosynthetic liner shall be covered with granular materials as soon as practicable after a section of liner has been approved by the Owner's Representative.

1.06 SUBMITTALS

A. Product Data:

- Aggregate source list: Submit a list of proposed aggregate sources.
- Shipping Tickets: Submit shipping tickets for the granular materials delivered to the site. Shipping tickets shall be according to paragraph 1004.01f of the IDOT Standard Specifications.

B. Test Reports.

 Submit results of grain size analysis (ASTM D422) and hydraulic conductivity testing (ASTM D2434) for gradations established by the Contractor that provide the specified hydraulic conductivity. Test results are required for each proposed source and gradation. Submit test results for each source demonstrating compliance with reactivity, soundness, and abrasion requirements specified herein.

C. Samples:

 Submit one sample per source for each gradation proposed for use on the project. Samples shall be at least one pound and shall be obtained and shipped according to ASTM D75. Submit samples at least 15 days prior to starting construction of the drainage layer and coarse aggregate encasement for leachate piping.

1.07 STORAGE AND HANDLING

A. Storage and handling of granular materials shall be according to paragraph 1004.01e of the IDOT Standard Specifications.

1.01

PART 2. PRODUCTS

2.01 MATERIALS

A. General

- Unless otherwise approved by the Owner's Representative, granular materials shall be obtained from sources listed on the current IDOT Approved Aggregate Source List (www.dot.il.gov\materials\) approvedaggregatesources.pdf).
- Coarse Granular materials shall be meet the Description of Gravel, as described in Section 1004.01(a)(1) of the IDOT Standard Specifications, and shwll be spherical to sub-discoidal, sub-rounded to well rounded particles as defined by AGI Data Sheet, 4th Edition, Sheet 8.4 – Comparison Charts for Estimating Roundness and Sphericity.
- Granular materials shall experience no more than 15 percent carbonate loss per ASTM D3042.
- Granular materials shall be free of deleterious material, and shall meet the Na₂SO₄ soundness and Los Angeles Abrasion Specifications for Class B quality aggregate per paragraph 1004.01 of the IDOT Standard Specifications.
- All material shall pass the 2 in. sieve, and no greater than 5 percent shall be retained on the No. 200 sieve.
- Granular materials shall be innocuous to alkali-silica reactivity, and shall exhibit internal expansions of less than 0.10 percent at 16 days after casting as determined by ASTM C 1260.

B. Gypsum Management Facility Granular Materials

Granular Materials for Drainage Layer

Gradation for granular material for drainage layer shall be as required to provide a minimum hydraulic conductivity (ASTM D2434) of 1 x 10⁻³ cm/sec.

Coarse Aggregate around Ring Drain Collection Piping

Coarse Aggregate used to encase the ring drain collection piping shall be IDOT Gradation CA 7 material as outlined in Article 1004.01 of the IDOT Standard Specifications for Road and Bridge Construction.

Filter Sand

Filter Sand used for protective cover over the ring drain collection system shall be IDOT Gradation FA 1, Class B or better according to Article 1003 of the IDOT Standard Specifications for Road and Bridge Construction.

Aggregate Base Course, Type B

Aggregate Base Course, Type B used for base material for all new access roads and shall be IDOT Gradation CA 2, in accordance with Section 1004.04 of the IDOT Standard Specifications for Road and Bridge Construction. The material shall originate from an IDOT approved source. The Na₂SO₄ soundness and Los Angeles Abrasion Specifications for Class B quality aggregate per paragraph 1004.01 of the IDOT Standard Specifications shall not apply.

Aggregate Surface Course, Type B

Aggregate Surface Course, Type B used for surface material for all new access roads and the McKinley Road relocation shall be IDOT Gradation CA 6, in accordance with Section 1004.04 of the IDOT Standard Specifications for Road and Bridge Construction. The material shall originate from an IDOT approved source. The Na₂SO₄ soundness and Los Angeles Abrasion Specifications for Class B quality aggregate per paragraph 1004.01 of the IDOT Standard Specifications shall not apply.

2.02 EQUIPMENT

Equipment for spreading and compacting granular materials shall be low ground pressure equipment to prevent damage to the underlying geosynthetic liners.

PART 3. EXECUTION

3.01 PROTECTION OF GEOSYNTHETICS

- A. Protection of the geosynthetic liners is critically important. Approved geosynthetic liner shall be covered by granular material as soon as practicable. Granular material shall be placed to a minimum thickness of 1 ft before any heavy equipment or loaded trucks are allowed on the lined area.
- No equipment will be permitted directly on the geosynthetic liner.
- C. Any damage to the geosynthetic liner system shall be repaired, as directed by the Owner's Representative, at the expense of the Contractor.

3.02 GRANULAR DRAINAGE LAYER (GYPSUM MANAGEMENT FACILITY)

A. Placement on Cell Floor

- The granular material shall be back-dumped on the geotextile cushion fabric in a sequence of operations beginning at the perimeter of the liner on the cell floor.
- 2. Placement of material on the fabric shall be accomplished by spreading dumped material off of previously placed material with a bulldozer blade or endloader, in such a manner as to prevent tearing or shoving of the cloth. Dumping of material directly on the fabric will only be permitted to establish an initial working platform. No vehicles or construction equipment shall be allowed on the fabric prior to placement of the granular blanket to a minimum thickness of 1 ft.

B. Placement on Cell Side Slopes

- Placement of granular material on cell side slopes shall be accomplished using methods and equipment similar to that specified for placement of material on cell floor.
- The Contractor may place gypsum underlain with separation geotextile fabric to buttress the granular material on the slope:
 - The Construction Quality Assurance (CQA) survey to certify thickness of drainage material shall be completed within the footprint of the gypsum stack before gypsum placement.

b. Separation geotextile fabric shall extend beyond the toe of gypsum buttress a sufficient distance to prevent contamination of the granular drainage layer. See Sections 02373 and 02320 for construction of separation geotextile fabric and gypsum, respectively.

3.04 COARSE AGGREGATE FOR ENCASEMENT OF RING DRAIN COLLECTION PIPING (GYPSUM MANAGEMENT FACILITY)

- A. The geotextile filter fabric for encasement of leachate collection piping shall be placed on the approved cushion geotextile fabric according to Section 02373 – Geotextiles.
- B. The coarse aggregate shall be placed on the encasement fabric to the width shown on the plans to the level of the bottom of the ring drain collection piping.
- C. Course aggregate shall be placed and tamped along the pipe during pipe installation. The coarse aggregate shall be placed longitudinally along the pipe in lifts not to exceed 8 in. thick to a height of at least the center of the pipe. The aggregate shall be maintained at equal elevation on each side of the pipe, and the first lift of material shall be mechanically tamped to ensure that the space under the pipe is completely filled. The top of pipe shall not be covered until the CQA survey certifies leachate piping grade has been completed.
- C. After the CQA survey has been completed, coarse aggregate material shall continue to be placed in lifts not to exceed 8 in. thick, as specified in the previous paragraph until the minimum cover height shown in the plans is attained.
- D. The running of trucks or heavy equipment over leachate piping shall be avoided until there is at least a 12 in. cover of Filter Sand over the completed geotextile envelop. Temporary ramps no steeper than 10H:1V transverse to the piping shall be provided for temporary equipment crossings until the first lift of gypsum is placed.

3.03 ROADWAY CONSTRUCTION

- Prepare the roadway subgrade as shown on the plans, in accordance with Section 02200 – Earthwork.
- Furnish Geotechnical Fabric for Ground Stabilization in accordance with Section 02373 – Geotextiles.
- C. Furnish Aggregate Base Course, Type B in accordance with Article 351 of the IDOT Standard Specifications for Road and Bridge Construction.

D. Furnish Aggregate Surface Course, Type B in accordance with Article 402 of the IDOT Standard Specifications for Road and Bridge Construction.

3.04 TESTING

- A. CQA gradation and permeability tests will be made by the Owner's Testing Consultant during the progress of the work as indicated in Appendix 2. The Contractor shall cooperate with the Testing Consultant and allow such tests to be performed.
- B. If tests indicate that an area of granular material or coarse aggregate does not meet the specified requirements, then the Contractor shall remove the material and replace it with suitable material.

3.05 FINISH GRADING

The granular drainage layer shall be fine graded to provide a smooth finish before a CQA survey of the completed portion of the drainage layer is requested. Ruts or erosion damage shall be repaired before placement of the separation geotextile fabric.

3.06 CONSTRUCTION TOLERANCES

The minimum thickness of drainage layer shall be 1 ft.

END OF SECTION 02315

PART 1. GENERAL

1.01 DESCRIPTION

This section pertains to furnishing and installing geotextile fabrics on prepared surfaces.

1.02 RELATED SECTIONS

The following sections contain items which are related to the work in this section:

- 02300 Earthwork
- 02315 Granular Materials
- 02800 HDPE Geomembrane

1.03 REFERENCES

The following references, or cited portions thereof, govern the work:

- Illinois Department of Transportation (IDOT): Standard Specifications for Road and Bridge Construction, adopted January 1, 2007.
- American Society for Testing and Materials (ASTM):
 - a. ASTM 3776 (1996; R 2002) Standard Test Method for Mass per Unit Area (Weight) of Fabric;
 - ASTM D 3786 (2001) Test Method for Hydraulic Bursting Strength of Textile Fabrics – Diaphragm Bursting Strength Tester Method;
 - ASTM D 4533 (2004) Test Method for Trapezoid Tearing Strength of Geotextiles;
 - ASTM D 4632 (1991; R 2003) Test Method for Grab Breaking Load and Elongation of Geotextiles;
 - e. ASTM D 4751 (2004) Test Method for Determining Apparent Opening Size of Geotextile;
 - f. ASTM D 4833 (2000) Test Method for Index Puncture Resistance of Geotextiles, Geomembranes, and Related Products;
 - g. ASTM D 4873 (2002) Guide for Identification, Storage, and Handling of Geosynthetic Rolls;
 - ASTM D 4884 (1996; R 2003) Test Method for Strength of Sewn or Thermally Bonded Seams of Geotextiles;
 - ASTM D5261-92(2003) Standard Test Method for Measuring Mass per Unit Area of Geotextiles

ASTM D6241-04 Standard Test Method for the Static Puncture Strength of Geotextiles and Geotextile-Related Products Using a 50-mm Probe

1.04 MEASUREMENT AND PAYMENT

- A. The Contractor shall be responsible for estimating the extent of geotextile fabric required to complete the work including fabric for laps, anchorage, repairs, and samples for Construction Quality Assurance (CQA) testing. The Contractor shall include the dollar amount associated with all geotextile construction in his Lump Sum Bid amount, except as specified in paragraph B.
- B. Geotextile fabric for ground stabilization, when directed by the Owner's Representative, will be paid for on a time and materials basis.
- C. No additional payment will be made for geotextile fabric for ground stabilization installed at the Contractor's discretion.

1.05 SUBMITTALS

A. Product Data

- The manufacturer's list of guaranteed properties for each geotextile fabric or geogrid proposed for use on the project shall be submitted.
- The manufacturer's installation guidelines shall be submitted.

B. Samples

Samples of geotextile fabrics shall be submitted for CQA prequalification testing. Sample size and sampling frequency are specified in Appendix 2.

C. Inventory

A copy of the roll inventory that identifies, as a minimum, manufacturer or supplier, product or style number, roll number, width, and length of roll as identified on the roll label shall be submitted.

1.06 STORAGE AND HANDLING

Geotextiles shall be stored and handled according to ASTM D4873.

PART 2. PRODUCTS

2.01 MATERIALS

A. Geotextile Fabric for Liner System

Geotextile fabrics for use in the cell liner system shall consist of non-woven filaments of polypropylene, polyester, or polyethylene. Stabilizers and/or inhibitors shall be added to the base polymer if necessary to make the filaments resistant to deterioration caused by ultraviolet light and heat exposure. Reclaimed or recycled fibers or polymer shall not be added to the formulation. Non-woven fabric may be needle-punched, heat-bonded, or a combination thereof. The filaments shall be dimensionally stable (i.e., filaments shall maintain their relative position with respect to each other) and resistant to delamination. The edges of the geotextile shall be finished to prevent the outer fiber from pulling away from the geotextile. The filaments shall be free from any chemical treatment or coating that might significantly reduce porosity and permeability.

Fabric shall have the following physical properties:

Physical Properties ⁽¹⁾	4 oz. (Separation)	6 oz. (PWRS)	16 oz. (CA Envelope)
Mass/Unit Area (oz/yd²) ASTM D5261	4.0	6.0	16.0
Grab Tensile Strength (lb.) ASTM D4632	115	160	380
Grab Elongation (%) ASTM D4632	50	50	50
Puncture Strength (lb.) ASTM D4833	65	85	240
Puncture (CBR) Strength (lb.) ASTM D6241	310	410	1025
Mullen Burst Strength (psi) ASTM D3786	210	280	750
Trapezoidal Tear Strength (lb.) ASTM D4533	50	60	150
Width (ft.)	15	15	15
Apparent Opening Size (AOS) Max. US Std. Sieve No. ASTM D4751	70	70	100
UV Resistance ⁽²⁾ (%) ASTM D4355	70	70	70
Roll Width (ft.)	15	15	15

Notes:

All Values listed are Minimum Average Roll Values (MARV) unless otherwise noted, calculated as the typical minus two standard deviations.

⁽²⁾ UV Resistance is a minimum value and not a MARV. Evaluation to be on 2.0 inch strip tensile specimens after 500 hours exposure.

Cushion Geotextile Fabric.

Cushion geotextile fabric shall consist of non-woven filaments of polypropylene, polyester, or polyethylene. Stabilizers and/or inhibitors shall be added to the base polymer if necessary to make the filaments resistant to deterioration caused by ultraviolet light and heat exposure. Reclaimed or recycled fibers or polymer shall not be added to the formulation. Non-woven fabric may be needle-punched, heat-bonded, or a combination thereof. The filaments shall be dimensionally stable (i.e., filaments shall maintain their relative position with respect to each other) and resistant to delamination. The edges of the geotextile shall be finished to prevent the outer fiber from pulling away from the geotextile. The filaments shall be free from any chemical treatment or coating that might significantly reduce porosity and permeability.

Fabric shall have the following physical properties:

Physical Properties ⁽¹⁾	10 oz. (Cushion)	
Mass per unit area (oz/yd²) ASTM D5261		
Grab Tensile Strength (lb.) ASTM D4632	230	
Grab Tensile Elongation (%) ASTM D4632	50	
Trapezoidal Tear Strength (lb.) ASTM D4533	95	
Puncture (CBR) Strength (lb.) ASTM D6241	700	
Puncture (CBR) Elongation (in.) ASTM D6241	1.5	
UV Resistance ⁽²⁾ (%) ASTM D4355	70	
Apparent Opening Size (Max.) (AOS) Sieve No ASTM D4751	912 .8	
Roll Width (ft.)	15	

Notes:

B. Geotechnical Fabric for Ground Stabilization

Geotechnical fabric for ground stabilization shall conform to Article 1080.02 of the IDOT Standard Specifications for Road and Bridge Construction.

All Values listed are Minimum Average Roll Values (MARV) unless otherwise noted, calculated as the typical minus two standard deviations.

⁽²⁾ UV Resistance is a minimum value and not a MARV. Evaluation to be on 2.0 inch strip tensile specimena after 500 hours exposure.

C. Thread for Seams

High strength thread should be used such that seam test should conform to ASTM D4884. The thread shall meet the chemical, ultraviolet, and physical requirements of the geotextile, and the color shall be different from that of the geotextile.

D. Securing Devices

Pins, staples, and other devices that project through the geotextile fabric are not permitted for fabrics installed above the geomembrane. Sandbags, stone, or other appropriate means approved by the Owner's Representative shall be used to prevent movement of the geotextile.

2.02 EQUIPMENT

A. Equipment for spreading and compacting granular materials shall be low ground pressure equipment to prevent damage to the underlying geosynthetic liners.

PART 3. EXECUTION

3.01 SAMPLES FOR CQA TESTING

- Geotextile fabric samples shall be obtained, identified and packaged from rolls designated by the Owner's Representative according to ASTM D4873.
- Samples shall be 3 ft. wide by the full roll width.

3.02 BASE PREPARATION

- A. Surface on which the geotextile will be placed shall be prepared to a relatively smooth surface condition, and shall be free from obstruction, debris, depressions, erosion features, or any irregularities that would prevent continuous, intimate contact of the geotextile with the entire surface. Rills, gullies, and ruts must be graded out of the surface before geotextile placement. Areas on which geotextile are to be placed shall be graded and/or dressed in accordance with Section 02200 Earthwork and Section 02315 Granular Drainage Materials. Immediately prior to placing the geotextile, the prepared base will be inspected by the Owner's Representative, and no material shall be placed thereon until that area has been approved.
- B. Geotextile cushion fabric will be installed directly on the geosynthetic liner. Jointly inspect the liner with the Owner's Representative before commencing fabric installation each day. Notify the Owner's Representative promptly of any damage or defects observed in the liner as fabric installation progresses. Do not place fabric in the damaged or defective area until the liner has been repaired and

approved by the Owner's Representative. Submit a daily inspection report identifying the area of fabric placement and certifying that there were no visible defects in the area of fabric placement.

C. Do not run heavy vehicle traffic directly on the geosynthetic liner or cushion geotextile. Use vehicles and equipment as specified in paragraph 2.02 to transport and deploy fabric on the liner. Operate the equipment with care, and place protective cover over the geomembrane, if necessary, to avoid damaging the liner. Route traffic and personnel over installed cushion fabric and use the installed fabric as a working platform to the greatest extent possible.

3.03 INSTALLATION

A. General Requirements:

- Geotextile fabric shall be unrolled and laid out following these requirements to the greatest extent practical:
 - Orient panels with the longest dimension parallel to the slope.
 - Minimize the number of seams in corners and odd-shaped areas.
 - Extend panels on slopes a minimum of 5 ft onto a horizontal surface.

Geotextile panels shall be unrolled using methods that will not damage the fabric and will protect underlying surface from damage. While unrolling, the geotextile fabric shall be visually inspected for imperfections and faulty or suspect areas marked. Ballast shall be placed on fabric to prevent wind uplift. Expansion and contraction should be allowed for by leaving slack.

Heavy vehicle traffic shall not be run directly on geotextile fabric. Fabric in areas of heavy traffic shall be protected with protective cover over the fabric.

2. Laps

Individual panels of geotextile fabric shall be lapped according to manufacturer's instructions and as specified herein. Provide a minimum overlap of 3 in. unless otherwise specified herein or in the plans. Shingle overlaps so that water or other material cannot run down the slope between the two layers of fabric.

Field Seams

Continuously sew all laps on slopes steeper than 10H:1V. This requirement does not apply to the heavy geotextile fabric for envelopment of coarse aggregate around leachate piping.

Defects and Repairs

Examine the installed geotextile fabric for defects, holes discontinuous seams, puckered or separated laps, etc. Repair defective laps and seams. Patch holes and defects according to manufacturer's recommendations and as directed by the Owner's Representative. Do not cover suspect or patched areas until they have been inspected and approved by the Owner's Representative.

B. Geotextile Fabric for Separation

- Use low ground pressure equipment to avoid rutting the granular material.
- Horizontal seams (parallel to top of slope) will be permitted on cell side slopes to facilitate staged construction of the drainage layer on the side slope.
- Extend separation geotextile fabric into and across the bottom of the anchor trench and complete backfill of the trench according to Section 02200.

C. Geotextile Fabric for Coarse Aggregate Envelope

- Geotextile for coarse aggregate envelope will be installed directly on the cushion fabric. Remove any foreign materials from the cushion fabric within the footprint of the coarse aggregate leachate piping encasement before installing the geotextile envelope. Place sufficient width to completely envelop the coarse aggregate and provide a longitudinal lap of at least 6 in.
- After the coarse aggregate encasement has been completed, according to Section 02315, wrap the geotextile around the mounded aggregate, and cover the lap with at least 6 in. of material before permitting vehicle or equipment on the fabric.
- Any ballast material other than coarse aggregate, according to Section 02315, that is placed within the envelope will require removal during coarse aggregate construction.

D. Geotechnical Fabric for Ground Stabilization

 Install Geotechnical Fabric for Ground Stabilization in accordance with Section 210 of the IDOT Standard Specifications for Road and Bridge Construction.

- If approved by the Owner's Representative, the Contractor may, at his own expense, install geotextile or geogrid for ground stabilization outside the limits designated by the Owner's Representative.
- Submit as-built drawings that clearly delineate limits and type of ground stabilization.

3.04 PROTECTION

- A. Protect installed fabric until it is covered by at least 1 ft. of overlying material.
- B. Any damage to the geotextile during its installation or during placement of overlying materials shall be replaced by the Contractor at no cost to the Owner. Unless otherwise noted, the work shall be scheduled so that the covering of the geotextile with a layer of the specified material is accomplished within 14 calendar days after placement of the geotextile. Failure to comply shall require replacement of geotextile. The geotextile shall be protected from damage prior to and during the placement of overlying materials. Before placement of overlying materials, the Contractor shall demonstrate that the placement technique will not cause damage to the geotextile.

3.05 TESTING AND INSPECTION

A. Prequalification Testing

Geotextiles are subject to CQA testing by the Owner's Testing Consultant to verify conformance with the manufacturer's list of guaranteed properties according to Appendix 2. The Contractor shall provide samples as specified herein. If tests indicate nonconformance to the list of guaranteed properties, provide additional samples as directed by the Owner's Representative to determine the extent of the non-conformance. Any fabric that does not conform to the list of guaranteed properties shall be removed from the site.

- B. Installed fabric shall be inspected by the Owner's Representative. No material shall be placed on the fabric, other than ballast, until the installation has been approved by the Owner's Representative. Ballast shall not obscure seams or significant length of unseamed laps. The Owner's Representative may require removal of ballast to inspect suspect areas.
- C. If the Owner's Representative suspects that completed work has been damaged by construction methods that do not conform to the specifications, he may require removal of completed work to verify the integrity of the underlying materials. The Contractor shall bear the cost of removal and subsequent repair as directed by the Owner's Representative.

END OF SECTION 02373

PART 1. GENERAL

1.01 DESCRIPTION

- A. This section covers furnishing and installation of a reinforced needlepunched Geosynthetic Clay Liner (GCL) at the Gypsum Management Facility and the CCB Management Facility.
- B. The work includes furnishing all equipment and materials, providing all labor, supervision, administration and management necessary to perform the work as specified herein and as shown on the plans.

1.02 RELATED SECTIONS

None.

1.03 REFERENCES

The following references, or cited portions thereof, govern the work

- American Society for Testing and Materials (ASTM):
 - a. ASTM D 4632 (1991; R 2003), Standard Test Method for Grab Breaking Load and Elongation of Geotextiles;
 - ASTM D 4643 (2000), Test Method for Determination of Water (Moisture) Content of Soil by the Microwave Oven Method;
 - ASTM D 5084 (2003), Test Method for Measurement of Hydraulic Conductivity of Saturated Porous Materials Using a Flexible Wall Permeameter;
 - d. ASTM D 5261 (1992; R 2003), Test Method for Measuring Mass Per Unit Area of Geotextiles;
 - e. ASTM D 5321 (2002), Test Method for Determining the Coefficient of Soil and Geosynthetic or Geosynthetic and Geosynthetic Friction by the Direct Shear Method;
 - f. ASTM D 5887 (2004), Test Method for Measurement of Index Flux Through Saturated Geosynthetic Clay Liner Specimens Using a Flexible Wall Permeameter;
 - ASTM D 5888 (1995; R 2002), Practice for Storage and Handling of Geosynthetic Clay Liners;
 - ASTM D 5889 (1997; R 2003), Practice for Quality Control of Geosynthetic Clay Liners;

- ASTM D 5890 (2002), Test Method for Swell Index of Clay Mineral Component of Geosynthetic Clay Liners;
- ASTM D 5891 (2002), Test Method for Fluid Loss of Clay Component of Geosynthetic Clay Liners.

1.04 SUBMITTALS

- A. With the bid, the Contractor shall furnish the following information:
 - Conceptual description of the proposed plan for placement of the GCL panels over the areas of installation.
 - GCL Manufacturer's Quality Control (MQC) Plan for documenting compliance with Sections 2.01 and 2.02 of these specifications.
 - GCL manufacturer's historical data for reinforced GCL of a) 10,000-hour creep shear testing per Section 2.01 D, and b) seam flow data at 2 psi confining pressure per Section 2.01 E.
 - A copy of GCL manufacturer's International Standards Organization (ISO) Quality Certificate of Registration.
 - Statement of experience from the proposed GCL supplier.
 - Statement of experience from the proposed GCL installer.
- B. At the Owner Representative's or Owner's request, the Contractor shall furnish:
 - 1. A representative sample of the GCLs.
 - A project reference list for the GCL(s) consisting of the principal details of at least ten projects totaling at least 10 million sq. ft (100,000 sq. meters) in size.
- C. Upon shipment, the Contractor shall furnish:
 - The GCL manufacturer's Quality Assurance/Quality Control (QA/QC) certifications to verify that the materials supplied for the project are in accordance with the requirements of this specification.
 - Inventory of materials received.
- D. As installation proceeds, the Contractor shall submit certificates of subgrade acceptance, signed by the Contractor and Construction Quality Assurance (CQA) Inspector (see Sections 1.06 and 3.03) for each area that is covered by the GCL.
- E. Warranty

After construction, the contractor shall submit material and installation warranty certificates.

1.05 OUALIFICATIONS

- A. GCL Manufacturer must have produced at least 10 million sq. ft. (1 million sq. meters) of GCL, with at least 8 million sq. ft. (800,000 sq. meters) installed.
- B. The GCL Installer must either have installed at least 1 million sq. ft. (100,000 sq. meters) of GCL, or must provide to the Engineer satisfactory evidence, through similar experience in the installation of other types of geosynthetics, that the GCL will be installed in a competent, professional manner.

1.06 CONSTRUCTION QUALITY ASSURANCE (CQA)

- A. The Owner shall provide a third-party inspector for CQA of the GCL installation. The inspector shall be an individual or company who is independent from the manufacturer and installer and who shall be responsible for monitoring and documenting activities, related to the CQA of the GCL throughout installation.
- B. Testing of the GCL as necessary to support the CQA effort shall be performed by a third party laboratory retained by the Owner and independent from the GCL manufacturer and installer.

WARRANTY

The geomembrane material shall be warranted, on a pro-rata basis against manufacturer's defects for a period of five (5) years from the date of liner installation. The installation shall be warranted against defects in workmanship for a period of (1) year from the date of liner completion.

PART 2. PRODUCTS

2.01 MATERIALS

- A. Acceptable products for the GCL are GCL Bentomat® SDN, as manufactured by CETCO, 1350 West Shure Drive, Arlington Heights, Illinois 60004 USA (847-392-5800), or an engineer-approved reinforced needlepunched GCL material equal to Bentomat SDN.
- B. The delineation of areas to receive GCL shall be agreed by the Installer and the Engineer prior to installation.
- C. The GCL and its components shall have the properties shown in the GCL Certified Properties table at the end of this section.
- D. The reinforced GCL shall have 10,000 hour test data for large-scale constant-load (creep) shear testing for related products under hydrated conditions. The

- displacement shall be 0.13 in. (3.3 mm) or less at a constant shear load of 250 psf (12 kPa) and a normal load of 500 psf (24 kPa).
- E. The reinforced GCL shall have seam test data from an independent laboratory showing that the seam flow with a grooved cut in one of the nonwoven geotextiles is less than 1 x 10⁻⁸ m³/m²/s at 2 psi hydraulic pressure.
- F. The minimum acceptable dimensions of full-size GCL panels shall be 150 ft. (45.7 m) in length. Short rolls [(those manufactured to a length greater than 70 ft. (21 meters) but less than a full-length roll)] may be supplied at a rate no greater than three (3) per truckload or three (3) rolls every 36,000 sq. ft. (3,500 sq. meters) of GCL, whichever is less.
- G. A 6-inch (150 mm) overlap guideline shall be imprinted on both edges of the upper geotextile component of the GCL as a means for providing quality assurance of the overlap dimension. Lines shall be printed in easily visible, nontoxic ink.

2.02 PRODUCT QUALITY DOCUMENTATION

The GCL manufacturer shall provide the Contractor or other designated party with manufacturing QA/QC certifications for each shipment of GCL. The certifications shall be signed by a responsible party employed by the GCL manufacturer and shall include:

- A. Certificates of analysis for the bentonite clay used in GCL production demonstrating compliance with the swell index and fluid loss parameters shown in the GCL Certified Properties tables.
- B. Manufacturer's test data for finished GCL product(s) of bentonite mass/area, GCL tensile strength and GCL peel strength (reinforced only) demonstrating compliance with the index parameters shown in the GCL Certified Properties tables.
- GCL lot and roll numbers supplied for the project (with corresponding shipping information).

2.03 PRODUCT LABELING

- A. Prior to shipment, the GCL manufacturer shall label each roll, identifying:
 - Product identification information (Manufacturer's name and address, brand product code).
 - Lot number and roll number.
 - Roll length, width and weight.

2.04 PACKAGING

- A. The GCL shall be wound around a rigid core whose diameter is sufficient to facilitate handling. The core is not necessarily intended to support the roll for lifting but should be sufficiently strong to prevent collapse during transit.
- All rolls shall be labeled and bagged in packaging that is resistant to photodegradation by ultraviolet (UV) light.

2.05 ACCESSORY BENTONITE

A. The granular bentonite sealing clay used for overlap seaming, penetration sealing and repairs shall be made from the same natural sodium bentonite as used in the GCL and shall be as recommended by the GCL manufacturer. Seaming of GCLs shall be conducted in accordance with the manufacturer's guidelines for each particular GCL. Please refer to the installation guidelines for Bentomat /Claymax GCLs.

PART 3. EXECUTION

3.01 SHIPPING AND HANDLING

- A. The rolls of GCL shall be packaged and shipped by appropriate means to prevent damage to the material and to facilitate off-loading.
- B. The Installation Supervisor shall be present during delivery and unloading of the GCL. A visual inspection of each roll should be made during unloading to identify if any packaging has been damaged. Rolls with damaged packaging should be marked and set aside for further inspection. The packaging should be repaired prior to being placed in storage. The Installation Supervisor shall prepare and submit an inventory that includes lot and roll number for materials received.
- C. The Installer is responsible for unloading the GCL. The Owner will make available equipment and operators employed at the site to assist with unloading. The Installer shall coordinate with the Owner to determine equipment availability and should contact the Manufacturer prior to shipment to ascertain the appropriateness of the proposed unloading methods and equipment.

3.02 STORAGE

A. Storage of the GCL rolls shall be the responsibility of the Installer. A dedicated storage area shall be provided by the Owner at the job site. Submit storage area requirements (size and preferred location) with bid documents.

- B. Rolls should be stored in a manner that prevents sliding or rolling from the stacks and may be accomplished by the use of chock blocks. Rolls should be stacked at a height no higher than that at which the lifting apparatus can be safely handled (typically no higher than four).
- C. All stored GCL materials and the accessory bentonite must be covered with a plastic sheet or tarpaulin until their installation.
- The integrity and legibility of the labels shall be preserved during storage.

3.03 EARTHWORK

- A. The low permeability soil layer upon which the GCL is installed shall be prepared and compacted prior to installation. The surface shall be smooth, firm, and unyielding, and free of:
 - Vegetation.
 - Construction debris.
 - Sticks.
 - Sharp rocks.
 - Void spaces.
 - Ice.
 - Abrupt elevation changes.
 - Standing water.
 - Cracks larger than 0.25 in. (6 mm) in width.
 - 10. Any other foreign matter that could contact the GCL.
- B. Immediately prior to GCL deployment, the low permeability soil layer shall be final-graded by the contractor to fill in all voids or cracks and then smooth-rolled to provide the best practicable surface for the GCL. At completion of this activity, no wheel ruts, footprints or other irregularities shall exist in the subgrade. Furthermore, all protrusions extending more than 0.5 in. (12 mm) from the surface shall either be removed, crushed or pushed into the surface with a smoothdrum compactor.
- On a continuing basis, the project CQA inspector shall certify acceptance of the subgrade before GCL placement.
- D. It shall be the Installer's responsibility thereafter to indicate to the Owner's Representative any change in the condition of the low permeability soil layer that could cause the subgrade to be out of compliance with any of the requirements listed in this Section. The Installation Supervisor shall certify in the daily report that no GCL was placed over visibly defective low permeability soil surface.
- E. At the top of sloped areas of the job site, an anchor trench for the GCL shall be excavated by the contractor in accordance with the project plans. The trench shall

be excavated and approved by the CQA Inspector prior to GCL placement. No loose soil shall be allowed at the bottom of the trench and no sharp corners or protrusions shall exist anywhere within the trench.

3.04 GCL PLACEMENT

- A. GCL rolls shall be delivered to the working area of the site in their original packaging. Immediately prior to deployment, the packaging shall be carefully removed without damaging the GCL. The orientation of the GCL (i.e., which side faces up) shall be in accordance with the Owner Representative's recommendations.
- B. Equipment which could damage the GCL, shall not be allowed to travel directly on it. If the installation equipment causes rutting of the subgrade, the subgrade must be restored to its originally accepted condition before placement continues.
- C. Care must be taken to minimize the extent to which the GCL is dragged across the subgrade in order to avoid damage to the bottom surface of the GCL. A temporary geosynthetic subgrade covering commonly known as a slip sheet or rub sheet may be used to reduce friction damage during placement.
- D. The GCL panels shall be placed parallel to the direction of the slope.
- E. All GCL panels shall lie flat on the underlying surface, with no wrinkles or folds, especially at the exposed edges of the panels.
- F. Only as much GCL shall be deployed as can be covered at the end of the working day with soil, a geomembrane, or a temporary waterproof tarpaulin. The GCL shall not be left uncovered overnight. If the GCL is hydrated when no confining stress is present, the Installer shall remove and replace the hydrated material as directed by the Owner Representative.

3.05 ANCHORAGE

A. As directed by the project drawings and specifications, the end of the GCL roll shall be placed in an anchor trench at the top of the slope. The front edge of the trench shall be rounded so as to eliminate any sharp corners. Loose soil shall be removed from the floor of the trench. The GCL shall cover the entire trench floor, but shall not extend up the rear trench wall.

3.06 SEAMING

A. The GCL seams shall be constructed by overlapping their adjacent edges according to the manufacturer's recommendations. Care should be taken to ensure that the overlap zone is not contaminated with loose soil or other debris.

- B. The minimum dimension of the longitudinal overlap should be 6 in. (150 mm) for Bentomat SDN. If the GCL is manufactured with a grooved cut in the nonwoven geotextile that allows bentonite to freely extrude into the longitudinal overlap then no bentonite-enhanced seam is required for this overlap. If the GCL does not have a grooved cut in one of the nonwoven geotextiles in the longitudinal overlap, then bentonite-enhanced seams are required as described below.
- C. End-of-roll overlapped seams shall be constructed with a minimum overlap of 24 in. (600 mm) for Bentomat SDN. Seams at the ends of the panels should be constructed such that they are shingled in the direction of the grade to prevent the potential for runoff flow to enter the overlap zone. End-of-roll overlapped seams for all reinforced GCL seams require bentonite-enhanced seams as described below.
- D. Bentonite-enhanced seams shall be constructed between the overlapping adjacent panels as follows. The underlying edge of the longitudinal overlap is exposed and then a continuous bead of granular sodium bentonite is applied along a zone defined by the edge of the underlying panel and the 6-inch (150 mm) line. The granular bentonite shall be applied at a minimum application rate of one quarter pound per lineal ft. (0.4 kg/m). A similar bead of granular sodium bentonite is applied at the end-of-roll overlap.

3.07 DETAIL WORK

- There shall be no penetrations through the GCL.
- B. Cutting the GCL should be performed using a sharp utility knife. Frequent blade changes are recommended to avoid damage to the geotextile components of the GCL during the cutting process.

3.08 DAMAGE REPAIR

A. If the GCL is damaged (torn, punctured, perforated, etc.) during installation, it may be possible, if approved by the Owner's Representative, to repair it by cutting a patch to fit over the damaged area. The patch shall be obtained from a new GCL roll and shall be cut to size such that a minimum overlap of 12 in. (300 mm) is achieved around all of the damaged area. Granular bentonite or bentonite mastic shall be applied around the damaged area prior to placement of the patch. It may be desirable to use an adhesive to affix the patch in place so that it is not displaced during cover placement. Patching shall be observed and approved by the Owner's Representative.

GCL CERTIFIED PROPERTIES

MATERIAL PROPERTY	TEST METHOD	TEST FREQUENCY ft ² (m ²)	REQUIRED VALUES		
Bentonite Swell Index ¹	ASTM D 5890	1 per 50 tonnes	24 mL/2g min.		
Bentonite Fluid Loss ¹	ASTM D 5891	1 per 50 tonnes	18 mL max.		
Bentonite Mass/Area ²	ASTM D 5993	40,000 ft ² (4,000 m ²)	0.75 lb/ft ² (3.6 kg/m ²) min		
GCL Grab Strength ³	ASTM D 6768	200,000 ft ² (20,000 m ²)	30 lbs/in (53 N/cm) MARV		
GCL Peel Strength ³	ASTM D 6496	40,000 ft ² (4,000 m ²)	2.5 lbs/in (4.4 N/cm) min		
GCL Index Flux ⁴	ASTM D 5887	Weekly	$1 \times 10^{-8} \text{m}^3/\text{m}^2/\text{sec max}$		
GCL Hydraulic Conductivity ⁴	ASTM D 5887	Weekly	5 x 10 ⁻⁹ cm/sec max		
GCL Hydrated Internal Shear Strength ⁵	ASTM D 5321 ASTM D 6243	Periodic	500 psf (24 kPa) typ @ 200 psf		

Notes

³ Bentonite mass/area reported at 0 percent moisture content.

Peak values measured at 200 psf (10 kPa) normal stress for a specimen hydrated for 48 hours. Site-specific materials, GCL products, and test conditions must be used to verify internal and interface strength of the proposed design.

END OF SECTION 02376

Bentonite property tests performed at a bentonite processing facility before shipment the manufacturer's production facilities.

³ All tensile strength testing is performed in the machine direction using ASTM D 6768. All peel strength testing is performed using ASTM D 6496. Upon request, tensile and peel results can be reported per modified ASTM D 4632 using 4 inch grips.

^{*} Index flux and permeability testing with deaired distilled/deionized water at 80 psi (551kPa) cell pressure, 77 psi (531 kPa) headwater pressure and 75 psi (517 kPa) tailwater pressure. Reported value is equivalent to 925 gal/scre/day. This flux value is equivalent to a permeability of 5x10° cm/sec for typical GCL thickness. Actual flux values vary with field condition pressures. The last 20 weekly values prior the end of the production date of the supplied GCL may be provided.

PART 1. GENERAL

1.01 DESCRIPTION

This section pertains to construction of the HDPE (High Density Polyethylene) Piping at the Gypsum Management Facility and the CCB Management Facility.

1.02 RELATED SECTIONS

None.

1.03 REFERENCES

The following references, or cited portions thereof, govern the work

A. American Society of Testing and Materials:

- ASTM D 2683 (2004); Specification for Socket-Type Polyethylene Fittings for Outside Diameter-Controlled Polyethylene Pipe and Tubing.
- ASTM D 3261 (2003); Specification for Butt Heat Fusion Polyethylene (PE) Plastic Fittings for Polyethylene (PE) Plastic Pipe and Tubing.
- ASTM D 3350 (2005); Specification for Polyethylene Plastics Pipe and Fittings Materials.
- ASTM F 412 (2001a); Terminology Relating to Plastic Piping System.
- ASTM F 1055 (1998); Specification for Electrofusion Type Polyethylene Fittings for Outside Diameter Controlled Polyethylene Pipe and Tubing.
- ASTM F 1056 (2004); Specification for Socket Fusion Tools for Use in Socket Fusion Joining Polyethylene Pipe or Tubing and Fittings.

1.04 SUBMITTALS

A. Qualifications

Submit qualifications of the Welding Supervisor who will be responsible for construction quality control of the pipe joining process.

B. Material Certifications

Submit manufacturer certifications that the pipe provided complies with the requirements herein.

C. Product Data

- Submit product data and operating instructions for pipe joining equipment.
- Submit pipe manufacturer's recommended procedures for storing, handling, and installing pipe and fittings.

1.05 QUALIFICATIONS

A. The Contractor or Subcontractor performing the work under this section shall have in his employ a Welding Supervisor who has completed a minimum of 1,000 ft of pipe joining work using the type of equipment proposed for use in this work. The Welding Supervisor shall be on site at all times during pipe line installation, and shall provide direct supervision over other employees.

1.06 WARRANTY

A. The pipe and fittings shall be warranted, on a pro-rata basis, against manufacturer's defects for a period of five (5) years from the date of pipe installation. The installation shall be warranted against defects in workmanship for a period of one (1) year from the date of completion of the leachate collection piping system.

PART 2. PRODUCTS

2.01 MATERIALS

A. Pipe

- Pipe material shall be High Density Polyethylene (HDPE) PE 3408, according to ASTM F412, with a cell class designation of 345464C, according to ASTM D3350. Iron pipe size (IPS) and standard dimension ratio (SDR) shall be as indicated in the plans.
- Size and spacing of holes in perforated pipe shall be as indicated in the plans.

B. Fittings

 Fittings shall be made of the same material, and shall have a pressure rating no less than 160 psi. Butt fusion, socket, or electrofusion fittings, according to ASTM D3261, ASTM D2683, and ASTM F1055, respectively, are acceptable.

2.02 EQUIPMENT

A. Butt Fusion Machine

The butt fusion machine shall include the following features:

- Facer with rotating planer block design.
- Heater faces coated by the manufacturer to prevent molten plastic from adhering to the heater face.
- Hydraulic-operated jaws suitable for use with the pipe sizes indicated in the plans.

B. Socket Fusion Equipment

Socket fusion heating tools and depth gauges shall be of the same manufacturer, unless they are all marked F1056, indicating compliance with ASTM F1056.

C. All equipment shall conform to any requirements specified in the pipe and socket manufacturer's installation instructions, and shall be approved by the Owner's Representative.

PART 3. EXECUTION

3.01 MATERIAL DELIVERY, STORAGE, AND HANDLING

- A. HDPE pipe and fittings shall be packaged and shipped by appropriate means to prevent damage to the material and to facilitate off-loading. The Owner will provide an on-site storage site. Storage site requirements (size and preferred location) shall be submitted with the bid documents.
- Storage and handling shall be according to manufacturer's recommendations.

3.02 BASE PREPARATION

All HDPE piping shall be installed on a layer of coarse aggregate placed by the Contractor in accordance with the plans. The grade of the coarse aggregate base shall be verified before installing the piping.

3.03 INSTALLATION

All pipe and fittings shall be installed according to the manufacturer's recommendations. Removal of weld beads is not required. Contractor shall place coarse aggregate along the pipe to provide lateral stability. Welds shall not be obscured until they have been approved by the Owner's Representative, the top of pipe shall not be covered until the Construction Quality Assurance (CQA) survey has been completed to verify conformance with specified tolerances.

3.04 INSPECTIONS

- A. The Owner's Representative shall shall be visually inspect pipe materials to verify that each pipe material is properly stamped (by the manufacturer) for ASTM acceptance before installation. Defective or damaged materials shall be removed from the site.
- B. Each weld and connection shall be visually inspected by the Owner's Representative. Defective welds shall be repaired as directed by the Owner's Representative and according to manufacturer's recommendations. Welds and connections shall not be covered until they have been approved by the Owner's Representative.

3.05 TOLERANCES

A. HDPE piping shall be located within 0.5 ft. of plan location, and elevation shall be within 0.1 ft. of plan elevation with no adverse slopes.

END OF SECTION 02936

PART 1. GENERAL

1.01 DESCRIPTION

- A. This section includes manufacturing, furnishing, and installing High Density Polyethylene (HDPE) Geomembranes for the Gypsum Management Facility and the Gypsum Management Facility Recycle Pond.
- B. The work includes furnishing all equipment and materials and providing all labor, supervision, administration and management necessary to perform the work as shown on the plans.

1.02 RELATED SECTIONS

- A. The following sections contain items which are related to the work in this section:
 - 02373 Geotextiles
 - 02376 Geosynthetic Clay Liner

1.03 REFERENCES

- A. The following references, or cited portions thereof, govern the work:
 - American Society for Testing and Materials (ASTM):
 - a. D 638, Standard Test Method for Tensile Properties of Plastics.
 - D 751, Standard Test Methods for Coated Fabrics.
 - D 792, Standard Test Methods for Density and Specific Gravity (Relative Density) of Plastics by Displacement.
 - D 1004, Standard Test Method for Initial Tear Resistance of Plastic Film and Sheeting.
 - D 1204, Standard Test Method for Linear Dimensional Changes of Non Rigid Thermoplastic Sheeting or Film at Elevated Temperature.
 - D 1238, Standard Test Method for Flow Rates of Thermoplastics by Extrusion Plastometer.
 - D 1505, Standard Test Method for Density of Plastics by Density-Gradient Technique.
 - D 1603, Standard Test Method for Carbon Black in Olefin Plastics.
 - D 3895, Test Method for Oxidative Induction Time of Polyolefins by Thermal Analysis.
 - D 4218, Test Method for Determination of Carbon Black Content in Polyethylene Compounds by the Muffle-Furnace Technique.

02800-1

- D 4437, Standard Practice for Determining the Integrity of Field Seams Used in Joining Flexible Polymeric Sheet Geomembranes.
- D 4833, Test Method for Index Puncture Resistance of Geotextiles, Geomembranes and Related Products.
- m. D 5199, Standard Test Method for Measuring Nominal Thickness of Smooth Geomembranes.
- D 5397, Standard Test Method for Evaluation of Stress Crack Resistance of Polyolefins using Notched Constant Tensile Load Test.
- D 5596, Standard Practice for Microscopical Examination of Pigment Dispersion in Plastic Compounds.
- D 5641, Standard Practice for Geomembrane Seam Evaluation by Vacuum Chamber.
- q. D 5721, Practice for Air-Oven Aging of Polyolefin Geomembranes.
- D 5820, Test Method for Air Testing.
- D 5885, Test Method for Oxidative Induction Time of Polyolefin Geosynthetics by High Pressure Differential Scanning Calorimetry.
- D 5994, Standard Test Method for Measuring Nominal Thickness of Textured Geomembranes
- D 6365, Standard Practice for the Nondestructive Testing of Geomembrane Seams using The Spark Test

Geosynthetic Research Institute (GRI):

- a. GRI GM 6, Pressurized Air Channel Test for Dual Seamed Geomembranes
- GRI GM 9, Cold Weather Seaming of Geomembranes
- GRI GM 10, Specification for Stress Crack Resistance of HDPE Geomembrane Sheet
- d. GRI GM 13, Test Properties, Testing Frequency and Recommended Warranty for High Density Polyethylene (HDPE) Smooth and Textured Geomembranes
- e. GRI GM 14, Test Frequencies for Destructive SeamTesting

1.04 SUBMITTALS

- A. Submit the following to the Engineer or Owner, for review and approval, within a reasonable time so as to expedite shipment or installation of the Geomembrane:
 - Documentation of manufacturer's qualifications as specified in subsection 1.05A of this Section.
 - Manufacturer's Quality Control program manual or descriptive documentation.
 - A material properties sheet, including at a minimum all properties specified in GRI GM 13, including test methods used.
 - Sample of the material.

- Documentation of Installer's qualifications, as specified below and in subsection 1.05B of this Section.
 - a. Submit a list of at least ten completed facilities. For each name and type of facility; its location; the date of installation; number of contact at the facility; type and thickness of geomembrane and; surface area of the installed geomembrane.
 - Submit resumes or qualifications of the Installation Supervisor,
 Master Seamer and Technicians to be assigned to this project.
 - c. Quality Control Program.
- Example Material Warranty and Liner Installation Warranty complying with subsections 1.07 and 1.08 of this Section.
- Resin Supplier's name, resin production plant identification, resin brand name and number, production date of the resin, resin Manufacturer's quality control certificates, and certification that the properties of the resin meet the requirements

B. Shop Drawings

- Submit copies of shop drawings for engineer's approval within a
 reasonable time so as not to delay the start of geomembrane installation.
 Shop drawings shall show the proposed panel layout identifying seams
 and details. Seams should generally follow direction of the slope. Butt
 seams or roll-end seams should not occur on a slope unless approved by
 the Owner's Representative. Butt seams on a slope, if allowed, should be
 staggered.
- Placement of geomembrane will not be allowed to proceed until Owner's Representative has received and approved the shop drawings.

C. Additional Submittals (In-Progress and at Completion)

- Manufacturer's warranty (refer to subsection 1.08).
- Geomembrane installation warranty (refer to subsection 1.09).
- Daily written acceptance of subgrade surface (refer to subsection 3.01.C).
- Low-temperature seaming procedures if applicable (refer to subsection 3.03.A)
- Prequalification test seam samples (refer to subsection 3.05.A.6).
- Field seam non-destructive test results (refer to subsection 3.05.B.1).
- Field seam destructive test results (refer to subsection 3.05.C.6).
- Daily field installation reports (refer to subsection 3.05.G).
- Installation record drawing, as discussed in subsection 3.05.G).

1.05 QUALITY CONTOL

A. Manufacturer's Qualifications:

The manufacturer of geomembrane of the type specified or similar product shall have at least five years experience in the manufacture of such geomembrane. In addition, the geomembrane manufacturer shall have manufactured at least 10,000,000 sq. ft. of the specified type of geomembrane or similar product during the last five years.

B. Installer's Qualifications:

- The Geomembrane Installer shall be the Manufacturer, approved Manufacturer's Installer or a contractor approved by the Owner's Representative to install the geomembrane.
- The Geomembrane Installer shall have at least three years experience in the installation of the specified geomembrane or similar. The Geomembrane Installer shall have installed at least 10 projects involving a total of 5,000,000 sq. ft. of the specified type of geomembrane or similar during the last three years.
- Installation shall be performed under the direction of a field Installation Supervisor who shall be responsible throughout the geomembrane installation, for geomembrane panel layout, seaming, patching, testing, repairs, and all other activities of the Geomembrane Installer. The Field Installation Supervisor shall have installed or supervised the installation and seaming of a minimum of 10 projects involving a total of 5,000,000 sq. ft. of geomembrane of the type specified or similar product.
- Seaming shall be performed under the direction of a Master Seamer (who may also be the Field Installation Supervisor or Crew Foreman) who has seamed a minimum of 3,000,000 sq. ft. of geomembrane of the type specified or similar product, using the same type of seaming apparatus to be used in the current project. The Field Installation Supervisor and/or Master Seamer shall be present whenever seaming, patching, other welding operations, and testing is performed.
- 5 All seaming, patching, other welding operations, and testing shall be performed by qualified technicians employed by the Geomembrane Installer.

1.06 DELIVERY, STORAGE AND HANDLING

- A. Each roll of geomembrane delivered to the site shall be labeled by the manufacturer. The label shall be firmly affixed and shall clearly state the manufacturer's name, product identification, material thickness, roll number, roll dimensions and roll weight.
- Geomembrane shall be protected from mud, dirt, dust, puncture, cutting or any other damaging or deleterious conditions.
- C. Rolls shall be stored away from high traffic areas. Continuously and uniformly support rolls on a smooth, level prepared surface.
- Rolls shall not be stacked more than three high.

1.07 PROJECT CONDITIONS

Geomembrane shall not be installed in the presence of standing water, while precipitation is occurring, during excessive winds, or when material temperatures are outside the limits specified in Section 3.03.

1.08 MATERIAL WARRANTY

As required by specification, or as required in GRI GM 13 (attachment A)

1.09 GEOMEMBRANE INSTALLATION WARRANTY

The Geomembrane Installer shall guarantee the geomembrane installation against defects in the installation and workmanship for 1 year commencing with the date of final acceptance.

1.10 GEOMEMBRANE PRE-CONSTRUCTION MEETING

- A. Geomembrane Pre-Construction Meeting shall be held at the site prior to installation of the geomembrane. At a minimum, the meeting shall be attended by the Geomembrane Installer, Owner, Owner's representative (Engineer and/or CQA Firm), and the General Contractor.
- B. Topics for this meeting shall include:
 - Responsibilities of each party.
 - Lines of authority and communication. Resolution of any project document ambiguity.
 - Methods for documenting, reporting and distributing documents and reports.
 - Procedures for packaging and storing archive samples.
 - Review of time schedule for all installation and testing.
 - Review of panel layout and numbering systems for panels and seams including details for marking on geomembrane.
 - Procedures and responsibilities for preparation and submission of as-built panel and seam drawings.
 - Temperature and weather limitations. Installation procedures for adverse weather conditions. Defining acceptable subgrade, geomembrane, or ambient moisture and temperature conditions for working during liner installation.
 - Subgrade conditions, dewatering responsibilities and subgrade maintenance plan.
 - Deployment techniques including allowable subgrade for the geomembrane.
 - Plan for controlling expansion/contraction and wrinkling of the geomembrane.
 - Covering of the geomembrane and cover soil placement.
 - Measurement and payment schedules.
 - Health and safety.
- C. The meeting shall be documented by the Owner's Representative and minutes shall be transmitted to all parties.

PART 2. PRODUCTS

2.01 SOURCE QUALITY CONTROL

Manufacturing Quality Control

- A. The test methods and frequencies used by the manufacturer for quality control/quality assurance of the above geomembrane prior to delivery, shall be in accordance with GRI GM 13, or modified as required for project specific conditions.
- B. The manufacturer's geomembrane quality control certifications, including results of quality control testing of the products, as specified in subsection 2.01.C of this Section, must be supplied to the Owner's Representative. The certification shall be signed by a responsible party employed by the manufacturer, such as the QA/QC Manager, Production Manager, or Technical Services Manager. Certifications shall include lot and roll numbers and corresponding shipping information.
- C. The Manufacturer will provide Certification that the geomembrane and welding rod supplied for the project have the same base resin and material properties.

2.02 GEOMEMBRANE

- A. The geomembrane shall consist of new, first quality products designed and manufactured specifically for the purpose of this work which shall have been satisfactorily demonstrated by prior testing to be suitable and durable for such purposes. The geomembrane rolls shall be seamless, high density polyethylene (HDPE- Density >0.94g/cm) containing no plasticizers, fillers or extenders and shall be free of holes, blisters or contaminants, and leak free verified by 100% in line spark or equivalent testing. The geomembrane shall be supplied as a continuous sheet with no factory seams in rolls. The geomembrane will meet the property requirements as shown in Table A. (GRI GM 13)
- Material shall be reviewed for conformance to the project specifications by the Owner's Representative
- The geomembrane seams shall meet the property requirements as shown in Table 2, (Attachment B).

PART 3. EXECUTION

3.01 SUBGRADE PREPARATION

Geomembrane installed over geosynthetic clay liner (GCL).

The area of GCL to be covered with geomembrane shall be jointly inspected daily with the Owner's Representative before commencing geomembrane installation for the day, and the condition of the GCL shall be continuously observed as geomembrane installation progresses. Rocks, stones, sticks, sharp objects and debris of any kind shall be removed from the surface of the GCL. The Owner's Representative shall be notified of any discontinuities, premature hydration, or

otherwise defective GCL. Geomembrane shall not be placed over suspect areas until they have been repaired to the satisfaction of the Owner's Representative. The Installation Supervisor shall certify daily in writing that the GCL surface was acceptable at the time of geomembrane installation.

B. Geomembrane installed over cushion dirt.

The area of cushion dirt to be covered with geomembrane shall be prepared in accordance with the Section 02200 – Earthwork. The surface shall be smooth and free of ruts and holes, rocks, stones, sticks, sharp objects and debris of any kind.

- C. The Geomembrane installer shall provide daily written acceptance for the surface to be covered by the geomembrane in that day's operations. The surface shall be maintained in a manner, during geomembrane installation, to ensure subgrade suitability.
- D. All subgrade damaged by construction equipment and deemed unsuitable by the Owner's Representative for geomembrane deployment shall be repaired prior to placement of the geomembrane. All repairs shall be reviewed by the Owner's Representative and approved by the Geomembrane Installer. This damage, repair, and the responsibilities of the contractor and Geomembrane Installer shall be defined in the preconstruction meeting.

3.02 GEOMEMBRANE PLACEMENT

- A. No geomembrane shall be deployed until the applicable certifications and quality control certificates listed in subsection 1.04 of this Section are submitted to and approved by the Owner's Representative. Should geomembrane material be deployed prior to approval by the Owner's Representative it will be at the sole risk of the Geomembrane Installer and/or Contractor. If the material does not meet project specifications it shall be removed from the work area at no cost to the owner.
- B. The geomembrane shall be installed to the limits shown on the project drawings and essentially as shown on approved panel layout drawings.
- C. No geomembrane material shall be unrolled and deployed if the material temperatures are lower than 0 degrees C (32 degrees F). Temperature limitations should be defined in the preconstruction meeting. Typically, only the quantity of geomembrane that will be anchored and seamed together in one day should be deployed.
- No vehicular traffic shall travel on the geomembrane other than an approved low ground pressure All Terrain Vehicle or equivalent.
- E. Sand bags or equivalent ballast shall be used as necessary to temporarily hold the geomembrane material in position under the foreseeable and reasonably - expected wind conditions. Sand bag material shall be sufficiently close- knit to prevent soil fines from working through the bags and discharging on the geomembrane.
- F. Geomembrane placement shall not be done if moisture prevents proper subgrade preparation, panel placement, or panel seaming. Moisture limitations

- should be defined in the preconstruction meeting.
- G. Damaged panels or portions of the damaged panels which have been rejected shall be marked and their removal from the work area recorded.
- H. The geomembrane shall not be allowed to "bridge over" voids or low areas in the subgrade. In these areas, the subgrade shall be prepared to allow the geomembrane to rest in intimate contact with the subgrade.
- Wrinkles caused by panel placement or thermal expansion should be minimized in accordance with section 1.10 B. 11.
- J. Considerations on Site Geometry: In general, seams shall be oriented parallel to the line of the maximum slope. In corners and odd shaped geometric locations, the total length of field seams shall be minimized. Seams shall not be located at low points in the subgrade.
- K. Overlapping: The panels shall be overlapped prior to seaming to whatever extent is necessary to effect a good weld and allow for proper testing. In no case shall this overlap be less than 75mm (3 in.).

3.03 SEAMING PROCEDURES

- Cold weather installations should follow guidelines as outlined in GRI GM9.
- No geomembrane material shall be seamed when liner temperatures are less than 0 degrees C (32 degrees F).
- C. No geomembrane material shall be seamed when the sheet temperature is above 75 degrees C (170 degrees F) as measured by an infrared thermometer or surface thermocouple.
- D. Seaming shall primarily be performed using automatic fusion welding equipment and techniques. Extrusion welding shall be used where fusion welding is not possible such as at pipe penetrations, patches, repairs and short (less than a roll width) runs of seams.
- E. Fishmouths or excessive wrinkles at the seam overlaps, shall be minimized and when necessary cut along the ridge of the wrinkles back into the panel so as to effect a flat overlap. The cut shall be terminated with a keyhole cut (nominal 10 mm (1/2 in) diameter hole) so as to minimize crack/tear propagation. The overlay shall subsequently be seamed. The key hole cut shall be patched with an oval or round patch of the same base geomembrane material extending a minimum of 150 mm (6 in.) beyond the cut in all directions.

3.04 PIPE AND STRUCTURE PENETRATION SEALING SYSTEM

- Provide penetration sealing system as shown in the Project Drawings.
- B. Penetrations shall be constructed from the base geomembrane material, flat stock, prefabricated boots and accessories as shown on the Project Drawings. The prefabricated or field fabricated assembly shall be field welded to the geomembrane as shown on the Project Drawings so as to prevent leakage. This assembly shall be tested as outlined in section 3.05.B. Alternatively, where field non destructive testing can not be performed, attachments will be field spark tested by standard holiday leak detectors in accordance with ASTM 6365 Spark testing should be done in areas where both air pressure testing and vacuum testing are not possible.
 - Equipment for Spark testing shall be comprised of but not limited to: A 02800-8

- hand held holiday spark tester and conductive wand that generates a high voltage.
- 2. The testing activities shall be performed by the Geomembrane Installer by placing an electrically conductive tape or wire beneath the seam prior to welding. A trial seam containing a non welded segment shall be subject to a calibration test to ensure that such a defect (non welded segment) will be identified under the planned machine settings and procedures. Upon completion of the weld, enable the spark tester and hold approximately 25mm (1 in) above the weld moving slowly over the entire length of the weld in accordance with ASTM 6365. If there is no spark the weld is considered to be leak free.
- A spark indicates a hole in the seam. The faulty area shall be located, repaired and retested by the Geomembrane Installer.
- Care should be taken if flammable gases are present in the area to be tested.

3.05 FIELD QUALITY CONTROL

The Owner's Representative shall be notified prior to all pre qualification and production welding and testing, or as agreed upon in the pre construction meeting.

A. Prequalification Test Seams

- Test seams shall be prepared and tested by the Geomembrane Installer to verify that seaming parameters (speed, temperature and pressure of welding equipment) are adequate.
- 2. Test seams shall be made by each welding technician and tested in accordance with ASTM D 4437 at the beginning of each seaming period. Test seaming shall be performed under the same conditions and with the same equipment and operator combination as production seaming. The test seam shall be approximately 3.3 meters (10 feet) long for fusion welding and 1 meter (3 feet) long for extrusion welding with the seam centered lengthwise. At a minimum, tests seams should be made by each technician 1 time every 4-6 hours; additional tests may be required with changes in environmental conditions.
- 3. Two 25 mm (1 in) wide specimens shall be die-cut by the Geomembrane Installer from each end of the test seam. These specimens shall be tested by the Geomembrane Installer using a field tensiometer testing both tracks for peel strength and also for shear strength. Each specimen shall fail in the parent material and not in the weld, "Film Tear Bond"(F.T.D. failure). Seam separation equal to or greater than 10% of the track width shall be considered a failing test.
- The minimum acceptable seam strength values to be obtained for all specimens tested are listed in Subsection 3.05.C.4 of this Section. All four specimens shall pass for the test seam to be a passing seam.
- If a test seam fails, an additional test seam shall be immediately conducted. If the additional test seam fails, the seaming apparatus shall be rejected and not used for production seaming until the deficiencies are

- corrected and a successful test seam can be produced.
- 6. A sample from each test seam shall be labeled. The label shall indicate the date, geomembrane temperature, number of the seaming unit, technician performing the test seam and pass or fail description. The sample shall then be given to the Owner's Representative for archiving.

B. Field Seam Non-destructive Testing

- All field seams shall be non-destructively tested by the Geomembrane Installer over the full seam length before the seams are covered. Each seam shall be numbered or otherwise designated. The location, date, test unit, name of tester and outcome of all non-destructive testing shall be recorded and submitted to the Owner's Representative.
- Testing should be done as the seaming work progresses, not at the
 completion of all field seaming. All defects found during testing shall be
 numbered and marked immediately after detection. All defects found
 should be repaired, retested and remarked to indicate acceptable
 completion of the repair.
- Non-destructive testing shall be performed using vacuum box, air pressure or spark testing equipment.
- Non-destructive tests shall be performed by experienced technicians familiar with the specified test methods. The Geomembrane Installer shall demonstrate to the Owner's Representative all test methods to verify the test procedures are valid.
- Extrusion seams shall be vacuum box tested by the Geomembrane Installer in accordance with ASTM D 4437 and ASTM D 5641 with the following equipment and procedures:
 - a. Equipment for testing extrusion seams shall be comprised of but not limited to: a vacuum box assembly consisting of a rigid housing, a transparent viewing window, a soft rubber gasket attached to the base, port hole or valve assembly and a vacuum gauge; a vacuum pump assembly equipped with a pressure controller and pipe connections; a rubber pressure/vacuum hose with fittings and connections; a plastic bucket; wide paint brush or mop; and a soapy solution.
 - The vacuum pump shall be charged and the tank pressure adjusted to approximately 35 kPa (5 psig).
 - c. The Geomembrane Installer shall create a leak tight seal between the gasket and geomembrane interface by wetting a strip of geomembrane approximately 0.3m (12 in) by 1.2m (48 in) (length and width of box) with a soapy solution, placing the box over the wetted area, and then compressing the box against the geomembrane. The Geomembrane Installer shall then close the bleed valve, open the vacuum valve, maintain initial pressure of approximately 35 kPa (5 psig) for approximately 5 seconds. The geomembrane should be continuously examined through the viewing window for the presence of soap bubbles, indicating a leak. If no bubbles appear after 5 seconds, the area shall be

- considered leak free. The box shall be depressurized and moved over the next adjoining area with an appropriate overlap and the process repeated.
- All areas where soap bubbles appear shall be marked, repaired and then retested.
- At locations where seams cannot be non destructively tested, such as pipe penetrations, alternate nondestructive spark testing (as outlined in section 3.04.B) or equivalent should be substituted.
- f. All seams that are vacuum tested shall be marked with the date tested, the name of the technician performing the test and the results of the test.
- Double Fusion seams with an enclosed channel shall be air pressure tested by the Geomembrane Installer in accordance with ASTM D 5820 and ASTM D 4437 and the following equipment and procedures:
 - a. Equipment for testing double fusion seams shall be comprised of but not limited to: an air pump equipped with a pressure gauge capable of generating and sustaining a pressure of 210 kPa (30 psig), mounted on a cushion to protect the geomembrane; and a manometer equipped with a sharp hollow needle or other approved pressure feed device.
 - b. The Testing activities shall be performed by the Geomembrane Installer. Both ends of the seam to be tested shall be sealed and a needle or other approved pressure feed device inserted into the tunnel created by the double wedge fusion weld. The air pump shall be adjusted to a pressure of 210 kPa (30 psig), and the valve closed,. Allow 2 minutes for the injected air to come to equilibrium in the channel, and sustain pressure for 5 minutes. If pressure loss does not exceed 28 kPa (4 psig) after this five minute period the seam shall be considered leak tight. Release pressure from the opposite end verifying pressure drop on needle to ensure testing of the entire seam. The needle or other approved pressure feed device shall be removed and the feed hole sealed.
 - c. If loss of pressure exceeds 28 kPa (4 psig) during the testing period or pressure does not stabilize, the faulty area shall be located, repaired and retested by the Geomembrane Installer.
 - Results of the pressure testing shall be recorded on the liner at the seam tested and on a pressure testing record.

C. Destructive Field Seam Testing

One destructive test sample per 150 linear m (500 linear ft) seam length or another predetermined length in accordance with GRI GM 14 shall be taken by the Geomembrane Installer from a location specified by the Owner's Representative. The Geomembrane Installer shall not be informed in advance of the sample location. In order to obtain test results prior to completion of geomembrane installation, samples shall be cut by the Geomembrane Installer as directed by the Owner's Representative as seaming progresses.

- 2. All field samples shall be marked with their sample number and seam number. The sample number, date, time, location, and seam number shall be recorded. The Geomembrane Installer shall repair all holes in the geomembrane resulting from obtaining the seam samples. All patches shall be vacuum box tested or spark tested. If a patch cannot be permanently installed over the test location the same day of sample collection, a temporary patch shall be tack welded or hot air welded over the opening until a permanent patch can be affixed.
- 3. The destructive sample size shall be 300 mm (12 in) wide by 1 m (36 in) long with the seam centered lengthwise. The sample shall be cut into three equal sections and distributed as follows: one section given to the Owner's Representative as an archive sample; one section given to the Owner's Representative for laboratory testing as specified in paragraph 5 below; and one section retained by the Geomembrane Installer for field testing as specified in paragraph 4 below.
- 4. For field testing, the Geomembrane Installer shall cut 10 identical 25 mm (1 in) wide replicate specimens from his sample. The Geomembrane Installer shall test five specimens for seam shear strength and five for peel strength. Peel tests will be performed on both inside and outside weld tracks. To be acceptable, 4 of 5 test specimens must pass the stated criteria in section 2.02 with less than 10% separation. If 4 of 5 specimens pass, the sample qualifies for testing by the testing laboratory if required.
- If independent seam testing is required by the specifications it shall be conducted in accordance with ASTM 5820 or ASTM D4437 or GRI GM
 6.
- Reports of the results of examinations and testing shall be prepared and submitted to the Owner's Representative.
- 7. For field seams, if a laboratory test fails, that shall be considered as an indicator of the possible inadequacy of the entire seamed length corresponding to the test sample. Additional destructive test portions shall then be taken by the Geomembrane Installer at locations indicated by the Engineer, typically 3 m (10 ft) on either side of the failed sample and laboratory seem tests shall be performed. Passing tests shall be an indicator of adequate seams. Failing tests shall be an indicator of non-adequate seams and all seams represented by the destructive test location shall be repaired with a cap-strip extrusion welded to all sides of the capped area. All cap-strip seams shall be non-destructively vacuum box tested until adequacy of the seams is achieved. Cap strip seams exceeding 50 M in length (150 FT) shall be destructively tested.

D. Identification of Defects

- Panels and seams shall be inspected by the Installer and Owner's Representative during and after panel deployment to identify all defects, including holes, blisters, undispersed raw materials and signs of contamination by foreign matter.
- E. Evaluation of Defects: Each suspect location on the liner (both in geomembrane 02800-12

seam and non-seam areas) shall be non-destructively tested using one of the methods described in Section 3.05.B. Each location which fails non-destructive testing shall be marked, numbered, measured and posted on the daily "installation" drawings and subsequently repaired.

- If a destructive sample fails the field or laboratory test, the Geomembrane Installer shall repair the seam between the two nearest passed locations on both sides of the failed destructive sample location.
- Defective seams, tears or holes shall be repaired by reseaming or applying a extrusion welded cap strip.
- Reseaming may consist of either:
 - Removing the defective weld area and rewelding the parent material using the original welding equipment; or
 - Reseaming by extrusion welding along the overlap at the outside seam edge left by the fusion welding process.
- Blisters, larger holes, and contamination by foreign matter shall be repaired by patches and/or extrusion weld beads as required. Each patch shall extend a minimum of 150 mm (6 in) beyond all edges of the defects.
- 5. All repairs shall be measured, located and recorded.
- F. Verification of Repairs on Seams: Each repair shall be non-destructively tested using either vacuum box or spark testing methods. Tests which pass the non-destructive test shall be taken as an indication of a successful repair. Failed tests shall be reseamed and retested until a passing test results. The number, date, location, technician and test outcome of each patch shall be recorded.
- G. Daily Field Installation Reports: At the beginning of each day's work, the Installer shall provide the Engineer with daily reports for all work accomplished on the previous work day. Reports shall include the following:
 - 1. Total amount and location of geomembrane placed;
 - Total length and location of seams completed, name of technicians doing seaming and welding unit numbers;
 - Drawings of the previous day's installed geomembrane showing panel numbers, seam numbers and locations of non-destructive and destructive testing:
 - Results of pre-qualification test seams;
 - 5. Results of non-destructive testing; and
 - Results of vacuum testing of repairs.
- H. Destructive test results shall be reported prior to covering of liner or within 48 hours.

3.06 LINER ACCEPTANCE

- A. Geomembrane liner will be accepted by the Owner's Representative when:
 - The entire installation is finished or an agreed upon subsection of the installation is finished;

- 2. All Installer's QC documentation is completed and submitted to the owner
- Verification of the adequacy of all field seams and repairs and associated geomembrane testing is complete.

3.07 ANCHOR TRENCH

Construct as specified on the project drawings.

3.08 DISPOSAL OF SCRAP MATERIALS

A. On completion of installation, the Geomembrane Installer shall dispose of all trash and scrap material in a location approved by the Owner, remove equipment used in connection with the work herein, and shall leave the premises in a neat acceptable manner. No scrap material shall be allowed to remain on the geomembrane surface.

PART 4. GRI GM13 SPECIFICATIONS

Geosynthetics Research Institute (GRI) Test Method GM13 – "Test Methods, Test Properties and Testing Frequency for High Density Polyethylene (HDPE) Smooth and Textured Geomembranes", Revision 8, Dated July 10, 2006.

ATTACHMENT A:

Minimum Average Weld Pr Geome	operties for mbranes (En			and 7	Textu	ired !	HDP	E
Property	Test Method	30 mil	40 mil	50 mil	60 mil	80 mil	100 mil	120 mil
Peel strength (fusion & extrusion) lb/in. Shear strength (fusion & extrusion) lb/in.	ASTM 4437 ASTM 4437	39 60	52 80	65 100	78 120	104 160	130 200	156 239

END OF SECTION 02800

DIVISION 2 - SITE WORK

Section 02936 - Topsoil, Seeding, and Mulching

PART 1. GENERAL

1.01 DESCRIPTION

This section pertains to seeding and placing mulch or erosion control blanket over seeded areas.

1.02 RELATED SECTIONS

- A. Specified elsewhere:
 - 02200 Earthwork

1.03 REFERENCES

The following reference or cited portions thereof, current at date of bidding documents unless otherwise specified, governs the work.

 Illinois Department of Transportation (IDOT): Standard Specifications for Road and Bridge Construction, adopted January 1, 2007.

1.04 SPECIFICATIONS

- A. Work shall conform to the applicable requirements of Sections 250 and 251 of Standard Specifications for Road and Bridge Construction and to the requirements hereinafter specified.
- Exceptions: All references in the IDOT specifications to methods of measurement and payment shall not apply.

1.05 WARRANTY

 Warranty for one (1) year plus one growing season from date of substantial completion shall be provided.

PART 2. PRODUCTS

2.01 MATERIALS

A. Seed: Seed shall conform to Article 1081.04 of the IDOT Standard Specifications. The composition of the Ameren Energy Resources Generating hay seeding mix shall

be as follows:

Seed Type	Pounds/Acre				
Vernal Alfalfa	12				
Wrangler Alfalfa	8				
Medium Red Clover	6				
Timothy	4				

- B. Mulch Material and Erosion Control Blanket: Mulch material shall conform to Article 1081.06 and the excelsior blanket/knitted straw mat shall conform to Article 1081.10 of the IDOT Standard Specifications.
- Fertilizer and agricultural ground limestone will not be permitted.

PART 3. EXECUTION

3.01 CONSTRUCTION

- A. Seed bed preparation and seeding methods shall conform to Section 250 of the IDOT Specifications. Seeding of areas disturbed by construction activities after September 30, 2008 may be deferred until Spring 2009.
- B. Seed shall be applied to the perimeter berm ditch, to disturbed portions of the perimeter berm, and to all disturbed earth surfaces outside of the existing perimeter berm. IDOT seeding mixture 7 shall be used on stockpiles. IDOT seeding mixture 1A shall be used on the gypsum stack perimeter earthen berm, the recycle pond dam embankment and on slopes that are 4H:1V or steeper. The Ameren hay seed mix shall be used on slopes flatter that 4H:1V.
- C. Application rates for IDOT seed mixtures shall be as specified in Section 250 of the IDOT Specifications. The application rate for the Ameren Energy Resources Generating's seed mix shall be as specified in the Ameren Energy Resources Generating's hay seeding mix.
- D. Seeded areas shall be mulched in accordance with Article 251.03. The Contractor may use either Method 2 or Method 3.

3.02 MAINTENANCE OF COMPLETED WORK

A. All areas seeded by the Contractor shall be maintained by the Contractor during the period between completion of such work and final completion and acceptance of the Contractor's work by the Owner. This maintenance shall be such that the completed work, at time of acceptance, complies in all respects with the requirements herein specified. B. The areas seeded will be required to germinate. If the seed does not germinate, the Contractor will be required to regrade and reseed at no additional cost.

END OF SECTION 02936

DIVISION 3 - CONCRETE Section 03100 - Concrete Formwork

PART 1. GENERAL

1.01 WORK INCLUDES

A. The complete installation of the formwork for cast-in-place concrete, with shoring, bracing and anchorage, openings for other work, form accessories, form stripping.

1.02 RELATED SECTIONS

- A. Section 03200 Concrete Reinforcement.
- B. Section 03300 Cast-In-Place Concrete.
- C. Section 03400 Concrete Embedment Liner.

1.03 REFERENCES

- ACI 347 Recommended Practice For Concrete Formwork.
- B. ACI 301 Specifications For Structural Concrete For Buildings.

1.04 DESIGN REQUIREMENTS

A. Design, engineer and construct formwork, shoring and bracing to conform to design and code requirements; resultant concrete to conform to required shape, line and dimension.

1.05 QUALITY ASSURANCE

Perform Work in accordance with ACI 347 and 301.

1.06 REGULATORY REQUIREMENTS

 Conform to applicable code for design, fabrication, erection and removal of formwork.

1.07 DELIVERY, STORAGE, AND HANDLING

 Store off ground in ventilated and protected manner to prevent deterioration from moisture.

1.08 COORDINATION

- Coordinate this Section with other Sections of work which require attachment of components of formwork.
- B. If formwork is placed which results in insufficient concrete cover over reinforcement, request instructions from Owner's Representative before proceeding.

PART 2. PRODUCTS

2.01 WOOD FORM MATERIALS

- A. Softwood Plywood: 3/4 in. PS 1-83 "B-B" (concrete form) plywood, Class I, exterior grade or better, mill-oiled and edge sealed with each piece bearing legible inspection trademark.
- B. Architectural Plywood: 3/4 in. PS 1-83 "B-B" plyform, Class I, with High Density smooth overlay, 1 surface, edge scaled with each piece bearing legible inspection trademark.

2.02 MANUFACTURERS - PREFABRICATED FORMS

- Weyerhauser Concrete Form.
- B. Georgia Pacific, G-P Exterior Soft Wood Plywood Product.
- C. Plywood and Door Corporation's Finn-Form.

2.03 PREFABRICATED FORMS

- A. Preformed Steel Forms: Minimum 16 gage matched, tight fitting, stiffened to support weight of concrete without deflection detrimental to tolerances and appearance of finished surfaces.
- B. Glass Fiber Fabric Reinforced Plastic Forms: Matched, tight fitting, stiffened to support weight of concrete without deflection detrimental to tolerances and appearance of finished concrete surfaces.

2.04 FORMWORK ACCESSORIES

A. Form Ties: Snap-off type, galvanized metal, adjustable length, 1 in. back break dimension, free of defects that could leave holes larger than 1 in. in concrete surface; Dayton-Sure Grip snap-in-form ties, as manufactured by Dayton Superior

- Corp., Symons Ties as manufactured by Symons Corporation, Snap-Tys as manufactured by Richmond Corporation. Ties shall be removed after forms are removed, and holes filled with mortar that matches the adjacent surfaces.
- B. Form Release Agent: Colorless mineral oil which will not stain concrete, or absorb moisture; by Magic Kote manufactured by Symons Manufacturing Co., Form Coat manufactured by Concrete Services Co., Formcel manufactured by Lambert Corp.
- C. Corners: Chamfered, wood strip type; 3/4 x 3/4 in. size on all exterior corners, 3 x 3 in. size where shown on the drawings; maximum possible lengths.
- D. Nails, Spikes, Lag Bolts, Through Bolts, Anchorages: Sized as required, of sufficient strength and character to maintain formwork in place while placing concrete.
- E. Concrete Embedment Liner, where required, shall be installed in accordance with Section 03400 – Concrete Embedment Liner.

PART 3. EXECUTION

3.01 EXAMINATION

 Verify lines, levels and centers before proceeding with formwork. Ensure that dimensions agree with drawings.

3.02 EARTH FORMS

Earth forms are not permitted, except for footings.

3.03 ERECTION - FORMWORK

- A. Erect formwork, shoring and bracing to achieve design requirements, in accordance with requirements of ACI 301. Metal forms shall be installed in strict accordance with manufacturer's directions and specifications.
- B. Provide bracing to ensure stability of formwork. Shore or strengthen formwork subject to overstressing by construction loads.
- C. Arrange and assemble formwork to permit dismantling and stripping. Do not damage concrete during stripping. Permit removal of remaining principal shores.
- Align joints and make watertight. Keep form joints to a minimum.
- Obtain approval before framing openings in structural members which are not indicated on drawings.

3.04 APPLICATION - FORM RELEASE AGENT

- Apply form release agent on formwork in accordance with manufacturer's recommendations.
- Apply prior to placement of reinforcing steel, anchoring devices, and embedded items.
- C. Do not apply form release agent where concrete surfaces will receive special finishes or applied coverings which are affected by agent.

3.05 INSERTS, EMBEDDED PARTS, AND OPENINGS

- Provide formed openings where required for items to be embedded in or passing through concrete work.
- Locate and set in place items which will be cast directly into concrete.
- C. Coordinate work of other Sections in forming and placing openings, slots, reglets, recesses, chases, sleeves, bolts, anchors, and other inserts.
- Install accessories in accordance with manufacturer's instructions, straight, level, and plumb. Ensure items are not disturbed during concrete placement.
- E. Provide temporary ports or openings in formwork where required to facilitate cleaning and inspection. Locate openings at bottom of forms to allow flushing water to drain.
- F. Close temporary openings with tight fitting panels, flush with inside face of forms, and neatly fitted so joints will not be apparent in exposed concrete surfaces.

3.06 FORM CLEANING

- Clean and remove foreign matter within forms as erection proceeds.
- Clean formed cavities of debris prior to placing concrete.
- C. Flush with water or use compressed air to remove remaining foreign matter. Ensure that water and debris drain to exterior through clean-out ports.
- D. During cold weather, remove ice and snow from within forms. Do not use de-icing salts or water to clean out forms. Use compressed air or other means to remove foreign matter.

3.07 FORMWORK TOLERANCES

Construct formwork to maintain tolerances required by ACI 301.

3.08 FIELD QUALITY CONTROL

- A. Inspect erected formwork, shoring, and bracing to ensure that work is in accordance with formwork design, and that supports, fastenings, wedges, ties, and items are secure.
- B. Do not reuse wood formwork more than three times for concrete surfaces to be exposed to view. Do no patch formwork.

3.09 FORM REMOVAL

- Do not remove forms or bracing until concrete has gained sufficient strength to carry its own weight and imposed loads.
- B. Loosen forms carefully. Do not wedge pry bars, hammers, or tools against finished concrete surfaces scheduled for exposure to view.
- C. Store removed forms in manner that surfaces to be in contact with fresh concrete will not be damaged. Discard damaged forms.

END OF SECTION 03100

DIVISION 3 - CONCRETE Section 03200 - Concrete Reinforcement

PART 1. GENERAL

1.01 WORK INCLUDES

 The complete installation of the reinforcing steel bars and accessories for cast-in-place concrete.

1.02 RELATED SECTIONS

- A. Section 03100 Concrete Formwork.
- B. Section 03300 Cast-in-Place Concrete.

1.03 REFERENCES

- A. ACI 301 Structural Concrete for Buildings.
- B. ACI 318 Building Code Requirements For Reinforced Concrete.
- C. ACI SP-66 American Concrete Institute Detailing Manual.
- D. ASTM A615 Deformed and Plain Billet Steel Bars for Concrete Reinforcement.
- E. CRSI Concrete Reinforcing Steel Institute Manual of Practice.

1.04 SUBMITTALS

- Submit under provisions of Section 01010.
- B. Shop Drawings: Indicate bar sizes, spacings, locations, and quantities of reinforcing steel, and bending and cutting schedules. Contract drawings shall not be reproduced as the basis for shop drawings.
- Manufacturer's Certificate: Certify that products meet or exceed specified requirements.

1.05 QUALITY ASSURANCE

- Perform Work in accordance with CRSI Manual of Standard Practice.
- Submit certified copies of mill test report of reinforcement materials analysis.

1.06 COORDINATION

Coordinate with placement of formwork, formed openings and other work.

PART 2. PRODUCTS

2.01 REINFORCEMENT

Reinforcing Steel: ASTM A615, 60 ksi yield grade; deformed billet steel bars.

2.02 ACCESSORY MATERIALS

- A. Tie Wire: Minimum 16 gage, annealed steel wire, epoxy coated when used with epoxy-coated reinforcement.
- Chairs, Bolsters, Bar Supports, Spacers: Sized and shaped for strength and support of reinforcement during concrete placement conditions.
- C. Special Chairs, Bolsters, Bar Supports, Spacers Adjacent to Weather Exposed Concrete Surfaces: Plastic coated steel type; size and shape as required.

2.03 FABRICATION

- Fabricate concrete reinforcing in accordance with CRSI Manual of Standard Practice and ACI SP-66.
- Splice reinforcement on at locations indicated on drawings. Indicate location of splices on shop drawings.

PART 3. EXECUTION

3.01 PLACEMENT

- Place, support and secure reinforcement against displacement. Do not deviate from required position. Clean reinforcement of foreign particles or coatings.
- Accommodate placement of formed openings.
- Conform to ACI 318 code for concrete cover over reinforcement.

3.02 FIELD QUALITY CONTROL

A. Contractor shall notify the Owner's Representative at least 24 hrs. in advance of concrete placement. Placement of reinforcing shall occur in such sequence that the Owner's Representative has sufficient time to inspect the correctness of the reinforcing within the placement area. The Owner's Representative retains the right to require necessary revisions be made before concrete is placed.

END OF SECTION 03200

DIVISION 3 - CONCRETE Section 03300 - Cast-In-Place Concrete

PART 1. GENERAL

1.01 WORK INCLUDES

 The complete installation of cast-in-place concrete structures, including joint scalants.

1.02 RELATED SECTIONS

- A. Section 03100 Concrete Formwork: Formwork and accessories.
- B Section 03200 Concrete Reinforcement.
- C. Section 03400 Concrete Embedment Liner

1.03 REFERENCES

- A. ACI 301 Structural Concrete for Buildings.
- B. ACI 302 Guide for Concrete Floor and Slab Construction.
- ACI 304 Recommended Practice for Measuring, Mixing, Transporting and Placing Concrete.
- D. ACI 305R Hot Weather Concreting.
- E. ACI 306R Cold Weather Concreting.
- F. ACI 308 Standard Practice for Curing Concrete.
- G. ACI 318 Building Code Requirements for Reinforced Concrete.
- H. ASTM C31 Concrete Test Specimens.
- ASTM C33 Concrete Aggregates.
- J. ASTM C94 Ready-Mixed Concrete.
- K. ASTM C150 Portland Cement.
- L. ASTM C260 Air Entraining Admixtures for Concrete.

M. ASTM C494 - Chemical Admixtures for Concrete.

1.04 SUBMITTALS

Product Data: Provide data on joint devices, attachment accessories, admixtures.

1.05 QUALITY ASSURANCE

- Perform Work in accordance with ACI 301.
- Acquire cement and aggregate from same source for all work.
- C. Conform to ACI 305R when concreting during hot weather.
- Conform to ACI 306R when concreting during cold weather.

1.06 COORDINATION

 Coordinate this Section with other Sections which require embedment of components in cast-in-place concrete.

1.07 PRODUCT DATA

- A. Submit proposed mix design to Owner's Representative for review prior to commencement of work. Identify source and provide material certificates for cement, fine and coarse aggregates. Provide recent laboratory gradation for fine and coarse aggregates and mix design information in accordance with ACI 301.
- B. Submit Construction joint plan.

PART 2. PRODUCTS

2.01 CONCRETE MATERIALS

- Cement: ASTM C150, Type I Normal Portland Type, Gray Color.
- B. Fine and Coarse Aggregates: ASTM C33.
- C. Water: Potable.

2.02 ADMIXTURES

A. Air Entrainment: ASTM C260.

- B. Chemical: ASTM C494. Maximum 0.05% Chloride Ion Contents.
- The use of calcium chloride in any concrete is not permitted.

2.03 ACCESSORIES

- A. Non-Shrink Grout: Premixed compound consisting of non-metallic aggregate, cement, water reducing and plasticizing agents; capable of developing minimum compressive strength of 2,400 psi in 48 hours and 7,000 psi in 28 days.
- B. Curing Compound: Dress and Seal No. 18 by L&M Construction Chemicals, MB-429 by Master Builders, or Sikagard Cure/Hard by the Sika Corporation.
- C. Epoxy Grouted Adhesive Anchors: Hilti, Red Head, Simpson, or Rawl.

2.04 CONCRETE MIX

- Mix concrete in accordance with ACI 304. Deliver concrete in accordance with ASTM C94.
- Select proportions for normal weight concrete in accordance with ACI 301.
- C. Provide normal weight concrete of the following characteristics:
 - Compressive strength at 28 days: 4,000 psi.
 - Slump: 4 in. A tolerance of up to 1 in. above the maximum shall be allowed for one batch in any five consecutive batches tested.
 - Water/cement ratios: 0.4 (max).
- D. Use accelerating admixtures in cold weather only when approved by Owner's Representative. Use of admixtures will not relax cold weather placement requirements.
- Use set-retarding admixtures during hot weather only when approved by Owner's Representative.
- F. Water-reducing admixtures may be used in all concrete except footings and in strict compliance with the manufacturer's directions.
- G. Add air-entraining agent to concrete mix for air content of 6% (± 1%).

PART 3. EXECUTION

3.01 EXAMINATION

- Verify requirements for concrete cover over reinforcement.
- B. Verify that anchors, seats, plates, reinforcement and other items to be cast into concrete are accurately placed, positioned securely, and will not cause hardship in placing concrete.

3.02 PLACING CONCRETE

- Place concrete in accordance with ACI 301.
- Notify Owner's Representative minimum of 24 hours prior to commencement of operations.
- Ensure reinforcement, inserts, and embedded parts are not disturbed during concrete placement.
- D. Maintain records of concrete placement. Record date, location, quantity, air temperature, and test samples taken.
- Place concrete continuously between predetermined expansion, control, and construction joints.
- F. When air temperature is between 80°F and 90°F, reduce the mixing and delivery time specified in ASTM C94 from 1-1/2 hours to 75 minutes. When the air temperature is above 90°F, reduce the mixing and delivery time to 60 minutes.
- G. Cold weather concreting. Comply with ACI 306 except as follows:
 - In freezing weather, provide suitable means for maintaining concrete temperature at a minimum of 70°F for three days, or 50°F for five days after placing.
 - Cooling of concrete to outside temperature: Not faster than 1° per hour for first day and 2° per hour thereafter until outside temperature is reached.
 - Maximum temperature of concrete produced with heated aggregated, heated water, or both, at any time during its production or transportation: 90°F.
 - Do not mix chemicals or other foreign materials in concrete to prevent freezing or to accelerate hardening of concrete, unless approved in writing by Owner's Representative.

- H. Hot weather concreting. Comply with ACI 305R.
 - ACI recommendations shall be observed when any combination of high air temperature, low relative humidity and wind velocity tend to impair the quality of fresh or hardened concrete.
 - Retarding and water reducing admixtures shall be approved in writing for each concrete mix design prior to placement.

3.03 CONCRETE FINISHING

- A. Provide exterior concrete formed surfaces to be left exposed with smooth rubbed finish in accord with ACI 301. All other formed surfaces shall have fins, projections and offsets removed.
- Provide Class A tolerances to exterior concrete slabs according to ACI 301.
 - Broom finish all exterior slabs. Broom out all tool marks.
- C. Pitch slabs to drain.

3.04 CURING AND PROTECTION

- Immediately after placement, protect concrete from premature drying, excessively hot or cold temperatures, and mechanical injury.
- B. Maintain concrete with minimal moisture loss at relatively constant temperature for a period necessary for hydration of cement and hardening of concrete in accordance with ACI 308.
- Cure and protect finished concrete slabs in accordance with ACI 308.

3.05 FIELD QUALITY CONTROL

- Field inspection and testing will be performed in accordance with ACI 301 and under provisions of Section 01010, paragraph 8.0.
- Testing firm will take cylinders, perform slump and air entrainment tests in accordance with ACI 301.
- Provide free access to Work and cooperate with appointed firm.
- Submit proposed concrete mix design to Owner's Representative firm for review 14 days prior to commencement of Work.

E. Testing frequency shall be as specified in Section 01010, paragraph 8, except that one additional test cylinder will be taken during cold weather concreting, cured on job site under same conditions as concrete it represents.

3.06 PATCHING

- Defective Concrete: Concrete not conforming to required lines, details, dimensions, tolerances or specified requirements.
- B. Repair or replacement of defective concrete will be determined by Owner's Representative and performed by the Contractor at no additional cost to the project.
- C. Do not patch, fill, touch-up, repair, or replace exposed concrete except upon express direction of Owner's Representative for each individual area.

END OF SECTION 03300

PART 1. GENERAL

1.01 WORK INCLUDES

 Specifications and guidelines for manufacturing and installing high-density polyethylene embedment liners.

1.02 RELATED SECTIONS

- A. Section 03100 Concrete Formwork.
- B. Section 03300 Cast-in-Place Concrete.

1.03 REFERENCES

- A. American Society for Testing and Materials (ASTM)
 - D 1505 Test Method for Density of Plastics by the Density-Gradient Technique
 - D 1603 Test Method for Carbon Black in Olefin Plastics
 - D 5199 <u>Standard Test Method for Measuring Nominal Thickness of Geotextiles and Geomembranes</u>
 - D 5596 Standard Test Method for Microscopic Evaluation of the Dispersion of Carbon Black in Polyolefin Geosynthetics
 - D 6693 Standard Test Method for Determining Tensile Properties of Nonreinforced Polyethylene and Nonreinforced Flexible Polypropylene Geomembranes
 - D 1204 Standard Test Method for Linear Dimensional Changes of Nongrid Thermoplastic Sheeting or Film at Elevated Temperature
 - D 696 Standard Test Method for Coefficient of Linear Thermal Expansion of Plastics Between -30°C and 30°C With a Vitreous Silica Dilatometer
 - D 746 Standard Test Method for Brittleness Temperature of Plastics and Elastomers by Impact
 - D 570 Standard Test Method for Water Absorption of Plastics
 - E 96 Standard Test Method for Water Vapor Transmission of Material

1.04 SUBMITTALS

A. All work for and in connection with the installation of the lining, field seaming and welding joints shall be completed in strict conformity with all applicable instructions and recommendations of the liner manufacturer. B. Included with the shipment of liner, submit certified test reports that the liner and material are manufactured in accordance with standards specified herein.

1.05 OUALIFICATIONS

- A. The HDPE liner specified in this section shall be furnished by a manufacturer who is fully experienced, reputable and qualified in the manufacturing of the materials. The manufacturer must at least 10 years of manufacturing experience.
- Locking devices must be extruded to the sheet as a one step process.
- C. Liner shall be GSE StudLiner as manufactured by GSE Lining Technology, Inc.
- D. Liner shall be 8 feet in width.
- E. Liner shall demonstrate a minimum pull-out strength of 14,000 psf.

1.06 COORDINATION

Coordinate with placement of formwork, formed openings and other work.

PART 2. PRODUCTS

2.01 ROLL DIMENSIONS

- A. Embedment sheets shall be produced in rolls that are 8.0 ft (2.4 m) in width and a thickness range of 80 mils (2.0 mm) to 200 mils (5.0 mm) in thickness. Roll lengths vary according to thickness.
- B. Locking studs of the same material as that of the liner shall be integrally extruded with the sheet. Stud spacing shall be on approximate 1.25 in (30 mm) centers, such that there are approximately 110 studs per square foot (1200 per square meter).

2.02 MATERIAL PROPERTIES

- A. The material used in the embedment liner and in all welding strips shall be made from 97-98% virgin high density polyethylene and 1.5-3% carbon black or pigmentation for the purpose of an otherwise specified color.
- Plasticizer shall not be added to the resin formulation.

- C. Embedment sheet and welding strips shall be free of holes, pinholes, bubbles, blisters, excessive contamination by foreign matter, and nicks and cuts on roll edges.
- D. The HDPE cap strips shall be made from HDPE, have good impact resistance and have an elongation sufficient to bridge up to 1/4 inch settling cracks.
- Cap strips shall be approximately 4 inches wide or greater and shall be equivalent to that of the liner.
- F. Material shall maintain a repairable state through it's lifecycle by methods approved and recommended by the manufacturer.
- G. Embedment sheets shall have the following physical properties when tested in accordance with Table I.
- H. Raw resin shall have the following properties when tested in accordance with Table 2.

Table 1: Material Properties

Property	Test Method	Nominal Value				Testing Frequency	
Thickness, mm (mil)	ASTM D 5199	2.00 (80)	3.00 (120)	4.00 (160)	5.00 (200)	Every 5th roll	
Density, g/cm ³	ASTM D 1505	0.94	0.94	0.94	0.94	1/100,000 ft ²	
Tensile Properties Strength@Yield,lb/in ² (MPa) Elongation @ Break, %	ASTM D 6693 Type IV, Dumbell G.L.= 2.0in.	2,200 (14.5) 500	2,200 (14.5) 500	2,200 (14.5) 500	2,200 (14.5) 500	1/100,000 ft ²	
Stud Pull-Out Strength ¹ , lb/ft ² (kN/m ²)		>14,000 (669.89)	>14,000 (669.89)	>14,000 (669.89)	>14,000 (669.89)	1/ product	
Carbon Black Content/ Pigment Content, % Black Liner Gray Liner	ASTM D 1603, mod. ASTM D 5630, mod.	2-3 1.5 – 2.5	2-3 1.5 – 2.5	2-3 1.5 – 2.5	2-3 1.5 – 2.5	1/100,000 ft ²	
Carbon Black Dispersion ²	ASTM D 5596	Note 2	Note 2	Note 2	Note 2	1/100,000 ft ²	
Notched Constant Tensile Load, hours	ASTM D 5397	400	400	400	400	1/ formulation	
Coefficient of Linear Thermal Expansion, per C	ASTM D 696	1.20E-04	1.20E-04	1.20E-04	1.20E-04	I/ product	
Low Temperature Brittleness, °C	ASTM D 746	-77	-77	-77	-77	1/ product	
Dimensional Stability, % (each direction)	ASTM D 1204	± 1.0	± 1.0	± 1.0	± 1.0	1/ product	
Water Absorption, %	ASTM D 570	0.1	0.1	0.1	0.1	1/ product	
Water Vapor Transmission, (g/m²/day)	ASTM E 96	<0.01	<0.01	< 0.01	<0.01	1/ product	

¹Note: Concrete must have a compressive strength of at least 5,000 lb/in¹ (34,500 kPa).
²Note: Dispersion only applies to near spherical agglomerates. 9 of 10 views shall be Category 1 or 2. No more than 1 view for category 3.

Table 2: Raw Material Properties

Property	Test Method	Value	Testing Frequency
Density, g/cm3	ASTM D 1505	0.932	1/ resin lot
Melt Flow, g/10 min	ASTM D 1238 (190/2.16)	≤1.0	1/ resin lot
OIT, minutes	ASTM D 3895 (1atm/200°C)	100	1/ formulation

2.03 MATERIAL SUPPLY

- Embedment sheets shall be supplied in roll form, sheets, pre-fabricated tubes or panels.
- Cap strips shall be supplied in 4 inch widths or greater.

PART 3. EXECUTION

3.01 PLACEMENT

- Place, support and secure reinforcement against displacement. Do not deviate from required position. Clean reinforcement of foreign particles or coatings.
- Accommodate placement of formed openings.
- Conform to ACI 318 code for concrete cover over reinforcement.

3.02 FIELD QUALITY CONTROL

A. Contractor shall notify the Owner's Representative at least 24 hrs. in advance of concrete placement. Placement of the Concrete Embedment Liner shall occur in such sequence that the Owner's Representative has sufficient time to inspect the correctness of the placement within the concrete formwork area. The Owner's Representative retains the right to require necessary revisions be made before concrete is placed.

END OF SECTION 03200



Appendix E: Operation & Maintenance Manual for #1 Ash Pond



Coffeen Power Station

Operational Procedure

X-XXX-XXXX--XXX

Operation & Maintenance Manual for #1 Ash Pond

(Bottom Ash Recycle Pond)

Effective Date: xx/xx/xxxx

Reason for Change: New Procedure

Approved By: x Date: xx/xx/xxxx

Responsible Department: Coffeen Power Station, Technical Services Department

John Romang

This entire document shall be in the field during procedure
performance.
The following portions of this procedure shall be in the field
during procedure performance:
from this procedure shall be in the field during
procedure performance.
No part of this procedure is required to be in the field during
procedure performance

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1.0 Purpose

1.1 This procedure is intended to ensure the safe and environmentally responsible operation and use of the #1 Ash Pond (Bottom Ash Recycle Pond) at the Coffeen Power Station. The primary purpose of the #1 Ash Pond is for the removal of bottom ash by settling and the recirculation of slag tank water. The pond is used to supply water to the Unit 1 and Unit 2 ash handling systems via the recycle pumps.

2.0 Scope

2.1 This procedure applies to all onsite personnel and the Dam Safety Group staff.

3.0 Responsibilities

- 3.1 Outside Unit Operator Checks the pond level and screens once a shift. Operates the facilities as described in this Operational Procedure. Reports any conditions noted during routine activities to the Shift Supervisor and Chemistry Department. Writes job requests if a problem is identified.
- 3.2 Shift Supervisor (SS) Calls the Chemistry Department when structural concerns or overflow conditions are reported. Make entries into the shift electronic log book (e-log) indicating the concern and actions taken.
- 3.3 Dam Safety Inspector Conducts weekly detailed dam safety inspections and provides a report with findings and recommendations. Make entries in e-log indicating the concern and actions taken.

4.0 Historical Information

- 4.1 The #1 Ash Pond was initially constructed to be a mixed ash deposition pond and was put in service in the mid-1960's. It is located east of the Main Building. It is a 23 acre pond with a maximum outer berm height of 41.5 feet above ground surface level (approximately elevation 637.5'). The pond overflow was located on the north east corner of the pond and discharged into the flume.
- 4.2 The #1 Ash Pond was converted to act as a closed loop system in the late 1970's when the dewatering bins were installed. The mixed ash was removed and deposited into the #2 ash pond during the closure of #2 pond. The #1 Ash Pond berms were modified and an inner berms was added to the pond to aid in dropping out bottom ash solids. Exterior berm elevation is approximately 637.5 feet.

- 4.3 The #1 Ash Pond was equipped with an emergency overflow at the outlet structure. When the pond level reaches approximately 6.5 feet from the top of the berm, it will overflow into the flume. Overflow will be reported to the EPA. In 2011 there was an assessment of the overflow pipe which showed no obstructions or damage.
- 4.4 In 2006, the bottom ash system was modified to directly sluice bottom ash into the pond, bypassing the retired dewatering bins. Bottom Ash is removed from the pond via an outside contractor on an as needed (typically daily) basis.

5.0 Water Supply

All water inlets to the pond are located on the west side of the pond.

The ash sluice lines (from the valve house) discharge to the pond. These lines are used to convey ash from the slag handling system to the #1 Ash Pond. These lines are the southern most of the pond inlets. HPSW system is routed to the pond (valve house sparger valves, floor drains at Unit 1 cyclone level).

The Slag Tank Overflow sump pumps discharge into the pond at the concrete culvert located directly east of the of the lime/soda ash silo. Also in this area, a small stainless line extends thru the concrete. This is the discharge of the sludge pumps at the Waste Treatment System in the Recycle Pump House building.

The recycle pump flow control valves discharge to the pond through a line located at the northwest corner of the pond. Also in the vicinity of this line is the discharge pipe of the recycle pump house sump pumps.

Water from the Unit 1 and Unit 2 oil water separators are typically routed to the pond via the Slag Tank Overflow Pump (STOP) House sumps. Water entering these sumps are floor and roof drains in the plant and the yard area immediately to the north of the main building.

6.0 Operations Requirements

Pond Level - Plant personnel shall monitor the level of the #1 Ash Pond on a daily basis. Pond level is maintained at approximately 1.0' to 1.5' at the water level staff gauge located on the pond side of the screens. The staff gauge has elevation 629.0' as the 0 elevation.

At 2.0' water level (elevation 631.0 feet), the pond overflows resulting in a sampling and analysis requirement for Total Suspended Solids and Oil and Grease with reporting of the results to the IEPA. If the pond is found at or above 2.0' on the pump side staff gauge, contact Chemistry immediately.

Water can be added to the pond from either the Unit 1 or Unit 2 Low Pressure Service Water (LPSW) headers via piping that discharges to the slag tank overflow trench.

Water can be drained from the pond via the water supply pipe to the dewatering bins. Opening this valve drains water from the recycle header which will remove water from the #1 Ash Pond.

Recycle Pump Intake Structure – Suction to the recycle pumps is supplied from the intake structure located at the west end of the north leg of the pond. This is the only water discharge point from the pond. Water level staff gauges are located upstream and downstream of the trash screens for determination of the screen differential. At 0.5′ differential, the screens should be cleaned. Level sensors are also installed upstream and downstream of the screens. Digital displays of the upstream and downstream levels are located along the north side of the catwalk leading out to the screen enclosure. These level sensors will generate a high screen differential alarm in the Control Room DCS. Check screen differential (should clean screens at 6 inches differential.) When the screens become plugged, suction to the recycle pumps is reduced. Call shift supervisor to report if screens needs to be cleaned.

Oil Boom – Plant personnel shall monitor the oil boom that is provided upstream of the intake structure. Check condition of oil booms across pond, at discharge, and across pond inlet. Booms should be replaced when they become oil saturated or damaged. Also check that booms have not come unattached from one another. Write JR to change out booms or to reconnect booms when required.

Emergency Conditions – If a condition arises where there is a possibility of an embankment failure, then the following procedures will be followed:

- 1. Notify the Supervising Engineer Dam Safety immediately.
- 7.0 Dam Safety Requirements
- 7.1 Dam Safety Inspections The plant's impoundment and flood prevention structures shall be inspected and maintained in a manner to ensure safe and environmentally responsible operations. A regular maintenance program shall be performed and shall consist of the following inspection items:

- 1. Earth embankments: Walk the crest, side slopes, and downstream toe of the dam concentrating on surface erosion, seepage, cracks, settlement, slumps, slides, and animal burrows. Frequency of inspection: Weekly.
- 2. Vegetation: Grass should be a thick vigorous growth to stabilize the earth embankment soils and prevent erosion form occurring. There should be NO trees on the earth embankment and none within a minimum of 20 feet of the embankment toe or other structures. Mowing frequency: Semiannually.
- 3. Well Readings: Record level of wells on the crest and toe of the berm. Frequency: Quarterly.
- 4. Special Inspections Special inspections of the levees and ash pond berms shall be performed after earthquakes, floods, water level exceedance in the ponds, or heavy rainfall events. Inspection and report shall be equal to an annual inspection level of detail. Water level in the pond should be noted after a heavy rainfall. Dam Safety staff shall accompany plant personnel on special inspections. Frequency: As required.
- 8.0 Maintenance Log
- 8.1 Dam & Berm Inspector shall enter on e-log under the Dam Safety tab all weekly inspections, any usual occurrences, and maintenance performed.
- 9.0 Contact Numbers

Plant Environmental Supervisor: John Romang / 217-534-7629
Plant Dam & Berm Inspector: Vito Passariello/ 217-534-7664
Plant Control Room: 217-534-7668 / 217-534-7669

Supervising Engineer Dam Safety: Steve Bluemner / 314-554-6298

Dam Safety Staff Contact: Mike Wagstaff / 314-554-6296

10.0 References



Appendix F: Operation and Maintenance Manual, Gypsum Management FacilityOperation (2015)

Operation and Maintenance Manual Coffeen Energy Center Gypsum Management Facility Montgomery County, Illinois

IDNR Permit # DS2014019 Dam Permit # IL50578 & IL50579

Prepared For:

ILLINOIS POWER GENERATING COMPANY
Coffeen Energy Center
134 CIPS Lane
Coffeen, Illinois 62017

Prepared By:

HANSON PROFESSIONAL SERVICES INC. 1525 South Sixth Street Springfield, Illinois 62703

Amended By:

DYNEGY OPERATING COMPANY 1500 Eastport Plaza Drive Collinsville, Illinois 62234

> Original: February 2008 Amended: March 2015

OPERATION AND MAINTENANCE MANUAL COFFEEN ENERGY CENTER GYPSUM MANAGEMENT FACILITY MONTGOMERY COUNTY, ILLINOIS

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SECTION 1.0 GENERAL

This operation and maintenance (O&M) manual outlines objectives, proposed policies, responsibilities, and procedures for Coffeen Energy Center personnel who are responsible for the management of the Coffeen Energy Center Gypsum Management Facility (GMF). The GMF incorporates two reservoirs, the Gypsum Pond and the Recycle Pond, for processing and storing gypsum.

1.1 REASONS FOR DEVELOPMENT AND DISSEMINATION OF THE O&M MANUAL

The State of Illinois Rivers, Lakes and Streams Act, (615 ILCS 5) Paragraph 23a includes the statement "The Department is authorized to carry out inspections of any dam within the State, and to establish standards and issue permits for the safe construction of new dams and the reconstruction, repair, operation and maintenance of all existing dams." (emphasis added).

Part 3702 of Section 17 of the Illinois Administrative Code, Chapter I entitled the "Construction and Maintenance of Dams" details the requirements to obtain a permit for the construction, operation, and maintenance of a dam. Section 3702.40 b) includes the following statements:

- "4) An applicant for a Class I or II dam shall submit an operational plan specifying the method and schedule for the operation of the dam and the routine operating procedures to keep the dam in good working order, including an emergency warning plan." and
- "5) As a condition of each permit, the dam owner shall submit a maintenance plan detailing the procedures and schedules to be followed to maintain the dam and its appurtenances in a reasonable state of repair."

Thus, it is a requirement of all dam owners who have dams which fall under the jurisdiction of the Illinois Department of Natural Resources Office of Water Resources (IDNR-OWR) to operate and maintain them safely.

As a dam owner, Illinois Power Generating Company (IPGC) Coffeen Energy Center is responsible for the safety of the public and for maintaining the structures at the facility for both safety and economy. The overall public interest is served by providing a document to serve as a basis for the safe and economical operation and maintenance of the dam during both emergency and day-to-day conditions.

1.2 GENERAL RESPONSIBILITIES CONCERNING DAMS

IPGC is responsible for the operation and maintenance of the Gypsum Pond Dam and the Recycle Pond Dam. These responsibilities include general maintenance (mowing, removing debris from decants, placing riprap where needed, etc.), operation, inspection and emergency action decisions.

SECTION 2.0 DEFINITIONS

Appurtenant Works - The structures or machinery auxiliary to dams which are built to operate and maintain dams; such as outlet works, spillways, gates, valves, channels, etc.

Boil - A stream of water discharging from the ground surface downstream of the dam carrying with it a volume of soil which is distributed around the hole formed by the discharging water.

Berm - A horizontal step or bench in the sloping profile of an embankment dam.

Breach - A break, gap, or opening (failure) in a dam which releases impoundment water.

Dam - A barrier built for impounding or diverting the flow of water.

Dike (Levee) - An embankment, usually applied to embankments or structures built to protect land from flooding.

Drain, Layer or Blanket - A layer of pervious material in a dam to facilitate the drainage of the embankment including such items as a toe drain, a weephole, and a chimney drain.

Drawdown - The resultant lowering of the water surface level due to the release of water from the impoundment.

Embankment - Fill material, usually rock or earth, placed with sloping sides.

Earthen Dam - Any dam constructed of excavated natural materials.

Emergency Action Plan - A predetermined plan of action to be taken to reduce the potential for property damage and loss of lives.

Failure - An incident resulting in the uncontrolled release of water from the dam.

Freeboard - The vertical distance between a stated water level and the top of the dam.

Gate or Valve - In general, a device in which a leaf or member is moved across the waterway to control or stop the flow.

Groin - The junction of the upstream or downstream face of the dam with the valley wall.

Maintenance - The upkeep, involving labor and materials, necessary for efficient operation of dams and their appurtenant works.

Operation - The administration, management, and performance needed to operate the dam and appurtenant works.

Operation and Maintenance Inspection - Inspections conducted by the dam operator. These inspections are frequent visual "Walk-around" inspections of the dam surface and appurtenant works.

Outlet - An opening through which water can freely discharge for a particular purpose from an impoundment.

Phreatic Surface - The upper surface of saturation in an embankment.

Piping - The progressive development of internal erosion by seepage, appearing downstream as a hole or seam, discharging water that contains soil particles.

Riprap - A layer of large stones, broken rock or precast blocks placed in a random fashion usually on the upstream slope of an embankment dam, on a reservoir shore, or on the sides of a channel as a protection against wave and ice action.

Silt/Sediment - Soil particles and debris in an impoundment.

Slump/Slide Area - A portion of earth embankment which moves downslope, sometimes suddenly, often with cracks developing.

Spillway System - A structure or structures over or through which flows are discharged. If the flow is controlled by gates, it is considered a controlled spillway. If the elevation of the spillway crest is the only control of the flows, it is considered an uncontrolled spillway.

Emergency Spillway - A spillway designed to operate very infrequently, only during exceptionally large floods, usually constructed of materials expected to erode slowly.

Principal Spillway - The main spillway which controls both normal and flood flows and is usually constructed of non-erodable materials.

Auxiliary Spillway - A spillway which works in conjunction with the principal spillway to control flood flows and is usually constructed of non-erodable materials.

Stilling Basin - A basin constructed to dissipate the energy of fast flowing water, such as from a spillway, and to protect the streambed from erosion.

Toe of Embankment - The junction of the face of the dam with the ground surface in the floodplain upstream or downstream of the dam.

SECTION 3.0 INFORMATION ABOUT THE DAMS

3.1 LOCATION

The Gypsum Pond Dam and Recycle Pond Dam are located in the NW 1/4 of Section 11, Township 7 North, Range 3 West of the Third Principal Meridian in Montgomery County, Illinois. More specifically, the dams are located approximately 1.5 miles south of Coffeen, Illinois. A map showing the location of the dams is included in Appendix A.

3.2 DESCRIPTION OF DAM AND APPURTENANCES

The gypsum pond perimeter earthen dam, the gypsum pond "gypsum" dam, and the recycle pond dam will all be regulated in accordance with 17 Illinois Administrative Code (IAC) Part 3702, Construction and Maintenance of Dams. The gypsum pond perimeter earthen dam, which will be lined with a dual high density polyethylene (HDPE) geomembrane system, will have a maximum embankment height of 13 ft and a maximum impounding capacity of 442 acre-ft (measured at the top of earthen dam elevation 632 ft). There will be an additional 123 acre-ft of incised storage. The total volume of gypsum stored within the completed gypsum pond dams will be approximately 2,478 acre-ft.

The dam for the recycle pond, which will be lined with a 60 mil HDPE geomembrane, will have a maximum embankment height of 16 ft and a maximum impounding capacity of 243 acre-ft (measured at the top of dam elevation 629 ft). There will be an additional 99 acre-ft of incised storage.

The gypsum pond will be divided into two sub-cells for the containment of scrubber sludge (gypsum). Discharges to the site will switch back and forth between the two sub-cells so that one sub-cell can be dewatered and raised while the other is in use. There will be two fixed decant pipes constructed in the gypsum stack – one for each sub-cell - which will discharge to stilling wells located adjacent to the perimeter ditches. The control elevation on the decant pipes will be maintained 5.0 ft below the lowest point on the stack cell crest. The decant pipes will enable the cells to be dewatered after storm events so that a minimum of 5.0 ft of freeboard will be maintained in each cell. A minimum of 4.7 ft of freeboard is required above the decant inlet to contain the Probable Maximum Flood (PMF) storm event in addition to peak wind generated waves.

The gypsum pond dam perimeter ditches will be located on the interior sides of the earthen dam. Runoff from the stack will be conveyed through the ditches to a transfer channel which will discharge into the recycle pond. The ditches will be trapezoidal in shape with a 15 ft bottom width, a maximum depth of 9 ft and a longitudinal slope of 0.0005 ft/ft. Side slopes will be 3H:1V. During operation, the ditches will be monitored for erosion. If erosion of the designed ditch geometry occurs, a geogrid will be used for stabilization.

The transfer channel between the gypsum pond dam and the recycle pond have a trapezoidal cross-section with 3H:1V side slopes will be lined with HDPE. The 500 ft long transfer channel will transition from a 32-ft bottom width at an invert elevation of 623.0 ft at the upstream end to a 60-ft bottom width at an invert elevation of 622.0 ft at the downstream end. The transfer channel will be fitted with stop logs capable of raising the discharge control elevation to 625.0 ft. To prevent degradation of the HDPE liner due to flow velocities, the transfer channel and a portion of the recycle pond dam will incorporate an additional sacrificial layer of HDPE.

The emergency spillway for the recycle pond will consist of three 6 ft by 6 ft precast reinforced concrete risers (drop inlets) with a top elevation of 624 ft (5 ft below the top of the dam). The recycle pond's HDPE liner will attach to the exterior sides of each riser. A 4-ft diameter HDPE outlet conduit will be constructed at each riser with an upstream invert of 615.0 ft and a downstream invert of 613.0 ft. Assuming a normal pool elevation of 624 ft (control elevation of the risers), the emergency spillway has been designed to pass the 24-hour PMF storm event with adequate freeboard to prevent overtopping of the recycle pond crest by wind generated waves. The emergency spillway has been provided in the event of accident or catastrophic rainfall only. It is not expected to be activated during the life of the facility. As designed, all discharges from the system will be through the pump house located on the southeast corner of the recycle pond.

3.3 SIZE AND HAZARD CLASSIFICATION

If a worst case failure of the gypsum pond dam were to occur, and the entire volume of the stack is released easterly into Coffeen Lake, the Coffeen Lake reservoir has adequate freeboard to accept this additional volume without overtopping the dam during flood events up to and including the 60 percent PMF. However, the power plant and several residences could potentially be impacted if the gypsum stack dam were to fail in a westerly direction. Considering the regulatory criteria established in Part 3702, the gypsum stack perimeter earthen dam and the gypsum stack "gypsum" dam are classified as intermediate-size Class I (high hazard potential) dams.

A failure of the recycle pond dam would discharge water to Coffeen Lake but it is not anticipated to result in loss of life or any significant economic damage. Breach analyses indicate that a failure of the recycle pond dam during a PMF event would be expected to result in an increase in the Coffeen Lake water surface elevation of not more than ½ inch. Accordingly, the recycle pond dam is classified as a small-size Class III (low hazard potential) dam.

3.4 PURPOSE OF THE DAMS

The dams will be used to dewater, store and dispose of flue gas desulphurization sludge (gypsum) from the Coffeen Power Station (the Plant). Gypsum will be transported to the Gypsum Pond Dam in slurry form (approximately 20 percent solids) and allowed to settle. Clarified process water will then be decanted to the recycle pond and returned to the Plant for reuse via a pipeline.

3.5 PERTINENT DATA

Pertinent data about the dams, appurtenant works, and reservoirs are presented in Table 3-1 and Table 3-2.

Table 3-1 Pertinent Data for the Gypsum Pond Earthen Dam (Based on the Construction of 2 Gypsum Cells)

Perimeter Ditches			Transfer Channel		
Bottom Width	15.00	feet	Bottom Width	32.00	feet
Top Width	73.50	feet	Top Width	86.00	feet
Depth	9.00	feet	Depth	9.00	feet
Outer Side Slope	3:1	H:V	Upstream Invert	623.00	feet
Inner Side Slope	3:1	H:V	Downstream Invert	622.00	feet
Upstream Invert	624.85	feet	Weir Elevation	625.00	feet
Downstream Invert	623.00	feet	Weir Length (at 2 ft height)	44.00	feet
Ditch slope	0.00050	ft/ft			
Bank Full Cross-sectional Area	378.00	sf	Dam		
Length of Each Ditch (Centerline)	3710.00	feet	Top of Dam Elevation	632	feet
Bank Full Volume of Each Ditch	32.19	acre-ft	Reservoir Surface Area	77.29	acres
Total Ditch length (Centerline)	7420.00	feet	Total Watershed Area	77.29	acres
Total Ditch Bank Full Volume	64.39	acre-ft	Dam Length	7720	feet
			Dam Height	13	feet
1.0 PMF Storm Event			0.5 PMF Storm Event		
Storm Duration	24	hours	Storm Duration	24	hours
Peak Outflow Discharge	1100.7	cfs	Peak Outflow Discharge	541.1	cfs
Total Discharge Volume	228.83	acre-ft	Total Discharge Volume	122.41	acre-ft
Peak WSEL in Perimeter Ditches	629.89	feet	Peak WSEL in Perimeter Ditches	628.23	feet
Freeboard over Max WSEL	2.11	feet	Freeboard over Max WSEL	3.77	feet
Wave Runup/Wind Setup	2.06	feet	Wave Runup/Wind Setup	2.06	feet
Adequate Freeboard?	YES		Adequate Freeboard?	YES	
100-yr Storm Event - Critical Duration			100-yr Storm Event - 24 Hour Du	ration	
Storm Duration	12	hours	Storm Duration	24	hours
Peak Outflow Discharge	92.6	cfs	Peak Outflow Discharge	62.9	cfs
Total Discharge Volume	50.91	acre-ft	Total Discharge Volume	57.01	acre-ft
Peak WSEL in Perimeter Ditches	626.07	feet	Peak WSEL in Perimeter Ditches	625.84	feet
Freeboard over Max WSEL	5.93	feet	Freeboard over Max WSEL	6.16	feet
Wave Runup/Wind Setup	2.06	feet	Wave Runup/Wind Setup	2.06	feet
Adequate Freeboard?	YES		Adequate Freeboard?	YES	

Note: The Critical Storm Duration is the duration of the rainfall event which produces the highest reservoir water surface elevation in the Gypsum Stack Perimeter Ditches for the given storm frequency. In each case, the starting normal pool elevation of the Recycle Pond is considered to be at elevation 624 ft.

Table 3-2 Pertinent Data for the Recycle Pond Dam (Based on the Construction of 2 Gypsum Cells)

Dam			3 Spillways- 6ft x 6ft inlet w/ 4ft d	ia outlet pip	e
Top of Dam Elevation	629	feet	Weir Length	22	feet
Invert of Reservoir Elevation	605	feet	Weir Elevation	624.00	feet
Reservoir Area at Invert	11.55	acres	Outlet Conduit Length	120	feet
Reservoir Area at Top of Dam	17.07	acres	Outlet Conduit Diameter (Inside)	48	inch
Total Reservoir Volume	341.91	acre-ft	Upstream Invert	615	feet
Volume at Elevation 624 ft	259.60	acre-ft	Downstream Invert	614	feet
Total Watershed Area	94.36	acres	Outlet Conduit Slope	0.00833	
Dam Length	3600	feet	·		
Dam Height	16	feet			
1.0 PMF Storm Event - Norma	l Pool at El	ev. 624 ft	1.0 PMF Storm Event - Normal	Pool at Elev	. 609 ft
Storm Duration	24	hours	Critical Storm Duration	24	hours
Peak Inflow	1261.6	cfs	Peak Inflow	1261.6	cfs
Peak Outflow	586.9	cfs	Peak Outflow	289.7	cfs
Peak Storage	315.47	acre-ft	Peak Storage	280.65	acre-ft
Peak WSEL (HEC-HMS)	627.45	feet	Peak WSEL (HEC-HMS)	625.34	feet
Freeboard over Peak WSEL	1.55	feet	Freeboard over Peak WSEL	3.66	feet
Wave Runup/Wind Setup	1.20	feet	Wave Runup/Wind Setup	1.20	feet
Adequate Freeboard?	YES		Water Released from Dam?	YES	
0.5 PMF Storm Event - Norma			0.5 PMF Storm Event - Normal		
Storm Duration	24	hours	Critical Storm Duration	24	hours
Peak Inflow	608.4	cfs	Peak Inflow	608.4	cfs
Peak Outflow	413.6	cfs	Peak Outflow	0	cfs
Peak Storage	286.48	acre-ft	Peak Storage	255.83	acre-ft
Peak WSEL (HEC-HMS)	625.69	feet	Peak WSEL (HEC-HMS)	623.75	feet
Freeboard over Peak WSEL	3.31	feet	Freeboard over Peak WSEL	5.25	feet
Wave Runup/Wind Setup	1.20	feet	Wave Runup/Wind Setup	1.20	feet
Adequate Freeboard?	YES		Water Released from Dam?	NO	
100-yr Storm Event - Normal	Pool at El	ev. 624 ft	100-yr Storm Event - Normal I	Pool at Elev.	619 ft
Critical Storm Duration	12	hours	Critical Storm Duration	24	hours
Peak Inflow	113.2	cfs	Peak Inflow	76.6	cfs
Peak Outflow	95.8	cfs	Peak Outflow	0	cfs
Peak Storage	269.36	acre-ft	Peak Storage	258.48	acre-ft
Peak WSEL (HEC-HMS)	624.63	feet	Peak WSEL (HEC-HMS)	623.94	feet
Freeboard over Peak WSEL	4.37	feet	Freeboard over Peak WSEL	5.06	feet
Wave Runup/Wind Setup					
wave Runup/wind Setup	1.20	feet	Wave Runup/Wind Setup Water Released from Dam?	1.20	feet

Note: The above variation in normal pool elevations for the Recycle Pond is for the purpose of documenting the water surface elevation which must be maintained in the recycle pond in order to prevent the release of water from the GMF for the above described storm events.

SECTION 4.0 OPERATIONS ACTIVITIES

4.1 INTRODUCTION

The operations plan describes the proposed operation of the Coffeen Gypsum Management Facility (GMF) which includes the gypsum pond and the recycle pond.

4.2 SITE OPERATIONS AND PERSONNEL

4.2.1 Site Operations

The GMF will receive gypsum slurry 24 hours per day, seven days per week. Routine facility maintenance and construction activities will generally be conducted during day shift hours. The crest widths for both the gypsum stack earthen dam and the recycle pond dam are 20 ft. In addition, multi-directional ramps are being provided for both structures so that they are readily accessible by inspection, maintenance and gypsum recovery equipment.

The Plant is a restricted access location. Additional fencing around the perimeter of the active sedimentation cells of the gypsum stack and the recycle pond will be erected to prevent unauthorized access to the GMF, which is also under surveillance by security personnel.

4.2.2 Personnel

The proposed GMF will be owned and operated by Ameren Energy Generating Company (Ameren). Corporate offices are located in St. Louis, Missouri. Overall responsibility for the GMF operation lies with Ameren management personnel.

4.3 GYPSUM MANAGEMENT FACILITY STARTUP

The major components of the proposed GMF consist of:

- The gypsum stack dam/impoundment;
- The recycle pond;
- The earthen transfer channel that connects the two structures and through which process water will be decanted from the gypsum stack into the recycle pond; and
- The recycle pond decant and pumphouse through which process water will be returned to the Plant for reuse.

Both the recycle pond and the gypsum stack dam will be constructed before gypsum is placed within the gypsum stack dam/impoundment.

Upon startup, it is likely that the gypsum stack impoundment will have no more than a few feet of water in the bottom to prevent the high density polyethylene (HDPE) geomembrane from moving. The gypsum slurry (approximately 20 percent solids) will be pumped from the

Plant to the gypsum stack via piping. The piping will be HDPE with a suitable pressure rating for the intended hydraulic and static head. The HDPE pipe will discharge the slurry into the impoundment, and gypsum will settle by gravity.

It will take approximately 10 months before the gypsum stack impoundment is filled to elevation 623 ft, the point where process water may begin flowing into the recycle pond via the HDPE-lined earthen channel connecting the two structures. As soon as water begins to fill the recycle pond, it will be pumped back to the Plant for reuse.

4.4 WATER BALANCE

The capacity of the recycle pond has been designed to accommodate all precipitation runoff from the entire gypsum pond/recycle pond area during a 2-week complete maintenance outage at the Coffeen Power Station (the Plant) followed by a 12-week outage of one of the two units. The runoff and excess water accumulated during this time can be stored within the recycle pond without discharging. The design is based on the maximum 3.5 month precipitation that has occurred in the area since 1950. This occurred in April, May, June and half of July, 1957 and consisted of 28.83 inches of rainfall.

The water balance has been carried out for the expected life of the Site. During the first nine or ten months of operation, the water balance is positive, meaning that there is more water entering the gypsum stack/recycle pond system through process water and precipitation than is leaving the system through process water return and evaporation. However, there is 15 ft of freeboard between the pump discharge and the emergency spillway. With proper water-level management, the water surface will remain well below discharge elevation. After this initial startup period, the water balance is negative, meaning that other water sources will need to be continually added to the process water makeup stream to maintain the volume necessary for transport of the gypsum slurry.

The water balance is of particular concern since the entire system is designed to be a closed loop with no discharges. (As previously noted, the recycle pond has been designed with an emergency spillway, but this is only to protect the structures in the event of an unforeseen accident or catastrophic rainfall event.) Table 3.5-2 lists the maximum water surface elevation allowed in the recycle pond in order to prevent the discharge of water for the 100-year storm event and the 0.5 PMF storm event.

4.5 GYPSUM MANAGEMENT FACILITY OPERATION

4.5.1 Routine Operations

Gypsum slurry will initially be discharged at the southwestern corner of the gypsum pond impoundment. Settled gypsum will gradually create a plane of material sloping gently towards the north end of the impoundment. Depending on the slope of the settled gypsum, the discharge pipe may be moved to other corners of the impoundment to evenly distribute the material. Care must be taken during the initial filling period so to ensure that the sand layer covering the ring drains is not disturbed. If necessary, the sand may be armored with larger washed aggregate or

the impoundment may be gradually filled with water to cover the sand prior to the discharge of gypsum slurry into the impoundment.

Once the gypsum plane reaches approximately elevation 627 ft (5 ft below the earthen dam crest), a track excavator or similar piece of equipment will be used to create the first gypsum berm and to form the perimeter ditch. Each gypsum berm will be approximately 10 ft in height and will effectively create a two-compartment impoundment within its perimeter. Gypsum for construction of the gypsum berm will be obtained from the settled material on the inside of the berm, creating an inner ditch. Gypsum slurry will then be discharged alternately into the inner ditch of each compartment. Gypsum will settle out into the inner ditch and clarified process water will flood the compartment to a depth of several feet. This water will be decanted to the perimeter ditch by way of an HDPE decant pipe which will discharge to a stilling well located at the toe of the gypsum stack.

As each compartment fills with settled gypsum, the discharge piping will be moved to the alternate compartment. The compartment, or sub-cell, that is not in service will be allowed to dewater and another gypsum berm will be constructed on top of the previous gypsum berm, effectively raising the gypsum stack another 10 ft. This alternating cycle of gypsum discharge, compartment dewatering and berm construction will continue. Gypsum will be deposited in the stack with an average dry density of approximately 74 lb/ft³. Drawing No. C-10201-25 provides a visual description of this process.

4.5.2 Piezometer Installation and Monitoring

The side slopes of the gypsum pond will be constructed with 3:1 side slopes. After consolidation of the settled gypsum over time, the final slopes should approach 3.75:1. The stability of each gypsum pond slope is <u>critically dependent</u> on the location of the phreatic surface which is anticipated to develop within the stack. Ring drains are intended to lower the phreatic surface so that it is located an adequate distance from the surface of the slope in order to maintain slope stability. In order to monitor the phreatic surface within the stack, piezometers will be installed on each side of the gypsum pond. The piezometers will be installed every 15 vertical feet up the slope (45 horizontal feet based on 3:1 side slopes) and will extend to a depth of at least 15 feet below the anticipated phreatic water surface elevation as shown in Figure 4-1. At the time of installation, each piezometer will be labeled with the "critical elevation" corresponding to the anticipated phreatic surface elevation at that location. The anticipated phreatic surface elevation is the water surface elevation which was used in the slope stability analysis of the gypsum pond. The water level in each piezometer will be read and recorded on a monthly basis. If at any time a reading is recorded higher than "critical elevation" for that specific peizometer, the design engineer must be contacted immediately for evaluation of the reading. Any readings above the "critical elevation" may be indicative of improper ring drain function and/or slope instability which could lead to a failure of the gypsum stack. Therefore, it is critical that the piezometers are installed in accordance with the construction plans and specifications and monitored in accordance with this manual. It may be necessary to install additional subdrainage to maintain the phreatic surface at the desired level within the gypsum stack.

Figure 4-1 Anticipated Phreatic Surface in Gypsum Pond	ł
Refer to figure at the end of the report text.	

4.6 DAM INSPECTIONS

The inspection program includes two types of dam inspections. The first is regularly conducted by the dam operator and is referred to as an Operation and Maintenance Inspection. The second type of inspection, referred to as the Engineering Inspection, is conducted by a qualified engineer approved by IPGC. All engineering inspection reports must be signed and sealed by an Illinois Registered Professional Engineer.

The dam operator will perform monthly Operation and Maintenance Inspections of the gypsum pond perimeter earthen dam <u>and the gypsum berms and side slopes</u> during the operating life of the structure. During these inspections, the gypsum stack ditches and the transfer channel will also be examined for signs of erosion and liner degradation. The "operating life of the structure" will be considered to cease upon covering of the gypsum with an HDPE/soil cover. Engineering Inspections will be conducted on an annual basis during the operating life of the structure and will continue after covering of the gypsum pond until authorization to abandon the structure is received from IDNR/OWR.

4.6.1 Operation and Maintenance Inspection

Occasional "walk-around" inspections of the dams and appurtenant works are to be made by the dam operator. During these inspections, a checklist of items to be maintained and items to be observed should be recorded. Appendix A provides an example of the Operation and Maintenance Inspection Checklist to be utilized for these inspections. If any of the following items are found to be unusual or are cause for concern, the Shift Supervisor should be notified and the Emergency Action Plan should be immediately consulted for guidance on an appropriate course of action.

Frequency: Operation and maintenance inspections will be performed by the dam operator on a monthly basis and also during and after unusual events such as heavy rainfall or an earthquake.

Inspection Items: During each inspection the following items should be noted in particular.

- 1. Water Level Maximum reservoir levels as a result of heavy rainfall should be recorded.
- 2. Earth Embankment Walk the crest, side slopes and downstream toe of the dam concentrating on surface erosion, seepage, cracks, settlement, slumps, slides, and animal burrows. These are described as follows:
 - Surface Erosion Removal of vegetative cover by water action or pedestrian or vehicle usage forming deep ruts or gullies.
 - Seepage The passage of water through and/or underneath the earth embankment abutment and natural groundline or at the contact between the embankment and

- outlet works. It can be indicated by cattails or other wet environmental vegetation, erosion, channelization, or slumping on the embankment face.
- Cracks Deep cracks usually indicate the movement of the dam and/or the foundation and can be in either the longitudinal (along the length of the dam) or transverse (across the dam) directions. Cracking can be an indicator of the beginning of slumps. Shallow cracks may develop during the summer when the surface soils of the embankment become severely dried and are usually of no concern in regard to the safety of the dam.
- Settlement Settlement is indicated by depressions or low spots and can be signs
 of consolidation of the dam or foundation or the loss of material beneath the
 settlement area.
- Slumps/Slides A slow or sudden movement of the earth embankment slope on either face toward the toe of the dam.
- If seepage indicates the presence of soil particles, or if deep cracks, settlement, slumps, or slides are noticed, a qualified engineer should be contacted immediately for consultation.
- Animal Burrows Animal burrows result in a loss of earth embankment material and can provide seepage paths for water through the embankment.
- 3. Gypsum Embankment Walk the crest, side slopes and downstream toe of the dam concentrating on surface erosion, seepage, cracks, settlement, slumps, slides and animal burrows. The descriptions for these are the same as for earth embankment.
- 4. Vegetation Grass should be a thick vigorous growth to stabilize the earth embankment soils and prevent erosion from occurring. Note the height of the grass; if greater than 1 foot a mowing of the area should be scheduled before the next inspection. There should be NO trees on the earth embankment and NONE within a minimum of 20 feet of the embankment toes or other structures. The gypsum embankment will not be seeded and is not expected to have any vegetation.
- 5. Gypsum Stack piezometers should be inspected for any damage or loss of function. Damaged piezometers must be promptly repaired or replaced since their function is critical to ensuring stability of the gypsum stack.
- 6. The water level in each Gypsum Stack piezometer must be measured and recorded during each monthly inspection. If the water level in any piezometer is above the "critical elevation" as discussed in Section 4.5.2 of this plan, the Ameren Technical Services Superintendent should be notified and the Emergency Action Plan should be immediately consulted for guidance on an appropriate course of action.

- 7. Gypsum Pond LD/LCRS Drains The change in location or amount of flows discharging from the Leak Detection/Leachate Collection Recovery System (LD/LCRS) should be recorded. If a significant change has occurred, a qualified engineer should be contacted for consultation.
- 8. Gypsum Stack Ring Drains The change in location or amount of flows discharging from the Ring Drains should be recorded. If a significant change has occurred, a qualified engineer should be contacted for consultation.
- 9. Gypsum Stack Fixed Decant Check the alignment and supports for the pipe. Record the amount of flows discharging from the pipe and any erosion or scour around the discharge point.
- 10. Gypsum Stack Perimeter Ditch The perimeter ditch should have a consistent prismatic shape for the entire length. Inspect the perimeter ditch for evidence of erosion, sediment deposition and irregularity in channel geometry, especially in the vicinity of siphon, decant or ring drain outfall structures. If irregularities are noted, repairs should be scheduled and completed.
- 11. Drawdown Facilities Check to make sure that the drawdown stop logs in the transfer ditch are undamaged, operating well and allowing for the free flow of water over them. Confirm during inspections the valves are opened and closed at least quarterly.
- 12. Transfer Channel Check for any debris or other obstructions which may block or restrict the free flow of water. Check for any pools or undulation of the floor of the channel.
- 13. Recycle Pond Decant Check for any debris or other obstructions around the Recycle Pond decant which may block or restrict the free flow of water. The emergency dewatering valve should be lubricated. If there is no return water in the pipe, the emergency dewatering valve should be exercised. Record the physical and operating conditions of the system.
- 14. Recycle Pond Drop Inlet Spillways Check for any debris or other obstructions around the inlet crest and at the bottom of the drop inlet which may block or restrict the free flow of water. Check for the development of any rusty areas on the concrete, and seepage, cracking, breaking, or spalling of the concrete. Check for settlement or cracking of the crest. Check for any debris in the pipes which may restrict the flow of water. Check for any tears or leaks in the HDPE liner covering the concrete.
- 15. Recycle Pond Rip Rap Basin Check for any debris or other obstructions in the riprap basin which may block or restrict the free flow of water. Check to make sure that the rip rap is remaining in a uniform position. Freeze/thaw action or flow over the rip rap may tend to lift or fracture, thus requiring replacement or leveling to maintain the necessary level of protection. NO trees or woody vegetation should be growing through the rip rap.

- 16. Fences Check for damage, accumulated debris, operation of gates and locks, and adequacy of locations (this may change with time as people access the area or development occurs in the area).
- 17. Perimeter Check the perimeter of the dams for a distance of at least 100 feet beyond the toe for signs of seepage or boils.
- 18. HDPE Liner Wherever exposed, the HDPE Liner should be inspected for tears, gouges, protrusions under the liner and abrasion.

Records: A log book of activities occurring at the dam is to be kept current by the dam operator. The log book should be reviewed during the Engineering Inspection. This book should contain at the least the following documentation:

- 1. Completed operation and maintenance inspection checklists
- 2. Readings from all piezometers on the Gypsum Stack
- 3. Additional visual observations
- 4. A list of maintenance performed
- 5. A list of any unusual occurrences at the dam
- 6. Copies of the engineering inspection reports

4.6.2 Engineering Inspection

The engineering inspection is to be conducted by a qualified engineer approved by Ameren. The inspection will provide a thorough evaluation of the dam condition and appurtenances. Appendix B is an example of the inspection report form which is to be utilized for these inspections.

Frequency: The Gypsum Pond Dam is a Class I, High Hazard Potential dam and is to be inspected by an Illinois Registered Professional Engineer at least once per year. The Recycle Pond Dam is classified as a Class III, Low Hazard Potential dams and is to be inspected by an Illinois Registered Professional Engineer at least once every five years.

Inspection Items: The engineer will thoroughly inspect all of the items noted in Section 4.6.1 Operation and Maintenance Inspection.

Records: The Dam Inspection Report form from IDNR-OWR "Guidelines and Forms for Inspection of Illinois Dams" (a copy of which is included in Appendix B), will be completed by the inspecting engineer and will be signed and sealed by an Illinois Registered Professional Engineer. This report will document problem areas and deficiencies; recommend remedial actions for problem areas; and establish time requirements for dealing with the problems. The original report will be retained in Dynegy Operating Company (DOC) files, and a copy of the report will be submitted to the Illinois Department of Natural Resources, Office of Water Resources.

4.6.3 Review of Emergency Action Plan

The emergency action plan should be reviewed annually to assure that all contacts, addresses and telephone numbers are current. Changes in the adjacent land use should also be noted and may dictate the need for revisions to the plan. Changes to the plan should be made as appropriate but only with the concurrence of the Montgomery County Emergency Services and Disaster Agency and of the Illinois Department of Natural Resources, Office of Water Resources. Copies of any revisions should also be forwarded to all personnel and known emergency responders that possess previous versions the plan.

SECTION 5.0 MAINTENANCE ACTIVITIES

Timely repairs are a must after problem areas have been identified. The dam operator is to perform the work required to correct items noted in the operation and maintenance inspections and engineering inspections. Such items include repairing erosion of the gypsum slopes, mowing, seeding, tree and brush removal, replacing rip rap, repairing fences and locks, clearing debris, etc. The maintenance activities specified in the following sections are minimum requirements. NOTE: NO alterations or repairs to structural elements should be made without the assistance of the Ameren Chief Dam Safety Engineer and the concurrence of the Illinois Department of Natural Resources, Office of Water Resources.

Debris: Remove all trash, logs and other debris which may obstruct flow into the principal spillway pipes and drop inlets, or block passage from their discharge channels.

Rip Rap: Replenish rip rap as needed to provide adequate protection against erosion.

Vegetation Control

1. Maintain a good grass cover on the embankment by seeding, fertilizing and mulching areas which are refilled, barren, or thinly vegetated. Seeding mixtures used for maintenance reseeding shall result in a cover compatible with adjacent cover. The seeding mixture specified at the time of the dam's construction was IDOT Standard Specifications Class 1A (Salt Tolerant Lawn Mixture) as follows:

IDOT Class 1A Salt To	olerant Lawn Mixture
Bluegrass	60 lb/acre
Perennial Ryegrass	20 lb/acre
Dawsons Red Fescue	20 lb/acre
Scaldis Hard Fescue	20 lb/acre
Fults Salt Grass	60 lb/acre

- 2. Grassed areas such as the embankment and the areas beyond the embankment toes for a distance of at least 20 feet should be mowed at least twice annually or at any time the height of the grass exceeds 1 foot.
- 3. All erosion areas will be filled and compacted, reseeded, fertilized and mulched to establish a thick erosion resistant cover.
- 4. Remove all trees and brush growing on the dam embankment to prevent development of a root system which could provide seepage paths. Herbicides utilized for tree and brush control are discussed in Appendix D.
- 5. Keep the riprap basin clear of weeds, brush, and trees.

6. Clear all brush and trees for a distance of approximately 20 feet beyond the toe of each dam.

Animal Damage: Fill rodent holes and other animal burrows with compacted clayey soil and reseed. If rodents become a nuisance, an effective rodent control program as approved by the Illinois Department of Natural Resources District Wildlife Biologist should be implemented.

Signs: All warning signs shall be maintained (repaired, painted, or replaced) as needed.

Gypsum Slopes: Erosion of the gypsum slopes will be evident with the presence of erosion rills. Erosion rills should be filled with additional gypsum material and graded to conform with the design slope.

Piezometers: All piezometers on the gypsum stack shall be inspected for signs of damage or displacement. Non-functioning piezometers shall immediately be replaced.

APPENDIX A LOCATION MAP

APPENDIX B OPERATION AND MAINTENANCE INSPECTION CHECKLIST

OPERATION AND MAINTENANCE INSPECTION CHECKLIST

Dam Name (circle one):	Gypsum Pond Dam	Recycle Pond Dam
Date:	Time:	
Name of Inspector:		
Reservoir Elevation:	feet	

<u>ITEM</u>	<u>NO</u>	<u>YES</u>	<u>IF YES</u>
Record Piezometer Readings for Gypsum Stack. Are any readings above the critical level? (see section 4.5.2 of O&M Manual)			Contact Manager, Environment & Chemistry and notify Hanson Professional Services
Note the condition of the Piezometers on the Gypsum Stack. Any damage?			Contact Manager, Environment & Chemistry
Deep Surface Cracks			Contact Manager, Environment & Chemistry
Slump or Slide on the upstream or downstream face			Contact Manager, Environment & Chemistry
Erosion from runoff, wave action or traffic			Repair and stabilize
Embankment, abutment or spillway seepage			Contact Manager, Environment & Chemistry
Seepage or flows of muddy water			Contact Manager, Environment & Chemistry
Uneven settlement			Contact Manager, Environment & Chemistry
Trees, brush or burrow holes on the embankment or in the riprap basin			Remove trees and brush, fill holes
Transfer channel or Spillway pipes blocked			Clear immediately
Damage to stop logs			Repair or replace
Damage to HDPE Liner			Repair and schedule engineer inspection
Settlement or displacement of Gypsum Pond fixed decant pipes or outlets			Schedule engineer inspection
Discharge from Gypsum Pond LD/LCRS Drains?			Record discharge rate for each outlet (time to fill bucket)
Discharge from Gypsum Pond Ring Drains?			Record discharge rate for each outlet (time to fill bucket)
Gypsum Stack Perimeter Ditch erosion			Schedule repair
Problems with Recycle Pond spillways			Contact Manager, Environment & Chemistry
Problems with Recycle Pond decant			Contact Manager, Environment & Chemistry
Height of grass (inches)		inches	If more than 1 foot, schedule mowing
Damage to fencing, gates and locks or other access restriction measures			Contact Manager, Environment & Chemistry
Confirm drawdown facilities are opened and closed at least quarterly.			Contact Manager, Environment & Chemistry

APPENDIX C ENGINEERING INSPECTION FORMS

Dam Inspection Report

Name of Dam	Dam Identification Number		
Permit Number	Class of Dam		
Location NW 1/4 Section 11			
Owner			
Name	Telephone Number (Day)		
Street	Telephone Number (Night)		
	County Montgomery		
City Zip Coo	de		
Type of Dam			
Type of Spillway			
Weather When Inspected			
Pool Elevation When Inspected			
Tailwater Elevation When Inspected			
	Inspection Personnel:		
	N. Total		
	Name Title		

Professional Engineer's Seal

The Department of Natural Resources is requesting information that is necessary to accomplish the statutory purpose as outlined under the River, Lakes and Streams Act, 615 ILCS 5 (1994 State Bar Edition). Submittal of this information is REQUIRED. Failure to provide the required information could result in the initiation of non-compliance procedures as outlined in Section 702.160 of the "Rules for Construction and Maintenance of Dams". This form has been approved by the State Forms Management Center.

CONDITION CODES

EC	-	Emergency Condition. A serious dam safety condition exists that needs immediate action. Emergency measures
	_	implemented as instructed by Chief Dam Safety Engineer; such as, pool draw down, work stoppage, plant stoppage.
<u>NE</u>	=	No evidence of a problem
GC	=	Good condition
<u>MM</u>	=	Item needing minor maintenance and/or repairs within the year, the safety or integrity of the item is not yet imperiled
<u>IM</u>	=	Item needing immediate maintenance to restore or ensure its safety or integrity. Remediation should be completed within 1 month.
EC	=	Emergency condition which if not immediately repaired or other appropriate measures taken could lead to failure of the dam
<u>OB</u>	=	Condition requires regular observation to ensure that the condition does not become worse
<u>NA</u>	=	Not applicable to this dam
<u>NI</u>	=	Not inspected - list the reason for non-inspection under deficiencies
EC	=	Emergency Condition. A serious dam safety condition exists that needs immediate action. Emergency measures implemented as instructed by Chief Dam Safety Engineer; such as, pool draw down, work stoppage, plant stoppage.

GYPSUM STACK - EARTH EMBANKMENT

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Surface Cracks			
Vertical and Horizontal Alignment of Crest			
Unusual movement or Cracking at or Beyond Toe			
Sloughing or Erosion of Outer Embankment Slopes			
Upstream Face Slope Protection (HDPE Liner)			
Seepage			
Animal Damage			

GYPSUM STACK - EARTH EMBANKMENT

(Continued)

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Vegetative Cover			

GYPSUM STACK - GYPSUM EMBANKMENT

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Surface Cracks			
Vertical and Horizontal Alignment of Crest			
Unusual movement or Cracking at or Beyond Toe			
Sloughing or Erosion of Outside Embankment Slopes			
Sloughing or Erosion of Inside Embankment Slopes			
Seepage			
Animal Damage			

GYPSUM STACK - GYPSUM EMBANKMENT (Continued)

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Condition of Piezometers on Gypsum Stack			
Piezometer Readings on Gypsum Stack Above Critical Level?			

<u>GYPSUM STACK – PERIMETER DITCH</u>

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Ditch Geometry (15 ft bottom width, 3:1 slopes, 8-9 ft depth)			
Concrete Apron at ring drain outlets			
Ring Drain Discharge Pipes			
Stilling Wells for Fixed Decants			

TRANSFER CHANNEL - (between gypsum stack and recycle pond)

Drop Inlet	Structure	X Overflow Spillway Structure	Gated
ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Debris			
Side Slope Stability			
HPDE Liner			
HDPE Liner Welds			
Stop Logs			
Differential Settlement			

<u>RECYCLE POND - EMBANKMENT</u>

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Surface Cracks			
Vertical and Horizontal Alignment of Crest			
Unusual movement or Cracking at or Beyond Toe			
Sloughing or Erosion of Outer Embankment Slopes			
Upstream Face Slope Protection (HDPE Liner)			
Seepage			
Animal Damage			

<u>RECYCLE POND - EMBANKMENT</u>

(Continued)

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Vegetative Cover			

RECYCLE POND - PRINCIPAL SPILLWAY (Left, Looking Downstream)

X Drop Inlet Structure		Overflow Spillway Structure	Gated		
ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE		
Alignment of Structure Walls					
Construction Joints					
Differential Settlement					
Erosion, Spalling, Cavitation					
Joint Separation					
Seepage Around or into Conduit					
Surface Cracks					

RECYCLE POND - PRINCIPAL SPILLWAY (Left, Looking Downstream)

(Continued)

X Drop Inlet	Structure	Overflow Spillway Structure	Gated
ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Structural Cracks			
	†		

RECYCLE POND - PRINCIPAL SPILLWAY (Center)

X Drop Inlet Structure		Overflow Spillway Structure	Gated
ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Alignment of Structure Walls			
Construction Joints			
Differential Settlement			
Erosion, Spalling, Cavitation			
Joint Separation			
Seepage Around or into Conduit			
Surface Cracks			

RECYCLE POND - PRINCIPAL SPILLWAY (Center)

(Continued)

X Drop Inlet Structure		Overflow Spillway Structure	Gated
ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Structural Cracks			

RECYCLE POND - PRINCIPAL SPILLWAY (Right, Looking Downstream)

X Drop Inlet Structure		Overflow Spillway Structure	Gated		
ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE		
Alignment of Structure Walls					
Construction Joints					
Differential Settlement					
Erosion, Spalling, Cavitation					
Joint Separation					
Seepage Around or into Conduit					
Surface Cracks					

<u>RECYCLE POND - PRINCIPAL SPILLWAY (Right, Looking Downstream)</u> (Continued)

X Drop Inlet Structure		Overflow Spillway Structure	Gated
ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Structural Cracks			

RECYCLE POND - ENERGY DISSIPATOR

X Principal Spillway		Outlet Works Type:	FHWA HEC-1	4, Riprap Basin	
ITEM	CONDITION	DEFICIENCIE:	S	RECOMMENDED REMEDIAL MEASU SCHEDULE	RES &
Riprap					
Outlet Channel					
Debris					

RECYCLE POND - DECANT STRUCTURE

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE				
Alignment							
Connection to Bollard							
Debris in Inlets							
Condition of Pipe							
Condition of Liner Beneath Pipe							
Connection to Ballast							
Connection of Pipe Boot to Liner							

RECYCLE POND - DECANT STRUCTURE

(continued)

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Seepage Around or into Conduit			

<u>RECYCLE POND – WATER LEVEL GAGE STRUCTURE</u>

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE				

APPENDIX D HERBICIDES

HERBICIDES

Site personnel should check with the Illinois Department of Natural Resources, Regional Fisheries Biologist and the Regional Wildlife Biologist before using any herbicide. Read the product label prior to use and follow the use directions and precautions accordingly.

On March 1, 1979 the U.S. Environmental Protection Agency (U.S.E.P.A.) halted the use of the herbicide 2, 4, 5-T in parks and recreation areas. The use of silvex (2, 4, 5-TP) around water has also been banned.

The Agronomy Department at the University of Illinois and the Aquatic Biology Section of the Department of Natural Resources, Office of Scientific Research and Analysis indicate that the herbicides containing the 2, 4-D or 2, 4-DP are legal for use in parks and recreation areas and effective for controlling brush and woody growth. Some examples of approved herbicides are:

- 1. Tordon RTU by DOW Chemical. (Can be obtained with blue dye.)
- 2. WEEDONE 170 by Union Carbide
- 3. WEEDONE, 2, 4-DP by Union Carbide
- 4. A 1% to 2% solution of ROUNDUP
- 5. Garlon by DOW Chemical
- 6. Banvel by Sandoz

Your distributor may carry brand name herbicides other than those listed above. Be certain that the product does not contain the ingredients 2, 4, 5-T or 2, 4, 5-TP. An example of an unacceptable product is ESTERON 2, 4, 5 by DOW Chemical.

APPENDIX E CONSTRUCTION DRAWINGS



Appendix G: Photos of 2015 Sloughing Repairs



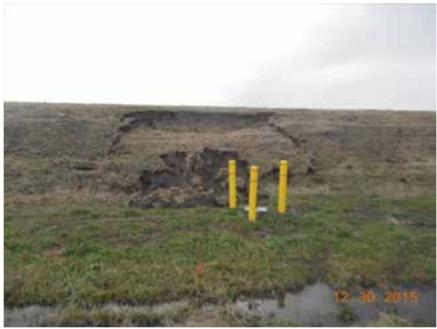


Figure G.1. Photo of 2015 sloughing prior to repairs.



Figure G.2. Photo of 2015 sloughing prior to repairs.

AECOM



Figure G.3. Photo of 2015 sloughing area after repairs.



Figure G.4. Photo of 2015 sloughing area after repairs.

ATTACHMENT C

Coffeen Power Plant – GMF Pond System's Chemical Constituents

In accordance with 35 I.A.C. 845.230(d)(2)(C), IPGC is submitting available/existing analyses of "the chemical constituents of all waste streams, chemical additives and sorbent materials entering or contained in" the GMF Stack Pond.

A list of the chemical constituents' analyses contained in the CCR surface impoundment can be found in Appendix A. As determined through antidegradation studies, this list contains chemical constituents found in the surface free liquid and the subsurface free liquids. IPGC is also including a list of chemical additives, sorbent materials and waste streams that were submitted in the facility's NPDES permit applications to IEPA within the past ten years at a minimum and/or listed in the current NPDES permit (IL0001554) in Appendix B.

Appendix A: Chemical Constituents Contained in the GMF Stack Pond

		GMF Pond System					
Pollutant	Units	Stack Pond Surface Free Liquids Average Concentration		Stack Pond Subsurface Free Liquids Average Concentration		Recycle Pond Surface Free Liquids Average Concentration	
Ammonia	mg/L		3045		1120		2885
Arsenic	mg/L	'	0.025	'	0.025	<	0.025
Barium	mg/L		0.057		0.058		0.060
Boron	mg/L		47.2		24.7		45.4
Cadmium	mg/L	<	0.034		0.016		0.034
Chloride	mg/L		1430		734		1355
Chromium	mg/L	<	0.010	<	0.005	<	0.008
Chromium (hexavalent)	mg/L	<	0.004	<	0.005		0.002
Copper	mg/L		0.013		0.008		0.013
Cyanide	mg/L	<	0.005	<	0.005	<	0.005
Fluoride	mg/L		43.2		22.3		40.3
Iron	mg/L		0.945		1.78		16.1
Lead	mg/L	<	0.015	<	0.015	<	0.015
Manganese	mg/L		59.1	<	30.4		58.7
Mercury	mg/L		0.00008		0.0005		0.00003
Nickel	mg/L		0.363		0.176		0.365
Nitrate	mg/L		36.5		37.6		35.8
Nitrite	mg/L		0.580		0.060		0.050
Nitrogen, Total Kjeldahl	mg/L		3000		1300	<	2850
Oil and Grease	mg/L	<	2.00	<	3.00	<	2.00
рН	SU		5.84		6.85		4.36
Phenols	mg/L	<	0.005	<	0.005	<	0.005
Phosphorus	mg/L		0.111		0.209	<	0.100
Selenium	mg/L		0.630		0.328		0.601
Silver	mg/L	٧	0.001	'	0.001	<	0.001
Sulfate	mg/L		16900		10100		16200
TDS	mg/L		23450		13500		22400
TSS	mg/L		11.4		18.5		6.6
Zinc	mg/L		0.307		0.134		0.363

^{*}Used https://calstormcompliance.com/ph-averaging-tool

Appendix B: List of Chemical Additives, Waste Streams and Sorbent Materials

Chemical Additives
Bulab 9626 (corrosion and scale control)
Bulab 7041 (Water soluble polycarboxylate)
Nalco 71D5 PLUS (Anti-foaming agent)

Waste Streams and Sorbent Materials*
Coal combustion byproduct Landfill Leachate
Closed Ash Pond 2 Dewatering
Wet FGD
Low Pressure Surface Water Makeup
Limestone Runoff Pond

^{*}No sorbent materials



Gypsum SDS Number: 1.0

Revision Date: 03/2018

Safety Data Sheet

Section 1 Identification of the Substance and of the Supplier

1.1 **Product Identifier**

Product Name/Identification:	FGD Gypsum
Synonyms:	Gypsum, calcium sulfate dihydrate, calcium sulphate dihydrate, gesso, alabaster, plaster of Paris.
Formula:	UVCB Substance

1.2 Relevant Identified Uses of the Substance or Mixture and Uses Advices Against

Relevant Identified Uses:	Component of wallboard, concrete, roofing material, bricks, cement kiln feed, agricultural amendment.
Uses Advised Against:	None known.

1.3 **Details of the Supplier of the SDS**

Manufacturer/Supplier:	Dynegy, Inc.
Street Address:	601 Travis Street, Suite 1400
City, State and Zip Code: Houston, TX 77002	
Customer Service Telephone:	800-633-4704

Preparation Date: 02/27/2018



Section 2 Hazards Identification

2.1 Classification of the Substance

GHS Classification(s) according to OSHA Hazard Communication Standard (29 CFR 1910.1200):

- STOT-SE Category 3 (Respiratory Irritation)
- STOT-RE Category 1 (Lungs)
- Carcinogen Category 1A

2.2 Label Elements

Labelling according to 29 CFR 1910.1200 Appendices A, B and C*		
Hazard Pictogram(s):		
Signal word:	DANGER	
	May cause respiratory irritation.	
Hazard Statement(s):	Causes damage to lungs after repeated/prolonged exposure via inhalation.	
	May cause cancer of the lungs.	
	Obtain special instructions before use.	
	Do not handle until all safety precautions have been read and understood. Do not breathe dust.	
	Wash hands thoroughly after handling.	
	Do not eat, drink or smoke when using this product.	
Precautionary	Wear protective gloves/protective clothing/eye protection/face protection.	
Statement(s):	Do not eat drink or smoke when using this product.	
	Use outdoors or in a well-ventilated area.	
	If inhaled: Remove to fresh air and keep comfortable for breathing. Get medical advice/attention if you feel unwell.	
	Store in a secure area.	
	Dispose of product in accordance with local/national regulations.	

The following elements may be present in trace amounts as oxides: aluminum, calcium, iron, magnesium, nickel, phosphorus, potassium, silicon, sulfur, titanium, and vanadium. The exact composition of the gypsum will be dependent on the fuel source and flue additives composed of many constituents.

Preparation Date: 02/27/2018



Listed Carcinogens:

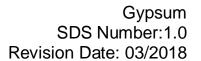
-Respirable Crystalline Silica

IARC: [Yes] NTP: [Yes] OSHA: [Yes] Other: (ACGIH) [Yes]

Section 3 Composition/Information on Ingredients

Substance	CAS No.	Percentage (%)	GHS Classification
Calcium sulfate, dihydrate	10104-14-1	90 - 99%	STOT – Single Exposure Category 3 (Respiratory Irritation)
Crystalline Silica	14808-60-7	≥0.1 - 3%	STOT – Repeated Exposure Category 1 (Lung) Carcinogen, Category 1A
Silica, crystalline respirable (RCS)	crystalline respirable (RCS) 14808-60-7		STOT – Repeated Exposure Category 1 (Lungs) Carcinogen Category 1A
Fly Ash	68131-74-8	<2%	STOT – Single Exposure Category 3 (Respiratory Irritation)

Footnote 1: The percentage of respirable crystalline silica has not been determined. Therefore, a GHS classification of Carcinogen, Category 1A has been assigned.





Section 4
First Aid Measures

4.1 Description of First Aid Measures

Inhalation: If product is inhaled and irritation of the nose or coughing occurs, remove fresh air. Get medical advice/attention if respiratory symptoms persist.	
Skin Contact:	If skin exposure occurs, wash with soap and water.
Eye Contact:	If product gets into the eye, rinse copiously with water for several minutes. Remove contact lenses, if present and easy to do. Seek medical attention/advice if irritation occurs or persists.
Ingestion:	No specific first aid measures are required.

4.2 Most Important Health Effects, Both Acute and Delayed

Acute Effects: Short-term airborne exposure to FGD gypsum dust may cause respiratory irritation. Direct exposure can dry and irritate the skin and cause dermatitis or eye irritation through mechanical abrasion.

Chronic Effects: Chronic (long-term) exposure to FGD gypsum may cause lung damage from repeated exposure. Prolonged inhalation of dusts containing respirable crystalline silica above certain concentrations may cause lung disease (silicosis) and lung cancer.

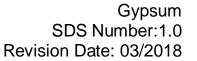
4.3 Indication of Any Immediate Medical Attention and Special Treatment Needed

Seek first aid or call a doctor or Poison Control Center if contact with eyes occurs and irritation remains after rinsing. Get medical advice if inhalation occurs and respiratory symptoms persist.

Section 5
Firefighting Measures

5.1 Extinguishing Media

Suitable Extinguishing Media:	Product is not flammable. Use extinguishing media appropriate for surrounding fire.	
Unsuitable Extinguishing Media:	Not applicable, the product is not flammable.	





5.2 Special Hazards Arising from the Substance or Mixture

Hazardous Combustion Products:	Above 1450°C (~2600°F), gypsum decomposes to calcium oxide and sulfur dioxide.
--------------------------------	--

5.3 Advice for Firefighters

and Precautions for Firefighters: approved or equivalent) and full protective gear.

Section 6 Accidental Release Measures

6.1 Personal Precautions, Protective Equipment and Emergency Procedures

Personal precautions/Protective Equipment:	See Section 8.2.2 Individual Protective Measures. For concentrations exceeding Occupational Exposure Levels (OELs), use a self-contained breathing apparatus (SCBA).
Emergency procedures:	Use scooping, water spraying/flushing/misting or ventilated vacuum cleaning systems to clean up spills. Do not use pressurized air.

6.2 Environmental Precautions

Environmental precautions:	Prevent contamination of drains or waterways and dispose according to local and national regulations.
----------------------------	---

6.3 Methods and Material for Containment and Cleaning Up

Methods and materials for containment and cleaning up:	Do not use brooms or compressed air to clean surfaces. Use dust collection vacuum and extraction systems. Large spills of dry product should be removed by a vacuum system. Dampened material should be removed by mechanical means and recycled or disposed of according to local and national regulations.
--	---

See Sections 8 and 13 for additional information on exposure controls and disposal.



Section 7 Handling and Storage

7.1 Precautions for Safe Handling

Practice good housekeeping. Use adequate exhaust ventilation, dust collection and/or water mist to maintain airborne dust concentrations below permissible exposure limits (note: respirable crystalline silica dust may be in the air without a visible dust cloud).

Do not permit dust to collect on walls, floors, sills, ledges, machinery, or equipment. Maintain and test ventilation and dust collection equipment. In cases of insufficient ventilation, wear a NIOSH approved respirator for silica dust when handling or disposing dust from this product. Avoid contact with skin and eyes. Wash or vacuum clothing that has become dusty. Avoid eating, smoking, or drinking while handling the material.

7.2 Conditions for Safe Storage, Including any Incompatibilities

Minimize dust produced during loading and unloading.

Section 8 Exposure Controls/Personal Protection

8.1 Control Parameters

OCCUPATIONAL EXPOSURE LIMITS					
SUBSTANCE		OSHA PEL TWA (mg/m³)	NIOSH REL TWA (mg/m³)	ACGIH TLV TWA (mg/m³)	CA - OSHA PEL (mg/m³)
Particulates Not Otherwise Regulated	Total	15	15	10	10
other wide regulated	Respirable	5	5	3	5
Respirable Crystalline Silica	Total Respirable	0.05-	0.05	0.025	0.05
Calcium Sulfate, anhydrous (CAS# 7778-18-9)	Total Dust	*	10	10	*
	Respirable	*	5	-	*
Note: In the absence of a CA-PEL, the value for Particulates Not Otherwise Regulated (PNOR) is applied.					

8.2 Exposure Controls

8.2.1 Engineering Controls



Gypsum SDS Number:1.0 Revision Date: 03/2018

Provide ventilation to maintain the ambient workplace atmosphere below the occupational exposure limit(s). Use general and local exhaust ventilation and dust collection systems as necessary to minimize exposure.

8.2.2 Personal Protective Equipment (PPE)

Respiratory protection:	Wear a NIOSH approved particulate respirator if exposure to airborne particulates is unavoidable and where occupational exposure limits may be exceeded. If airborne exposures are anticipated to exceed applicable PELs or TLVs, a self-contained breathing apparatus or airline respirator is recommended.	
Eye and face protection:	If eye contact is possible, wear protective glasses with side shields. Avoid contact lenses.	
Hand and skin protection:	Wear gloves and protective clothing. Wash hands with soap and water after contact with material.	



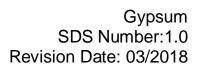
Section 9
Physical and Chemical Properties

9.1 Information on Basic Physical and Chemical Properties

Property: Value	Property: Value		
Appearance (physical state, color, etc.): White or gray cake-like material	Upper/lower flammability or explosive limits: Not applicable		
Odor: Odorless	Vapor Pressure (Pa): Not applicable		
Odor threshold: Not applicable	Vapor Density: Not applicable		
pH (25 °C) (in water): 6 - 8	Specific gravity or relative density: 2.0 - 2.9		
Melting point/freezing point (°C): 128	Water Solubility: 0.1 – 0.3%		
Initial boiling point and boiling range (°C): >163	Partition coefficient: n-octane/water: Not determined		
Flash point (°C): Not determined	Auto ignition temperature (°C): Not applicable		
Evaporation rate: Not applicable	Decomposition temperature (°C): 1450		
Flammability (solid, gas): Nonflammable/non-combustible	Viscosity: Not applicable		

Section 10 Stability and Reactivity

10.1 Reactivity:	Avoid contact with strong acids or oxidizers and diazomethane.
10.2 Chemical stability:	The material is stable under normal use conditions.
10.3 Possibility of hazardous reactions:	The material is a relatively stable, inert material; polymerization will not occur.
10.4 Conditions to avoid:	Product can become airborne in moderate winds. Dry material should be stored in silos. Materials stored out of doors should be covered or maintained in a damp condition.
10.5 Incompatible materials:	Acids, ammonium salts, diazomethane, phosphorus and aluminum metal.
10. 6 Hazardous decomposition products:	None known.





Section 11 Toxicological Information

11.1 Information on Toxicological Effects

Acute oral toxicity Acute dermal toxicity No data. Acute inhalation toxicity	
Acute inhalation toxicity Inhalation LC50: > 3.26 mg/L	
Skin corrosion/irritation Respiratory/skin sensitization Carcinogenicity Reproductive toxicity Not irritating or corrosive to skin based on 4-hour, semi-occlusive exposure to rabbits. Not irritating or corrosive to skin based on 4-hour, semi-occlusive exposure to rabbits. Not positive responses in rabbits based upon 24-, 48-, and 72-homean scores for corneal opacity, iritis, conjunctival redness/eder nean scores for corneal opacity iritis, conjunctival redness/eder nean scores for corneal opacity, iritis, conjunctival redness/eder nean scores for corneal opacity. Several in vitro and in vivo mutagenicity assays determined that calcium sulfate, dihydrate.	
exposure to rabbits. Eye damage/irritation Respiratory/skin sensitization Not a respiratory or dermal sensitizer. Several in vitro and in vivo mutagenicity assays determined that calcium sulfate, dihydrate was non-mutagenic, with and without metabolic activation. No data on calcium sulfate, dihydrate. Carcinogenic studies were conducted based on the non-neoplastic effects noted in the oral inhalation repeated dose studies as well as the negative mutage assays. Respirable crystalline silica has been identified as a carcinogen NTP, IARC, ACGIH and OSHA. No significant developmental or reproductive toxicity were identified in rabbits after exposure to either calcium sulfate, dehydrate or calcium sulfate, dihydrate.	
mean scores for corneal opacity, iritis, conjunctival redness/eder Not a respiratory or dermal sensitizer. Several in vitro and in vivo mutagenicity assays determined that calcium sulfate, dihydrate was non-mutagenic, with and without metabolic activation. No data on calcium sulfate, dihydrate. Carcinogenic studies were conducted based on the non-neoplastic effects noted in the oral inhalation repeated dose studies as well as the negative mutage assays. Respirable crystalline silica has been identified as a carcinogen NTP, IARC, ACGIH and OSHA. No significant developmental or reproductive toxicity were identified in rabbits after exposure to either calcium sulfate, dehydrate or calcium sulfate, dihydrate.)
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Carcinogenicity Carcin	
Carcinogenicity Carcin	
Reproductive toxicity No significant developmental or reproductive toxicity were identified in rabbits after exposure to either calcium sulfate, dehydrate or calcium sulfate, dihydrate.	and nicity
	ied
STOT-SE Acute toxicity testing did not result in direct organ toxicity after a single exposure to calcium sulfate, dihydrate. However, as the form tested was not indicated, FGD gypsum dust may result in mechanisms.	
A repeat dose oral toxicity study (35-45 days) with calcium sulfate dihydrate conducted using rats reported a NOAEL for males of 1 mg/kg/day on the basis of decreased total protein, albumin, blood urea nitrogen, and creatinine levels observed at the 300 and 1,0 mg/kg/day dose groups. No effects were observed in females. Repeated inhalation exposures to high levels of respirable crystates silica may result in lung damage (silicosis) and lung cancer. Aspiration Hazard A repeat dose oral toxicity study (35-45 days) with calcium sulfated high study and selection in the protein and support in the sulfated high selection in the protein and support in the sulfated high selection in the protein and support in the protei	00 d 00



Section 12 Ecological Information

12.1 Toxicity

Calcium sulfate (CAS# 7778-18-9) ¹			
Taviaitu ta Fiab	Fathead Minnow (<i>Pimephales promelas</i>): Acute, 7-day LC50 > 1,970 mg/L Acute, 96-hour NOEC = 1,470 mg/L		
Toxicity to Fish	Rainbow trout (<i>Oncorhynchus mykiss</i>): Chronic, 30-day NOEC (survival, growth, reproduction) = 732 mg/L Bluegill (<i>Lepomis macrochirus</i>):		
	Acute, 96-hour LC50 = 2,890 mg/L Water Flea (<i>Ceriodaphnia dubia</i>): Acute, 96-hour NOEC = 1447.4 mg/L		
Toxicity to Aquatic Invertebrates	Water Flea (<i>Daphnia magna</i>): Acute, 48-hour LC50 = 1,970 mg/L Chronic, 21-day NOEC (and 42-day post-hatch): 1,600 mg/L		
Toxicity to Aquatic Algae and Plants	Algae (<i>Navicula seminulum; Nitzschia linearis</i>): Chronic, 96-hour LC50 and EC50 (growth) = 3,200 mg/L		

¹The aquatic toxicity of sulfate has been shown to be dependent on water hardness, generally decreasing as hardness increases.

12.2 Persistence and Degradability

Not relevant for inorganic materials.

12.3 Bioaccumulative Potential

This material does not contain any compounds that that would bioaccumulate up the food chain.

12.4 Mobility in Soil

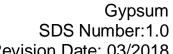
No data available.

12.5 Results of PBT and vPvB Assessment

This material does not contain any compounds classified as "persistent, bioaccumulative or toxic" nor as "very persistent/very bioaccumulative".

12.6 Other Adverse Effects

None known.





Revision Date: 03/2018

Section 13 Disposal Considerations

See Sections 7 and 8 above for safe handling and use, including appropriate industrial hygiene practices.

Dispose of all waste product and containers in accordance with federal, state and local regulations.

Section 14 Transport Information

	Shipping Name:	Not Regulated
Regulatory entity:	Hazard Class:	Not Regulated
U.S. DOT	ID Number:	Not Regulated
	Packing Group:	Not Regulated



Section 15 **Regulatory Information**

Safety, Health and Environmental Regulations/Legislation Specific for the Mixture 15.1

- TSCA Inventory Status
 - FGD gypsum as well as listed impurities are on the TSCA Inventory.
- California Proposition 65

The following substances are known to the State of California to be carcinogens and/or reproductive toxicants:

- Respirable crystalline silica
- State Right-to-Know (RTK)

Component	CAS	MA ^{1,2}	NJ ^{3,4}	PA ⁵	RI ⁶
Gypsum; calcium sulfate;	7778-18-9 or	Yes	Yes	Yes	No
calcium sulfate dihydrate	10101-41-4				
Calcium carbonate	1317-65-3	Yes	Yes	Yes	No
Silica-crystalline (SiO2), quartz	14808-60-7	Yes	Yes	Yes	No

Section 16

Other Information, Including Date of Preparation or Last Revision

Indication of Changes 16.1

Date of preparation or last revision: February 27, 2018

Abbreviations and Acronyms 16.2

ACGIH: American Conference of Industrial Hygienists

California CA:

Chemical Abstract Services CAS: **Coal Combustion Product** CCP: CFR: Code of Federal Regulations EPA: **Environmental Protection Agency**

FGD: Flue Gas Desulturization

GHS: Globally Harmonized System of Classification and Labelling

IARC: International Agency for Research on Cancer

Concentration estimated to result in the mortality of 50% of an animal population LC50:

LD50: Dose estimated to result in the mortality of 50% of an animal population

Massachusetts MA: NA: Not Applicable NJ: **New Jersey**

¹ Massachusetts Department of Public Health, no date ² 189th General Court of The Commonwealth of Massachusetts, no date ³ New Jersey Department of Health and Senior Services, 2010a ⁴ New Jersey Department of Health, 2010b

⁵ Pennsylvania Code, 1986

⁶ Rhode Island Department of Labor and Training, no date



Gypsum SDS Number:1.0 Revision Date: 03/2018

NOEC: No observed effect concentration

NIOSH: National Institute of Occupational Safety and Health

• NOx: Nitrogen oxides

NTP: US National Toxicology Program
 OEL: Occupational Exposure Limit

OSHA: Occupational Safety and Health Administration

PA: Pennsylvania

PBT: Persistent, Toxic and Bioaccumulative

PEL: Permissible exposure limit
 PPE: Personal Protective Equipment
 REL: Recommended exposure limit

RI: Rhode Island

RCS: Respirable Crystalline Silica

RTK: Right-to-Know

SCBA: Self-contained breathing apparatus

SDS: Safety Data SheetSTEL: Short-term exposure limit

STOT-RE: Specific target organ toxicity-repeated exposure
 STOT-SE: Specific target organ toxicity-single exposure

TLV: Threshold limit value

TSCA: Toxic Substances Control Act
 TWA: Time-weighted average
 UEL: Upper explosive limit

• UVCB: Unknown or Variable Composition/Biological

U.S.: United States

• U.S. DOT: United States of Department of Transportation

16.3 Other Hazards

Hazardous Materials Identification System (HMIS)							
Degree of hazard (0= low, 4 = extreme)							
Health:	1*	Flammability:	0	Physical Hazards:	0	Personal protection:**	

^{*} Chronic Health Effects

DISCLAIMER:

This SDS has been prepared in accordance with the Hazard Communication Rule 29 CFR 1910.1200. Information herein is based on data considered to be accurate as of date prepared. No warranty or representation, express or implied, is made as to the accuracy or completeness of this data and safety information. No responsibility can be assumed for any damage or injury resulting from abnormal use, failure to adhere to recommended practices, or from any hazards inherent in the nature of the product.

^{**} Appropriate personal protection is defined by the activity to be performed. See Section 8 for additional information.

ATTACHMENT D

Memorandum



Date: 25 October 2021

Subject: 35 Ill. Admin. Code Part 845 - Fault Area Location Demonstration for GMF Pond at

Coffeen Power Plant

Illinois Power Generating Company operates the coal fired Coffeen Power Plant (Plant) located in Montgomery County, Illinois. The Coffeen GMF Pond is an existing surface impoundment storing coal combustion residuals (CCR). The requirements for the GMF Pond are found in 35 Ill. Admin. Code (I.A.C.) 845 (Part 845).

This memorandum addresses the requirements of Section 845.320 Fault Areas, which states:

Section 845.320 Fault Areas

- a) Existing and new CCR surface impoundments, and all lateral expansions of CCR surface impoundments must not be located within 60 meters (200 feet) of the outermost damage zone of a fault that has had displacement in Holocene time unless the owner or operator demonstrates that an alternative setback distance of less than 60 meters (200 feet) will prevent damage to the structural integrity of the CCR surface impoundment.
- b) The owner or operator of the CCR surface impoundment must obtain a certification from a qualified professional engineer stating that the demonstration meets the requirements of subsection (a).

Pursuant to Section 845.210(d)(2), for existing CCR surface impoundments, the owner or operator of the CCR surface impoundment may use a previously completed location restriction demonstration required by Section 845.300 (Placement Above the Uppermost Aquifer), Section 845.310 (Wetlands), Section 845.320 (Fault Areas), Section 845.330 (Seismic Impact Zones), and Section 845.340 (Unstable Areas), provided that the previously completed assessments meet the applicable requirements of those Sections.

The previous fault area demonstration was certified by a qualified professional engineer stating that the demonstration meets the requirements of 40 C.F.R. § 257.62. The requirements described in 40 C.F.R. § 257.62 are nearly identical to the requirements contained in I.A.C. Section 845.320. Pursuant to Section 845.210(d)(2), a certification is not required for this demonstration. The previously completed fault area demonstration is included in Attachment D.



HALEY & ALDRICH, INC. 6500 Rockside Road Suite 200 Cleveland, OH 44131 216.739.0555

MEMORANDUM

16 October 2018 File No. 129788

SUBJECT:

Location Restriction Demonstration - Fault Areas

Coffeen Power Station GMF Gypsum Stack Pond

Coffeen, Illinois

Illinois Power Generating Company operates the coal-fired Coffeen Power Station (Plant) located near Coffeen, Illinois. The GMF Gypsum Stack Pond (Unit) is an existing coal combustion residuals (CCR) surface impoundment. This demonstration addresses the requirements of 40 CFR §257.62 (Fault Areas) of the U.S. Environmental Protection Agency's (EPA) rule entitled Hazardous and Solid Waste Management System; Disposal of Coal Combustion Residuals from Electric Utilities. 80 Fed. Reg. 21,302 (Apr. 17, 2015) (promulgating 40 CFR §257.62); 83 Fed. Reg. 36,435 (July 30, 2018) (amending 40 CFR §257.62).

§257.62(a): New CCR landfills, existing and new CCR surface impoundments, and all lateral expansions of CCR units must not be located within 60 meters (200 feet) of the outermost damage zone of a fault that has had displacement in Holocene time unless the owner or operator demonstrates by the dates specified in paragraph (c) of this section that an alternative setback distance of less than 60 meters (200 feet) will prevent damage to the structural integrity of the CCR unit.

A review of available data from the U.S. Geologic Survey, the Illinois State Geological Survey, and other available information was completed for this demonstration. The nearest known mapped faults are the Crown Fault, which is located approximately 31 miles northwest, and the Centralia Fault zone, located approximately 35 miles southeast of the Unit. The timeframe of the most recent activity on these faults is not known. Based on the available published geologic data and information reviewed, there are no active faults or fault damage zones that have had displacement in Holocene time reported or indicated within 200 feet of the Unit.

Coffeen Power Station – GMF Gypsum Stack Pond Location Restriction – Fault Areas 16 October 2018 Page 2

§257.62(b): The owner or operator of the CCR unit must obtain a certification from a qualified professional engineer or approval from the Participating State Director or approval from EPA where EPA is the permitting authority stating that the demonstration meets the requirements of paragraph (a) of this section.

I, Steven F. Putrich, being a Registered Professional Engineer in good standing in the State of Illinois, do hereby certify, to the best of my knowledge, information, and belief, that the information contained in this certification has been prepared in accordance with the accepted practice of engineering. I certify, for the above-referenced CCR Unit, that the demonstration that the CCR Unit is not located within 60 meters (200 feet) of the outermost damage zone of a fault that has had a displacement in Holocene time as included in the CCR Rule Location Restrictions Evaluation memorandum dated 12 October 2018 meets the requirements of 40 CFR §257.62(a).

Signed:

Consulting Engineer

Print Name:

Steven F. Putrich

Illinois License No.:

62048779

Title:

Vice President

Company:

Haley & Aldrich, Inc.

Professional Engineer's Seal:





Memorandum



Date: 25 October 2021

Subject: 35 Ill. Admin. Code Part 845 - Placement Above the Uppermost Aquifer Location

Demonstration for the GMF Pond at Coffeen Power Plant

Illinois Power Generating Company operates the coal fired Coffeen Power Plant (Plant) located in Montgomery County, Illinois. The Coffeen GMF Pond is an existing surface impoundment storing coal combustion residuals (CCR). The requirements for the GMF Pond are found in 35 Ill. Admin. Code (I.A.C.) 845 (Part 845).

This memorandum addresses the requirements of Section 845.300 Placement Above the Uppermost Aquifer, which states:

Section 845.300 Placement Above the Uppermost Aquifer

- a) Existing and new CCR surface impoundments, and all lateral expansions of CCR surface impoundments must, be constructed with a base that is located at least 1.52 meters (five feet) above the upper limit of the uppermost aquifer, or must demonstrate that there will not be an intermittent, recurring, or sustained hydraulic connection between any portion of the base of the CCR surface impoundment and the uppermost aquifer due to normal fluctuations in groundwater elevations (including the seasonal high water table).
- b) <u>The owner or operator of the CCR surface impoundment must obtain a certification from a qualified professional engineer stating that the demonstration meets the requirements of subsection (a).</u>

Pursuant to Section 845.210(d)(2), for existing CCR surface impoundments, the owner or operator of the CCR surface impoundment may use a previously completed location restriction demonstration required by Section 845.300 (Placement Above the Uppermost Aquifer), Section 845.310 (Wetlands), Section 845.320 (Fault Areas), Section 845.330 (Seismic Impact Zones), and Section 845.340 (Unstable Areas), provided that the previously completed assessments meet the applicable requirements of those Sections.

The previous upper aquifer demonstration was certified by a qualified professional engineer stating that the demonstration meets the requirements of 40 C.F.R. § 257.60. The requirements described in 40 C.F.R. § 257.60 are nearly identical to the requirements contained in I.A.C. Section 845.300. Pursuant to Section 845.210(d)(2), a certification is not required for this demonstration. The previously completed upper aquifer demonstration is included in Attachment D.



HALEY & ALDRICH, INC. 6500 Rockside Road Suite 200 Cleveland, OH 44131 216.739.0555

MEMORANDUM

16 October 2018 File No. 129788

SUBJECT:

Location Restriction Demonstration - Placement Above Uppermost Aquifer

Coffeen Power Station GMF Gypsum Stack Pond

Coffeen, Illinois

Illinois Power Generating Company operates the coal-fired Coffeen Power Station (Plant) located near Coffeen, Illinois. The GMF Gypsum Stack Pond (Unit) is an existing coal combustion residuals (CCR) surface impoundment. This demonstration addresses the requirements of 40 CFR §257.60 (Placement above the uppermost aquifer) of the U.S. Environmental Protection Agency's (EPA) rule entitled Hazardous and Solid Waste Management System; Disposal of Coal Combustion Residuals from Electric Utilities. 80 Fed. Reg. 21,302 (Apr. 17, 2015) (promulgating 40 CFR §257.60); 83 Fed. Reg. 36,435 (July 30, 2018) (amending 40 CFR §257.60).

§257.60(a): New CCR landfills, existing and new CCR surface impoundments, and all lateral expansions of CCR units must be constructed with a base that is located no less than 1.52 meters (five feet) above the upper limit of the uppermost aquifer, or must demonstrate that there will not be an intermittent, recurring, or sustained hydraulic connection between any portion of the base of the CCR unit and the uppermost aquifer due to normal fluctuations in groundwater elevations (including the seasonal high water table). The owner or operator must demonstrate by the dates specified in paragraph (c) of this section that the CCR unit meets the minimum requirements for placement above the uppermost aquifer.

Haley & Aldrich reviewed available information provided by Vistra including historic record drawings, design drawings, and boring logs, and based on review and evaluation of the information provided, the results do not demonstrate compliance with the requirements of 40 CFR §257.60(a).

Coffeen Power Station – GMF Gypsum Stack Pond Location Restriction – Placement Above Uppermost Aquifer 16 October 2018 Page 2

§257.60(b): The owner or operator of the CCR unit must obtain a certification from a qualified professional engineer or approval from the Participating State Director or approval from EPA where EPA is the permitting authority stating that the demonstration meets the requirements of paragraph (a) of this section.

I, Steven F. Putrich, being a Registered Professional Engineer in good standing in the State of Illinois, do hereby certify, to the best of my knowledge, information, and belief, that the information contained in this certification has been prepared in accordance with the accepted practice of engineering. I certify that the above-referenced CCR Unit does not meet the requirements of 40 CFR §257.60(a).

Signed:

Consulting Engineer

Print Name:

Steven F. Putrich

Illinois License No.:

62048779

Title:

Vice President

Company:

Haley & Aldrich, Inc.

Professional Engineer's Seal:





Memorandum



Date: 25 October 2021

Subject: 35 Ill. Admin. Code Part 845 – Seismic Impact Zone Location Demonstration for

GMF Pond at Coffeen Power Plant

Illinois Power Generating Company operates the coal fired Coffeen Power Plant (Plant) located in Montgomery County, Illinois. The Coffeen GMF Pond is an existing surface impoundment storing coal combustion residuals (CCR). The requirements for the GMF Pond are found in 35 Ill. Admin. Code (I.A.C.) 845 (Part 845).

This memorandum addresses the requirements of Section 845.330 Seismic Impact Zone, which states.

Section 845.330 Seismic Impact Zones

- a) Existing and new CCR surface impoundments, and all lateral expansions of CCR surface impoundments must not be located in seismic impact zones unless the owner or operator demonstrates that all structural components including liners, leachate collection and removal systems, and surface water control systems, are designed to resist the maximum horizontal acceleration in lithified earth material for the site.
- b) The owner or operator of the CCR surface impoundment must obtain a certification from a qualified professional engineer stating that the demonstration meets the requirements of subsection (a).

Pursuant to Section 845.210(d)(2), for existing CCR surface impoundments, the owner or operator of the CCR surface impoundment may use a previously completed location restriction demonstration required by Section 845.300 (Placement Above the Uppermost Aquifer), Section 845.310 (Wetlands), Section 845.320 (Fault Areas), Section 845.330 (Seismic Impact Zones), and Section 845.340 (Unstable Areas), provided that the previously completed assessments meet the applicable requirements of those Sections.

The previous seismic impact zone demonstration was certified by a qualified professional engineer stating that the demonstration meets the requirements of 40 C.F.R. § 257.63. The requirements described in 40 C.F.R. § 257.63 are nearly identical to the requirements contained in I.A.C. Section 845.330. Pursuant to Section 845.210(d)(2), a certification is not required for this demonstration. The previously completed seismic impact zone demonstration is included in Attachment D.



HALEY & ALDRICH, INC. 6500 Rockside Road Suite 200 Cleveland, OH 44131 216.739.0555

MEMORANDUM

16 October 2018 File No. 129788

SUBJECT:

Location Restriction Demonstration - Seismic Impact Zone

Coffeen Power Station
GMF Gypsum Stack Pond

Coffeen, Illinois

Illinois Power Generating Company operates the coal-fired Coffeen Power Station (Plant) located near Coffeen, Illinois. The GMF Gypsum Stack Pond (Unit) is an existing coal combustion residuals (CCR) surface impoundment. This demonstration addresses the requirements of 40 CFR §257.63 (Seismic Impact Zones) of the U.S. Environmental Protection Agency's (EPA) rule entitled Hazardous and Solid Waste Management System; Disposal of Coal Combustion Residuals from Electric Utilities. 80 Fed. Reg. 21,302 (Apr. 17, 2015) (promulgating 40 CFR §257.63); 83 Fed. Reg. 36,435 (July 30, 2018) (amending 40 CFR §257.63).

§257.63(a): New CCR landfills, existing and new CCR surface impoundments, and all lateral expansions of CCR units must not be located in seismic impact zones unless the owner or operator demonstrates by the dates specified in paragraph (c) of this section that all structural components including liners, leachate collection and removal systems, and surface water control systems, are designed to resist the maximum horizontal acceleration in lithified earth material for the site.

A Seismic Impact Zone is defined in the CCR Rule (40 CFR §257.63) as "an area having a 2% or greater probability that the maximum expected horizontal acceleration, expressed as a percentage of the earth's gravitational pull (g), will exceed 0.10 g in 50 years". The 2014 US Geological Survey Hazard Map raw data for the Coffeen Power Station GMF Gypsum Stack Pond indicates that the maximum expected horizontal acceleration for 2 percent probability of exceedance in 50 years is 0.21g.

The results of our evaluation indicate that the Unit is in compliance with 40 CFR §257.63(a). Although the Unit is located in a seismic impact zone, it satisfies the demonstration requirements of 40 CFR §257.63(a). The AECOM report entitled "CCR Certification Report: Initial Structural Stability Assessment, Initial Safety Factor Assessment, and Initial Inflow Design Flood Control System Plan for the GMF Pond at Coffeen Power Station" dated October 2016 (AECOM Report), includes engineering analysis, calculations, and findings that support the requirements of 40 CFR §257.63(a), and provides documentation that those requirements have been evaluated by AECOM for the subject CCR unit.

Coffeen Power Station – GMF Gypsum Stack Pond Location Restriction – Seismic Impact Zone 16 October 2018 Page 2

§257.63(b): The owner or operator of the CCR unit must obtain a certification from a qualified professional engineer or approval from the Participating State Director or approval from EPA where EPA is the permitting authority stating that the demonstration meets the requirements of paragraph (a) of this section.

I, Steven F. Putrich, being a Registered Professional Engineer in good standing in the State of Illinois, do hereby certify, to the best of my knowledge, information, and belief, that the information contained in this certification has been prepared in accordance with the accepted practice of engineering. I certify, that the CCR Unit is located in a seismic impact zone as included in the CCR Rule Location Restrictions Evaluation memorandum dated 12 October 2018 and satisfies all requirements of 40 CFR §257.63(a).

By providing this certification demonstration statement, we are not stating or inferring that we have verified or certified the details, assumptions, calculations and/or site condition models developed by AECOM in the subject report; those elements of the report are considered the professional opinions and determinations of AECOM.

Signed:

Consulting Engineer

Print Name:

Steven F. Putrich

Illinois License No.:

62048779

Title:

Vice President

Company:

Haley & Aldrich, Inc.

Professional Engineer's Seal:





Memorandum



Date: 25 October 2021

Subject: 35 Ill. Admin. Code Part 845 – Unstable Areas and Floodplains Location Standard

Demonstration for GMF Pond at Coffeen Power Plant

Illinois Power Generating Company (IPGC) is the owner of the coal fired Coffeen Power Plant (Plant) located in Montgomery County, approximately 2 miles south of the city of Coffeen, Illinois. The GMF Pond is an existing surface impoundment storing coal combustion residuals (CCR). The requirements for the GMF Pond are found in 35 Ill. Admin. Code (I.A.C.) 845 (Part 845).

This certification addresses the requirements of Part 845, Section 845.340 Unstable Areas and Floodplains, which states:

<u>Section 845.340 (a):</u> An existing or new CCR surface impoundment, or any lateral expansion of a CCR surface impoundment must not be located in an unstable area unless the owner or operator demonstrates that recognized and generally accepted engineering practices have been incorporated into the design of the CCR surface impoundment to ensure that the integrity of the structural components of the CCR surface impoundment will not be disrupted.

<u>Section 845.340 (b):</u> The owner or operator must consider all the following factors, at a minimum, when determining whether an area is unstable: 1) On-site or local soil conditions, including but not limited to liquefaction, that may result in significant differential settling; 2) On-site or local geologic or geomorphologic features; and 3) On-site or local human-made features or events (both surface and subsurface).

Pursuant to Section 845.210(d)(2), for existing CCR surface impoundments, the owner or operator of the CCR surface impoundment may use a previously completed location restriction demonstration required by Section 845.300 (Placement Above the Uppermost Aquifer), Section 845.310 (Wetlands), Section 845.320 (Fault Areas), Section 845.330 (Seismic Impact Zones), and Section 845.340 (Unstable Areas), provided that the previously completed assessments meet the applicable requirements of those Sections.

The previous unstable area demonstration was certified by a qualified professional engineer stating that the demonstration meets the requirements of 40 C.F.R. § 257.64. The requirements described in 40 C.F.R. § 257.64 are nearly identical to the requirements contained in I.A.C. Section 845.340 (a). Pursuant to Section 845.210(d)(2), a certification is not required for the unstable area demonstration. The previously completed unstable area demonstration meeting the requirements of Section 845.340 (a) is included in Attachment D.

Memorandum (cont'd)



I.A.C. Part 845 – Unstable Areas and Floodplains Certification for GMF Pond at Coffeen Power Plant 25 October 2021 Page 2

<u>Section 845.340 (c):</u> An existing or new CCR surface impoundment, or any lateral expansion of a CCR surface impoundment, must not be located in a floodplain unless the owner or operator demonstrates that recognized and generally accepted engineering practices have been incorporated into the design of the CCR surface impoundment to ensure that the CCR surface impoundment will not restrict the flow of the base flood, reduce the temporary water storage capacity of a floodplain, or result in washout of CCR, so as to pose a hazard to human life, wildlife, or land or water resources.

The boundaries of the impoundment were determined by a survey conducted by a professional surveyor licensed in the State of Illinois. The surveyed boundaries were compared to the existing floodplain boundary from the effective FEMA Flood Hazard Boundary Map (FHBM) Number 170992 0009 A, dated January 9, 1981. The location of the Coffeen GMF Pond is outside of the FEMA 100-year floodplain, therefore, it was determined that the Coffeen GMF Pond is not located within the floodplain.

<u>Section 845.330 (d):</u> The owner or operator of the CCR surface impoundment must obtain a certification from a qualified professional engineer stating that the demonstration meets the requirements of subsections (a) and (c).

Pursuant to Section 845.210(d)(2), for existing CCR surface impoundments, the owner or operator of the CCR surface impoundment may use a previously completed location restriction demonstration required by Section 845.340 (Unstable Areas) and demonstrated in Attachment D. Therefore, a new certification is not included for Section 845.340 (a). Certification for Section 845.340 (c) is included below.

Memorandum (cont'd)



I.A.C. Part 845 – Unstable Areas and Floodplains Certification for GMF Pond at Coffeen Power Plant 25 October 2021 Page 3

35 Illinois Administration Code Part 845: Standards for the Disposal of Coal Combustion Residuals in Surface Impoundments Section 845.340 (c): Certification

I, <u>Sarah Espinosa</u>, being a Professional Engineer in good standing in the State of Illinois, do hereby certify, to the best of my knowledge, information, and belief that this floodplain demonstration meets the requirements of 35 Ill. Adm. Code 845.340(c).

Sarah Espinosa, P.E.

Printed Name

October 21, 2021



Date



HALEY & ALDRICH, INC. 6500 Rockside Road Suite 200 Cleveland, OH 44131 216.739.0555

MEMORANDUM

16 October 2018 File No. 129788

SUBJECT:

Location Restriction Demonstration - Unstable Areas

Coffeen Power Station
GMF Gypsum Stack Pond

Coffeen, Illinois

Illinois Power Generating Company operates the coal-fired Coffeen Power Station (Plant) located near Coffeen, Illinois. The GMF Gypsum Stack Pond (Unit) is an existing coal combustion residuals (CCR) surface impoundment. This demonstration addresses the requirements of 40 CFR §257.64 (Unstable Areas) of the U.S. Environmental Protection Agency's (EPA) rule entitled Hazardous and Solid Waste Management System; Disposal of Coal Combustion Residuals from Electric Utilities. 80 Fed. Reg. 21,302 (Apr. 17, 2015) (promulgating 40 CFR §257.64); 83 Fed. Reg. 36,435 (July 30, 2018) (amending 40 CFR §257.64).

§257.64(a): An existing or new CCR landfill, existing or new CCR surface impoundment, or any lateral expansion of a CCR unit must not be located in an unstable area unless the owner or operator demonstrates by the dates specified in paragraph (d) of this section that recognized and generally accepted good engineering practices have been incorporated into the design of the CCR unit to ensure that the integrity of the structural components of the CCR unit will not be disrupted.

§257.64(b): The owner or operator must consider all of the following factors, at a minimum, when determining whether an area is unstable:

- (1) On-site or local soil conditions that may result in significant differential settling;
- (2) On-site or local geologic or geomorphologic features; and
- (3) On-site or local human-made features or events (both surface and subsurface).

Determination of compliance with §257.64(b)(1) - Conditions associated with the potential for significant differential settlement due to liquefaction were not identified in the area where the Plant is located. A separate report completed by AECOM entitled "CCR Certification Report: Initial Structural Stability Assessment, Initial Safety Factor Assessment, and Initial Inflow Design Flood Control System Plan for the GMF Pond at Coffeen Power Station" dated October 2016 concluded that a 1-ft thick layer of liqueifiable material existed below the impoundment and post-liquefaction stability analyses performed by AECOM concluded that the impoundment slopes would remain stable if that layer liqueified during the design seismic event.

Determination of compliance with $\S257.64(b)(2)$ - Based on available United States Geological Survey (USGS) and Illinois State Geological Survey (ISGS) information, karst topography or physiographic features such as sinkholes, vertical shafts, sinking streams, caves, large springs, or blind valleys do not

Coffeen Power Station – GMF Gypsum Stack Pond Location Restriction – Unstable Areas 16 October 2018 Page 2

exist at the Plant. To evaluate the susceptibility of landslides, we reviewed readily available USGS and ISGS data. The USGS data indicates that the Plant is in an area of low landslide incidence and the closest document landslide is more than 10 miles from the site. Accordingly, it is our opinion that the Unit is not located in an area that has high susceptibility to landslides.

Determination of compliance with §257.64(b)(3) - An inactive underground mine is located below the Unit, however an evaluation by AECOM (2016) concluded that the presence of this underground mine does not negatively affect the stability of the Unit.

§257.64(c): The owner or operator of the CCR unit must obtain a certification from a qualified professional engineer or approval from the Participating State Director or approval from EPA where EPA is the permitting authority stating that the demonstration meets the requirements of paragraph (a) of this section.

I, Steven F. Putrich, being a Registered Professional Engineer in good standing in the State of Illinois, do hereby certify, to the best of my knowledge, information, and belief, that the information contained in this certification has been prepared in accordance with the accepted practice of engineering. I certify, for the above-referenced CCR Unit, that the demonstration indicating the CCR Unit is not located in an unstable area as included in the CCR Rule Location Restrictions Evaluation memorandum dated 12 October 2018 meets the requirements of 40 CFR §257.64(a).

Signed:

Consulting Engineer

Print Name:

Steven F. Putrich

Illinois License No.:

62048779

Title:

Vice President

Company:

Haley & Aldrich, Inc.

Professional Engineer's Seal:





Memorandum



Date: 25 October 2021

Subject: 35 I.A.C. Admin. Code Part 845 - Wetland Location Demonstration for GMF Pond at

Coffeen Power Plant

Illinois Power Generating Company operates the coal fired Coffeen Power Plant (Plant) located in Montgomery County, Illinois. The Coffeen GMF Pond is an existing surface impoundment storing coal combustion residuals (CCR). The requirements for the GMF Pond are found in 35 Ill. Admin. Code (I.A.C.) 845 (Part 845).

This memorandum addresses the requirements of Section 845.310 Wetlands, which states:

Section 845.310 Wetlands

- a) Existing and new CCR surface impoundments, and all lateral expansions of CCR surface impoundments must not be located in wetlands unless the owner or operator demonstrates [that the requirements listed in 845.310(a)(1) through (5) are met.]
- b) The owner or operator of the CCR surface impoundment must obtain a certification from a qualified professional engineer stating that the demonstration meets the requirements of subsection (a).

Pursuant to Section 845.210(d)(2), for existing CCR surface impoundments, the owner or operator of the CCR surface impoundment may use a previously completed location restriction demonstration required by Section 845.300 (Placement Above the Uppermost Aquifer), Section 845.310 (Wetlands), Section 845.320 (Fault Areas), Section 845.330 (Seismic Impact Zones), and Section 845.340 (Unstable Areas), provided that the previously completed assessments meet the applicable requirements of those Sections.

The previous wetlands demonstration was certified by a qualified professional engineer stating that the demonstration meets the requirements of 40 C.F.R. § 257.61. The requirements described in 40 C.F.R. § 257.61 are nearly identical to the requirements contained in I.A.C. Section 845.310. Pursuant to Section 845.210(d)(2), a certification is not required for this demonstration. The previously completed wetlands demonstration is included in Attachment D.



HALEY & ALDRICH, INC. 6500 Rockside Road Suite 200 Cleveland, OH 44131 216.739.0555

MEMORANDUM

16 October 2018 File No. 129788

SUBJECT:

Location Restriction Demonstration - Wetland Areas

Coffeen Power Station GMF Gypsum Stack Pond

Coffeen, Illinois

Illinois Power Generating Company operates the coal-fired Coffeen Power Station (Plant) located near Coffeen, Illinois. The GMF Gypsum Stack Pond (Unit) is an existing coal combustion residuals (CCR) surface impoundment. This demonstration addresses the requirements of 40 CFR §257.61 (Wetlands) of the U.S. Environmental Protection Agency's (EPA) rule entitled Hazardous and Solid Waste Management System; Disposal of Coal Combustion Residuals from Electric Utilities. 80 Fed. Reg. 21,302 (Apr. 17, 2015) (promulgating 40 CFR §257.61); 83 Fed. Reg. 36,435 (July 30, 2018) (amending 40 CFR §257.61.

§257.61(a): New CCR landfills, existing and new CCR surface impoundments, and all lateral expansions of CCR units must not be located in wetlands, as defined in §232.2 of this chapter, unless the owner or operator demonstrates by the dates specified in paragraph (c) of this section that the CCR unit meets the requirements of paragraphs (a)(1) through (5) of this section.

Based on a review of the U.S. Fish and Wildlife Service's National Wetland Inventory mapping, 0.5-meter resolution aerial imagery (2015) and the results of on-site field assessments, the Unit is not located in wetlands as defined by 40 CFR §232.2.

Coffeen Power Station – GMF Gypsum Stack Pond Location Restriction – Wetland Areas 16 October 2018 Page 2

§257.61(b): The owner or operator of the CCR unit must obtain a certification from a qualified professional engineer or approval from the Participating State Director or approval from EPA where EPA is the permitting authority stating that the demonstration meets the requirements of paragraph (a) of this section.

I, Steven F. Putrich, being a Registered Professional Engineer in good standing in the State of Illinois, do hereby certify, to the best of my knowledge, information, and belief, that the information contained in this certification has been prepared in accordance with the accepted practice of engineering. I certify, for the above-referenced CCR Unit, that the CCR Unit is not located in wetlands as included in the CCR Rule Location Restrictions Evaluation memorandum dated 12 October 2018 and, therefore, meets the requirements of 40 CFR §257.61(a).

Signed:

Consulting Engineer

Print Name:

Steven F. Putrich

Illinois License No.:

62048779

Title:

Vice President

Company:

Haley & Aldrich, Inc.

Professional Engineer's Seal:





ATTACHMENT E



ATTACHMENT F

Illinois Power Generating Company

COFFEEN POWER PLANT MONTGOMERY COUNTY, ILLINOIS

Emergency Action Plan (EAP)

40 CFR § 257.73(a)(3), 35 Ill. Adm. Code 845.520 Coal Combustion Residual (CCR) Impoundments & Related Facilities

- Gypsum Management Facility (GMF) Pond (NID # IL50579) (IEPA # W1350150004-03)
- GMF Recycle Pond (NID # IL50578) (IEPA # W1350150004-04)
- Ash Pond No. 1 (NID # IL50722) (IEPA # W1350150004-01)
- Ash Pond No. 2 (NID # IL50723) (IEPA # W1350150004-02)

Revision Date: September 16, 2021

Qualified Professional Engineer Certification; Emergency Action Plan for the Coffeen Power Plant GMF Pond, GMF Recycle Pond, Ash Pond 1 and Ash Pond 2

In accordance with 40 C.F.R. § 257.73(a)(3)(iv) and 35 III. Adm. Code 845.520(e), the owner or operator of a CCR unit that is required to prepare a written Emergency Action Plan under 40 C.F.R. § 257.73(a)(3) and 35 III. Adm. Code 845.520(a) must obtain a certification from a qualified professional engineer stating that the written Emergency Action Plan meets the requirements of 40 C.F.R. § 257.73(a) (3) and 35 III. Adm. Code 845.520.

- I, ____Phil Morris_, being a Professional Engineer in good standing in the State of Illinois, do hereby certify, to the best of my knowledge, information, and belief that:
 - 1. the information contained in this Emergency Action Plan was prepared in accordance with the accepted practice of engineering; and
 - this Emergency Action Plan meets the requirements of 40 C.F.R. § 257.73(a)(3) and 35 Ill. Adm. Code 845.520.

Phil Morris

9/27/21

Senior Director, Corporate Environmental

Date

062-058768
REGISTERED
PROFESSIONAL
ENGINEER
9/OF
LINO19

COFFEEN POWER PLANT EMERGENCY ACTION PLAN CCR IMPOUNDMENTS & RELATED FACILITIES

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COFFEEN POWER PLANT EMERGENCY ACTION PLAN CCR IMPOUNDMENTS & RELATED FACILITIES

1 STATEMENT OF PURPOSE

The Coffeen Power Plant (Power Plant) is located near the town of Coffeen in Montgomery County, Illinois. The location of the Power Plant is shown in Figure 1-1. The Power Plant is a coal-fired electricity producing power plant owned and operated by the Illinois Power Generating Company (IPGC), a subsidiary of Dynegy. This Emergency Action Plan (EAP) was prepared in accordance with 40 CFR § 257.73(a)(3) and 35 Ill. Adm. Code 845.520 and covers the following Coal Combustion Residual (CCR) surface impoundments located at the site:

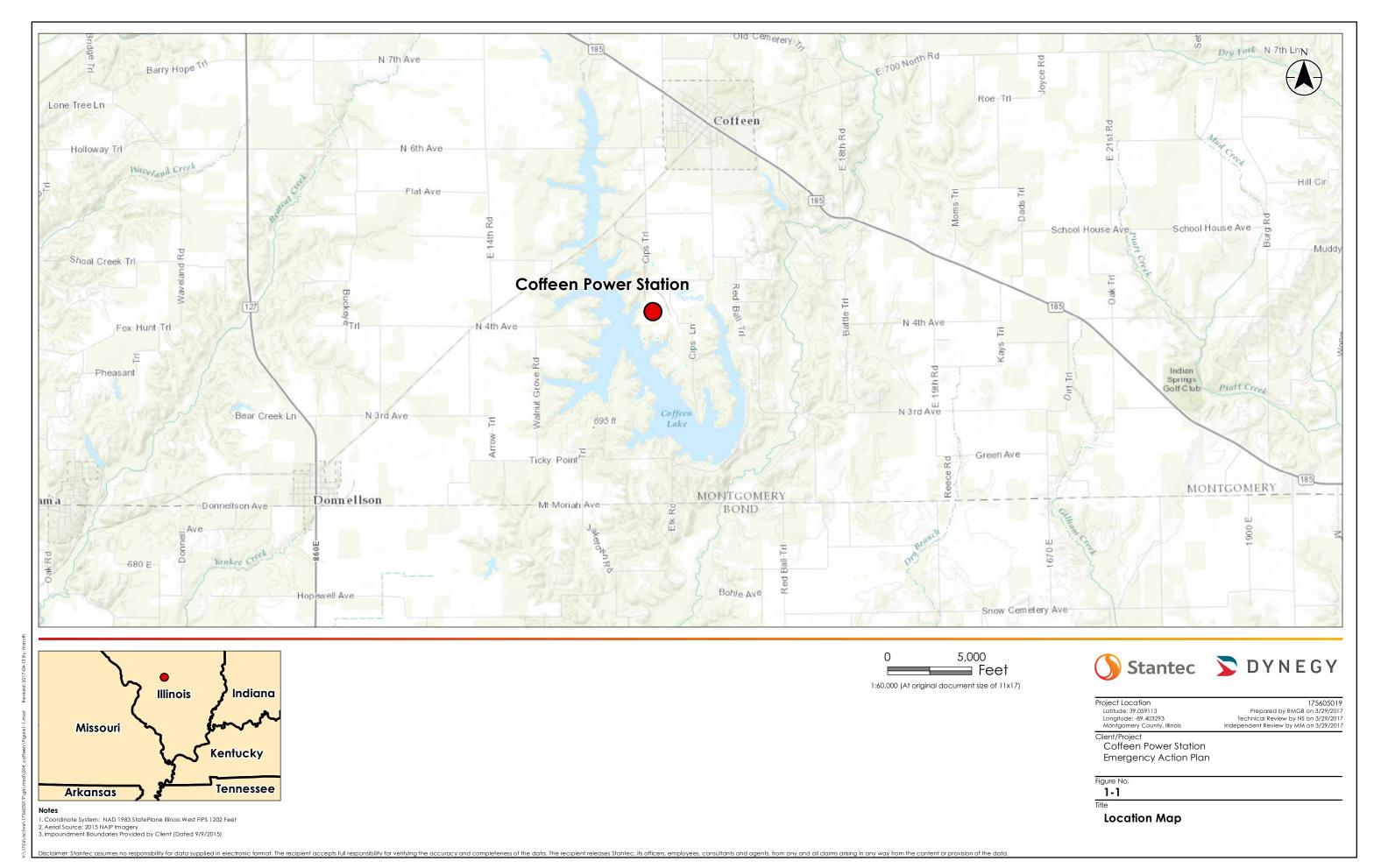
- Gypsum Management Facility (GMF) Pond (NID # IL50579) (IEPA # W1350150004-03)
- GMF Recycle Pond (NID # IL50578) (IEPA # W1350150004-04)
- Ash Pond No. 1 (NID # IL50722) (IEPA # W1350150004-01)
- Ash Pond No. 2 (Capped/Closed) (NID # IL50723) (IEPA # W1350150004-02)

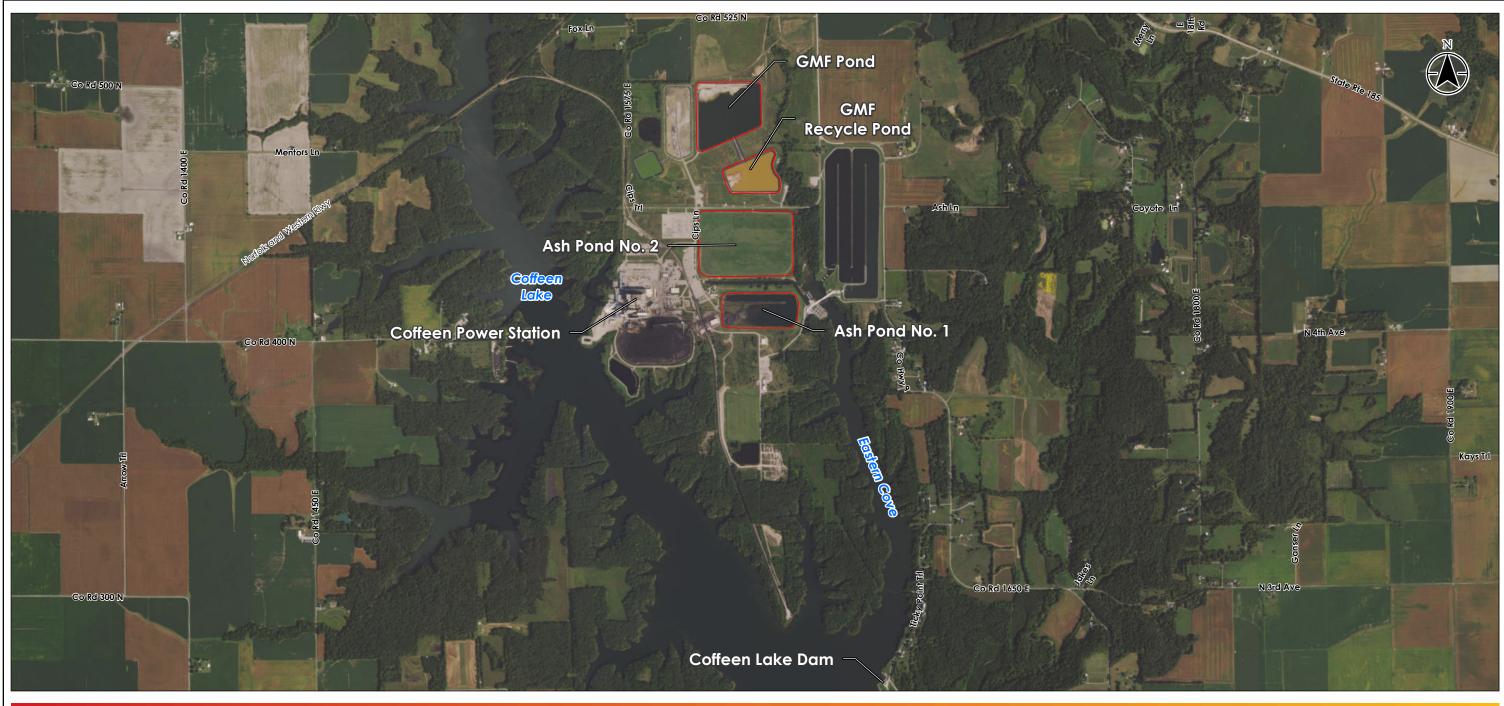
The locations of these impoundments are shown in Figure 1-2. Section 6 of this EAP includes a description of each impoundment.

The purpose of this Emergency Action Plan (EAP) is to:

- Safeguard the lives, as well as to reduce property damage, of citizens living within potential downstream flood inundation areas of CCR impoundments and related facilities at the Coffeen Power Plant.
- Define the events or circumstances involving the CCR impoundments and related facilities at the Coffeen Power Plant that pose a safety hazard or emergency and how to identify those conditions.
- 3. Define responsible persons, their responsibilities, and notification procedures in the event of a safety emergency.
- 4. Provide list of emergency responders.
- 5. Identify emergency actions in the event of a potential or imminent failure of the impoundments.
- 6. Identify the downstream area that would be affected by failure of the impoundments.
- Provide for effective facility surveillance, prompt notification to local Emergency
 Management Agencies, citizen warning and notification responses, and preparation should an
 emergency occur.

Information provided by Illinois Power Generating Company was utilized and relied upon in preparation of this Emergency Action Plan.







Legend

CCR Surface Impoundment Boundary



1:24,000 (At original document size of 11x17)





Project Location Latitude: 39.059113 Longitude: -89.403293 Montgomery County, Illinois

175605019 Prepared by RMGB on 3/29/2017 Technical Review by NS on 3/29/2017 Independent Review by MM on 3/29/2017

Client/Project
Coffeen Power Station Emergency Action Plan

Figure No.

1-2

CCR Impoundments

Coordinate System: NAD 1983 StatePlane Illinois West FIPS 1202 Feet
 Aerial Source: 2015 NAIP Imagery
 Impoundment Boundaries Provided by Client (Dated 9/9/2015)

claimer. Stantec assumes no responsibility for data supplied in electronic format. The recipient accepts full responsibility for verifying the accuracy and completeness of the data. The recipient releases Stantec, its officers, employees, consultants and agents, from any and all claims arising in any way from the content or provision of the data.

2 COMMUNICATION

To facilitate understanding among everyone involved in implementing this EAP, four response levels are used to identify the condition of an impoundment. These are:

Response Levels:

- <u>Level 0</u>: Normal conditions and routine operations, including surveillance and initial investigation of unusual conditions and effects of storm events.
- <u>Level 1</u>: Potentially hazardous condition exists, requiring investigation and possible corrective action.
- <u>Level 2</u>: Potential failure situation is developing; possible mode of failure is being assessed; corrective measures are underway.
- <u>Level 3</u>: Failure is occurring or is imminent, public protective actions are required.

The 4-Step Incident Response Process is outlined in Figure 2-1. This should be used in conjunction with the Notification Flowchart (Figure 2-2) and EAP Decision Tree (Figure 2-3). Section 4 provides guidance tables for determining Response Levels and a table providing emergency actions to be taken given various situations. Table 2-1 lists contact information for the emergency responders.

Figure 2-1. Summary/Sequence of Tasks 4-Step Incident Response Process

Step 1: Detection, Evaluation, and Response Level Determination

Sequence of Tasks:

- Notify EAP Coordinator, Plant Manager, and Dam Safety Manager of unusual condition detected and confer on next steps needed.
- Conduct technical evaluation of conditions as needed.
- Determine Response Level based on evaluation. (Table 4-1)
- Reset Response Level as revised evaluations warrant.

Step 2: Notification

Sequence of Tasks:

- Notify authorities, designated personnel, and external response partners of change in Response Level, using the Notification Flowchart. (Figure 2-2)
- Re-notify authorities, designated personnel, and external response partners as Response Level is changed.

Step 3: Emergency Actions

Sequence of Tasks:

- Perform emergency actions with goal of saving the impoundment and minimizing impacts to life, property, and environment. (Table 4-3)
- Take continuous actions to include situation assessment, information sharing, remediation, and public safety advisories or warnings, as warranted.
- Revise action plan as changes in conditions warrant.

Step 4: Follow-up

Sequence of Tasks:

- Document conditions and decisions in the Emergency Incident Log.
- Notify authorities, designated personnel, and external response partners that condition is stabilized, limit incident termination declarations to conditions at the site.
- Conduct and document after-action review of incident and response.

Figure 2-2. Notification Flowchart Initial Detector
(Internal) **Initial Detector** (External) 911 Asset Closure Manager **EAP Coordinator** (Environmental Manager) **Determine Response Level Response Level** Dam Safety Manager Level 0 Level 1 Montgomery County ESDA / EMA Coordinator Office: (217) 532-9564 Level 2 Home: (217) 532-6437 Cell: (217) 254-6437 Level 3 **IPGC** Corporate **Affected Parties Local / County Police, Fire & Rescue** Montgomery County 911 Center Director: Greg Nimmo **Onsite Personnel** (217) 532-9564 (OR 911) Montgomery County Sheriff: Jim Vazzi (217) 532-9511 Coffeen Police Department (217) 534-2216 Coffeen Fire Department (217) 534-2410

Coffeen Power Plant, Montgomery County, Illinois

Figure 2-3. EAP Response Process Decision Tree

Note: At any given below, if failure is imminent or actively occurring CALL 911 IMMEDIATELY to notify emergency responders and then continue with process afterwards.

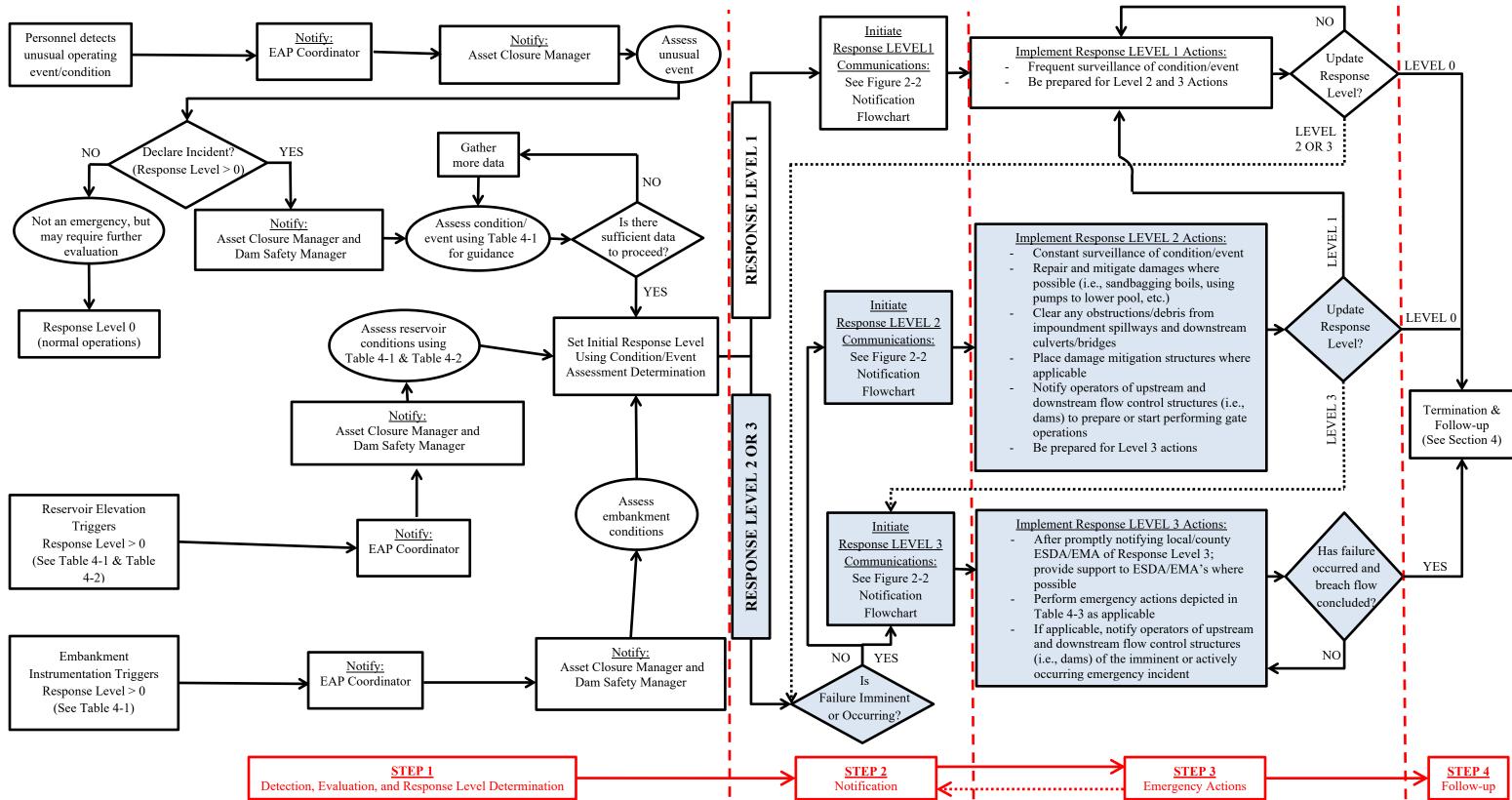


Table 2-1. EAP Emergency Responders

Position / Entity	Contact Ir	ıformation	
	al Contacts		
Coffeen Power Plant	Pho	ne #	
Asset Closure Manager	(903) 5	77-5207	
Environmental Manager (EAP Coordinator)	(217) 34	41-7319	
Control Room	(217) 53	34-7621	
IPGC Corporate Operations	Con	tact	
Dam Safety Manager		(618) 792-8488	
Extern	al Contacts		
Local/County ESDA/EMA, Police, & Fire	Phone #	Alternate Phone #	
Montgomery County 911 Emergency Communication Center	911	(217) 532-9564	
Montgomery County Sheriff	(217) 532-9511		
Montgomery County ESDA/EMA Coordinator	(217) 532-9564	(217) 254-6437	
Coffeen Police Department	(217) 534-2216		
Coffeen Fire Department	(217) 534-2410		
Montgomery County Ambulance	(217) 532-9562		
Montgomery County Engineer	(217) 532-6109		
State Emergency Management Agencies & Organizations	Contact Phone #		
IDNR-OWR Dam Safety Section Manager	(217) 782-4427		
Coffeen Lake State Fish & Wildlife Area	(217) 537-3351		
Illinois Conservation Police	(309) 338-1017		
Illinois State Police	(309) 833-4046		

3 EAP ROLES AND RESPONSIBILITIES

Table 3-1 provides a summary of the EAP roles during an emergency event.

Table 3-1. Summary of EAP Roles

Entity	Role Description
IPGC Emergency Response Team (ERT)	 ERT: IPGC personnel responsible for EAP implementation, distribution, updates/maintenance, and training activities. The ERT is comprised of the following roles: IPGC Corporate: IPGC corporate entity, committee, team, or position with relevant responsibility for a given generating power plant. Asset Closure Management: Personnel responsible for the management of the closure of the Power Plant. Dam Safety Manager: Personnel that is most knowledgeable about the design and technical operation of facilities at a given power plant. EAP Coordinator: Personnel responsible for implementing the EAP and associated activities.
Montgomery County ESDA/EMA	 Receive Response Level reports from <u>IPGC Corporate</u> through <u>EAP Coordinator</u>. Coordinate emergency response activities with local/county authorities: police, fire and rescue, etc. Coordinate notification of public as necessary through established channels, which may include doorto-door contact. Coordinate notification activities to affected parties within inundation areas. Evaluate risk to areas beyond the inundation areas, communicate needs to the <u>IPGC Corporate</u> and/or <u>EAP Coordinator</u>, and coordinate aid as appropriate. Responsible for declaring termination of an emergency condition off-site upon receiving notification of an emergency status termination from the <u>IPGC Corporate</u>. If necessary, coordinate with <u>State ESDA/EMA</u>.
Local/County Police, Fire & Rescue	 Receive alert status reports from the <u>ERT</u> or the Montgomery County <u>ESDA/EMA</u>. If necessary, notify affected parties and general public within inundation areas (see Section 7). Render assistance to Montgomery County <u>ESDA/EMA</u>, as necessary. Render assistance to <u>IPGC Corporate</u> and <u>Power Plant Management</u>, as necessary.

4 EAP RESPONSE

The 4-Step Incident Response Process is shown in Figure 2-1. The Decision Tree shown in Figure 2-3 provides a flowchart for the various elements of the response process. Upon reaching Step 4 of the response process (termination and follow-up), the EAP Coordinator is responsible for notifying the ESDA/EMA's that the condition of the dam/impoundment has been stabilized. The purpose of this section is to provide specific information that can be used during a response. This information is provided in the following tables:

- Table 4-1 provides guidance for determining the response level.
- Table 4-2 provides impoundment pool level trigger elevations.
- Table 4-3 lists emergency actions to be taken depending on the situation.

Table 4-1. Guidance for Determining the Response Level

	Table 4-1. Guidance for Determining the Response Level	
Event	Situation	Response Level
	Primary spillway flow is not causing active erosion and impoundment water surface elevation is below auxiliary spillway crest elevation (if equipped).	Level 0
	Impoundment water surface elevation is at or above auxiliary spillway crest elevation (if equipped). No active erosion caused by spillway flow.	Level 1
	Spillway flow actively causing minor erosion that is not threatening the control section or dam/impoundment stability.	Level 2
Spillway flow (See Table 4-2 for	Spillway flow that could result in flooding of people downstream if the reservoir level continues to rise.	Level 2
relevant elevations)	Abnormal operation of the spillway system due to blockage or damage that could lead to flooding.	Level 2
	Spillway flow actively eroding the soil around the spillway that is threatening the control section (e.g., undermining) or dam/impoundment stability.	Level 3
	Spillway flow that is flooding people downstream.	Level 3
Embankment	Impoundment water surface elevation at or below typical normal pool fluctuation elevation.	Level 0
Overtopping	Impoundment water surface elevation above typical high pool fluctuation elevation.	Level 1
(See Table 4-2 for relevant elevations)	Impoundment water surface elevation within 2 feet of the embankment crest elevation	Level 2
	Impoundment water surface elevation at or above embankment crest elevation.	Level 3
	New seepage areas in or near the dam/impoundment with clear flow.	Level 1
Seepage	New seepage areas with cloudy discharge or increasing flow rate.	Level 2
	Heavy seepage with active erosion, muddy flow, and/or sand boils.	Level 3
G: 11 1	Observation of new sinkhole in impoundment area or on embankment.	Level 2
Sinkholes	Rapidly enlarging sinkhole and/or whirlpool in the impoundment.	Level 3
Embankment cracking	New cracks in the embankment greater than ¼ inch wide without seepage.	Level 1
	Any crack in the embankment with seepage.	Level 2
oracking .	Enlarging cracks with muddy seepage.	Level 3

Table 4-1. Guidance for Determining the Response Level

Event	Situation	Response Level
	Visual signs of movement/slippage of the embankment slope.	Level 1
Embankment movement	Detectable active movement/slippage of the embankment slope or other related effects (tension cracking, bulges/heaves, etc.) that could threaten the integrity of the embankment.	Level 2
	Sudden or rapidly proceeding slides of the embankment slopes.	Level 3
Embankment Monitoring Equipment	Instrumentation readings beyond historic normal.	Level 1
(piezometers, inclinometers, surface	Instrumentation readings indicate the embankment is susceptible to failure.	Level 2
displacement mounts, etc.)	Instrumentation readings indicate embankment is at threshold of failure or is currently failing.	Level 3
	Measurable earthquake felt or reported on or within 100 miles of the impoundment.	Level 1
Earthquake or another event	Earthquake or other event resulting in visible damage to the impoundment or appurtenances.	Level 2
another event	Earthquake or other event resulting in uncontrolled release of water or materials from the impoundment.	Level 3
Security	Verified bomb threat or other physical threat that, if carried out, could result in damage to the impoundment.	Level 2
threat	Detonated bomb or other physical damage that has resulted in damage to the impoundment or appurtenances.	Level 3
	Damage to impoundment or appurtenance with no impact to the functioning of the impoundment.	Level 1
Sabotage/ vandalism	Modification to the impoundment or appurtenances that could adversely impact the functioning of the impoundment. This would include unauthorized operation of spillway facilities.	Level 2
	Damage to impoundment or appurtenances that has resulted in seepage flow.	Level 2
	Damage to impoundment or appurtenances that has resulted in uncontrolled water release.	Level 3

Table 4-2. Impoundment Trigger Elevations

I	Embankment	Auxiliary Spillway	Normal Pool Fluctuation	
Impoundment	Crest Elevation	Crest Elevation	Typical	High
GMF Pond	632.0 ft.	N/A	621 ft.	626 ft.
GMF Recycle Pond	629.0 ft.	624.1 ft.	610 ft.	623 ft.
Ash Pond No.1	637.5 ft.	631.0 ft.	629 ft.	633 ft.
Ash Pond No.2	638.0 ft.	N/A	N/A	N/A

Notes: Elevations are in reference to NAVD88

Table 4-3. Step 3: Emergency Actions

Condition	Description of Condition	Action to be Taken
High Water Level/ Large Spillway Release	Not applicable to capped impoundments. See Table 4-1 and Table 4-2 for elevations and triggering water levels associated with the impoundments and spillways covered by this EAP.	 Assess cause of increased reservoir stage, especially during fair weather conditions. Determine Response Level. Make proper notifications as outlined in the Figure 2-2 Notification Flowchart. Perform additional tasks as determined through consultation with the ERT. Make notifications if condition worsens such that downstream flooding is imminent. Response Level 0: require enhanced surveillance 3 times per day Response Level 1: contact internal chain of command and external response partners as necessary; inspect impoundment minimum 1 time per hour Response Level 2: contact internal chain of command; notify ESDA/EMA's and notify external response partners. ESDA/EMA's notify affected parties. Response Level 3: contact internal chain of command; notify ESDA/EMA's and notify external response partners. ESDA/EMA's notify affected parties of emergency incident.
Seepage	Localized new seepage or boil(s) observed along downstream face / toe of earthen embankment with muddy discharge and increasing but controllable discharge of water.	 Measure and record feature dimensions, approximate flow rate, and relative location to existing surface features. Take photos. Document location on a site plan and in inspection notes. Determine Response Level. Make proper notifications as outlined in the Figure 2-2 Notification Flowchart. ERT (with Dam Safety Manager as lead) to determine mitigation actions. The following actions may apply: Place a ring of sandbags with a weir at the top towards the natural drainage path to monitor flow rate. If boil becomes too large to sandbag, place a blanket filter over the area using non-woven filter fabric and pea gravel. Attempt to contain flow in such a manner (without performing any excavations) that flow rates can be measured. Stockpile gravel and sand fill for later use, if necessary. Inspect the embankment and collect piezometer, water level and seepage flow data daily unless otherwise instructed by the Dam Safety Manager. Record any changes of conditions. Carefully observe embankment for signs of depressions, seepage, sinkholes, cracking or movement. Maintain continuous monitoring of feature. Record measured flow rate and any changes of condition, including presence or absence of muddy discharge. Make notifications as outlined in the lower portion of the Notification Flowchart (Figure 2-2) if condition worsens such that failure is imminent.

Table 4-3. Step 3: Emergency Actions

Condition	Description of Condition	Action to be Taken
Sabotage and Miscellaneous Other Issues	Criminal action with significant damage to embankment or structures where significant repairs are required and the integrity of the facility is compromised—condition appears stable with time.	 Contact law enforcement authorities and restrict all access (except emergency responders) to impoundment. Restrict traffic on embankment crest to essential emergency operations only. Determine Response Level. Make internal notifications as outlined in the upper portion of the Notification Flowchart (Figure 2-2). In conjunction with the Dam Safety Manager, assess extent of damage and visually inspect entire embankment and ancillary structures for additional less obvious damage. Based on inspection results, confirm if extent of damage to various components of the impoundment warrants a revised Response Level and additional notifications. Perform additional tasks as directed by the ERT. Make notifications if conditions worsen.
Embankment Deformation	Cracks: New longitudinal (along the embankment) or transverse (across the embankment) cracks more than 6 inches deep or more than 3 inches wide or increasing with time. New concave cracks on or near the embankment crest associated with slope movement.	 Measure and record feature dimensions, approximate flow rate, and relative location to existing surface features. Take photos. Document location on a site plan and in inspection notes. Restrict traffic on embankment crest to essential emergency operations only. Determine Response Level. Make notifications as outlined in the Figure 2-2 Notification Flowchart. ERT (with Dam Safety Manager as lead) to determine mitigation actions. The following actions may apply: Place buttress fill against base of slope immediately below surface feature. Stockpile additional fill. Place sandbags as necessary around crack area to divert any storm water runoff from flowing into crack(s). As directed by the Dam Safety Manager, additional inspection and monitoring of the dam may be required. Items may include: inspect the dam on a schedule determined by the Dam Safety Manager; collect piezometer and water level data; and record any changes of condition. Carefully observe dam for signs of depressions, seepage, sinkholes, cracking or movement. Make notifications as outlined in the Figure 2-2 Notification Flowchart if conditions worsen such that failure is imminent.
	Slides / Erosion: Deep slide / erosion (greater than 2 feet deep) on the embankment that may also extend beyond the embankment toe but does not encroach onto the embankment crest and appears stable with time.	 Measure and record feature dimensions, approximate flow rate, and relative location to existing surface features. Take photos. Document location on a site plan and in inspection report. Restrict traffic on embankment crest to essential emergency operations only. Determine the Response Level. Make notifications as outlined in the Figure 2-2 Notification Flowchart. ERT (with Dam Safety Manager as lead) to determine mitigation actions. Additional actions may include the following items. a) Place sandbags as necessary around slide area to divert any storm water runoff from flowing into slide(s). b) Increase inspections of the dam; collect piezometer and water level data; and record any changes of condition. During inspections, carefully observe dam for signs of depressions, seepage, sinkholes, cracking or movement. Make notifications as outlined in the Figure 2-2 Notification Flowchart if conditions worsen such that failure is imminent.

Table 4-3. Step 3: Emergency Actions

Condition	Description of Condition	Action to be Taken
Embankment Deformation (cont.)	Sinkholes: Small depression observed on the embankment or within 50 feet of the embankment toe that is less than 5 feet deep and 30 feet wide or which is increasing with time.	 Slowly open drain gates to lower pool elevation. Measure and record feature dimensions, approximate flow rate, and relative location to existing surface features. Take photos. Document location on a site plan and in inspection notes. Restrict traffic on embankment crest to essential emergency operations only. Determine Response Level. Make notifications as outlined in the Figure 2-2 Notification Flowchart. ERT (with Dam Safety Manager as lead) to determine mitigation actions. Additional actions may include the following items: a) Backfill the depression with relatively clean earth fill (free of organic materials) generally even with surrounding grade and slightly mounded (6 to 12 inches higher) in the center in order to shed storm water away from the depression. Stockpile additional fill. b) Increase inspections of the dam; collect piezometer and water level data daily unless otherwise instructed by Dam Safety Manager; and record any changes of condition. Carefully observe dam for signs of depressions, seepage, sinkholes, cracking or movement. Make notifications as outlined in the Figure 2-2 Notification Flowchart if conditions worsen such that failure is imminent.
Gate Malfunction or Failure	Sluice gate damaged structurally (sabotage, debris, etc.) with uncontrolled release of water at a constant volume. Condition appears stable.	 Close any other gates, if open. Determine Response Level. Make notifications as outlined in the Figure 2-2 Notification Flowchart. Obtain instructions from the Dam Safety Manager to determine if there are other methods to stop or slow down the flow of water. If conditions worsen such that failure is imminent, make notifications as outlined in the lower portion of the Figure 2-2 Notification Flowchart.

5 PREPAREDNESS

The intent of this section is to provide information that will be utilized during a response. Established emergency supplies and locations, suppliers, and equipment are provided in Table 5-1. Supplier contact information is listed in Table 5-2.

A coordination meeting shall be conducted annually between representatives of the Illinois Power Generating Company and local emergency responders. This meeting may be in the form of a face-to-face meeting, tabletop exercise, or additional training regarding the EAP.

Table 5-1. Emergency Supplies and Equipment

Flashlights Yes Contact EAP Coordinator for location(s). Generator Yes Contact EAP Coordinator for location(s). Contact Grand Rental Station for additional generators (see Table 5-2). Extension Cords Yes Contact EAP Coordinator for location(s). Fire extinguishers Yes Contact EAP Coordinator for location(s). Floodlights Yes Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Backhoe Yes Backhoe/tractor and 650 John Deere Track hoe available. Contact EAP Coordinator for location(s). Dozer Yes D7 & D10 Crawler Tractors and two rubber-tired dozers (844B & 834B). Contact EAP Coordinator for location(s). Large Equipment Rental (excavating equipment, large earth moving, etc.) Pump and Hoses Yes Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Contact United Rental (excavating equipment, large earth moving, etc.) Pump and Hoses Yes Contact EAP Coordinator for location(s). Contact The Curry Companies or Rain for Rent for high-capacity portable pumps (see Table 5-2). Sandbags Yes Contact EAP Coordinator for location(s). Contact The Curry Companies or Rain for Rent for high-capacity portable pumps (see Table 5-2). Contact EAP Coordinator for location(s). Contact Great Western Bag Co. for additional sandbags (see Table 5-2). Contact EAP Coordinator for location(s). Contact Great Western Bag Co. for additional sandbags (see Table 5-2). Contact EAP Coordinator for location(s). Contact Isted suppliers in Table 5-20 for gavel, sand, and riprap fill as necessary. Contact EAP Coordinator for location(s). Contact Vandalia Ready Mix and/or Greenville Ready Mix for additional concrete (see Table 5-2). Geotextile Filter Fabric Yes Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Should be maintained in close proximity to any features that might require immediate access. Personal Flotation Devices Yes Contact EAP Coordinator for location(s).	Item	On-site (Yes/No/Occasionally)	Remarks
Extension Cords Yes Contact EAP Coordinator for location(s). Fire extinguishers Yes Contact EAP Coordinator for location(s). Floodlights Yes Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Contact Grand Rental Station for additional emergency lighting (see Table 5-2). Backhoe Yes Backhoe Yes Backhoe/tractor and 650 John Deere Track hoe available. Contact EAP Coordinator for location(s). Dozer Yes D7 & D10 Crawler Tractors and two rubber-tired dozers (844B & 834B). Contact EAP Coordinator for location(s). Dump Truck Yes Contact EAP Coordinator for location(s). Contact EAP Coordinator for availability and location(s). Contact United Rental (excavating equipment, large earth moving, etc.) Pump and Hoses Yes Contact EAP Coordinator for location(s). Contact The Curry Companies or Rain for Rent for high-capacity portable pumps (see Table 5-2). Sandbags Yes Contact EAP Coordinator for location(s). Contact The Curry Companies or Rain for Rent for high-capacity portable pumps (see Table 5-2). Fill (Stone, aggregate, sand) Yes Contact EAP Coordinator for location(s). Contact United sudditional sandbags (see Table 5-2). Contact EAP Coordinator for location(s). Contact United See Table 5-2 for gravel, sand, and riprap fill as necessary. Contact EAP Coordinator for location(s). Contact United See Table 5-2 for gravel, sand, and riprap fill as necessary. Two rolls of 10-ounce, non-woven filter fabric available. Contact EAP Coordinator for location(s). Plastic Sheeting Yes Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s).	Flashlights	Yes	Contact EAP Coordinator for location(s).
Fire extinguishers Yes Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Contact Grand Rental Station for additional emergency lighting (see Table 5-2). Backhoe Yes Backhoe/tractor and 650 John Deere Track hoe available. Contact EAP Coordinator for location(s). Dozer Yes D7 & D10 Crawler Tractors and two rubber-tired dozers (844B & 834B). Contact EAP Coordinator for location(s). Dump Truck Yes Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Contact EAP Coordinator for availability and location(s). Contact United Rental (excavating equipment, large earth moving, etc.) Pump and Hoses Yes Contact EAP Coordinator for availability and location(s). Contact United Rentals (see Table 5-2) and/or other nearby large equipment rental providers for additional large equipment as necessary. Contact EAP Coordinator for location(s). Contact The Curry Companies or Rain for Rent for high-capacity portable pumps (see Table 5-2). Sandbags Yes Contact EAP Coordinator for location(s). Contact Great Western Bag Co. for additional sandbags (see Table 5-2). Fill (Stone, aggregate, sand) Yes Contact EAP Coordinator for location(s). Contact listed suppliers in Table 5-2 for gravel, sand, and riprap fill as necessary. Concrete/grout Yes Contact EAP Coordinator for location(s). Contact Vandalia Ready Mix and/or Greenville Ready Mix for additional concrete (see Table 5-2). Two rolls of 10-ounce, non-woven filter fabric available. Contact EAP Coordinator for location(s). Plastic Sheeting Yes Contact EAP Coordinator for location(s). Should be maintained in close proximity to any features that might require immediate access.	Generator	Yes	
Floodlights Yes Contact EAP Coordinator for location(s). Contact Grand Rental Station for additional emergency lighting (see Table 5-2). Backhoe Yes Backhoe/tractor and 650 John Deere Track hoe available. Contact EAP Coordinator for location(s). Dozer Yes D7 & D10 Crawler Tractors and two rubber-tired dozers (844B & 834B). Contact EAP Coordinator for location(s). Dump Truck Yes Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Contact EAP Coordinator for availability and location(s). Contact United Rentals (see Table 5-2) and/or other nearby large equipment rental providers for additional large equipment as necessary. Pump and Hoses Yes Contact EAP Coordinator for location(s). Contact The Curry Companies or Rain for Rent for high-capacity portable pumps (see Table 5-2). Sandbags Yes Contact EAP Coordinator for location(s). Contact Great Western Bag Co. for additional sandbags (see Table 5-2). Fill (Stone, aggregate, sand) Yes Contact EAP Coordinator for location(s). Contact Great Western Bag Co. for additional sandbags (see Table 5-2). Concrete/grout Yes Contact EAP Coordinator for location(s). Contact listed suppliers in Table 5-2 for gravel, sand, and riprap fill as necessary. Concrete/grout Yes Contact EAP Coordinator for location(s). Contact Vandalia Ready Mix and/or Greenville Ready Mix for additional concrete (see Table 5-2). Two rolls of 10-ounce, non-woven filter fabric available. Contact EAP Coordinator for location(s). Plastic Sheeting Yes Contact EAP Coordinator for location(s). Should be maintained in close proximity to any features that might require immediate access.	Extension Cords	Yes	Contact EAP Coordinator for location(s).
Backhoe Yes Backhoe/tractor and 650 John Deere Track hoe available. Contact EAP Coordinator for location(s). Dozer Yes D7 & D10 Crawler Tractors and two rubber-tired dozers (844B & 834B). Contact EAP Coordinator for location(s). Dump Truck Yes Contact EAP Coordinator for location(s). Large Equipment Rental (excavating equipment, large earth moving, etc.) Pump and Hoses Yes Contact EAP Coordinator for location(s). Contact United Rentals (see Table 5-2) and/or other nearby large equipment rental providers for additional large equipment as necessary. Pump and Hoses Yes Contact EAP Coordinator for location(s). Contact The Curry Companies or Rain for Rent for high-capacity portable pumps (see Table 5-2). Sandbags Yes Contact EAP Coordinator for location(s). Contact Great Western Bag Co. for additional sandbags (see Table 5-2). Fill (Stone, aggregate, sand) Yes Contact EAP Coordinator for location(s). Contact listed suppliers in Table 5-2 for gravel, sand, and riprap fill as necessary. Concrete/grout Yes Contact EAP Coordinator for location(s). Contact Vandalia Ready Mix and/or Greenville Ready Mix for additional concrete (see Table 5-2). Geotextile Filter Fabric Yes Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Rope Yes Contact EAP Coordinator for location(s). Should be maintained in close proximity to any features that might require immediate access.	Fire extinguishers	Yes	Contact EAP Coordinator for location(s).
Dozer Yes Coordinator for location(s). Dozer Yes D10 Crawler Tractors and two rubber-tired dozers (844B & 834B). Dump Truck Yes Contact EAP Coordinator for location(s). Large Equipment Rental (excavating equipment, large earth moving, etc.) Pump and Hoses Yes Contact EAP Coordinator for availability and location(s). Contact United Rentals (see Table 5-2) and/or other nearby large equipment rental providers for additional large equipment as necessary. Contact EAP Coordinator for location(s). Contact The Curry Companies or Rain for Rent for high-capacity portable pumps (see Table 5-2). Sandbags Yes Contact EAP Coordinator for location(s). Contact Great Western Bag Co. for additional sandbags (see Table 5-2). Fill (Stone, aggregate, sand) Yes Contact EAP Coordinator for location(s). Contact listed suppliers in Table 5-2 for gravel, sand, and riprap fill as necessary. Concrete/grout Yes Contact EAP Coordinator for location(s). Contact Vandalia Ready Mix and/or Greenville Ready Mix for additional concrete (see Table 5-2). Geotextile Filter Fabric Yes Two rolls of 10-ounce, non-woven filter fabric available. Contact EAP Coordinator for location(s). Plastic Sheeting Yes Contact EAP Coordinator for location(s). Should be maintained in close proximity to any features that might require immediate access.	Floodlights	Yes	
Dump Truck Yes Contact EAP Coordinator for location(s). Dump Truck Yes Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Contact EAP Coordinator for availability and location(s). Contact United Rentals (see Table 5-2) and/or other nearby large equipment rental providers for additional large equipment as necessary. Pump and Hoses Yes Contact EAP Coordinator for location(s). Contact The Curry Companies or Rain for Rent for high-capacity portable pumps (see Table 5-2). Sandbags Yes Contact EAP Coordinator for location(s). Contact Great Western Bag Co. for additional sandbags (see Table 5-2). Fill (Stone, aggregate, sand) Yes Contact EAP Coordinator for location(s). Contact listed suppliers in Table 5-2 for gravel, sand, and riprap fill as necessary. Concrete/grout Yes Contact EAP Coordinator for location(s). Contact Vandalia Ready Mix and/or Greenville Ready Mix for additional concrete (see Table 5-2). Two rolls of 10-ounce, non-woven filter fabric available. Contact EAP Coordinator for location(s). Plastic Sheeting Yes Contact EAP Coordinator for location(s).	Backhoe	Yes	
Large Equipment Rental (excavating equipment, large earth moving, etc.) Pump and Hoses Yes Contact EAP Coordinator for availability and location(s). Contact United Rentals (see Table 5-2) and/or other nearby large equipment rental providers for additional large equipment as necessary. Pump and Hoses Yes Contact EAP Coordinator for location(s). Contact The Curry Companies or Rain for Rent for high-capacity portable pumps (see Table 5-2). Sandbags Yes Contact EAP Coordinator for location(s). Contact Great Western Bag Co. for additional sandbags (see Table 5-2). Contact EAP Coordinator for location(s). Contact listed suppliers in Table 5-2 for gravel, sand, and riprap fill as necessary. Concrete/grout Yes Contact EAP Coordinator for location(s). Contact Vandalia Ready Mix and/or Greenville Ready Mix for additional concrete (see Table 5-2). Two rolls of 10-ounce, non-woven filter fabric available. Contact EAP Coordinator for location(s). Plastic Sheeting Yes Contact EAP Coordinator for location(s). Should be maintained in close proximity to any features that might require immediate access.	Dozer	Yes	
Rental (excavating equipment, large earth moving, etc.) Pump and Hoses Yes Contact EAP Coordinator for location(s). Contact United Rentals (see Table 5-2) and/or other nearby large equipment rental providers for additional large equipment as necessary. Yes Contact EAP Coordinator for location(s). Contact The Curry Companies or Rain for Rent for high-capacity portable pumps (see Table 5-2). Sandbags Yes Contact EAP Coordinator for location(s). Contact Great Western Bag Co. for additional sandbags (see Table 5-2). Fill (Stone, aggregate, sand) Yes Contact EAP Coordinator for location(s). Contact listed suppliers in Table 5-2 for gravel, sand, and riprap fill as necessary. Contact EAP Coordinator for location(s). Contact Vandalia Ready Mix and/or Greenville Ready Mix for additional concrete (see Table 5-2). Two rolls of 10-ounce, non-woven filter fabric available. Contact EAP Coordinator for location(s). Plastic Sheeting Yes Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Should be maintained in close proximity to any features that might require immediate access.	Dump Truck	Yes	Contact EAP Coordinator for location(s).
Rain for Rent for high-capacity portable pumps (see Table 5-2). Sandbags Yes Contact EAP Coordinator for location(s). Contact Great Western Bag Co. for additional sandbags (see Table 5-2). Fill (Stone, aggregate, sand) Yes Contact EAP Coordinator for location(s). Contact listed suppliers in Table 5-2 for gravel, sand, and riprap fill as necessary. Concrete/grout Yes Contact EAP Coordinator for location(s). Contact Vandalia Ready Mix and/or Greenville Ready Mix for additional concrete (see Table 5-2). Two rolls of 10-ounce, non-woven filter fabric available. Contact EAP Coordinator for location(s). Plastic Sheeting Yes Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Should be maintained in close proximity to any features that might require immediate access.	Rental (excavating equipment, large earth	Occasionally	Rentals (see Table 5-2) and/or other nearby large equipment rental providers
Additional sandbags (see Table 5-2). Fill (Stone, aggregate, sand) Yes Contact EAP Coordinator for location(s). Contact listed suppliers in Table 5-2 for gravel, sand, and riprap fill as necessary. Concrete/grout Yes Contact EAP Coordinator for location(s). Contact Vandalia Ready Mix and/or Greenville Ready Mix for additional concrete (see Table 5-2). Two rolls of 10-ounce, non-woven filter fabric available. Contact EAP Coordinator for location(s). Plastic Sheeting Yes Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Should be maintained in close proximity to any features that might require immediate access.	Pump and Hoses	Yes	
(Stone, aggregate, sand) Concrete/grout Yes Contact EAP Coordinator for location(s). Contact Vandalia Ready Mix and/or Greenville Ready Mix for additional concrete (see Table 5-2). Two rolls of 10-ounce, non-woven filter fabric available. Contact EAP Coordinator for location(s). Plastic Sheeting Yes Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Should be maintained in close proximity to any features that might require immediate access.	Sandbags	Yes	
Concrete/grout Yes and/or Greenville Ready Mix for additional concrete (see Table 5-2). Two rolls of 10-ounce, non-woven filter fabric available. Contact EAP Coordinator for location(s). Plastic Sheeting Yes Contact EAP Coordinator for location(s). Contact EAP Coordinator for location(s). Should be maintained in close proximity to any features that might require immediate access.		Yes	
Coordinator for location(s). Plastic Sheeting Yes Contact EAP Coordinator for location(s). Rope Yes Contact EAP Coordinator for location(s). Should be maintained in close proximity to any features that might require immediate access.	Concrete/grout	Yes	
Rope Yes Contact EAP Coordinator for location(s). Should be maintained in close proximity to any features that might require immediate access.	Geotextile Filter Fabric	Yes	
proximity to any features that might require immediate access.	Plastic Sheeting	Yes	Contact EAP Coordinator for location(s).
Personal Flotation Devices Yes Contact EAP Coordinator for location(s).	Rope	Yes	
	Personal Flotation Devices	Yes	Contact EAP Coordinator for location(s).

Table 5-2. Supplier Addresses

	Table 5-2. Supplier Addre		
Supply/Rental Item(s)	Supplier Contact Information	Distance from Site (miles)	Address
Sandbags	Great Western Bag Co. (314) 421-0498 (days) (314) 993-5287 (nights/weekends)	66	1416 N. Broadway St. Louis, MO
Gravel, Sand, & Riprap	Fuller Brothers Ready Mix (217) 532-2422	11	935 Ash Street Hillsboro, IL
Gravel, Sand, & Riprap	Vandalia Sand and Gravel (618) 283-4029	20	Route 140 Vandalia, IL
Gravel, Sand, & Riprap	Central Illinois Materials (618) 283-3259	20	RR 2 Vandalia, IL
Gravel, Sand, Riprap & High- Capacity Portable Pumps	The Curry Companies Brian Fenton: (217) 854-3101	40	21149 Route 4 Carlinville, IL
High-Capacity Portable Pumps	Rain for Rent Mark ByBee: (618) 931-0901	60	3711 Horseshoe Lake Road Granite City, IL
High-Capacity Portable Pumps	Linden and Company (800) 383-4811	145	800 W. Deerbrook Peoria, IL 61615
High-Capacity Portable Pumps	Heartland Pumps (618) 985-5110	120	1800 Supply Road, Suite A Carterville, IL 62918
Emergency Lighting, 5,000 to 8,500watt Generators, Concrete Mixers, Compact Excavators, Skid Steers, Portable Pumps, & Plate Compactors	Grand Rental Station Fairview Heights, IL (618) 277-7750 (866) 645-0218 (after hours)	60	5612 N. Illinois Street Fairview Heights, IL
Emergency Lighting, 4,000watt Generators, Concrete Mixers, Tractor Backhoes/Loaders, Compact Excavators, Skid Steers, Portable Pumps, & Compactors (Plate & Vibratory)	Grand Rental Station Litchfield, IL (217) 324-4000 (866) 645-0218 (after hours)	20	1105 West Weir Street Litchfield, IL 62056
Concrete (Ready Mix Concrete Supplier)	Vandalia Ready Mix (618) 283-1600	20	1021 Janette Drive Vandalia, IL 62471
Concrete (Ready Mix Concrete Supplier)	Greenville Ready Mix (618) 664-1340	17	1311 S. 4 th Street Greeneville, IL 62246
Large Earthmoving Equipment (25,000 to 50,000 lb. Track hoe Excavators & 3.0 to 3.4 CY Wheel Loaders)	<u>United Rentals</u> (618) 345-6050	60	5076 Mid America Court Collinsville, IL 62234

6 FACILITY/IMPOUNDMENT DESCRIPTION

The impoundments included in this EAP are described as follows and illustrated in Figure 1-2. Table 6-1 contains additional geometric details for each impoundment.

The Coffeen Power Plant is located in Montgomery County, Illinois approximately 1.5 miles south of Coffeen, Illinois. The plant is located on the east bank of Coffeen Lake, which is an impoundment created by Coffeen Lake Dam.

The GMF Pond is located northeast of power plant and north of the GMF Recycle Pond. The GMF Pond consists of a single pond formed by an earthen embankment around its perimeter. The earthen embankment crest elevation is currently 13-feet above grade; however, its final design has a crest elevation 100-feet above grade. The final design was used in the breach analysis and corresponding inundation map because the final design is more conservative than existing conditions (larger volume, greater dam height, etc.). The GMF Pond is used to dewater, store and dispose of gypsum from the Power Plant's flue gas desulphurization system. The GMF Pond discharges via a HDPE lined transfer channel into the GMF Recycle Pond. The transfer channel has a trapezoidal cross section with 3H:1V side slopes, a bottom width of 32 feet, a depth of 9-feet, and is 500-feet long.

The GMF Recycle Pond is located northeast of the power plant and south of the GMF Pond. The GMF Recycle Pond consists of a single pond formed by an earthen embankment around its perimeter. The pond is used to dewater, store and dispose of gypsum. It also is used to retain stormwater discharge from the GMF Pond transfer channel. The pool level is controlled by a recycle pump system that is located at the southeast corner. There is an emergency spillway located at the northeast corner that discharges into a creek that runs along the east side of the pond and discharges into the eastern cove of Coffeen Lake (Eastern Cove).

Ash Pond No. 1 is located east of the power plant and consists of a single pond formed by earthen embankments around the perimeter. The pool level is controlled by a recycle pump system that is located at the northwest corner. The emergency spillway consists of a pipe that connects to the top of the recycle pump intake pipe. The emergency spillway discharges into the cooling water discharge channel to the north which feeds into the eastern cove of Coffeen Lake. The stored material settled in the bottom of the pond consists of primarily bottom ash and boiler slag.

Ash Pond No. 2 is located east of the power plant, north of Ash Pond No. 1 and west of the Cooling Water Pond. Ash Pond No. 2 was closed by leaving CCR in place and constructing a final cover system. The boundaries of these impoundments encompass a total area of approximately 60 acres.

Table 6-1. Power Plant Impoundment Characteristics

Feature/Parameter	GMF Pond	GMF Recycle Pond	Ash Pond No.1	Ash Pond No.2
Maximum Embankment Height	13.0 feet	20.0 feet	41.5 feet	28.0 feet
Length of Dam	5,060 feet	3,600 feet	4,300 feet	6,400 feet
Crest Width	20 feet	20 feet	N/A	N/A
Crest Elevation	632.0 feet	629.0 feet	637.5 feet	638.0 feet
Reservoir Area at Top of Dam	37.6 acres	17.0 acres	N/A	N/A
Storage Capacity at Top of Dam	442 acre-feet	324 acre-feet	N/A	N/A
Primary Spillway Type	Trapezoidal Channel	Recycle Pump	Recycle Pump (48" dia. steel intake pipe)	Stormwater let- down structures are now the spillways
Primary Spillway Crest Elevation	623.0 feet	610.0 feet	N/A	N/A
Storage Capacity at Primary Spillway Elevation	1,150 acre-feet	49.7 acre-feet	N/A	N/A
Reservoir Area at Normal Water Surface Elevation	27.0 acres	10.4 acres	N/A	N/A
Auxiliary Spillway Type	None	(3x) 6'x6' conc. risers to (3x) 4'dia. HDPE pipes	N/A	N/A
Auxiliary Spillway Crest Elevation	N/A	624.1 feet	N/A	N/A

Notes: Elevations are in reference to NAVD88

7 BREACH INUNDATION MAPS AND POTENTIAL IMPACTS

Inundation maps for GMF Pond, GMF Recycle Pond, Ash Pond No.1, and Ash Pond No.2 potential breach scenarios are provided in the following pages. It is the Montgomery County ESDA/EMA's responsibility to keep a current list of affected parties/properties to contact in the case of emergencies that result in Response Level 2 or 3. This list should encompass all properties within and adjacent to the probable inundation extents shown in the provided inundation maps.

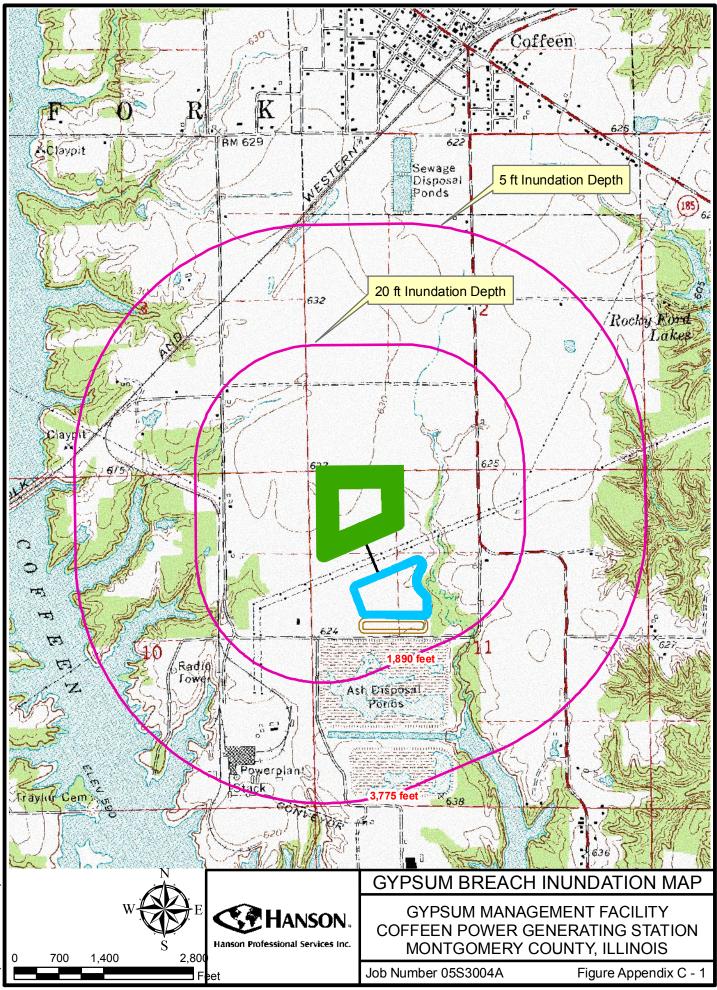
The methodology used to identify probable inundation extents for potential breach scenarios varied as a function of the impoundment size, location, surrounding topography, and surrounding structures/facilities/waterbodies.

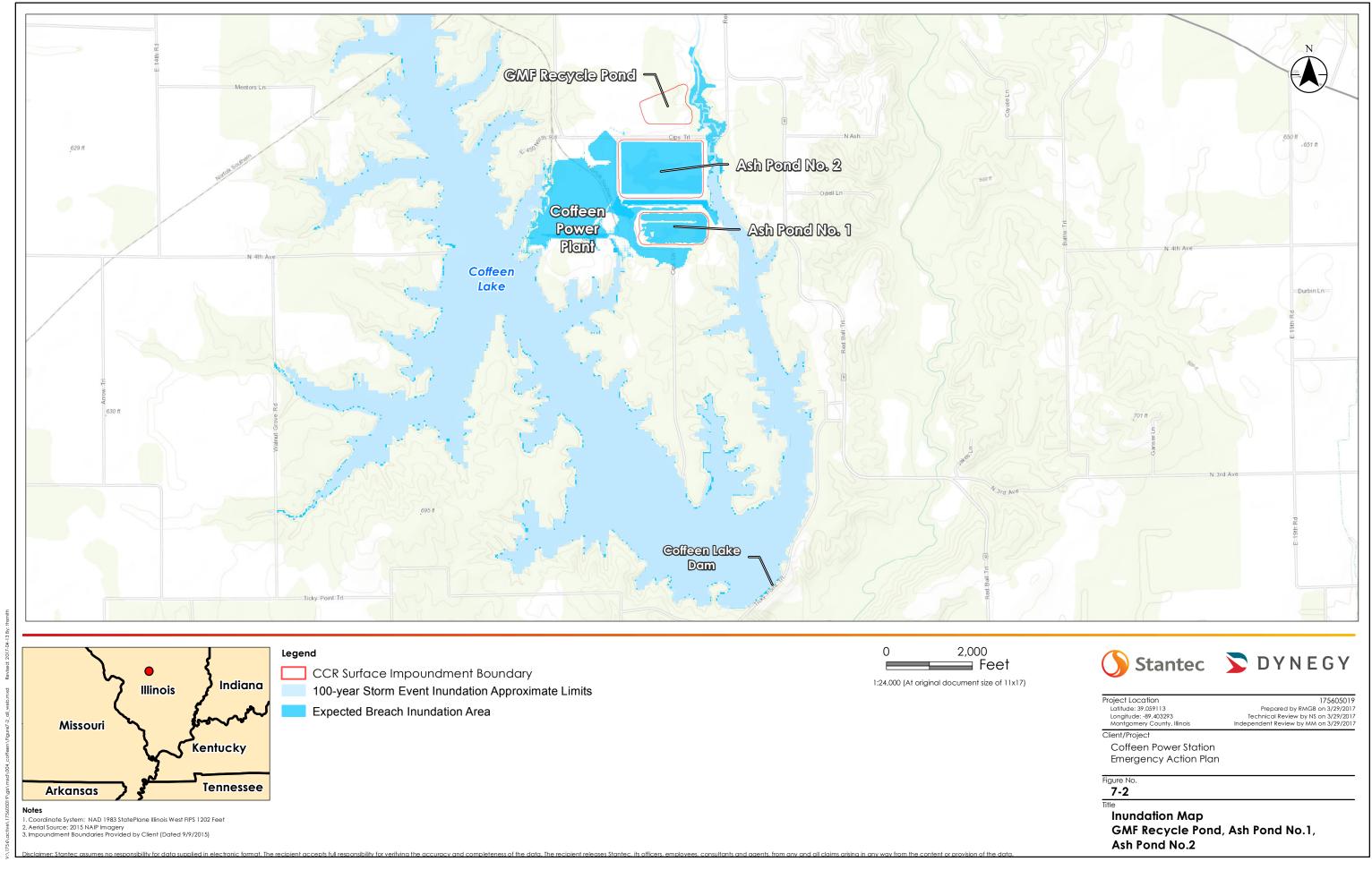
The GMF Pond inundation map was developed by Hanson Professional Services Inc. (2007) using final design conditions (100-feet tall and 2,478 acre-feet of stored volume) and an approximate method of computing the inundation limits of gypsum slurry by computing a runout distance on a constant slope. It was assumed that a breach of the earthen perimeter embankment would cause saturated gypsum material to liquefy and release towards downstream areas in a semi-circular pattern. This breach scenario was simulated at multiple locations along the earthen perimeter dike and the corresponding results were used to create the inundation map shown as Figure 7-1.

The GMF Recycle Pond breach analysis consisted of a Probable Maximum Precipitation (PMP) failure scenario at the eastern embankment. The resultant breach discharges were modeled downstream using 1D and 2D capabilities of HEC-RAS. The approximate inundation area is illustrated in the inundation map shown as Figure 7-2.

The Ash Pond No. 1 breach analysis consisted of PMP failure scenarios of the embankment near the northwest and northeast corners of the pond. The breach discharges were modeled downstream using 1D and 2D capabilities of HEC-RAS. The approximate inundation area is illustrated in the inundation map shown as Figure 7-2.

The Ash Pond No. 2 breach analysis consisted of a failure scenario where the stored volume liquefies and breaches the embankment near the southwest corner of the pond. The breach discharge was modeled downstream using 2D capabilities of HEC-RAS. The approximate inundation area is illustrated in the inundation map shown as Figure 7-2.





ATTACHMENT G

CCR Fugitive Dust Control Plan

for Coffeen Power Plant

Prepared for:

Illinois Power Generating Company

Coffeen Power Plant 134 Cips Lane Coffeen, IL 62017

Prepared by:

Burns & McDonnell Kansas City, Missouri

Amendment 1

October 2021

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1 Introduction

This Coal Combustion Residuals (CCR) fugitive dust control plan has been prepared for the retired Coffeen Power Plant, located in Montgomery County, Illinois. This plan addresses the air criteria in 40 C.F.R. § 257.80 of the United States Environmental Protection Agency's CCR rule, which requires the owner or operator of a CCR unit to "adopt measures that will effectively minimize CCR from becoming airborne at the facility" and to "prepare and operate in accordance with a CCR fugitive dust control plan." The plan also addresses the air criteria in 35 I.A.C. 845.500 of the Illinois Environmental Protection Agency's CCR rule, which contains similar requirements to the federal CCR rule.

1.1 Facility Information

- Facility Name: Coffeen Power Plant (Retired)
- Facility Address: 134 Cips Lane, Coffeen, IL 62017
- Owner/Operator: Illinois Power Generating Company

1.2 Certification

The owner or operator must obtain a certification from a qualified professional engineer that the initial CCR fugitive dust control plan, or any subsequent amendment of it, meets the requirements of 40 C.F.R. § 257.80 and 35 I.A.C. 845.500. See 40 C.F.R. § 257.80(b)(7); 35 I.A.C. 845.500(b)(7).

I certify under penalty of law that, to the best of my knowledge, this plan meets the requirements of 40 C.F.R. § 257.80 and 35 I.A.C. 845.500. This certification is based on my review of the document and conditions at the site and on my inquiry of the person or persons who managed the preparation of this document.

John R. Hesemann
Printed Name of Qualified Professional Engineer

1062-058523

| Signature of Qualified Professional Engineer and Date

| O62.058523 - Illinois - Expires 11/30/2021 | Registration Number and State

| O62.058523 - Illinois - Expires 11/30/2021 | Registration Number and State

2 CCR Fugitive Dust Control Measures and Appropriateness

CCR fugitive dust has the potential to become airborne at the facility during periods of CCR management in the CCR units, CCR handling and CCR transport. Areas at the facility that have the potential for airborne CCR fugitive dust are CCR surface impoundments, a CCR landfill, CCR handling equipment and CCR transport in trucks. This section identifies and describes the control measures selected and adopted by the facility to minimize CCR from becoming airborne at the facility and explains how the selected measures are applicable and appropriate for site conditions. The control measures may be adjusted or modified based on observed effectiveness of minimizing CCR from becoming airborne and weather conditions.

2.1 Management of CCR in the CCR Units

The facility manages CCR in surface impoundments and a landfill located at the facility. Table 2-1 below identifies CCR fugitive dust control measures that have been selected for use by the facility during CCR management in the CCR units, including placement of CCR into the CCR units, and explains how the selected measures are applicable and appropriate for site conditions. The facility will use the identified measures during CCR management in the CCR units to minimize CCR from becoming airborne at the facility.

CCR Activity	CCR Fugitive Dust Control Measure	Applicability and Appropriateness of Control Measure
	Condition CCR to be emplaced in the landfill before emplacement.	Conditioning CCR placed in the landfill allows CCR to bind together and thus minimizes the potential for CCR fugitive dust generation when CCR is managed in the landfill. The added moisture content will prevent wind dispersal of the CCR but will not result in free liquids. Use of conditioned CCR also achieves at least equivalent performance to conventional daily cover in terms of preventing wind entrainment.
Management of CCR in the facility's CCR units	Wet management of CCR bottom ash and flue gas desulfurization materials in CCR surface impoundments.	Wet management of CCR minimizes the potential for CCR fugitive dust generation.
	Water areas of exposed CCR in CCR units, as necessary.	Water will be applied to areas of exposed CCR to maintain moisture content to minimize the potential for CCR fugitive dust generation in excessively dry or windy conditions.
	Naturally occurring grass vegetation in areas of exposed CCR in CCR surface impoundments.	Vegetation provides a wind screen and/or cover and reduces wind entrainment of CCR.
	Reduce or halt operations during high wind events, as necessary.	Reducing or halting operations during high wind events minimizes the potential for CCR fugitive dust generation.

Table 2-1. Control Measures for CCR Management in CCR Units

2.2 Handling of CCR

Bottom ash may be periodically removed from the CCR surface impoundments and remains sufficiently wet during and after handling activities, including dewatering, associated with transfer of the CCR. Table 2-2 below identifies CCR fugitive dust control measures that have been selected for use by the facility during handling of CCR and explains how the selected measures are applicable and appropriate for site conditions. The facility will use the identified measures when handling CCR to minimize CCR from becoming airborne at the facility.

CCR Activity	CCR Fugitive Dust Control Measure	Applicability and Appropriateness of Control Measure
Handling of CCR at the facility	CCR bottom ash removed from CCR surface impoundments and loaded into trucks for transport remains conditioned during handling.	Conditioned CCR allows CCR to bind together and thus minimizes the potential for CCR fugitive dust generation when CCR is handled.
	CCR fly ash to be emplaced in the landfill is conditioned before emplacement.	Conditioning allows CCR to bind together and thus minimizes the potential for CCR fugitive dust generation.
	Condition CCR materials to be transported offsite before they are loaded into trucks, as necessary.	Conditioning allows CCR to bind together and thus minimizes the potential for CCR fugitive dust generation while loading CCR into trucks and during transport.
	Reduce or halt operations during high wind events, as necessary.	Reducing or halting operations during high wind events minimizes the potential for CCR fugitive dust generation.

Table 2-2. Control Measures for Handling CCR

2.2.1 Conditioning of CCR Prior to Emplacement in CCR Landfill

Conditioned CCR is CCR that has been wetted with water or an appropriate chemical dust suppressant. Water or a chemical dust suppressant is added to raise the moisture content of the CCR to prevent wind dispersal but will not result in free liquids. Conditioning allows for the CCR to bind together, which minimizes the potential for CCR fugitive dust.

CCR generated offsite that is authorized for placement in the facility's landfill, is conditioned in a pug mill or otherwise conditioned prior to emplacement into the facility's landfill. Therefore, all CCR that is added to the facility's landfill is emplaced in the landfill as conditioned CCR.

2.3 Transportation of CCR

CCR is transported via truck at the facility using a combination of paved and unpaved facility roads. Table 2-3 below identifies CCR fugitive dust control measures that have been selected for use by the facility during transport of CCR. The facility will use the identified measures when transporting CCR to minimize CCR from becoming airborne at the facility.

CCR Activity	CCR Fugitive Dust Control Measure	Applicability and Appropriateness of Control Measure
	Condition CCR to be emplaced in the landfill before emplacement.	Conditioning CCR increases moisture content of the CCR and minimizes the potential for CCR fugitive dust generation during CCR transport (and emplacement in the landfill).
	Condition CCR materials to be transported offsite before they are loaded into trucks, as necessary.	Conditioning allows CCR to bind together and thus minimizes the potential for CCR fugitive dust generation while loading CCR into trucks and during transport.
	Cover or enclose trucks used to transport CCR fly ash.	Covering or enclosing trucks transporting CCR on facility roads minimizes the potential for CCR fugitive dust generation from the CCR transport trucks.
	Limit the speed of vehicles to no more than 15 mph on facility roads.	Limiting the speed of vehicles traveling on facility roads minimizes the potential for CCR fugitive dust generation from the CCR transport trucks.
Transportation of CCR at the facility	Cover or enclose trucks used to transport CCR other than fly ash, as necessary.	Covering or enclosing trucks transporting CCR on facility roads minimizes the potential for CCR fugitive dust generation from the CCR transport trucks.
	Sweep or rinse CCR off of the outside of the trucks transporting CCR, as necessary.	Removing CCR present on the outside of the truck minimizes the potential for movement of the truck or wind to cause CCR fugitive dust to become airborne.
	Remove CCR, as necessary, deposited on facility road surfaces during transport.	Removing CCR deposited on facility road surfaces as a result of transport minimizes the potential for CCR fugitive dust generation from vehicle traffic.
	Condition CCR haul roads, including landfill roads, with water or dust suppressant as necessary.	Watering CCR haul roads, as well as landfill roads, minimizes the potential for dust generation to occur as a result of CCR hauling traffic and heavy equipment use.
	Reduce or halt operations during high wind events, as necessary.	Reducing or halting operations during high wind events minimizes the potential for CCR fugitive dust generation.

Table 2-3. Control Measures for Transportation of CCR

3 Procedures for Periodic Assessment of Effectiveness of the Plan

The facility conducts inspections associated with CCR fugitive dust control. The facility also uses the procedures identified in section 5 of this plan to log every citizen complaint involving CCR fugitive dust events at the facility. These inspections and the investigations of citizen complaints will be used to periodically assess the effectiveness of the CCR fugitive dust control plan per 40 C.F.R. § 257.80(b)(4) and 35 I.A.C. 845.500(b)(3).

The facility routinely performs inspections to verify the effectiveness of the CCR fugitive dust control measures used at the facility. Inspections are conducted during daylight working hours and include observing for the presence of CCR fugitive dust emissions from vehicles transporting CCR on facility roads, CCR handling and CCR management activities, including CCR placement in CCR units. Inspection records include information such as the name of the person conducting the inspection, the date and time of the inspection, the results of the inspection, and any corrective action taken.

When a CCR fugitive dust event is observed or a citizen complaint involving a CCR fugitive dust event at the facility is received, current CCR management practices will be reviewed to see that the selected control measures are being properly implemented. If the control measures are not being properly implemented, relevant operating personnel will be notified and, as warranted, retrained in the proper implementation of CCR fugitive dust control measures. If appropriate, use of revised and/or additional control measures will be evaluated. As warranted, revised and/or additional control measures found to be applicable and appropriate to control CCR fugitive dust emissions will be incorporated into an amended CCR fugitive dust control plan.

The plan also will be reassessed in the event of material changes in site conditions potentially resulting in CCR fugitive dust becoming airborne at the facility.

Coffeen

4 Recordkeeping, Notification, Internet Site

The written CCR fugitive dust control plan, any amendment of the written plan, and the annual CCR fugitive dust control report required by 40 C.F.R. § 257.80(c) and 35 I.A.C. 845.500(c) will be placed in the facility's written operating record and posted to the company's CCR website in accordance with 40 C.F.R. § 257.105(g), § 257.107(g), and 845.800(d)(7), (14) and 845.810(e). Notification of the availability of the CCR fugitive dust control plan, any amendment of the plan, and the annual CCR fugitive dust control report will be provided to IEPA in accordance with 40 C.F.R. § 257.106(g). Any amendment of the fugitive dust control plan will be submitted to IEPA in accordance with 845.500(b)(5).

Additionally, pursuant to 845.500(b)(6), this fugitive dust control plan is being placed in facility's operating record and posted to the company's CCR website prior to the submission of any permits for the Coffeen Power Plant.

5 Procedures to Log Citizen Complaints

In the event the owner or operator of the facility receives a citizen complaint involving a CCR fugitive dust event at the facility, relevant information about the complaint will be logged. Information that will be recorded includes, as applicable:

- Date/Time the complaint is received
- Date/Time and duration of the CCR fugitive dust event
- Description of the nature of the CCR fugitive dust event
- Name of the citizen entering the complaint
- Address & phone number of citizen entering the complaint
- Name of the personnel who took the complaint
- All actions taken to assess and resolve the complaint

All citizen complaints involving CCR fugitive dust events at the facility will be investigated promptly. As deemed appropriate or necessary, corrective measures will be taken and a follow-up response will be provided to the complainant.

Pursuant to 35 I.A.C. 845.500(b)(2), quarterly reports will be submitted to IEPA no later than 14 days from the end of the quarter for all complaints received in that quarter. At a minimum, the quarterly report will include the date of the complaint, the date of the incident, the name and contact information of the complainant (if given), and all actions taken to assess and resolve the complaint.

6 Amendments

The written CCR fugitive dust control plan may be amended at any time provided the revised plan is placed in the facility's operating record as required by 40 C.F.R. § 257.105(g)(1) and 845.500(b)(6). Any amendment of the fugitive dust control plan will be submitted to IEPA in accordance with 845.500(b)(5). The written CCR fugitive dust control plan must be amended whenever there is a change in conditions that would substantially affect the written plan in effect.

Amendment Number and Date	Pages or Section	Description of Amendment	Professional Engineer Certifying Plan
Version 0 October 2015		Initial Plan	Wendy M. Pennington
Amendment 1 October 2021	Various	Administrative changes and adjustments to site condition controls as appropriate.	John R. Hesemann

Table 6-1. CCR Fugitive Dust Control Plan Amendments

ATTACHMENT H

Intended for

Illinois Power Generating Company

Date

October 25, 2021

Project No.

1940100806-002

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT

GMF GYPSUM STACK POND COFFEEN POWER PLANT COFFEEN, ILLINOIS



HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT GMF GYPSUM STACK POND

Project Name Coffeen Power Plant GMF Gypsum Stack Pond

Project No. **1940100806-002**

Recipient Illinois Power Generating Company

Document Type Hydrogeologic Site Characterization Report

Revision FINAL

Date October 25, 2021

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•	
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ACRONYMS AND ABBREVIATIONS

°F degrees Fahrenheit

§ Section % percent

35 I.A.C. Title 35 of the Illinois Administrative Code 40 C.F.R. Title 40 of the Code of Federal Regulations

AP1 Ash Pond No. 1
AP2 Ash Pond No. 2
bgs below ground surface
CCR coal combustion residuals
CCR Rule 40 C.F.R. § 257 Subpart D
cm/s centimeters per second
CPP Coffeen Power Plant

DA deep aquifer

DCU deep confining unit

ESRI Environmental Systems Research Institute, Inc.

FEMA Federal Emergency Management Agency

ft/day feet per day ft/ft feet per foot

q horizontal acceleration

GMF GSP Gypsum Management Facility Gypsum Stack Pond

GMF RP Gypsum Management Facility Recycle Pond

GMP Groundwater Monitoring Plan
GMZ Groundwater Management Zone
GWPS Groundwater Protection Standard
Hanson Professional Services, Inc.

HCR Hydrogeologic Site Characterization Report

HDPE high-density polyethylene

HELP Hydrologic Evaluation of Landfill Performance

HUC Hydrologic Unit Code

ID identification

IDNR Illinois Department of Nature Resources
IEPA Illinois Environmental Protection Agency

ILWATERIllinois Water and Related WellsIPGCIllinois Power Generating CompanyISASIllinois State Archaeological SurveyISGSIllinois State Geological Survey

LCU lower confining unit

LF Landfill

mg/L milligrams per liter

mil millimeter

msl above mean sea level

NAVD88 North American Vertical Datum of 1988

NGVD29 National Geodetic Vertical Datum of 1929

NID National Inventory of Dams

NPDES National Pollutant Discharge Elimination System

No. number

NRT Natural Resource Technology, Inc.

NRT/OBG NRT, an OBG Company

Part 845 Standards for the Disposal of Coal Combustion Residuals in Surface

Impoundments: 35 I.A.C. § 845

pcf pounds per cubic foot

PMP potential migration pathways

Ramboll Ramboll Americas Engineering Solutions, Inc.

SI Surface Impoundments

Site combined area including GMF RP and GMF GSP

SSURGO Soil Survey Geographic Database

SU Standard Units

TDS total dissolved solids UCU upper confining unit

USEPA United States Environmental Protection Agency

USFWS United States Fish and Wildlife Service

USGS United States Geological Survey

EXECUTIVE SUMMARY

This Hydrogeologic Site Characterization Report (HCR) for the Gypsum Management Facility (GMF) Gypsum Stack Pond (GSP) expands upon the hydrogeology and groundwater quality data presented in previous hydrogeologic investigation reports prepared for the Coffeen Power Plant (CPP) (Natural Resource Technology [NRT], 2017; Hanson Professional Services, Inc. [Hanson], 2009; Hanson, 2016). This report has been assembled to satisfy the information and analysis requirements of Title 35 of the Illinois Administrative Code (35 I.A.C.) Section (§) 845.620 as summarized in **Table ES-1**. The conceptual site model includes hydrogeologic and groundwater quality data specific to the GMF GSP, which has been collected from 2015 to 2021. The GMF GSP is part of the CPP, which is two miles south of the city of Coffeen, Illinois and about eight miles southeast of the city of Hillsboro, Illinois.

The CPP operated as a coal-fired power plant from 1964 until November 2019 and has five coal combustion residuals (CCR) management units. The CCR unit that is the subject of this report is: GMF GSP, Vistra Identification [ID] Number [No.] 103, Illinois Environmental Protection Agency [IEPA] ID No. W1350150004-03, and National Inventory of Dams [NID] No. IL50579. The GMF GSP is a 77-acre, lined surface impoundment (SI) used to manage CCR waste streams at the CPP.

CPP is located between the two lobes of Coffeen Lake (**Figure 1-1**), which was formed in 1963 by damming the McDavid Branch of the East Fork of Shoal Creek. Coffeen Lake encompasses approximately 1,100 acres and was created to provide a source of cooling water for the CPP. Coffeen Lake borders the CPP to the west, east, and south, and agricultural land is located to the north.

Unlithified material present above the bedrock in the vicinity of the CPP was categorized into hydrostratigraphic units for this HCR. In addition to the CCR Unit, the hydrostratigraphic units occur in the following order (from surface downward) and include:

- **Upper Confining Unit (UCU):** Composed of the Roxana and Peoria Silts (Loess Unit) and the upper clayey portion of the Hagarstown Member which are classified as silts to clayey silts and gravelly clay below the surficial soil. Construction of the GMF GSP required the excavation and removal of this layer within the unit footprint and the UCU has been eroded east of the GMF GSP, near the Unnamed Tributary.
- **Uppermost Aquifer:** The uppermost aquifer is the Hagarstown Member which is classified as primarily sandy to gravelly silts and clays with thin beds of sands. Similar to the Loess Unit, the Hagarstown Member was excavated to facilitate construction of the GMF GSP and the Hagarstown Member is also absent in some locations near the Unnamed Tributary.
- Lower Confining Unit (LCU): Comprised of the Vandalia Member, Mulberry Grove Member, and Smithboro Member. These units include a sandy to silty till with thin, discontinuous sand lenses, a discontinuous and limited extent sandy silt which has infilled prior erosional features, and silty to clayey diamicton, respectively.
- **Deep Aquifer (DA):** Sand and sandy silt/clay units of the Yarmouth Soil, which include accretionary deposits of fine sediment and organic materials, typically less than five feet thick and discontinuous across the CPP.

• **Deep Confining Unit (DCU):** Comprised of the Banner Formation, generally consists of clays, silts, and sands. The Lierle Clay Member is the upper layer of the Banner Formation which was encountered at the CPP.

Bedrock of the Bond Formation which consists of limestone and calcareous clays and shale, was not encountered in the borings advanced at the CPP.

Flow of groundwater from central portions of the CPP to Coffeen Lake or the Unnamed Tributary through the uppermost aquifer are the primary pathways for contaminant migration. Groundwater elevations are primarily controlled by surface topography, geologic unit topography, and water levels within Coffeen Lake and the Unnamed Tributary. A groundwater divide trending north-south is observed running through the approximate center of the CPP. Phreatic surfaces or water elevations within the SIs are generally consistent and have not been observed to fluctuate with groundwater elevations, indicating limited hydraulic connection with the SIs.

35 I.A.C. § 845 parameters were monitored in the uppermost aquifer monitoring wells at the GMF GSP as part of the Title 40 of the Code of Federal Regulations (40 C.F.R.) § 257 and IEPA groundwater monitoring programs from 2015 to 2021. These data were supplemented with installation and sampling of additional wells installed in 2021. The results indicate the following parameters were detected at concentrations/measurements greater than (or less than for pH) the applicable 35 I.A.C. § 845.600 groundwater protection standards (GWPSs) and are considered potential exceedances:

- Arsenic in downgradient uppermost aquifer well G215.
- Beryllium in downgradient uppermost aquifer well G209.
- Cobalt in downgradient uppermost aguifer wells G209, G213, and G217.
- Lead in downgradient uppermost aquifer wells G209 and G213.
- pH (low) in downgradient uppermost aquifer well G206.
- Sulfate in downgradient uppermost aguifer well G215 and in downgradient DA well G206D.
- Thallium in downgradient uppermost aquifer well G209.
- Total dissolved solids (TDS) in downgradient DA well G206D.

Groundwater monitoring results were compared to the applicable 35 I.A.C. § 845.600 GWPSs to determine potential exceedances. Potential exceedances include results reported during the background groundwater monitoring or prior period that are greater than the GWPS. The results are considered potential exceedances because the results were compared directly to the standard and did not include an evaluation of background groundwater quality or utilize the statistical methodologies proposed in the groundwater monitoring plan (GMP) provided in the Operating Permit application. Exceedances will be determined following IEPA approval of the GMP.

TABLE ES-1. PART 845 REQUIREMENTS CHECKLIST

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT GMF GYPSUM STACK POND COFFEEN, ILLINOIS

Part 845 Reference	Individual Part 845 Components Reviewed for Completeness	Location of Information in HCR
845.620(b)	The hydrogeologic site characterization shall include but not be limited to the following:	
845.620(b)(1)	Geologic well logs/boring logs;	Table 3-1 Figure 3-1 Appendix C
845.620(b)(2)	Climatic aspects of the site, including seasonal and temporal fluctuations in groundwater flow;	Sections 3.2.4 & 3.3.1 Table 3-3 Figures 3-2 through 3-5
845.620(b)(3)	Identification of nearby surface water bodies and drinking water intakes;	Sections 3.3.2 & 5.2 Appendix B
845.620(b)(4)	Identification of nearby pumping wells and associated uses of the groundwater;	Section 5.1 Appendix B
845.620(b)(5)	Identification of nearby dedicated nature preserves;	Section 5.3 Appendix B
845.620(b)(6)	Geologic setting;	Sections 2.4 & 2.5 Figures 2-2 through 2-4
845.620(b)(7)	Structural characteristics;	Section 2.4.3 Figure 2-5
845.620(b)(8)	Geologic cross-sections;	Figure 2-7
845.620(b)(9)	Soil characteristics;	Section 2.3 Figure 2-2 Tables 2-1 & 2-4
845.620(b)(10)	Identification of confining layers;	Sections 3.2.1

TABLE ES-1. PART 845 REQUIREMENTS CHECKLIST

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT GMF GYPSUM STACK POND COFFEEN, ILLINOIS

Part 845 Reference	Individual Part 845 Components Reviewed for Completeness	Location of Information in HCR
845.620(b)(11)	Identification of potential migration pathways;	Section 3.2.1 & 3.2.3
845.620(b)(12)	Groundwater quality data;	Section 4.2 Table 4-1
845.620(b)(13)	Vertical and horizontal extent of the geologic layers to a minimum depth of 100 feet below land surface, including lithology and stratigraphy;	Section 2.8 Figures 2-7 & 2-8 Appendix C
845.620(b)(14)	A map displaying any known underground mines beneath a CCR surface impoundment;	Section 2.7 Appendix B
845.620(b)(15)	Chemical and physical properties of the geologic layers to a minimum depth of 100 feet below land surface;	Section 2.8 Tables 2-1, 2-2, & 2-4 Appendices D & F
845.620(b)(16)	Hydraulic characteristics of the geologic layers identified as migration pathways and geologic layers that limit migration, including:	Sections 3.2.4.1, 3.2.5, & 3.2.6 Tables 3-2 to 3-4 Appendices D & F
845.620(b)(16)(A)	water table depth;	Section 3.2.4 Figures 3-3 & 3-4 Appendix E
845.620(b)(16)(B)	hydraulic conductivities;	Sections 3.2.5 Tables 2-1 & 3-3 Appendices D & F
845.620(b)(16)(C)	effective and total porosities;	Section 2.5.1 Table 2-1

TABLE ES-1. PART 845 REQUIREMENTS CHECKLIST

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT GMF GYPSUM STACK POND COFFEEN, ILLINOIS

Part 845 Reference	Individual Part 845 Components Reviewed for Completeness	Location of Information in HCR
845.620(b)(16)(D)	direction and velocity of groundwater flow; and	Section 3.2.4 to 3.2.6 Tables 3-2 & 3-4 Figures 3-3 & 3-4
845.620(b)(16)(E)	map of the potentiometric surface;	Figures 3-3 & 3-4
845.620(b)(17)	Groundwater classification pursuant to 35 I.A.C. § 620; and	Section 3.2.7

[O: LDC 06/15/21, U: LDC 08/19/21; C: EJT 08/19/21; U:KLT 8/24/21, C: LDC 09/17/21]

Notes:

--= reference to main regulation 35 I.A.C. § 620 = Title 35 of the Illinois Administrative Code, Part 620

HCR = Hydrogeologic Site Characterization Report



1. INTRODUCTION

1.1 Overview

In accordance with requirements of the Standards for the Disposal of Coal Combustion Residuals in Surface Impoundments: 35 I.A.C. § 845 (Part 845) (IEPA, 2021), Ramboll Americas Engineering Solutions, Inc. (Ramboll) has prepared this HCR on behalf of CPP (**Figure 1-1**), operated by Illinois Power Generating Company (IPGC). This report will apply specifically to the CCR Unit referred to as the GMF GSP. However, information gathered to evaluate other CCR units on site regarding geology, hydrogeology, and groundwater quality is included, where appropriate. The GMF GSP is a lined impoundment with an underdrain system that covers an area of approximately 77 acres. This HCR includes Part 845 content requirements specific to 35 I.A.C. Part 845.620(b) (Hydrogeologic Site Characterization) for the GMF GSP at CPP.

1.2 Part 845 Description

Part 845 contains comprehensive rules for the design, construction, operation, corrective action, closure, and post closure care of SIs containing CCR. CCR is commonly referred to as coal ash, and CCR SIs are commonly referred to as coal ash ponds. This rule includes GWPSs applicable to each CCR SI at the waste boundary and requires each owner or operator to monitor groundwater. IEPA's rule includes a permitting program as well as all federal standards for CCR SIs promulgated by the United States Environmental Protection Agency (USEPA). In addition, the rule includes procedures for public participation, closure alternatives analyses, and closure prioritization. The rule also includes financial assurance requirements for CCR SIs.

1.3 Previous Investigations and Reports

Numerous hydrogeologic investigations have been performed concerning the CCR Units located at the CPP. The information presented in this HCR includes data collected in support of the monitoring well network established for development of the GMP and supplements comprehensive data collection and evaluations from prior hydrogeologic investigation reports (recent to oldest), including, but not limited to, the following:

- NRT, January 24, 2017. Hydrogeologic Site Characterization Report, Ash Pond 2, Coffeen Power Station, Coffeen, Illinois.
 - Summarizes the results of numerous hydrogeologic investigations that have been performed at the Site, including recent data collected to comply with 40 C.F.R. § 257 Subpart D (CCR Rule) as well as comprehensive data collection and evaluations from prior hydrogeologic investigation reports.
- NRT, January 24, 2017. Groundwater Management Zone Application, Coffeen Ash Pond No. 2, Coffeen Power Station, Coffeen, Illinois.
 - Establishes a three-dimensional Groundwater Management Zone (GMZ) containing groundwater being managed to mitigate a potential release of CCR constituents from Ash Pond No. 2 (AP2).
- NRT, January 24, 2017. Groundwater Monitoring Plan. Coffeen Power Station, Coffeen, Illinois.
 - The plan describes the groundwater monitoring and reporting to be completed in support of the Closure Plan for AP2.

• NRT, January 24, 2017. Hydrostatic Modeling Report. Coffeen Power Station, Coffeen, Illinois.

Utilized the Hydrologic Evaluation of Landfill Performance (HELP) model to predict percolation from AP2 and to evaluate AP2 hydrostatic conditions in response to the proposed cover system as described in the Revised 30% Closure Design Package.

• NRT, January 24, 2017. Groundwater Modeling Report. Coffeen Power Station, Coffeen, Illinois.

Included simulations of the site hydrology, the extent of CCR leachate impacts on groundwater and the effect of pond closure on groundwater quality.

• Hanson, April 16, 2016. Corrective Action Plan.

A plan to remediate groundwater exceedances around AP2 and other units. Proposed plan includes reduction in leachate within AP2, enhanced cover system on AP2, and a GMZ.

• AECOM, April 2016. Revised 30% Closure Design Package for Coffeen Power Station Ash Pond No. 2.

A 30% design package for closure of AP2 including the design basis and summary in addition to preliminary construction costs and schedule.

• Hanson, April 2016. Uppermost Aquifer Considerations.

A discussion of the construction of the gypsum pond and relation to the uppermost aquifer in the vicinity of the site.

• Hanson, 2015. G153 Assessment.

Evaluation of manganese, sulfate, and TDS concentrations that were identified at concentrations greater than Class I Groundwater Standards at well G153, which concluded that elevated concentrations were also found upgradient of the Storm Water Runoff Pond and intrawell standards should be utilized.

• Hanson, July 2011. Hydrogeologic Report.

Supports permit applications for the GMF GSP and GMF Recycle Pond (GMF RP).

In conjunction with this report, a GMP is being prepared for the GMF GSP.

1.4 Site Location and Background

The CPP is located in Montgomery County, in central Illinois, within Section 11 Township 7 North and Range 7 East. The CPP is approximately two miles south of the city of Coffeen and about eight miles southeast of the city of Hillsboro, IL (Figure 1-1). The GMF GSP is located between the two lobes of Coffeen Lake (identified as "Coffeen Lake" and "Unnamed Tributary" on Figures 1-1 and 1-2) to the west, east, and south, and is bordered by agricultural land to the north. The approximately 1,100-acre Coffeen Lake was built by damming the McDavid Branch of the East Fork of Shoal Creek in 1963 for use as an artificial cooling lake for the CPP. Historically, several coal mines were operated at depth in the vicinity of the CPP as well as a US Minerals processing facility located to the north. Figure 1-2 is a site map showing the location of Ash Pond No. 1 (AP1), AP2, GMF GSP (Part 845 regulated CCR Unit and subject of this HCR), GMF RP, and Landfill (LF). The GMF RP is located south and immediately adjacent to the GMF GSP; therefore, the geology and hydrogeology are similar and results from the 845 investigations from both units are included and discussed in this report, and in the HCR prepared for the GMF RP. The combined area including the GMF RP and GMF GSP will hereinafter be referred to as the Site and data from both units will be utilized in portions of Sections 2 and 3. Groundwater quality

data in **Section 4** includes only data from monitoring wells specific to the subject unit (GMF GSP).

1.5 Site History and CCR Units

The CPP was a coal-fired electrical generating plant that began operation in 1964. The plant initially burned bituminous coal from Illinois and CCR from the coal fired units was disposed of in AP1. AP2 was also utilized in the early 1970's and AP1 was reconstructed in 1978. Both of these units were used until the mid-1980's. Beginning in 2010, CCR material was placed in the LF and GMF Units.

GMF Gypsum Stack Pond (GMF GSP): The 77-acre GMF GSP received blowdown from the air emission scrubbers and was put into operation in 2010. Construction of the GMF GSP was in accordance with Water Pollution Control Permit 2008-EA-4661 and features a composite 60-millimeter (mil) high-density polyethylene (HDPE) liner with 3 feet of recompacted soil with a hydraulic conductivity of 1×10^{-7} centimeters per second (cm/s), with internal piping and drains to collect contact water. Construction of the unit required excavation to approximately 603 feet and installation of a groundwater underdrain system to eliminate inward pressure on the liner prior to placement of CCR. The GMF GSP underdrain was actively pumped during construction, but is not actively pumped. IPGC ceased receipt of waste to the GMF GSP prior to April 11, 2021.

GMF Recycle Pond (GMF RP): The 17-acre GMF RP received blowdown from the air emission scrubbers and was put into operation in 2010. Construction of the GMF RP was in accordance with Water Pollution Control Permit 2008-EA-4661 and features a composite 60-mil HDPE liner with 3 feet of recompacted soil with a hydraulic conductivity of 1×10^{-7} cm/s, with internal piping and drains to collect contact water. Construction of the unit required excavation to approximately 601 feet and installation of a groundwater underdrain system to eliminate inward pressure on the liner prior to placement of CCR. The GMF RP underdrain is a passive, gravity drained system. IPGC ceased receipt of waste to the GMF RP prior to April 11, 2021.

Ash Pond No. 1 (AP1): This SI (also known as the Bottom Ash/ Recycle Pond) is a reclaimed ash pond that was reconstructed utilizing the existing earthen berms with reinforcement, as provided by Water Pollution Control Permit 1978-EA-389, issued by IEPA on May 26, 1978. AP1 (existing unlined SI) covers an area of approximately 23 acres, has berms up to 41 feet above the surrounding land surface, and a volume of 300 acre-feet. It primarily received bottom ash and low volume wastes from floor drains in the main power block building. Several years ago, air heater wash and boiler chemical cleaning wastes were directed to AP1, but this practice was discontinued. The bottom ash was periodically removed for beneficial uses by a third-party contractor. Sluicing of waste to AP1 ceased prior to November 4, 2019.

Ash Pond No. 2 (AP2): AP2 is a closed (IEPA approved) SI with a surface area of approximately 60 acres and berms 47 feet higher than the surrounding land surface. AP2 was originally removed from service and capped in the mid 1980's. A clay and soil cap was placed on the surface of the pond with contouring and drainage provided to direct storm water to four engineered revetment down drain structures. Prior to capping, this pond was identified as Outfall 004 in the facility National Pollutant Discharge Elimination System (NPDES) operating permit, IL0000108. Additional closure activities include the construction of a geomembrane cover system that began in July 2019 and was completed on November 17, 2020. The construction was completed in accordance with the Closure and Post Closure Care Plan approved by IEPA on January 30, 2018.

Landfill (LF): Fly ash was managed in a permitted composite lined landfill constructed in 2010. The LF has an active groundwater underdrain system that is currently being pumped. Additionally, the ash landfill leachate collection system is restricted by rule to no more than one foot of leachate on the composite liner. An IEPA groundwater monitoring program is in effect for the GMF (under Bureau of Water) and Ash Landfill (under Bureau of Land).

The approximate dates of construction of each successive stage of the CCR Units at the CPP are summarized in **Table A** below (AECOM, 2016).

Table A. History of Construction

Date	Event
1964	Construction of AP1 (formerly identified as the Bottom Ash Recycle Pond)
1971	Construction of AP2
1978-1979	Installation of internal embankment and new recycle intake structure in AP1 and abandonment of existing outfall structure
1984-1985	Closure of AP2 by installing a clay cover
2000	Installation of a sheet pile wall to facilitate construction of drainage flume along the northeast corner of AP1
2006	Bottom ash system modified in AP1
2008-2010	Construction of the GMF GSP and the GMF RP
2009	Installation of well dewatering system in AP2
2015	Notice of intent to close AP2
2015	Closure plans for AP1, AP2, GMF GSP, GMF RP, and LF submitted to IEPA
2018	IEPA approved Closure and Post-Closure Care Plan for AP2
2020	Completion of closure of AP2 with geomembrane cover system

2. REGIONAL AND LOCAL GEOLOGY

2.1 Topography

The CPP and embankments surrounding the GMF GSP are located at an elevation of approximately 632 feet North American Vertical Datum of 1988 (NAVD88) with the surrounding areas having low topographic relief, generally at an elevation of around 625 to 630 feet NAVD88 (**Figure 2-1**). East and south of the GMF GSP, towards the Unnamed Tributary and Coffeen Lake, the elevation decreases to less than 590 feet NAVD88.

Topographic maps drawn prior to construction (1947) indicate the areas of the CPP were generally from 600 to 640 feet above mean sea level (msl), with the elevations near the GMF GSP ranging from 620 to 640 feet msl (**Appendix A**). No former major drainage features run through the current extent of the GMF GSP.

2.2 Regional Geomorphology

The CPP is located in the central portion of the Springfield Plain of the Till Plains section, the largest physiographic division in Illinois, covering approximately four-fifths of the state. It is characterized by level to undulatory till plains with a few morainic ridges in a late youthful stage of erosion. The Springfield Plain includes the level to gently undulating portion of the Illinoian drift-sheet in central and south-central Illinois (Leighton et al., 1948; Zuehls et al., 1984). Distinguishing features include flatness and shallow drainage features. Moraines in this region are low and broad. Drainage systems are well developed, and the valleys tend to be shallow, broadly alluviated, and terraced (Leighton et al., 1948). Streams in the western portion of the Springfield Plain primarily flow westward, ultimately into the Mississippi River, while streams in the eastern portion flow eastward ultimately into the Wabash River.

2.3 Soils

Surficial soils at the CPP and vicinity are shown on **Figure 2-2** and based on Montgomery County soil survey data available in the Soil Survey Geographic Database (SSURGO) by the United States Department of Agriculture's Natural Resources Conservation Service provided by Environmental Systems Research Institute, Inc.'s (ESRI's) web hosted layer. Former soils underlying the CPP, not including the Fill and CCR within the limits of the GMF GSP are identified as: Herrick Biddle-Piasa silt loams (0 to 2 percent slopes) on the northwest boundary of the unit, surrounding background monitoring wells G270 and G280; Cowden-Piasa silt loams (0 to 2 percent slopes) southwest of the unit; Cowden silt loam (0 to 2 percent slopes) through the center of the unit; Marine silt loam (2 to 5 percent slopes) and Fishook silt loam (2 to 5 percent slopes, eroded) to the north and south of the unit, and through the eastern portion of the unit; and Atlas silt loam (5 to 10 percent slopes, eroded) east of the unit. Marine silt loam (2 to 5 percent slopes) is also found immediately east of the Unnamed Tributary.

2.4 Regional Geology

2.4.1 Unlithified Deposits

Pleistocene deposits of unlithified glacial diamictons, lacustrine/alluvial deposits, and windblown loess overlie Pennsylvanian-age bedrock throughout central Illinois. The most extensive glacial deposits are those from the Illinoian Stage which cover much of the state and are present at the CPP. Windblown (aeolian) deposits, the Peoria and Roxana Silts, cover the glacial deposits over a

majority of the state. These units are fine-grained deposits blown from river valleys by prevailing winds (Hansel and Johnson, 1996).

Surficial deposits, as reported and mapped on a regional scale by the Illinois State Geological Survey (ISGS), are the Vandalia Member (**Figure 2-3**), although the Hagarstown Member has been identified in the vicinity of the CPP. The general sequence of unlithified Quaternary deposits, depicted on **Figure 2-4**, from ground surface down is:

- **Loess Unit**: The loess unit is comprised of the Peoria and Roxana Silts. The Peoria Silt is generally classified and described as light yellow-tan to gray, fine sandy silt. The Roxana Silt is predominately silt-sized material, but can be sandier in localized areas and the base of this unit is often colluvium of silt, and sand (Hansel and Johnson, 1996).
- Glasford Formation: Till members present in the surrounding area include the Hagarstown Member, the Vandalia Member, the Mulberry Grove Member, and the Smithboro Member (youngest to oldest). The Hagarstown Member is bounded at the top by the Sangamon Soil. The Vandalia Member is described as a sandy till with thin lenticular bodies of silt, sand, and gravel. It is calcareous, except where weathered, generally gray, and moderately compact. The member consists of gravelly till, poorly sorted gravel, well sorted gravel, and sand. The Mulberry Grove Member is intermittent at the CPP, and is described as a calcareous gray silt and fine sand containing some fossil mollusks. The Smithboro Member is described as a gray, compact, silty till. The Smithboro is bounded below by the Yarmouth Soil (Willman and Frey, 1970).
- **Banner Formation**: Composed primarily of glacial tills and intercalated outwash of sand, gravel, and silt. Members differentiated in western Illinois include the Yarmouth Soil and the Lierle Clay (Hanson, 2009).

2.4.2 Bedrock

Unlithified deposits at the CPP and surrounding areas, described in **Section 2.4.1**, are underlain by rocks belonging to the Pennsylvanian Bond Formation (Kolata, 2005). Detailed descriptions of the Pennsylvanian strata of Illinois were published by Willman et al. (1975). The Bond Formation includes all strata from the base of the Shoal Creek Limestone Member or the LaSalle Limestone Member to the top of the Millersville Limestone Member or the Livingstone Limestone Member. It is overlain by the Mattoon Formation and underlain by the Modesto Formation. It varies from less than 150 feet thick in eastern Illinois to over 300 feet thick in southeastern Illinois, averaging about 250 feet. The Bond Formation is characterized by a high percentage of limestone and calcareous clays and shales. The Bond and Modesto Formations of the McLeansboro Group also contain multiple thin (typically less than 2 feet) intermittent coal beds. The upper formation of the Kewanee Group is the Carbondale Formation which contains multiple coal beds, including the Herrin (No. 6) Coal, of varying thicknesses (up to 7 feet) (ISGS, 2020). It is bound by thick limestone members (up to 50 feet), the thickest and purest limestones in the Pennsylvanian System of Illinois. Gray shales constitute the greatest part of the formation, although thick channel sandstones are developed locally.

The elevation of the bedrock surface in the area ranges from 450 to 500 feet msl. The bedrock surface slopes gently towards the west into a minor bedrock valley that runs north-south (Herzog et al., 1994). Well logs indicate that the lithology of the uppermost bedrock is predominantly shale (Zeizel, 1959).

2.4.3 Structure

The major geologic structural features of Illinois are shown on **Figure 2-5**. There are no major structural features in Montgomery County. The nearest major structural feature to CPP is the Louden Anticline, which is north-south trending and located approximately 25 miles east of the CPP. Smaller-scale structural features within Montgomery County include the Hillsboro North and Hillsboro South Domes, which are located approximately 15 miles north of the CPP. The Crown Fault, which is a left-lateral fault, and the Girard Fault, which is a northeast dipping normal fault, are located approximately 31 miles northwest of the CPP.

Located south of the CPP in Bond County are the Ayers and Woburn Anticlines and the Greenville Dome. The Ayers Anticline is located approximately 10 miles south of the CPP and trends east-west. The Woburn Anticline is located approximately 10 miles southeast of the CPP and trends north-south. The Greenville Dome is located approximately 15 miles south of the CPP (Nelson, 1995). A review of the available data from the United States Geological Survey (USGS, 2010), ISGS, and other available structural information was completed by Haley & Aldrich, Inc., (2018) for the Location Restriction Demonstration to address the requirements of 40 C.F.R. § 257.62 (Fault Areas). The review found that the nearest known mapped fault is the Crown Fault referenced above, which is located approximately 31 miles northwest of the CPP, and the Centralia Fault zone, located approximately 35 miles southeast of the CPP. The timeframe of the most recent activity on these fault zones is unknown. There are no known active faults or fault damage zones that have had displacement in Holocene time reported or indicated within 200 feet of the GMF GSP (**Figure 2-5**).

2.4.4 Seismic Setting

The nearest areas of present-day fault related, seismic activity are the Northern Illinois Seismic Source Zone, the Wabash Valley Fault Zone near southwestern Indiana, and the New Madrid Fault Zone along the Ohio and Mississippi River Valleys in southeastern Illinois. No recent earthquake epicenters are located in Montgomery County. A magnitude 3.80 earthquake occurred approximately 15 miles south of CPP in Bond County in 1981 and a magnitude 3.60 earthquake occurred approximately 20 miles southeast of CPP in Fayette County in 1990.

35 I.A.C. § 845.330 requires that existing and new CCR SIs and lateral expansions of existing SIs must not be located in seismic impact areas, unless owners or operators demonstrate that the SI is designed to resist the maximum horizontal acceleration(g) in lithified earth material. This requirement is identical to that in 40 C.F.R. § 257.63. The definition of a seismic impact zone is "areas having a 2 percent or greater probability that the maximum expected horizontal acceleration, expressed as a percentage of the earth's gravitation pull, will exceed 0.10 g in 50 years." Although the GMF GSP is located within a seismic impact zone, it satisfies the demonstration requirements of 35 I.A.C. § 845.330. The AECOM report titled "CCR Certification Report: Initial Structural Stability Assessment, Initial Safety Factor Assessment, and Initial Inflow Design Flood Control System Plan for the GMF Pond at Coffeen Power Station", dated October 2016, includes engineering analysis, calculations, and findings that support the requirements of 40 C.F.R. § 257.63 (Haley & Aldrich, Inc., 2018), and, by extension, 35 I.A.C. § 845.330.

2.4.5 Mining Activities

Several coal mines, both strip and underground types, previously operated in Montgomery County, Illinois. A survey to identify historic mining activities was conducted for a 1,000-meter

radius around the Site. Based on records obtained from ISGS, three mines were identified within a 1,000-meter radius of the GMF GSP. A map showing the extent of historic mines is provided in **Appendix B**.

In the southeast portion of the CPP is the Hillsboro Mine (ISGS Mine No. 871), which was operated as a room and pillar panel mine. Operations began in 1964 under the Truax-Traer Coal Company. The mine was purchased by the Consolidation Coal Company in 1971 and production ceased in 1983. An approximately 5- to 7-foot-thick seam of Herrin Coal was mined at approximately 500 feet below ground surface (bgs) (ISGS, 2019). The mine showed indications of small-scale faulting, roof stability issues and floor heaving. The GMF GSP falls within the buffer zone of the Hillsboro Mine.

To the north/northwest is the Clover Leaf No. 4 Mine (ISGS Mine No. 442), which was operated as a room and pillar panel mine. Operations began in 1906 under the Clover Leaf Mining Company. Production discontinued in 1924 under Clover Leaf Coal Company ownership. An approximately 6- to 8-foot-thick seam of Herrin Coal was mined at approximately 510 feet bgs (ISGS, 2019). The GMF GSP directly overlies the southernmost portion of the Clover Leaf No. 4 Mine, and is entirely within the buffer zone of the mine.

To the northeast is the Clover Leaf No. 1 Mine (ISGS Mine No. 3001), which was operated as a room and pillar mine. Operations began in 1889 under the Coffeen Coal & Coke Company. The mine was purchased by the Clover Leaf Coal Company in 1901, and production ceased in 1908. An approximately 7- to 8-foot-thick seam of Herrin Coal was mined at approximately 534 feet bgs (ISGS, 2019). The GMF GSP does not overlie the Clover Leaf No. 1 Mine nor its corresponding buffer zone.

2.5 Site Geology

The Quaternary deposits in the vicinity of the CPP consist mainly of diamictons and interbedded outwash deposits that were deposited during Illinoian and Pre-Illinoian glaciations. The CPP geology summarized below is from a combination of the Hydrogeologic Monitoring Plan (NRT, an OBG Company [NRT/OBG], 2017) and a field investigation performed in 2021 to collect additional data for the discussion of vertical and horizontal lithology, stratigraphy, chemical properties, and physical properties of geologic layers to a minimum of 100 feet bgs as specified in 35 I.A.C. § 845.620(b). Field investigation locations are shown on **Figure 2-6** and cross-sections are included in **Figure 2-7**. Soil boring logs and well construction logs are provided in **Appendix C**. Samples for geotechnical analysis were collected from interpreted geologic units and composited to obtain a representative sample of the entire geologic unit prior to submittal (**Table 2-1**). The unconsolidated deposits and bedrock which occur at the CPP include the following units (beginning at the ground surface):

- **CCR:** CCR consisting of gypsum is present within the GMF GSP and GMF RP and non-CCR fill material consisting of silt, clay, and sand comprises the berms surrounding the GMF GSP and GMF RP.
- **Loess Unit**: Clays and silts, including undifferentiated Roxana Silt and Peoria Silt with thicknesses ranging from 1 to 16 feet, where present at the CPP.
- Hagarstown Member: The Hagarstown Member (consisting of gravelly clay till and sandy
 materials in contact with the Vandalia Member (also referred to as Hagarstown Beds) has
 been separated into two units for this discussion: the first unit, consisting of the gravelly clay

till and the second unit consisting of sandy material overlying the Vandalia Member. The Upper Hagarstown Member is up to 6 feet thick, while the sandy portions, where present, are generally less than 3 feet thick, although thicknesses up to 7 feet have been observed north of the LF.

- Vandalia Member: Sandy, silt, or clay till that is generally greater than 15 feet thick.
- **Mulberry Grove Member**: Gray silt and sandy silt/clay unit found between the Vandalia Till and the Smithboro Till. Generally thin and not laterally continuous across the CPP.
- Smithboro Member: Thick, gray compacted silty clay diamicton.
- **Yarmouth Soil**: Sand and sandy silt/clay, which include accretionary deposits of fine sediment and organic materials, typically less than 5 feet thick and not laterally continuous.
- **Lierle Clay Member:** Clay and silt with some sand which is the upper portion of the Banner Formation. No borings advanced on site penetrated the full thickness of the Lierle Clay.

2.5.1 Fill and CCR

Gypsum scrubber waste and other non-CCR wastes are present within the GMP GSP. The gypsum grab sample locations are shown on **Figure 2-6.** The elevation at the top of the fill layer estimated from the topographic surface (**Figure 2-1**) within the limits of the GMF GSP is from approximately 610 to 624 feet NAVD88.

Borings were not advanced during the 2021 investigation in the GMF GSP due to safety concerns, and two grab samples of gypsum were collected near the northwest corner of the GMF GSP. Based on a topographic survey conducted in 2021 and the base of the GMF GSP (**Figure 2-8**), gypsum thickness is estimated from approximately 4 feet at the southern extent of the pond up to a maximum depth of 17 feet in the northern extent of the pond. This is consistent with the gypsum thickness estimated at a maximum of 16 feet in 2016 (AECOM, 2016).

The geotechnical testing results are summarized in **Table 2-1** and the geotechnical laboratory report is included in **Appendix D**. Geotechnical testing results from the CCR material in the grab sample indicated the following:

- Moisture content is 25.3 percent.
- Dry density is 78.0 pounds per cubic feet (pcf).
- Particle size distribution is 0 percent gravel, 22 percent sand, 78 percent fines.
- The sample was not analyzed for specific gravity, so porosity was not calculated. Based on grain size of the sample the estimated porosity of the sample is 3 to 19 percent (Fetter, 2001).

Solids samples collected from the gypsum grab sample were also collected for chemical analysis. The results of solids samples collected from within the GMF GSP are summarized in **Table 2-2**. Additionally, the NE Riser (**Figure 3-1**), which collects contact water from process piping located in the northeast corner of the GMF GSP, was sampled as a representative source water location in 2021. The results of source water samples collected from the GMF GSP are summarized in **Table 2-3**.

2.5.2 Loess Unit

The Loess Unit is the uppermost unlithified unit identified at the CPP. This unit is comprised of the combined Roxana and Peoria Silt and extends from beneath the topsoil, derived from the loess, to the top of the Hagarstown Member. The loess has been classified as silt or clayey silt, with minor amounts of sand. The Loess Unit is generally considered unsaturated, and the uppermost aquifer is recharged by precipitation that percolates through this unit.

The top of the Loess Unit was typically encountered from approximately 606 to 628 feet NAVD88. Loess Unit thickness ranges from 0 feet (absent) to 16 feet. Construction of the LF, GMF GSP, and GMF RP required the excavation and removal of this layer within the unit footprints. The Loess Unit is typically thickest to the north, and is absent near historic drainage features to the south.

During the 2021 investigation of the GMF GSP, the Loess Unit was typically encountered from 2 to 5 feet bgs, at elevations of approximately 626 to 629 feet NAVD88, and was generally 8 to 14 feet thick, where present near the GMF GSP and GMF RP. The Loess Unit was absent in borings G283 and G285, located near the Unnamed Tributary.

The geotechnical testing results are summarized in **Table 2-1** and geotechnical laboratory report is included in **Appendix D**. Geotechnical testing results from the Loess Unit indicated the following:

- Average moisture content is 20.6 percent and ranges from 20.2 to 21.2 percent.
- Average calculated porosity is 35.2 percent and ranges from 35.1 to 35.3 percent.
- Average dry density is 104.9 pcf and ranges from 103.4 to 105.9 pcf.
- Average specific gravity is 2.58 and ranges from 2.56 to 2.60.
- Particle size distribution is 0 percent gravel, 20 percent sand, and 80 percent fines (46 to 50 percent silt, and 30 to 34 percent clay).

Soil samples collected from the Loess Unit were also submitted to an analytical laboratory for chemical analysis. The results of this chemical analysis are summarized in **Table 2-4**.

2.5.3 Hagarstown Member

The Hagarstown Member (also referred to as Hagarstown Beds) exhibits two units; the first unit, consisting of the gravelly clay till and the second consisting of sandy material overlying the Vandalia Member. The clay till portion had varying thicknesses ranging from approximately 2 to 6 feet as observed adjacent to, south, and west of the Pond. Based on the historic topographic map (**Appendix A**), the Hagarstown Member is not present in former drainage features.

During construction of the LF, GMF GSP, and GMF RP, the Loess Unit and the Hagarstown Member were excavated to facilitate construction and eliminate groundwater flow into excavations. The excavations were backfilled with structural fill and an underdrain system was installed to mitigate inward hydraulic pressure and potential liner uplift damage before the CCR units were filled. The LF underdrain system remains but is no longer actively pumped. The GMF GSP underdrain system has not been actively pumped since construction was completed. The GMF RP gravity underdrain remains in place.

Where present, the sandy portion of the Hagarstown Member is generally 2 to 4 feet thick. The composition of the sandy portion of the Hagarstown unit varies across the CPP and has been

classified as gravelly till, poorly sorted gravel, well sorted gravel, sand, and silty sand. The elevation of the top of the Hagarstown generally declines as the unit approaches Coffeen Lake or other topographic drainage features.

During the 2021 investigation, the sandy portion of the Hagarstown Member near the GMF GSP was generally encountered at 18 feet bgs, at an elevation of approximately 612 feet NAVD88, and was generally 2 to 4 feet thick, where present (**Figure 2-7**).

The geotechnical testing results are summarized in **Table 2-1** and the geotechnical laboratory report is included in **Appendix D**. Geotechnical testing results from the Hagarstown Member indicated the following:

- Average moisture content of samples collected near the GMF GSP is 13.4 percent and ranges from 10.1 to 16.6 percent.
- Average calculated porosity of samples collected near the GMF GSP is 28.6 percent and ranges from 26.5 to 30.6 percent.
- Average dry density of samples collected near the GMF GSP is 115.0 pcf and ranges from 112.6 to 117.4 pcf.
- Average specific gravity of samples collected near the GMF GSP is 2.58 and ranges from 2.56 to 2.60.
- Particle size distribution of samples collected near the GMF GSP is 0 percent gravel, 46 to 57 percent sand, and 43 to 54 percent fines (28 to 34 percent silt and 15 to 20 percent clay).

Soil samples collected from the Hagarstown Member were also submitted to an analytical laboratory for chemical analysis. The results of this chemical analysis are summarized in **Table 2-4**.

2.5.4 Vandalia Member

The Vandalia (Till) Member is a sandy/silty till with thin, discontinuous lenses of silt, sand, and gravel. The Vandalia Member was encountered in all borings advanced at the CPP. The Vandalia Member typically ranged in thickness from 11.7 feet in the northern portion of the CPP, to 31.0 feet between the GMF GSP and the GMF RP. Similar to the observed top elevation of the Hagarstown Member, the top of the Vandalia Member declines in elevation near Coffeen Lake and topographic drainage features. This unit is relatively thick throughout the CPP, with an average thickness of over 15 feet (Hanson, 2009).

During the 2021 investigation, the Vandalia Member was encountered from 1.5 to 34 feet bgs, at elevations of approximately 598 to 608 feet NAVD88, where present. The geotechnical testing results are summarized in **Table 2-1** and the geotechnical laboratory report is included in **Appendix D**. Geotechnical testing results from the Vandalia Member indicated the following:

- Average moisture content is 8.6 percent and ranges from 8.3 to 8.9 percent.
- Average calculated porosity is 18.6 percent and ranges from 17.7 to 19.4 percent.
- Average dry density is 132.1 pcf and ranges from 131.4 to 132.7 pcf.
- Average specific gravity is 2.60 and ranges from 2.56 to 2.64.

 Particle size distribution is 0 percent gravel, 46 to 47 percent sand, and 53 to 54 percent fines.

Soil samples collected from the Vandalia Member were also submitted to an analytical laboratory for chemical analysis. The results of this chemical analysis are summarized in **Table 2-4**.

2.5.5 Mulberry Grove Member

The Mulberry Grove (Silt) Member typically consists of a thin, lenticular unit of gray sandy silt (Willman et al., 1975). It represents the interval between the retreat of the glacier that deposited the Smithboro Member and the advance of the glacier that deposited the Vandalia Member. At the CPP, the Mulberry Grove Member is represented by pockets (generally less than 2 feet thick) of gray sandy silt. This unit was absent in many borings through the central portion of the Site from south to north. Where sampled, the Mulberry Grove Member ranged in thickness from 0.5 to 4.9 feet near the GMF GSP (Hanson, 2009). During the 2021 investigation, the Mulberry Grove Silt was not encountered in the borings near the GMF GSP. These silts appear to be deposited in depressions found in the surface of the underlying Smithboro Member.

2.5.6 Smithboro Member

The Smithboro (Till) Member is described as a gray, compact, silty, clayey diamicton. The Smithboro Member ranges in thickness from 6.7 to 21.2 feet northwest of the landfill.

The geotechnical testing results are summarized in **Table 2-1** and the geotechnical laboratory report is included in **Appendix D**. Geotechnical testing results from the Smithboro Member indicated the following:

- The moisture content is 14.2 percent and ranges from 13.4 to 14.9 percent.
- Porosity is 27.3 percent and ranges from 26.2 to 28.4 percent.
- Dry density is 118.6 pcf and ranges from 116.6 to 120.6 pcf.
- Specific gravity is 2.62 and ranges from 2.61 to 2.62.
- Particle size distribution is 0 percent gravel, 26 to 29 percent sand, and 72 to 74 percent fines.

Soil samples collected from the Smithboro Member were also submitted to an analytical laboratory for chemical analysis. The results of this chemical analysis are summarized in **Table 2-4**.

2.5.7 Yarmouth Soil

The Yarmouth Soil is described as the weathered zone on the Kansan drift, but in some places, it consists of accretionary deposits of fine sediment and organic material that accumulated in poorly drained areas on the surface of the Kansan deposits. Historical borings in the northern portion of the CPP which encountered the Yarmouth were summarized previously by Hanson (2009) as ranging in thickness from 0 feet (absent) to 5.1 feet.

During the 2021 investigation, the Yarmouth Soil was encountered from 46 to 55 feet bgs, at an elevation from approximately 565 to 577 feet NAVD88, and was 1 to 3 feet thick, where present. The measured thickness was consistent with previous investigations.

The geotechnical testing results are summarized in **Table 2-1** and the geotechnical laboratory report is included in **Appendix D**. Geotechnical testing results from the Yarmouth Soil indicated the following:

- Moisture content of the G206D sample is 14.9 percent.
- No samples were analyzed for dry density; therefore, average porosity was not calculated for the Yarmouth Soil. Based on material type encountered in the borings, the effective porosity is expected to range from 10 to 28 percent (Fetter, 2001).
- Specific gravity of the G206D sample is 2.75.
- Particle size distribution of the G206D sample is 0 percent gravel, 78 percent sand, 13 percent silt, and 9 percent clay.

Soil samples collected from the Yarmouth Soil were also submitted to an analytical laboratory for chemical analysis. The results of this chemical analysis are summarized in **Table 2-4**.

2.5.8 Lierle Clay Member/ Banner Formation

The Lierle Clay Member is the uppermost member of the Kansan Stage Banner Formation. It is described as an accretion gley with clay, silt, and some sand. It was encountered by Hanson (2009) in all but a few borings on site. During the 2021 investigation, borings G206D and SB289 encountered the Lierle Clay at 54 and 57 feet bgs, at approximately 571 feet NAVD88 in both borings. No borings penetrated the full thickness of the Banner Formation near the GMF GSP.

The geotechnical testing results from the sample collected at SB289 are summarized in **Table 2-1** and the geotechnical laboratory report is included in **Appendix D**. Geotechnical testing results from the Lierle Clay Member indicated the following:

- Moisture content of the sample collected from SB289 is 25 percent.
- Calculated porosity of the sample collected from SB289 is 40.0 percent.
- Dry density of the sample collected from SB289 is 98.4 pcf.
- Specific gravity of the sample collected from SB289 is 2.63.
- Particle size distribution of the sample collected from SB289 is 0 percent gravel, 21 percent sand, 36 percent silt, and 43 percent clay.

Soil samples collected from the Lierle Clay were also submitted to an analytical laboratory for chemical analysis. The results of this chemical analysis are summarized in **Table 2-4**.

2.5.9 Bedrock

Pennsylvanian-age Bond Formation bedrock was not encountered in any borings advanced at the CPP, so site-specific information is not available.

3. REGIONAL AND LOCAL HYDROGEOLOGY

3.1 Regional Hydrogeology

The water table conforms more or less to the topographic features of the land surface. Recharge occurs in the uplands and flows towards drainage features. Moderate thicknesses of unconsolidated materials fill shallow valleys or are present on the uplands bordering the main valleys. These materials contain thin and discontinuous deposits of sand and gravel. Potable water in Montgomery County is primarily serviced by the Hillsboro and Litchfield Water Departments. Surface water of Lake Glenn Shoals and Old Hillsboro Lake serves Hillsboro, Illinois and the surrounding communities (*e.g.*, Coffeen) (Hillsboro, 2021). Groundwater for domestic and farm supplies is obtained locally in this area from wells drilled in sand and gravel, but in some places good water-yielding deposits are absent and water from the unconsolidated material is obtainable only with large-diameter dug wells (Selkregg et al., 1957).

3.2 Site Hydrogeology

Over 100 monitoring wells have been installed since 2006 to monitor groundwater conditions around the five CCR units at the CPP for both State and Federal programs. Three monitoring wells were installed in 2008 and fourteen monitoring wells (one replacement) were installed in 2010 around the GMF GSP. One replacement well was installed near the GMF GSP in 2017 to meet requirements of the CCR Rule. In 2021, one additional well and one soil boring were installed to provide information to meet requirements of Part 845. A summary of the current monitoring well networks, and construction details, is included in **Table 3-1** and locations shown on **Figure 3-1**. This section discusses the recently (2021) collected information, focusing on the existing well network and monitoring wells installed after 2015 around the GMF GSP, as well as appropriate historical data from wells installed prior to 2015.

3.2.1 Hydrostratigraphic Units

Six hydrostratigraphic units have been identified at the CPP based on stratigraphic relationships and common hydrogeologic characteristics, and are summarized as follows:

- **CCR**: This unit is composed of CCR, consisting primarily of gypsum scrubber waste. This also includes earthen fill deposits of predominantly silt and clay materials from on-site excavations that were used to construct berms and roads surrounding the various impoundments across the CPP.
- **UCU**: Consists of the Loess Unit and the upper clayey portion of the Hagarstown Member which has generally lower vertical permeability and generally greater than 60 percent fines (**Table 2-1**). This Unit was encountered across most of the CPP, with the exception of the eastern edges of the SIs near the Unnamed Tributary where the unit was eroded following deposition or locations where it has been excavated for construction.
- **Uppermost Aquifer**: This unit consists primarily of sand and sandy silts and clays at the base of the Hagarstown Member and, in some locations, the uppermost weathered sandy clay portion of the Vandalia Member. This unit is absent in several locations due to weathering and in others due to excavation during construction of the CCR Unit. The hydraulic characteristics of the Hagarstown Member indicate the unit has a moderate hydraulic conductivity.

- **LCU**: This unit is composed of the sandy clay till of the Vandalia Member, the silt of the Mulberry Grove Formation, and the compacted clay till of the Smithboro Member. The unit underlies the uppermost aquifer and was encountered in all boring locations on the CPP. Results from laboratory tests completed for vertical hydraulic conductivity indicate the Vandalia Member has a very low vertical hydraulic conductivity.
- **DA**: This unit consists primarily of sandy silt and sands of the Yarmouth Soil, which are thin (less than 5 feet) and discontinuous across the CPP.
- **DCU**: This unit underlies the DA and is composed of the Banner Formation, of which the thick Lierle Clay is the first encountered unit. No boring penetrated the full thickness of this formation.

3.2.2 Uppermost Aquifer

The base of the Hagarstown Member is identified as the uppermost aquifer on Site. The sandy clay and sand of the uppermost aquifer is confined except where site excavations and ravines extend through the Loess Unit into the Hagarstown Member. The top of the uppermost aquifer was evaluated with respect to the location restrictions in 2018 (Haley & Aldrich, Inc., 2018) and provided in **Figure 3-2**. The top of the uppermost aquifer occurs at an elevation of 609 to 611 feet and was removed below the footprint of the GMF GSP (**Figure 2-7**). The base of the uppermost aquifer and the material on which the GMF GSP liner was placed is the top of the LCU which is comprised of the low permeability Vandalia Member, Mulberry Grove Member, and Smithboro Till.

3.2.3 Potential Migration Pathways

Potential migration pathways (PMPs) were interpreted using the lithologic composition and hydrogeologic properties (hydraulic conductivity and hydraulic position with respect to the unit) of the screened materials. In addition to the physical properties, the analytical results from the baseline groundwater monitoring performed in wells screened in the confining units and DA were used to identify PMPs. The uppermost aquifer is the first occurrence of groundwater and therefore the PMPs identified are in geologic units located below the uppermost aquifer. Monitoring well G206D is considered a DA PMP monitoring location and monitors the potential migration of impacts through the LCU.

3.2.4 Water Table Elevation and Groundwater Flow Direction

The NE Riser was utilized during the 2021 investigation to collect contact water samples from the GMF GSP. A transducer was installed near the NE Riser during the 2021 investigation to monitor pond water levels in the GMF GSP. The water elevations in the GMF GSP showed minimal variation, with elevations from approximately 625 to 627 feet NAVD88 (**Appendix E**).

No monitoring wells were installed in the UCU during 2021 investigation activities and no wells have historically been installed across solely the UCU because it is not present or is unsaturated in portions of the CPP.

During the 2021 Part 845 investigation, groundwater elevations in the uppermost aquifer ranged from approximately 591 to 625 feet NAVD88 across the CPP (**Appendix E**). Groundwater elevations were typically highest towards the northern extent of the CPP, near the GMF GSP and GMF RP, except monitoring well G307 south of AP1, which consistently had the highest groundwater elevation. Groundwater elevations were lowest near the Unnamed Tributary and

east of AP1 towards Coffeen Lake. Groundwater elevations in the vicinity of the GMF GSP were typically from 617 to 622 ft NAVD88 (**Figures 3-3 and 3-4**). No seasonal variation has been observed in the uppermost aquifer monitoring wells, and any seasonal responses may be muted by the proximity and hydraulic connection to Coffeen Lake.

Overall groundwater flow within the uppermost aquifer is divided towards the two lobes of Coffeen Lake. The groundwater divide runs approximately through the center of the CPP, with groundwater east of the divide flowing east to southeast towards the Unnamed Tributary or the eastern lobe and groundwater west of the divide flowing west to southwest towards the western lobe. Groundwater flows east to southeast across the GMF GSP (**Figures 3-3 and 3-4** and **Appendix E**) toward the Unnamed Tributary. Based on the elevations of the Tributary (**Figure 2-7**) and groundwater elevations measured east of the tributary (**Figures 3-3 and 3-4**) the tributary is a hydraulic barrier and prevents groundwater migration east of the Unnamed Tributary. Although elevations vary seasonally, the groundwater flow direction in the uppermost aquifer is consistent and likely controlled by the proximity and hydraulic connection to Coffeen Lake.

Monitoring wells G206D, G275D, and G314D are screened across the DA. Groundwater elevation within the DA typically ranges from approximately 567 to590 feet NAVD88. G206D is nearest the GMF GSP and typically has groundwater elevations ranging from about 584 to 591 feet NAVD88. Groundwater contour maps are not generated for the DA; however, groundwater flow within the DA is expected to generally follow subsurface topography for the unit.

3.2.4.1 Vertical Hydraulic Gradient

Vertical hydraulic gradients were calculated using available groundwater elevation data from March through July 2021 and from historic readings from 2017 to 2019 at nested well locations within the uppermost aquifer, LCU, and DA. Vertical hydraulic gradients for the GMF GSP are presented in **Table 3-2**. Vertical hydraulic gradients for other nested well locations at the CPP, discussed below, are presented in **Appendix E**. The results of the vertical hydraulic gradient calculations between hydrostratigraphic units are summarized below:

- Uppermost aquifer to upper LCU (Vandalia Member)
 - In 2021, vertical gradients in well nest G405/T408, located north of AP2, were consistently downward, with an average vertical gradient of 0.03 feet per foot (ft/ft). From 2017 to 2020, vertical gradients in well nest G405/T408 varied between upward and downward, with an average (downward) vertical gradient of 0.04 ft/ft.
 - In 2021, vertical gradients in well nest G406/T409, located near the southwest corner of AP2, were consistently upward, with an average vertical gradient of -0.18 ft/ft. From 2017 to 2020, vertical gradients in well nest G406/T409 varied between upward and downward, with an average (downward) vertical gradient of 0.02 ft/ft.
 - Vertical hydraulic gradients indicate there is intermittent migration of groundwater from the uppermost aquifer downward into the LCU in the vicinity of well nests G405/T408 and G406/T409.

- · Uppermost aquifer to DA
 - During 2021, vertical gradients at well nest G206/G206D, located near the southwest corner of the GMF GSP, were consistently strongly downward, with an average vertical gradient of 1.01 ft/ft.
- Upper LCU (Vandalia Member) to Lower LCU (Smithboro Member)
 - In 2021, vertical gradients at well nest T408/G45D, located north of AP2, were consistently downward with an average vertical gradient of 0.20 ft/ft. The direction is consistent with measurements from 2017 to 2020 although less than the average downward gradient measured (2.02 ft/ft).
 - In 2021, vertical gradients at well nest T409/G46D, located near the southwest corner of AP2, were downward with an average vertical gradient of 0.25 ft/ft. This direction is consistent with measurements from 2017 to 2020, although less than the average downward vertical gradient measured (1.28 ft/ft).
- LCU to DA
 - During 2021, vertical gradients in well nest G314/G314D were consistently strongly downward, with an average vertical gradient of 1.32 ft/ft.

Vertical hydraulic gradients indicate there is consistently downward migration of groundwater in most areas of the CPP, with the exception being northwest of AP1, where consistent upward gradients were measured between the upper LCU and UA in 2021.

3.2.4.2 Impact of Existing Ponds and Ash Saturation

During construction of the GMF GSP, the Loess Unit and the Hagarstown Member were excavated within the footprint of the GMF GSP. Groundwater surface does not appear to be affected by water levels in the GMF GSP, which is hydraulically isolated by a composite HDPE liner with 3 feet of recompacted soil with a hydraulic conductivity of 1×10^{-7} cm/s. Changes in pond elevations in 2021 are minimal, and do not result in, or vary with, corresponding changes in groundwater elevations. Saturated gypsum has been observed within the GMF GSP. As discussed above, the water within the GMF GSP is hydraulically isolated from surrounding groundwater; therefore, the thickness of saturated gypsum within the pond will vary with the level of water maintained in the pond.

3.2.4.3 Impact of Coffeen Lake on Groundwater Flow

Groundwater contour maps prepared from elevation data measured in monitoring wells indicate groundwater elevations can be variable but flow directions are generally consistent. Groundwater generally flows from the center of the CPP west towards Coffeen Lake, and east towards the Unnamed Tributary, the eastern lobe of Coffeen Lake, and the discharge flume, resulting in a groundwater divide (high) running through the middle of the CPP.

Construction of the LF, GMF GSP, and GMF RP required removal of the Hagarstown Member, in effect removing the aquifer beneath the footprint of these units (Hanson, 2016). It is uncertain whether these constructed units significantly limit lateral groundwater flow, either by creating no flow zones or by capturing groundwater via their dewatering (NRT, 2017).

3.2.5 Hydraulic Conductivities

3.2.5.1 Field Hydraulic Conductivities

Field hydraulic conductivity tests were performed by Hanson in 2021 as part of characterization efforts to complete Part 845 requirements. Individual field hydraulic conductivity test results conducted at the GMF GSP are summarized in **Table 3-3** and the field hydraulic conductivity data is included in **Appendix F**. The results of the tests are summarized as follows:

- Uppermost aquifer: Hydraulic conductivities near the GMF GSP ranged from 2.5×10^{-4} to 4.0×10^{-3} cm/s. Tests had a geometric mean of 1.4×10^{-3} cm/s. This is generally consistent, although higher than tests conducted prior to 2017 as part of CCR Rule characterization efforts that indicated hydraulic conductivities varied from 1.7×10^{-5} to 2.1×10^{-3} cm/s with a geometric mean of 2.9×10^{-4} cm/s (NRT, 2017).
- LCU: Hydraulic conductivities across the CPP ranged from 1.2 x 10⁻⁴ and 4.5 x 10⁻³ cm/s. Tests had a geometric mean of 7.2 x 10⁻⁴ cm/s (NRT, 2017). No monitoring wells near the GMF GSP are screened within the LCU. Monitoring wells with the highest hydraulic conductivities were located near the GMF RP and wells with the lowest hydraulic conductivities were located near AP1. Prior to 2017, field hydraulic conductivity tests completed in the LCU for monitoring well and temporary piezometers (G45D, G46D, T408, and T409) indicate horizontal conductivities from 4.0 x 10⁻⁸ and 3.4 x 10⁻⁵. The elevated hydraulic conductivity values (10⁻⁴ to 10⁻³ cm/s) in wells near the GMF GSP relative to other areas of the CPP are likely not representative of the primary LCU lithology, but instead reflect the isolated and discontinuous sandy lenses in which the wells are screened (NRT, 2017).
- DA: Geometric mean hydraulic conductivity at DA well G314D, near AP1, was 8.7×10^{-5} cm/s and was slightly lower than tests completed in the northern portion of the CPP in 2009 that resulted in hydraulic conductivity values ranging from 1.3×10^{-4} to 1.7×10^{-3} cm/s, with a geometric mean of 4.4×10^{-4} cm/s (NRT, 2017). Field hydraulic conductivity testing was not performed on DA monitoring well G206D, located near the GMF GSP.
- No monitoring wells are screened only within the DCU, and no field hydraulic conductivity tests were conducted for the DCU.

3.2.5.2 Laboratory Hydraulic Conductivities

Falling head permeability tests (ASTM D5084 Method F) were performed in the laboratory on nine samples collected primarily from CCR and confining units at the CPP during the 2021 investigations. Samples collected from locations near the GMF GSP are shown on **Figure 2-6**. The geotechnical laboratory report is provided in **Appendix D**. The results samples are summarized in **Table 2-1** and discussed below.

- CCR: One geotechnical sample of CCR (gypsum) was collected as a grab sample near the NE Riser and the vertical hydraulic conductivity is 8.9 x 10⁻⁴ cm/s.
- UCU:
 - The 2021 sitewide geometric mean of vertical hydraulic conductivities of three samples collected from the UCU is 2.5×10^{-8} cm/s, which is consistent with historically reported values. Vertical hydraulic conductivity of a sample collected from SB289 near the GMF GSP is 1.1×10^{-8} cm/s.

- Geotechnical tests conducted prior to 2017 indicated UCU vertical hydraulic conductivity values ranging from 1.3×10^{-8} to 5.0×10^{-7} cm/s, with a geometric mean of 1.0×10^{-7} cm/s (NRT, 2017).
- Uppermost Aquifer: One geotechnical sample of uppermost aquifer material was collected from G275D, near the GMF RP, with a vertical hydraulic conductivity of 1.6×10^{-4} cm/s. No uppermost aquifer samples collected near the GMF GSP were analyzed for vertical hydraulic conductivity.

• LCU:

- The 2021 sitewide geometric mean of vertical hydraulic conductivities of three samples collected from the LCU is 1.8×10^{-7} cm/s. Vertical hydraulic conductivities from 2021 are consistent with those observed historically. No LCU samples collected near the GMF GSP were analyzed for vertical hydraulic conductivity.
- Intermittently present within the LCU is the Mulberry Grove Member. Historic vertical hydraulic conductivities of the Mulberry Grove Member were measured as 1.6×10^{-6} and 1.9×10^{-6} cm/s.
- Historic laboratory tests reported LCU hydraulic conductivity values ranging from 6.8×10^{-9} to 4.5×10^{-6} cm/s, with a geometric mean of 3.0×10^{-8} cm/s (NRT, 2017).
- DA: No laboratory vertical hydraulic conductivity tests were completed during 2021 on DA materials.
- **DCU:** No laboratory vertical hydraulic conductivity tests were completed during 2021 on DCU materials. Historic vertical hydraulic conductivity tests were performed on samples collected north and west of the GMF GSP. Vertical hydraulic conductivities of 6.8 x 10⁻⁹ and 4.5 x 10⁻⁶ cm/s were reported (NRT, 2017).

3.2.6 Horizontal Groundwater Gradients and Flow Velocity

Horizontal gradient and flow velocities are calculated using the flow path from R104 to G215 (**Table 3-4**). Horizontal gradients are from 0.003 to 0.005 ft/ft, equating to a minimum flow velocity of 0.05 feet/day (ft/day) and a maximum flow velocity of 0.11 ft/day. Average calculated flow velocity across the GMF GSP is 0.08 ft/day; however, the flow velocity is not representative of actual groundwater flow conditions since the uppermost aquifer is not present beneath the entire footprint of the GMF GSP.

3.2.7 Groundwater Classification

Per 35 I.A.C. § 620.210, groundwater within the uppermost aquifer at the GMF GSP meets the definition of a Class I – Potable Resource Groundwater based on the following criteria:

- Groundwater in the uppermost aquifer is located 10 feet or more below the land surface and
- Within a geologic material which is capable of a hydraulic conductivity of 1 x 10⁻⁴ cm/s or greater using a slug test (**Table 3-3**).

3.3 Surface Water Hydrology

3.3.1 Climate

Average climatic data was obtained from the National Oceanic and Atmospheric Administration National Centers for Environmental Information Climate Data Online. The data was recorded from 2001 to May 2021 from Hillsboro, Illinois, which is located approximately eight miles northwest of CPP. The data includes monthly maximum and monthly minimum daily temperatures (degrees Fahrenheit [°F]) and average rainfall for each month calculated from daily values collected over the 20-year period. The data is summarized in **Table B** below.

Table B: Average Monthly Temperature Extremes and Precipitation for Hillsboro, Illinois

	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual
Maximum Temperature (°F)	38.4	42.8	54.8	67.2	77.2	86.1	88.4	86.0	82.3	69.5	54.2	44.9	66.3
Minimum Temperature (°F)	22.3	24.5	34.9	44.3	55.4	64.2	67.2	64.7	58.6	46.6	34.5	28.3	45.8
Precipitation (inches)	1.94	2.14	2.78	5.72	4.18	4.64	3.71	3.37	2.77	3.29	2.88	2.95	40.4

https://www.ncei.noaa.gov/orders/cdo/2651630.csv

3.3.2 Surface Waters

The primary surface water body in the area, Coffeen Lake, is comprised of two lobes (identified as "Coffeen Lake" and "Unnamed Tributary" on **Figure 1-2**). The main body of Coffeen Lake is immediately adjacent to CPP on the west and south and the Unnamed Tributary borders CPP to the east.

In 1963, a 75 foot-high earthen dam was built across the McDavid Branch of East Fork Shoal Creek, creating Coffeen Lake for use as an artificial cooling lake for CPP. Coffeen Lake covers approximately 1,100 acres. The lake is part of the Shoal Creek Watershed (Hydrologic Unit Code [HUC] 07140203), which encompasses approximately 916 square miles. The average depth of Coffeen Lake is approximately 19 feet and the maximum depth is approximately 59 feet (Illinois Department of Nature Resources [IDNR], 2014). The average elevation of Coffeen Lake is approximately 591 feet NAVD88.

A USGS stream gage (USGS 05593900) for East Fork Shoal Creek near Coffeen, Illinois (latitude 39.1347 degrees north, longitude 89.3525 degrees west) is located approximately 6.5 miles northeast (upstream) of CPP. The gage datum elevation is 574.76 feet National Geodetic Vertical Datum of 1929 (NGVD29) (574.39 feet NAVD88). Daily gage heights for the period of January 1, 2018 through March 30, 2021 are shown below in **Figure A** (USGS, 2021a). The gage height of approximately 2 feet, representing approximate baseflow, occurs at an elevation of about 576.39 feet NAVD88.

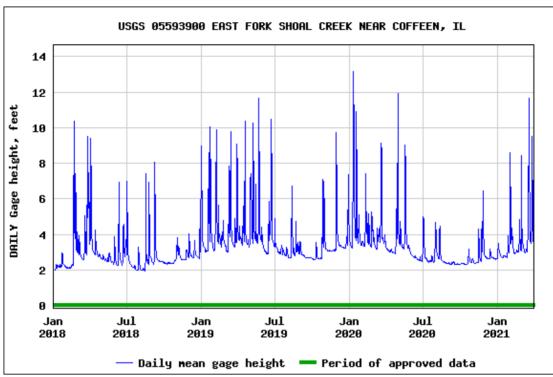


Figure A. Daily Gage Height of East Fork Shoal Creek Near Coffeen, Illinois (USGS 05593900)

Historically, Coffeen Lake received water discharge from both CPP and the Hillsboro Mine in additional to natural precipitation and drainage from East Fork Shoal Creek. At present, Coffeen Lake receives discharge from CPP under NPDES Permit No. IL 0000108. Additionally, an emergency spillway, located at the northeast corner of the GMF RP, discharges to the Unnamed Tributary, east of the CPP.

4. GROUNDWATER QUALITY

4.1 Summary of Groundwater Monitoring Activities

In accordance with the IEPA Water Pollution Control Permit No. 2020-EO-65043 the GMF GSP has been sampled since 2008. The monitoring program includes quarterly sampling and analysis of dissolved indicator parameters, and annual sampling for total concentrations. In 2015, additional well installation and groundwater sampling was initiated to meet the requirements of 40 C.F.R. § 257. Groundwater samples were collected and totals analyses were completed for Appendix III and Appendix IV parameters. In 2021, additional borings and wells were installed to comply with Part 845; wells were sampled for the parameters listed in 35 I.A.C. § 845.600. A review and summary of data from the annual IEPA groundwater sampling, 40 C.F.R. § 257, and Part 845 monitoring programs is included in the evaluation of groundwater quality at the Site.

4.1.1 40 C.F.R. § 257 Program Monitoring and Well Network

The 40 C.F.R. § 257 monitoring well network consists of seven monitoring wells screened in the uppermost aquifer, including two background monitoring wells (G200 and R201) and five compliance wells (G206, G209, G212, G215, and G218). The boring logs, well construction forms, and other related monitoring well forms for the 40 C.F.R. § 257 monitoring well network are available in the Operating Records as required by 40 C.F.R. § 257.91 for the CCR Unit, and included in **Appendix C** of this HCR. The well locations are shown on **Figure 3-1**.

40 C.F.R. § 257 monitoring well network groundwater samples are collected and analyzed for the laboratory parameters from Appendix III and Appendix IV of 40 C.F.R. § 257 as summarized in **Table C** below.

Table C. 40 C.F.R. § 257 Groundwater Monitoring Program Parameters

Field Parameters ¹			
Groundwater Elevation	рН		
Appendix III Paramete	ers (Total, except TDS)		
Boron	Chloride	Sulfate	
Calcium	TDS	Fluoride	
Appendix IV Paramete	rs (Total)		
Antimony	Beryllium	Cobalt	Molybdenum
Arsenic	Cadmium	Lead	Selenium
Barium	Chromium	Lithium	Thallium
Radium 226 and 228 com	nbined		

¹Dissolved oxygen, temperature, specific conductance, oxidation/reduction potential, and turbidity were recorded during sample collection.

4.1.2 IEPA Groundwater Monitoring

Routine quarterly groundwater monitoring is completed for a monitoring well network that includes wells for both the GMF GSP and GMF RP. The IEPA monitoring well network consists of thirty-one monitoring wells screened in the uppermost aquifer (G102, G103, R104, G105, G106, G200, G205, G206, G207, G208, G209, G210, G211, G212, G213, G214, G215, G216, G217, G218, G270, G271, G272, G273, G274, G275, G276, G277, G279, G280 and R201) in

accordance with IEPA Water Pollution Control Permit No. 2020-EO-65043. The boring logs, well construction forms for the IEPA monitoring well network are included in **Appendix C** of this HCR. The IEPA monitoring well network well locations are shown on **Figure 3-1**. Quarterly and annual samples are analyzed for the following field and laboratory parameters listed in **Table D** below.

Table D. IEPA Groundwater Monitoring Program Parameters

Field Parameters ¹			
pH		Elevation of Groundwater Surface	Specific Conductance
Depth to Water (beloground surface)	ow measuring point, below	Elevation of Measuring Point	Temperature
Metals (Dissolved))		
Antimony	Cadmium	Manganese	Thallium
Arsenic	Chromium	Mercury	Vanadium
Aluminum	Cobalt	Molybdenum	Zinc
Barium	Copper	Nickel	
Beryllium	Iron	Selenium	
Boron	Lead	Silver	
Inorganics (Disso	lved)		
Chloride	Fluoride	TDS	
Cyanide	Sulfate		
Other (Total)			
Phenols			

Note: Parameters are monitored as dissolved quarterly, and as dissolved and total annually.

4.1.3 Part 845 Well Installation and Groundwater Monitoring

In 2021, one additional monitoring well (G206D), one source sample collection point (NE Riser), and one soil boring (SB289) were installed around the GMF GSP to assess the vertical and horizontal lithology, stratigraphy, chemical properties, and physical properties of geologic layers to a minimum of 100 feet bgs as specified in 35 I.A.C. § 845.620(b). The boring logs, well construction forms, and other related monitoring well forms for the monitoring well network are included in **Appendix C** of this HCR.

Prospective Part 845 monitoring wells were sampled for eight rounds from February to August 2021 and the results were assessed for selection of the GMF GSP Part 845 monitoring well network presented in the GMP. Samples were collected from the new monitoring points and analyzed for 35 I.A.C. § 845.600 parameters summarized in **Table E** below. Part 845 groundwater monitoring results are discussed below in **Section 4.2**.

¹Dissolved oxygen, oxidation/reduction potential, and turbidity were recorded during sample collection.

Table E. Part 845 Groundwater Monitoring Program Parameters

Field Parameters ¹			
рН	Groundwater Elevation	Turbidity	
Metals (Total)			
Antimony	Boron	Cobalt	Molybdenum
Arsenic	Cadmium	Lead	Selenium
Barium	Calcium	Lithium	Thallium
Beryllium	Chromium	Mercury	
Inorganics (Total)			
Fluoride	Sulfate	Chloride	TDS
Other (Total)			
Radium 226 and 228 co	mbined		

¹ Dissolved oxygen, temperature, specific conductance, and oxidation/reduction potential were recorded during sample collection.

4.2 Groundwater Monitoring Results and Analysis

Groundwater data collected from the GMF GSP 40 C.F.R. § 257 network monitoring wells from 2015 to 2021 were supplemented with sampling of additional locations in 2021 and evaluated with respect to standards included in 35 I.A.C. § 845.600(a)(1). This data set was selected because it includes parameters (total metals) consistent with the parameter list in 35 I.A.C. § 845.600(a)(1). Based on this data set there were no concentrations of antimony, barium, boron, cadmium, calcium, chloride, fluoride, lithium, molybdenum, radium 226 and 228 combined, or selenium greater than the GWPSs. Results indicate that the parameters discussed in the following sections were detected at concentrations greater than the applicable 35 I.A.C. § 845.600(a)(1) standards and are considered potential exceedances^[1]. A summary of groundwater analytical data is provided in **Table 4-1**. Field parameters are included in **Table 4-2** and groundwater elevations are provided in **Appendix E**.

4.2.1 Arsenic

Arsenic has been detected intermittently at concentrations greater than the GWPS (0.010 milligrams per liter [mg/L]) at one downgradient uppermost aquifer well (G215). Arsenic was also detected above the GWPS in background uppermost aquifer well G200 during one event in October 2015. Arsenic concentrations in the downgradient uppermost aquifer wells listed above ranged from non-detect (at a reporting limit of 0.001) to 0.11 mg/L. Arsenic concentrations in the background uppermost aquifer well noted above ranged from non-detect to 0.038 mg/L mg/L.

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^[1] Potential exceedances include results reported during the eight rounds of baseline groundwater monitoring that are greater than the applicable 35 I.A.C. § 845.600(a)(1) standards. The results are considered potential exceedances because they were compared directly to the standard and did not include an evaluation of background groundwater quality or apply the statistical methodologies proposed in the Groundwater Monitoring Plan (GMP). For simplicity, "GWPS" will be used hereafter in discussing potential exceedances. Exceedances will be determined following IEPA approval of the GMP.

The DA monitoring well did not have concentrations greater than the arsenic GWPS.

4.2.2 Beryllium

Beryllium was detected at concentrations greater than the GWPS (0.004 mg/L) in downgradient uppermost aquifer well G209 during one event in October 2015. Beryllium was also detected above the GWPS in background uppermost aquifer well R201 during one event in February 2016. Beryllium concentrations in the downgradient uppermost aquifer wells listed above ranged from non-detect (at a reporting limit of 0.001) to 0.0042 mg/L. Beryllium concentrations in the background uppermost aquifer well noted above ranged from non-detect to 0.0067 mg/L.

The DA monitoring well did not have concentrations greater than the beryllium GWPS.

4.2.3 Cobalt

Cobalt was detected infrequently at concentrations greater than the GWPS (0.006 mg/L) in three downgradient uppermost aquifer wells (G209, G213, and G217). Cobalt has also been detected intermittently above the GWPS in background uppermost aquifer well G200. Cobalt concentrations in the downgradient uppermost aquifer wells listed above ranged from non-detect (at a reporting limit of 0.002) to 0.016 mg/L. Cobalt concentrations in the background uppermost aquifer well noted above ranged from non-detect to 0.053 mg/L.

The DA monitoring well did not have concentrations greater than the cobalt GWPS.

4.2.4 Lead

Lead was detected infrequently at concentrations greater than the GWPS (0.0075 mg/L) in two downgradient uppermost aquifer wells (G209 and G213). Lead has been detected in upgradient uppermost aquifer well G102. Lead has also been detected intermittently above the GWPS in background uppermost aquifer well G200. Lead concentrations in the downgradient uppermost aquifer wells listed above ranged from non-detect (at a reporting limit of 0.001) to 0.029 mg/L. Lead concentrations in the upgradient uppermost aquifer well noted above ranged from non-detect to 0.0097 mg/L. Lead concentrations in the background uppermost aquifer well noted above ranged from non-detect (at a reporting limit of 0.0005) to 0.082 mg/L.

The DA monitoring well did not have concentrations greater than the lead GWPS.

4.2.5 pH

During one event in June 2016, pH was detected at measurements less than the lower GWPS (6.5 Standard Units [SU]) at downgradient uppermost aquifer well G206. Measurements of pH have ranged from 6.2 to 7.5 SU at this well.

None of the uppermost aquifer wells had measurements greater than the upper pH GWPS.

The DA monitoring well did not have measurements less than the lower pH GWPS or higher than the upper pH GWPS.

4.2.6 Sulfate

Sulfate is a primary indicator parameter for CCR leachate impacts on groundwater quality. Sulfate has been detected infrequently at concentrations greater than the GWPS (400 mg/L) at

downgradient uppermost aquifer well G215. Sulfate concentrations at G215 ranged from 56 to 490 mg/L, with a median concentration of 120 mg/L.

Sulfate has been detected occasionally at concentrations greater than the GWPS at downgradient DA well G206D. Sulfate concentrations at G206D ranged from 250 to 600 mg/L, with a median concentration of 510 mg/L.

4.2.7 Thallium

Thallium was detected above the GWPS (0.002 mg/L) in downgradient uppermost aquifer well G209 during one event in October 2015. Thallium concentrations in the downgradient uppermost aquifer wells listed above ranged from non-detect (at a reporting limit of 0.001) to 0.0035 mg/L.

The DA monitoring well did not have concentrations greater than the thallium GWPS.

4.2.8 Total Dissolved Solids

During 2021 groundwater monitoring events, TDS was detected above the GWPS (1,200 mg/L) in downgradient DA well G206D during two events in March and April. Concentrations at G206D range from 1,200 to 1,300 mg/L, with a median concentration of 1,250 mg/L.

None of the uppermost aquifer wells had concentrations greater than the TDS GWPS.

5. EVALUATION OF POTENTIAL RECEPTORS

5.1 Water Well Survey

A water well survey was conducted for a 1,000-meter radius around the GMF GSP. Based on State of Illinois records obtained from the ISGS Illinois Water and Related Wells (ILWATER) Map¹ there are sixteen Illinois water wells located within 1,000 meters of the GMF GSP. These included eleven monitoring wells, four farm/domestic wells, and one industrial use well. A map of wells in the vicinity of the GMF GSP is presented in **Appendix B**.

5.2 Surface Water

A search was performed utilizing the United States Fish and Wildlife Service (USFWS) Wetlands Mapper² and the USGS National Map ³ for surface water bodies within 1,000 meters of the GMF GSP. The predominant surface water body in the region is Coffeen Lake and associated tributaries. Coffeen Lake consists of two lobes which are located approximately 1,300 feet west, 3,700 feet south, and 150 feet east and downgradient from the GMF GSP. A USGS stream gage (USGS 05593900) for the East Fork Shoal River near Coffeen, Illinois is located 6.5 miles north and east (upstream) of CPP.

Additional surface waters indicated in the USFWS Wetland Mapper and USGS National Map include several man-made freshwater ponds ranging from a roughly 0.6 acre freshwater forested/shrub wetland located northwest of the GMF GSP, several man-made freshwater ponds ranging from 0.1 to 0.8 acres, and one emergent wetland approximately 1.6 acres in size to the southeast of the GMF GSP. A map of wetlands and surface waters in the vicinity of the GMF GSP is presented in **Appendix B**.

The USGS National Map places CPP within the Shoal Creek watershed (HUC 07140203), which is part of the Middle Kaskaskia River Watershed (USGS, 2021b).

A Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map for Montgomery County, Illinois (Map No. 170992 0009 A, effective: January 9, 1981) is available in **Appendix G**. The GMF GSP does not occur within the special flood hazard zones identified on the 1981 FEMA map. The flood hazard areas shown on the map are defined as those areas subject to inundation by the 1 percent annual chance flood (*i.e.*, 100-year flood), also known as the base flood, that has a 1 percent chance of being equaled or exceeded in any given year.

5.3 Nature Preserves, Historic Sites, Endangered/Threatened Species

A comprehensive search of the IDNR Natural Heritage Database⁴ for natural areas and protected areas within 1,000 meters of the GMF GSP was performed. No natural or protected areas within the IDNR database were identified within 1,000 meters of the GMF GSP. A list of sites identified at the county level is found in **Appendix B**.

¹ ISGS ILWATER Map:

https://prairieresearch.maps.arcqis.com/apps/webappviewer/index.html?id=e06b64ae0c814ef3a4e43a191cb57f87

² USFWS Wetlands Mapper: https://www.fws.gov/wetlands/data/mapper.html

³ USGS National Map: https://apps.nationalmap.gov/viewer/

⁴ IDNR Natural Heritage Database:

https://www2.illinois.gov/dnr/conservation/NaturalHeritage/Pages/NaturalHeritageDatabase.aspx

The IDNR Natural Heritage Database Threatened and Endangered Species by County⁵ lists eleven threatened and endangered species as located within Montgomery County, including six endangered and five threatened species. Habitats for endangered or threatened species are identified at the county level only.

Additionally, a search of the IDNR Historic Preservation Division⁶ database for historic sites in the vicinity of the Site yielded six results at the county level located within Montgomery County. Four of these sites were identified from the Illinois Natural Areas Inventory and two were identified from the Illinois Nature Preserves list. None of these sites fall within 1,000 meters of the GMF GSP. The Illinois State Archaeological Survey (ISAS)⁷ databases that do not require credentials to access were also searched and yielded no results within 1,000 meters of the GMF GSP.

⁵ Illinois Threatened and Endangered Species by County: https://www2.illinois.gov/dnr/ESPB/Documents/ET by County.pdf

⁶ IDNR Historic Preservation Division: https://www2.illinois.gov/dnrhistoric/Pages/default.aspx

⁷ ISAS: https://www.isas.illinois.edu/

6. CONCLUSIONS

Based on extensive site investigation and monitoring, the GMF GSP has been characterized and a detailed site conceptual model has been developed. Results of these hydrogeologic studies were summarized and updated to include geologic, hydrogeologic, and groundwater quality data collected with a focus on the GMF GSP (Part 845 regulated CCR Unit and subject of this HCR).

Results of these hydrogeologic studies were reintroduced in this HCR and updated to include geologic, hydrogeologic, and groundwater quality data collected with a focus on the GMF (Part 845 regulated CCR Unit and subject of this HCR). The data were summarized and evaluated for changes in groundwater conditions since the previous investigations; available groundwater quality data for the GMF GSP collected from 2015 to present was compared to the Part 845 GWPS.

The results of the hydrogeologic and groundwater quality evaluation are:

- There are eight principal unlithified units above the bedrock in the vicinity of the GMF GSP, these include the following in descending order:
 - CCR: CCR consisting primarily of gypsum scrubber waste is present within the GMF GSP and non-CCR fill material consisting of silt, clay, and sand comprises the berms surrounding the GMF GSP.
 - Loess Unit: Clays and silts, including undifferentiated Roxana Silt and Peoria Silt with thicknesses ranging from 1 to 16 feet, where present. Construction of the GMF GSP required the excavation and removal of this layer within the unit footprint.
 - Hagarstown Member: The Hagarstown Member (consisting of gravelly clay till and sandy materials in contact with the Vandalia Member (also referred to as Hagarstown Beds) which has been subdivided into two units: the first unit consists of the gravelly clay till and the second unit consists of sandy material overlying the Vandalia Member. The Upper Hagarstown Member is up to 6 feet thick, while the sandy portions, where present, are generally less than 3 feet thick. During construction of the GMF GSP, the Hagarstown Member was excavated to facilitate construction and eliminate groundwater flow into excavations.
 - Vandalia Member: Sandy, silt till, or clay till that is generally greater than 15 feet thick.
 - Mulberry Grove Member: Gray silt and sandy silt/clay unit found between the Vandalia Member and the Smithboro Member. Generally thin and not laterally continuous across the Site.
 - **Smithboro Member**: Thick, gray compacted silty clay diamicton.
 - Yarmouth Soil: Sand and sandy silt/clay, which include accretionary deposits of fine sediment and organic materials, typically less than 5 feet thick and not laterally continuous.
 - Lierle Clay Member: Clay and silt with some sand which is the upper portion of the Banner Formation. No borings advanced on site penetrated the full thickness of the Lierle Clay.
 - Bedrock: Was not encountered in any deep borings advanced at the GMF GSP.

- Unlithified materials, described above, in the vicinity of the CPP were categorized into hydrostratigraphic units for this HCR. In addition to the CCR Unit, the hydrostratigraphic units occur in the following order (from surface downward) and include:
 - UCU: Composed of the Loess Unit and clayey portions of the Hagarstown Formation which are classified as silts to clayey silts and gravelly clay below the surficial soil. The UCU has been eroded east of the GMF GSP, near the Unnamed Tributary.
 - Uppermost Aquifer: The uppermost aquifer is the Hagarstown Member which is classified
 as primarily sandy to gravelly silts and clays with thin beds of sands. Similar to the Loess
 Unit, the Hagarstown is also absent in some locations near the Unnamed Tributary.
 - LCU: Comprised of the Vandalia Member, Mulberry Grove Member, and Smithboro Member. These units include a sandy to silty till with thin, discontinuous sand lenses, a discontinuous and limited extent sandy silt which has infilled prior erosional features, and silty to clayey diamicton, respectively.
 - DA: Sand and sandy silt/clay units of the Yarmouth Soil, which include accretionary deposits of fine sediment and organic materials, typically less than 5 feet thick and discontinuous across the Site.
 - DCU: Comprised of the Banner Formation, generally consists of clays, silts, and sands. The Lierle Clay Member is the upper layer of the Banner Formation which was encountered at the Site.
- The elevations of water within the GMF GSP are greater than the surrounding areas; however, the GMF GSP is lined and water elevation within the GMF GSP does not vary coincidentally with surrounding groundwater elevations.
- Groundwater flow within the uppermost aquifer is divided towards the two lobes of Coffeen
 Lake. The groundwater divide runs approximately through the center of the CPP property,
 with groundwater east of the divide flowing east to southeast towards the Unnamed Tributary
 or the eastern lobe and groundwater west of the divide flowing west to southwest towards the
 western lobe. Groundwater flows east to southeast across the GMF GSP.
- Vertical gradients measured near the site indicate downward flow from the uppermost aquifer
 to the LCU and DA. The DA has been identified as a PMP due to the presence of downward
 gradients and the relatively higher hydraulic conductivities measured in the DA.
- As determined by the detailed geologic information provided for the GMF GSP, and the hydrogeologic and groundwater quality data, groundwater within the uppermost aquifer at the GMF GSP is classified as Class I – Potable Resource Groundwater.
- Potential exceedances of 35 I.A.C. § 845.600 GWPSs were detected in monitoring wells downgradient of the GMF GSP in the various hydrostratigraphic units as follows:
 - Arsenic in downgradient uppermost aquifer well G215. Arsenic was also detected in background uppermost aquifer well G200.
 - Beryllium in downgradient uppermost aquifer well G209. Beryllium was also detected in background uppermost aquifer well R201.
 - Cobalt in downgradient uppermost aquifer wells G209, G213, and G217. Cobalt was also detected in background uppermost aquifer well G200.

- Lead in downgradient uppermost aquifer wells G209 and G213. Lead was also detected in upgradient uppermost aquifer well G102 and in background uppermost aquifer well G200.
- pH (lower limit) in downgradient uppermost aquifer well G206.
- Sulfate in downgradient uppermost aquifer well G215 and in downgradient DA well G206D.
- Thallium in downgradient uppermost aquifer well G209.
- TDS in downgradient DA well G206D.

Groundwater results are considered potential exceedances because they were compared directly to the standard and did not include an evaluation of background groundwater quality or apply the statistical methodologies proposed in the GMP.

This HCR satisfies Part 845 content requirements specific to 35 I.A.C. § 845.620(b) (Hydrogeologic Site Characterization) for the GMF GSP at the CPP.

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TABLES

TABLE 2-1. GEOTECHNICAL DATA SUMMARY

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT GMF GYPSUM STACK POND COFFEEN, ILLINOIS

Sample ID	Field Location ID	Top of Sample (ft bgs)	Bottom of Sample (ft bgs)	Moisture Content (%)	Dry Density (pcf)	Specific Gravity	Calculated Porosity ¹ (%)	Vertical Hydraulic Conductivity (cm/s)	LL	PL	ΡΙ	USCS	Gravel (%)	Sand (%)	Fines (%)
Loess Unit															,
G206D/Comp 1	G206D	4	16	20.5	105.3	2.60	35.1		42	18	24	CL	0	20	80
SB289/Comp 1	SB289	4	16	21.2	103.4	2.56	35.3		40	18	22	CL	0	20	80
SB289, ST5	SB289	8	10	20.2	105.9			1.1E-08				CL/CH			
Hagarstown Membe	er														
G206D/Comp 2	G206D	18.8	20	10.1	117.4	2.56	26.5		22	15	7	SC	0	57	43
SB289/Comp 2	SB289	18	22	16.6	112.6	2.60	30.6		23	14	9	CL	0	46	54
Vandalia Member															
G206D/Comp 3	G206D	20	31.8	8.3	131.4	2.56	17.7		21	13	8	CL	0	46	54
SB289/Comp 3	SB289	22	32.9	8.9	132.7	2.64	19.4		19	12	7	ML	0	47	53
Smithboro Member															
G206D/Comp 4	G206D	34	52	13.4	120.6	2.62	26.2		29	14	15	CL	0	29	72
SB289/Comp 4	SB289	34	52	14.9	116.6	2.61	28.4		31	15	16	CL	0	26	74
Yarmouth Soil															
G206D/Comp 5	G206D	54	58	14.9		2.75			18	12	6	SM	0	78	22
Lierle Clay													•		
SB289/Comp 5	SB289	54	60	25	98.4	2.63	40.0		48	18	30	CL	0	21	79
CCR							-								
Gypsum	NE Riser	grab	grab	25.3	78.0			8.9E-04	NP	NP	NP	Gypsum	0	22	78
Natara		<u> </u>	<u> </u>				-]	O:KLT, QC:		O, QC:KLT 8/9/2	1; U:KLT 8/1		

Notes:

¹ Porosity calculated as relationship of bulk density (p_b) to particle density (p_d) $(n = 100[1-(p_b/p_d)])$

-- = not analyzed

% = Percent

bgs = below ground surface

cm/s = centimeters per second

ft = foot/feet

GMF = Gypsum Management Facility

LL = Liquid limit

NP = Non Plastic

pcf = pounds per cubic foot

PI = Plasticity Index

PL = Plastic Limit

USCS = Unified Soil Classification System

CH = Fat Clay

CL = Lean Clay

ML = Silt

SC = Clayey Sand

SM = Silty Sand

TABLE 2-2. CCR ANALYTICAL RESULTS

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT GMF GYPSUM STACK POND COFFEEN, ILLINOIS

Sample Location	Sample Depth (ft BGS)	Sample Date	Antimony (mg/kg)	Arsenic (mg/kg)	Barium (mg/kg)	Beryllium (mg/kg)	Boron (mg/kg)	Cadmium (mg/kg)	Calcium (mg/kg)	Chloride (mg/kg)	Chromium (mg/kg)	Cobalt (mg/kg)	Fluoride (mg/kg)	Lead (mg/kg)	Lithium (mg/kg)	Mercury (mg/kg)	Molybdenum (mg/kg)	Selenium (mg/kg)	Sulfate (mg/kg)	Thallium (mg/kg)
GSP Gypsum 1	0-0	01/29/2021	<1.5	<0.51	6.6	<0.51	13	<0.51		25	<2	<1	13	0.67	<2.6	<0.1	1.2	<0.51	19000	<0.51
GSP Gypsum 2	0-0	03/09/2021	<3	<1	13	<1	<10	<1	130000	260	<4	<2	7.6	<1	<5		<1	<1	15000	<1

Notes:

< = concentration is less than the concentration shown, which corresponds to the reporting limit for the method.</p>

-- = data not available
BGS = below ground surface
CCR = coal combustion residuals

ft = feet

mg/kg = milligrams per kilogram

generated 10/05/2021, 2:11:53 PM CDT



TABLE 2-3. LEACHATE ANALYTICAL RESULTS

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT GMF GYPSUM STACK POND COFFEEN, ILLINOIS

Sample Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)
NE Riser	03/31/2021	<0.003	0.0021	0.046	<0.001	36	0.002	490	1300	<0.004	0.0048	31.9	<0.001	0.16	<0.0002	0.036	7.2	1.41	0.63	10000	0.0016
NE Riser	04/21/2021	<0.003	0.0023	0.041	<0.001	44	0.0022	530	1500	<0.004	0.0064	31.4	<0.001	0.16	<0.0002	0.04	7.3	0.0394	0.61	10000	0.002
NE Riser	05/05/2021	<0.003	0.0024	0.041	<0.001	45	0.0021	530	1600	<0.004	0.0062	31.6	<0.001	0.17	<0.0002	0.039	7.2	0.137	0.55	12000	0.0025
NE Riser	05/18/2021	<0.003	0.0027	0.041	<0.001	45	0.0022	550	1300	<0.004	0.0062	32.5	<0.001	0.18	<0.0002	0.04	7.2	0.609	0.56	12000	0.0019
NE Riser	06/14/2021																	0.412			
NE Riser	07/27/2021	<0.003	0.0022	0.042	< 0.001	41	0.0022	540	1400	<0.004	0.0064	1.66	<0.001	0.18	0.00024	0.041	7.1	0.363	0.52	11000	0.0023

Notes:

Field readings are reported with as many significant figures as provided by analytical laboratory.

-- = data not available

< = concentration is less than the concentration shown, which corresponds to the reporting limit for the method.</p>

mg/L = milligrams per liter

pCi/L = picocuries per liter

SU = standard units generated 10/05/2021, 2:12:06 PM CDT

RAMBOLL

TABLE 2-4. SOIL ANALYTICAL RESULTS

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT GMF GYPSUM STACK POND COFFEEN, ILLINOIS

Sample Location	Geologic Unit	Sample Depth (ft BGS)	Sample Date	Antimony (mg/kg)	Arsenic (mg/kg)	Barium (mg/kg)	Beryllium (mg/kg)	Boron (mg/kg)	Cadmium (mg/kg)	Chloride (mg/kg)	Chromium (mg/kg)	Cobalt (mg/kg)	Fluoride (mg/kg)	Lead (mg/kg)	Lithium (mg/kg)	Mercury (mg/kg)	Molybdenum (mg/kg)	Selenium (mg/kg)	Sulfate (mg/kg)	Thallium (mg/kg)
G206D	Fill/Loess Unit	4-16	01/25/2021	<3.6	9.1	65	<1.2	<12	<1.2	46	10	7.7	4.7	4.9	6.8	<0.24	<1.2	<1.2	300	<1.2
G206D	Hagarstown Member	18.8-20	01/25/2021	<3.6	3.1	110	<1.2	<12	<1.2	14	6.2	3	<3	4.8	<6.1	<0.24	<1.2	<1.2	26	<1.2
G206D	Vandalia Till Member	20-31.8	01/25/2021	<3.3	1.3	33	<1.1	<11	<1.1	<11	9.4	4	2.8	5.9	13	<0.22	<1.1	<1.1	<11	<1.1
G206D	Smithboro Till Member	34-52	01/25/2021	<3.6	3.8	89	<1.2	<12	<1.2	<12	11	5.3	3.6	8	11	<0.24	1.4	<1.2	<12	<1.2
G206D	Yarmouth Soil/Lierle Clay	54-58	01/25/2021	<3.6	1.9	160	<1.2	<12	<1.2	<12	11	6.4	7.6	12	8.4	<0.24	<1.2	<1.2	<12	<1.2
SB289	Loess Unit	4-16	01/26/2021	<2.8	<0.95	30	<0.95	<9.5	<0.95	14	4.9	<1.9	3.9	3.8	<4.7	<0.19	<0.95	<0.95	23	<0.95
SB289	Hagarstown Member	18-22	01/26/2021	<2.8	1.3	29	<0.95	<9.5	<0.95	17	6.7	6.8	0.47	5.4	6.8	<0.19	<0.95	<0.95	61	<0.95
SB289	Vandalia Member	22-32.9	01/27/2021	<3	1.8	12	<1	<10	<1	<10	4.8	2.5	<2.5	2.8	5.9	<0.2	<1	<1	16	<1
SB289	Smithboro Member	34-52	01/27/2021	<3	2.6	34	<1	<10	<1	<10	7.6	3.5	3.4	6.3	8.9	<0.2	<1	<1	<10	<1
SB289	Lierle Clay	54-60	01/27/2021	<3	2.8	110	<1	<10	<1	<10	11	4.7	5.7	13	7.1	<0.2	<1	<1	<10	<1

Notes:

< = concentration is less than the concentration shown, which corresponds to the reporting limit for the method.</p>
BGS = below ground surface
ft = foot or feet
mg/kg = milligrams per kilogram

generated 10/05/2021, 2:12:14 PM CDT



TABLE 3-1. MONITORING WELL LOCATIONS AND CONSTRUCTION DETAILSHYDROGEOLOGIC SITE CHARACTERIZATION REPORT

Well Number	HSU	Date Constructed	Top of PVC Elevation (ft)	Measuring Point Elevation (ft)	Measuring Point Description	Ground Elevation (ft)	Screen Top Depth (ft BGS)	Screen Bottom Depth (ft BGS)	Screen Top Elevation (ft)	Screen Bottom Elevation (ft)	Well Depth (ft BGS)	Bottom of Boring Elevation (ft)	Screen Length (ft)	Screen Diameter (inches)	Latitude (Decimal Degrees)	Longitude (Decimal Degrees)
G045D	LCU	08/17/2016	623.81	623.81	Top of PVC	620.94	31.88	41.52	589.06	579.42	41.92	578.90	9.6	2	39.064349	-89.396281
G046D	LCU	08/19/2017	625.24	625.24	Top of PVC	621.91	41.61	51.26	580.30	570.65	51.65	569.90	9.7	2	39.060305	-89.398524
G101	UA	02/02/2010		627.60	Top of Disk	625.27	15.68	20.32	609.59	604.95	20.89	603.40	4.6	2	39.071386	-89.400107
G102	UA	04/28/2006		629.04	Top of Disk	626.18	12.02	16.78	614.16	609.40	17.15	609.00	4.8	2	39.071387	-89.398991
G103	UA	02/15/2010		633.80	Top of Disk	627.94	15.88	20.67	612.06	607.27	21.09	606.90	4.8	2	39.070412	-89.399107
G104	UA	02/15/2010		632.94	Top of Disk	627.96	14.91	19.61	613.05	608.35	20.08	605.80	4.7	2	39.069451	-89.399104
G105	UA	02/16/2010		632.08	Top of Disk	626.86	16.11	20.90	610.75	605.96	21.37	604.40	4.8	2	39.068491	-89.3991
G106	UA	02/16/2010		631.15	Top of Disk	625.96	14.37	18.96	611.59	607.00	19.44	605.50	4.6	2	39.06753	-89.399097
G107	UA	02/17/2010	630.22	630.22	Top of Disk	628.20	13.87	18.50	614.33	609.70	19.00	607.50	4.6	2	39.067106	-89.399646
G108	UA	02/12/2010		630.22	Top of Disk	625.58	16.82	21.50	608.76	604.08	22.00	603.60	4.7	2	39.066984	-89.400035
G109	UA	02/11/2010		629.76	Top of Disk	624.79	15.39	19.93	609.40	604.86	20.50	604.30	4.5	2	39.067045	-89.400423
G110	UA	02/11/2010		629.65	Top of Disk	624.81	15.05	19.59	609.76	605.22	20.16	604.70	4.5	2	39.067172	-89.400704
G111	UA	02/11/2010		629.90	Top of Disk	625.28	14.61	19.15	610.67	606.13	19.72	605.60	4.5	2	39.067292	-89.40097
G119	UA	02/09/2010		631.55	Top of Disk	626.57	17.29	21.83	609.28	604.74	22.38	604.20	4.5	2	39.068986	-89.401213
G120	UA	02/08/2010		631.87	Top of Disk	627.21	15.10	19.62	612.11	607.59	20.21	605.10	4.5	2	39.069479	-89.401214
G121	UA	02/04/2010		632.83	Top of Disk	627.94	16.79	21.47	611.15	606.47	21.95	603.80	4.7	2	39.069781	-89.401216
G122	UA	02/04/2010		632.69	Top of Disk	628.05	16.51	21.05	611.54	607.00	21.66	606.20	4.5	2	39.070098	-89.401218
G123	UA	02/04/2010		632.96	Top of Disk	628.12	20.94	25.46	607.18	602.66	26.07	602.10	4.5	2	39.070399	-89.401219
G124	UA	02/03/2010		633.39	Top of Disk	628.70	15.98	20.51	612.72	608.19	21.06	606.70	4.5	2	39.070715	-89.40122
G125	UA	02/03/2010		633.51	Top of Disk	628.85	17.03	21.56	611.82	607.29	22.04	606.80	4.5	2	39.071003	-89.401221
G126	UA	02/10/2010		625.39	Top of Disk	622.96	12.89	17.43	610.07	605.53	18.00	605.00	4.5	2	39.067304	-89.401274
G151	UA	12/19/2011		625.93	Top of Disk	622.82	15.34	19.84	607.48	602.98	20.46	602.40	4.5	2	39.0672	-89.40159
G152	UA	12/20/2011		626.52	Top of Disk	623.06	13.59	18.09	609.47	604.97	18.57	604.50	4.5	2	39.066275	-89.401289



TABLE 3-1. MONITORING WELL LOCATIONS AND CONSTRUCTION DETAILSHYDROGEOLOGIC SITE CHARACTERIZATION REPORT

Well Number	HSU	Date Constructed	Top of PVC Elevation (ft)	Measuring Point Elevation (ft)	Measuring Point Description	Ground Elevation (ft)	Screen Top Depth (ft BGS)	Screen Bottom Depth (ft BGS)	Screen Top Elevation (ft)	Screen Bottom Elevation (ft)	Well Depth (ft BGS)	Bottom of Boring Elevation (ft)	Screen Length (ft)	Screen Diameter (inches)	Latitude (Decimal Degrees)	Longitude (Decimal Degrees)
G153	UA	12/15/2011	626.35	626.40	Top of Disk	623.23	15.90	20.34	607.33	602.89	20.80	602.50	4.4	2	39.065857	-89.402567
G154	UA	12/16/2011		626.35	Top of Disk	623.52	14.26	18.76	609.26	604.76	19.10	603.50	4.5	2	39.067089	-89.403574
G155	UA	12/19/2011		625.86	Top of Disk	622.89	15.09	19.58	607.80	603.31	23.23	599.70	4.5	2	39.067493	-89.402659
G200	UA	02/25/2008		625.94	Top of Disk	623.27	12.19	16.98	611.08	606.29	17.36	605.30	4.8	2	39.075139	-89.395009
G201	UA	02/25/2008	627.15	627.15	Top of Riser	624.19	13.01	17.80	611.18	606.39	18.15	606.00	4.8	2	39.075141	-89.397829
G205	UA	02/21/2008		624.34	Top of Disk	622.10	10.04	14.53	612.06	607.57	15.07	606.10	4.5	2	39.068596	-89.394147
G206	UA	10/14/2010		632.82	Top of Disk	630.53	17.51	21.92	613.02	608.61	22.42	606.50	4.4	2	39.067399	-89.398548
G206D	DA	01/25/2021	634.14	634.14	Top of PVC	631.41	49.20	59.00	582.21	572.41	59.39	571.41	9.8	2	39.067428	-89.398493
G207	UA	10/08/2010		633.21	Top of Disk	630.61	18.24	22.77	612.37	607.84	23.30	606.60	4.5	2	39.067568	-89.397952
G208	UA	10/07/2010		633.16	Top of Disk	630.57	17.53	22.06	613.04	608.51	22.60	606.60	4.5	2	39.067743	-89.397402
G209	UA	10/07/2010		632.91	Top of Disk	630.57	17.74	22.28	612.83	608.29	22.81	606.60	4.5	2	39.067923	-89.39685
G210	UA	10/06/2010		632.99	Top of Disk	630.48	19.39	23.93	611.09	606.55	24.46	605.50	4.5	2	39.068088	-89.396322
G211	UA	10/11/2010		632.64	Top of Disk	630.31	17.34	21.88	612.97	608.43	22.41	606.30	4.5	2	39.068263	-89.395792
G212	UA	10/11/2010		632.89	Top of Disk	630.59	16.74	21.29	613.85	609.30	21.81	606.60	4.6	2	39.06843	-89.395318
G213	UA	10/12/2010		632.81	Top of Disk	630.34	16.75	21.29	613.59	609.05	21.82	606.30	4.5	2	39.068585	-89.394822
G214	UA	10/14/2010		632.85	Top of Disk	630.39	17.75	22.14	612.64	608.25	22.65	606.40	4.4	2	39.068919	-89.393982
G215	UA	10/13/2010		633.06	Top of Disk	630.48	19.41	23.80	611.07	606.68	24.31	606.20	4.4	2	39.069309	-89.39394
G216	UA	10/13/2010		632.76	Top of Disk	630.28	20.04	24.42	610.24	605.86	24.93	604.30	4.4	2	39.069765	-89.393946
G217	UA	10/12/2010		633.10	Top of Disk	630.67	20.49	24.88	610.18	605.79	25.38	604.70	4.4	2	39.07034	-89.393959
G218	UA	10/12/2010		633.11	Top of Disk	630.64	20.33	24.77	610.31	605.87	25.27	604.60	4.4	2	39.070876	-89.393956
G270	UA	02/26/2008		625.86	Top of Disk	623.73	13.13	17.92	610.60	605.81	18.27	605.50	4.8	2	39.066564	-89.397403
G271	UA	09/10/2009		625.57	Top of Disk	622.89	9.96	14.31	612.93	608.58	14.79	606.90	4.4	2	39.065007	-89.395587
G272	UA	09/10/2009		623.81	Top of Disk	620.72	9.11	13.98	611.61	606.74	14.32	606.40	4.9	2	39.064989	-89.394785



TABLE 3-1. MONITORING WELL LOCATIONS AND CONSTRUCTION DETAILS HYDROGEOLOGIC SITE CHARACTERIZATION REPORT

Well Number	HSU	Date Constructed	Top of PVC Elevation (ft)	Measuring Point Elevation (ft)	Measuring Point Description	Ground Elevation (ft)	Screen Top Depth (ft BGS)	Screen Bottom Depth (ft BGS)	Screen Top Elevation (ft)	Screen Bottom Elevation (ft)	Well Depth (ft BGS)	Bottom of Boring Elevation (ft)	Screen Length (ft)	Screen Diameter (inches)	Latitude (Decimal Degrees)	Longitude (Decimal Degrees)
G273	UA	09/10/2009		623.02	Top of Disk	620.17	9.08	14.56	611.09	605.61	15.10	604.20	5.5	2	39.064985	-89.393973
G274	UA	09/16/2009		624.04	Top of Disk	621.67	12.90	17.67	608.77	604.00	18.06	603.60	4.8	2	39.064991	-89.393198
G275	UA	09/16/2009		618.26	Top of Disk	616.14	8.22	12.62	607.92	603.52	13.19	603.00	4.4	2	39.065151	-89.392561
G275D	DA	01/14/2021	620.31	620.31	Top of PVC	617.52	49.76	59.55	567.76	557.97	59.89	517.80	9.8	2	39.065121	-89.392595
G276	UA	09/16/2009		632.00	Top of Disk	629.14	22.41	27.22	606.73	601.92	27.65	601.10	4.8	2	39.065534	-89.392617
G277	UA	09/14/2009		623.08	Top of Disk	620.79	14.29	18.77	606.50	602.02	19.24	600.80	4.5	2	39.065927	-89.392572
G278	UA	09/11/2009	631.19	631.17	Top of Disk	628.85	18.93	23.70	609.92	605.15	24.06	604.80	4.8	2	39.066737	-89.393161
G279	UA	09/10/2009		632.04	Top of Disk	629.19	22.40	26.79	606.79	602.40	27.30	601.20	4.4	2	39.067156	-89.392998
G280	UA	02/26/2008	625.35	625.35	Top of Riser	623.11	12.79	17.63	610.32	605.48	17.98	605.10	4.8	2	39.067216	-89.394992
G281	UA	09/08/2015		626.36	Top of Disk	623.82	15.51	20.16	608.31	603.66	20.30	603.50	4.7	2	39.065405	-89.399322
G283	LCU	01/14/2021	610.75	610.75	Top of PVC	608.30	8.39	18.17	599.91	590.13	18.36	589.90	9.8	2	39.064645	-89.392119
G284	UA	02/03/2021	618.42	618.42	Top of PVC	615.33	8.08	12.85	607.25	602.48	13.23	601.30	4.8	2	39.065487	-89.390631
G285	LCU	01/25/2021	613.52	613.52	Top of PVC	610.54	13.68	23.45	596.86	587.09	23.83	584.50	9.8	2	39.066513	-89.391474
G286	UA	01/18/2021	613.13	613.13	Top of PVC	609.97	3.37	8.16	606.60	601.81	8.50	600.00	4.8	2	39.067277	-89.391883
G287	UA	01/20/2021	617.45	617.45	Top of PVC	614.34	5.43	10.25	608.91	604.09	10.59	602.50	4.8	2	39.068297	-89.392388
G288	UA	01/19/2021	620.07	620.07	Top of PVC	617.08	7.59	12.26	609.49	604.82	12.75	603.10	4.7	2	39.067834	-89.390082
G301	UA	09/04/2015		622.65	Top of Disk	620.88	11.31	15.96	608.96	604.31	16.21	604.10	4.7	2	39.05951	-89.395415
G302	UA	09/04/2015		620.04	Top of Disk	618.52	13.21	17.86	604.74	600.09	18.39	599.60	4.7	2	39.059544	-89.393192
G303	UA	08/26/2010		622.02	Top of Disk	619.33	10.00	20.00	609.07	599.07	20.40	598.70	10	2	39.057144	-89.391721
G304	UA	08/26/2010		626.72	Top of Disk	623.32	10.00	20.00	613.32	603.32	20.40	602.90	10	2	39.057205	-89.395663
G305	UA	05/03/2016	625.67	625.67	Top of PVC	623.23	13.44	18.27	609.10	604.27	18.50	604.10	4.8	2	39.056558	-89.396798
G306	UA	05/03/2016	625.91	625.91	Top of PVC	623.57	13.07	17.68	609.77	605.16	17.90	604.80	4.6	2	39.056494	-89.393556
G307	UA	07/27/2016	624.60	624.60	Top of PVC	624.73	12.96	17.80	609.12	604.28	18.22	603.90	4.8	2	39.057214	-89.395545



TABLE 3-1. MONITORING WELL LOCATIONS AND CONSTRUCTION DETAILS HYDROGEOLOGIC SITE CHARACTERIZATION REPORT

Well Number	HSU	Date Constructed	Top of PVC Elevation (ft)	Measuring Point Elevation (ft)	Measuring Point Description	Ground Elevation (ft)	Screen Top Depth (ft BGS)	Screen Bottom Depth (ft BGS)	Screen Top Elevation (ft)	Screen Bottom Elevation (ft)	Well Depth (ft BGS)	Bottom of Boring Elevation (ft)	Screen Length (ft)	Screen Diameter (inches)	Latitude (Decimal Degrees)	Longitude (Decimal Degrees)
G307D	LCU	01/19/2021	624.88	624.88	Top of PVC	622.51	48.98	58.75	573.53	563.76	59.60	562.50	9.8	2	39.05721	-89.39552
G308	UA	01/18/2021	624.59	624.59	Top of PVC	621.59	10.10	14.89	611.49	606.70	15.24	605.80	4.8	2	39.057379	-89.397134
G309	UA	01/21/2021	625.88	625.88	Top of PVC	622.77	12.97	17.75	609.80	605.02	18.10	604.70	4.8	2	39.058508	-89.397243
G310	UA	02/09/2021	622.87	622.87	Top of PVC	619.89	10.24	15.03	609.65	604.86	15.38	604.00	4.8	2	39.059532	-89.396907
G311	UA	01/13/2021	621.04	621.04	Top of PVC	618.32	9.27	14.04	609.05	604.28	14.40	603.90	4.8	2	39.059513	-89.394363
G311D	LCU	01/12/2021	621.24	621.24	Top of PVC	618.39	50.16	60.10	568.23	558.29	60.58	557.80	9.9	2	39.059513	-89.394312
G312	UA	01/15/2021	619.78	619.78	Top of PVC	616.92	9.79	14.58	607.13	602.34	14.93	601.70	4.8	2	39.059558	-89.391983
G313	UA	02/05/2021	614.30	614.30	Top of PVC	611.51	6.30	11.11	605.21	600.40	11.46	599.50	4.8	2	39.058773	-89.391124
G314	LCU	02/05/2021	613.88	613.88	Top of PVC	611.11	14.56	19.58	596.55	591.53	20.02	591.10	5	2	39.05782	-89.390964
G314D	DA	02/04/2021	613.70	613.70	Top of PVC	610.87	39.34	49.11	571.53	561.76	49.47	510.60	9.8	2	39.057852	-89.390958
G315	UA	01/14/2021	623.52	623.52	Top of PVC	620.94	9.69	14.48	611.25	606.46	14.85	605.00	4.8	2	39.057165	-89.393667
G316	LCU	02/26/2021	602.59	602.59	Top of PVC	599.64	10.02	14.82	589.62	584.82	15.16	583.90	4.8	2	39.057847	-89.389698
G317	UA	02/12/2021	641.93	641.93	Top of PVC	638.85	30.14	34.93	608.71	603.92	35.28	602.90	4.8	2	39.056727	-89.390148
G401	UA	09/14/2015		625.57	Top of Disk	623.03	14.36	18.79	608.67	604.24	19.29	603.70	4.4	2	39.060259	-89.395295
G402	UA	08/27/2010		613.37	Top of Disk	610.36	10.00	20.00	600.36	590.36	20.40	590.00	10	2	39.060207	-89.391712
G403	UA	09/11/2015		626.47	Top of Disk	623.81	13.11	17.78	610.70	606.03	18.15	605.70	4.7	2	39.063167	-89.398779
G404	UA	05/01/2007		615.67	Top of Disk	613.57	6.42	11.17	607.15	602.40	11.62	601.60	4.8	2	39.064329	-89.392493
G405	UA	05/01/2007		623.63	Top of Disk	621.40	9.01	13.76	612.39	607.64	14.21	607.20	4.8	2	39.064345	-89.396234
G406	UA	08/19/2016	625.36	625.36	Top of PVC	621.86	13.56	18.37	608.30	603.49	18.75	603.10	4.8	2	39.060309	-89.398508
G407	UA	08/16/2016	621.32	621.32	Top of PVC	618.35	13.78	18.61	604.57	599.74	19.04	598.40	4.8	2	39.061574	-89.402004
G410	UA	02/23/2018		619.79	Top of Disk	617.21	8.89	13.68	608.32	603.53	14.09	603.10	4.8	2	39.061572	-89.403763
G411	UA	02/22/2018		623.25	Top of Disk	620.49	11.21	16.07	609.28	604.42	16.47	604.00	4.9	2	39.063979	-89.404033
MW01D	DA	05/03/2006	609.02	609.02	Top of PVC	607.08	33.29	38.05	573.79	569.03	38.41	567.10	4.8	2	39.067068	-89.402747



TABLE 3-1. MONITORING WELL LOCATIONS AND CONSTRUCTION DETAILSHYDROGEOLOGIC SITE CHARACTERIZATION REPORT

Well Number	HSU	Date Constructed	Top of PVC Elevation (ft)	Measuring Point Elevation (ft)	Measuring Point Description	Ground Elevation (ft)	Screen Top Depth (ft BGS)	Screen Bottom Depth (ft BGS)	Screen Top Elevation (ft)	Screen Bottom Elevation (ft)	Well Depth (ft BGS)	Bottom of Boring Elevation (ft)	Screen Length (ft)	Screen Diameter (inches)	Latitude (Decimal Degrees)	Longitude (Decimal Degrees)
MW02S	UA	05/05/2006	627.12	627.12	Top of PVC	624.16	10.34	15.12	613.82	609.04	15.51	608.70	4.8	2	39.071017	-89.403648
MW02D	LCU	05/05/2006	626.99	626.99	Top of PVC	624.14	22.03	26.83	602.11	597.31	27.22	596.90	4.8	2	39.071031	-89.403649
MW03D	DA	04/27/2006	629.01	629.01	Top of PVC	625.86	52.29	57.06	573.57	568.80	57.40	567.90	4.8	2	39.071386	-89.398976
MW04S	UA	05/11/2006	625.89	625.89	Top of PVC	622.63	9.83	14.26	612.80	608.37	14.77	607.90	4.4	2	39.075356	-89.399232
MW05S	UA	05/17/2006	625.95	625.95	Top of PVC	622.65	12.66	17.41	609.99	605.24	17.71	604.90	4.8	2	39.075866	-89.40333
MW05D	DA	05/17/2006	625.91	625.91	Top of PVC	622.65	45.57	50.33	577.08	572.32	50.72	568.70	4.8	2	39.075863	-89.403313
MW06S	UA	05/04/2006	626.15	626.15	Top of PVC	623.37	11.04	15.62	612.33	607.75	16.08	607.30	4.6	2	39.078189	-89.403644
MW07S	UA	05/09/2006	627.60	627.60	Top of PVC	624.90	9.91	13.79	614.99	611.11	14.39	610.50	3.9	2	39.0786	-89.399383
MW08S	UA	05/10/2006	628.01	628.01	Top of PVC	625.09	11.51	16.00	613.58	609.09	16.60	608.00	4.5	2	39.080234	-89.399079
MW09S	UA	05/03/2006	627.62	627.62	Top of PVC	624.70	11.21	15.62	613.49	609.08	16.20	608.50	4.4	2	39.079954	-89.394899
MW09D	LCU	05/03/2006	627.61	627.61	Top of PVC	624.68	45.81	50.57	578.87	574.11	51.00	570.70	4.8	2	39.07994	-89.394899
MW10S	UA	05/02/2006	624.45	624.45	Top of PVC	621.43	11.28	15.76	610.15	605.67	16.30	605.10	4.5	2	39.07601	-89.394068
MW10D	LCU	05/01/2006	624.47	624.47	Top of PVC	621.33	41.74	46.57	579.59	574.76	47.02	572.60	4.8	2	39.075995	-89.39407
MW11S	UA	04/28/2006	625.27	625.27	Top of PVC	622.04	8.89	13.63	613.15	608.41	14.08	608.00	4.7	2	39.071888	-89.393913
MW11D	LCU	04/28/2006	625.52	625.52	Top of PVC	622.19	28.31	33.04	593.88	589.15	33.50	585.90	4.7	2	39.071888	-89.393894
MW12S	UA	05/10/2006	625.31	625.31	Top of PVC	622.24	10.61	15.18	611.63	607.06	15.61	606.60	4.6	2	39.068514	-89.394199
MW12D	DA	05/10/2006	625.21	625.21	Top of PVC	622.24	42.46	46.99	579.78	575.25	47.47	572.20	4.5	2	39.068501	-89.394199
MW13S	UA	05/09/2006	625.96	625.96	Top of PVC	622.80	11.43	16.23	611.37	606.57	16.62	606.20	4.8	2	39.066297	-89.40118
MW13D	DA	05/09/2006	625.86	625.86	Top of PVC	622.85	49.81	54.60	573.04	568.25	55.00	567.90	4.8	2	39.066293	-89.401163
MW14S	UA	05/02/2006	626.88	626.88	Top of PVC	624.62	12.26	17.02	612.36	607.60	17.38	607.20	4.8	2	39.069153	-89.400442
MW15S	UA	04/25/2006	626.66	626.66	Top of PVC	623.83	14.41	19.16	609.42	604.67	19.62	604.20	4.8	2	39.069772	-89.397088
MW15D	LCU	04/25/2006	626.44	626.44	Top of PVC	623.83	33.68	38.45	590.15	585.38	38.80	585.00	4.8	2	39.06977	-89.397073
MW16S	UA	04/25/2006	629.47	629.47	Top of PVC	626.32	14.59	19.41	611.73	606.91	19.76	606.40	4.8	2	39.073571	-89.397006



TABLE 3-1. MONITORING WELL LOCATIONS AND CONSTRUCTION DETAILSHYDROGEOLOGIC SITE CHARACTERIZATION REPORT

Well Number	HSU	Date Constructed	Top of PVC Elevation (ft)	Measuring Point Elevation (ft)	Measuring Point Description	Ground Elevation (ft)	Screen Top Depth (ft BGS)	Screen Bottom Depth (ft BGS)	Screen Top Elevation (ft)	Screen Bottom Elevation (ft)	Well Depth (ft BGS)	Bottom of Boring Elevation (ft)	Screen Length (ft)	Screen Diameter (inches)	Latitude (Decimal Degrees)	Longitude (Decimal Degrees)
MW16D	DA	04/25/2006	629.38	629.38	Top of PVC	626.37	45.90	50.34	580.47	576.03	50.78	575.40	4.4	2	39.073571	-89.397036
MW17S	UA	05/04/2006	630.56	630.56	Top of PVC	627.28	14.02	23.56	613.26	603.72	24.11	603.20	9.5	2	39.07715	-89.396978
MW17D	DA	05/04/2006	630.29	630.29	Top of PVC	627.47	48.82	53.32	578.65	574.15	53.87	573.60	4.5	2	39.077151	-89.396958
MW18S	UA	05/11/2006	628.66	628.66	Top of PVC	625.69	11.31	15.79	614.38	609.90	16.40	609.30	4.5	2	39.077033	-89.401698
MW20S	UA	05/01/2007	622.90	622.90	Top of PVC	620.26	8.41	13.22	611.85	607.04	13.67	604.30	4.8	2	39.064968	-89.394322
R104	UA	10/08/2010		632.84	Top of Disk	629.03	14.59	19.32	614.44	609.71	19.85	609.20	4.7	2	39.069474	-89.399109
R201	UA	10/08/2010		626.34	Top of Disk	624.02	14.59	19.32	609.43	604.70	19.85	604.20	4.7	2	39.075142	-89.397855
R205	UA	03/20/2017		624.52	Top of Disk	621.91	11.32	16.01	610.59	605.90	16.42	605.50	4.7	2	39.068593	-89.394164
T127	UA	02/10/2010		630.96	Top of Disk	625.53	17.53	22.07	608.00	603.46	22.64	602.90	4.5	2	39.068119	-89.40121
T128	UA	02/09/2010	631.03	630.93	Top of Disk	626.27	16.53	21.04	609.74	605.23	21.64	602.20	4.5	2	39.068532	-89.401211
T202	UA	10/15/2010		628.63	Top of Disk	626.22	12.27	16.65	613.95	609.57	17.21	608.20	4.4	2	39.071776	-89.397705
T408	LCU	08/17/2016	624.08	624.08	Top of PVC	621.09	20.66	25.49	600.43	595.60	25.92	595.20	4.8	2	39.064353	-89.396307
T409	LCU	08/19/2016	625.01	625.01	Top of PVC	621.85	21.79	26.59	600.06	595.26	26.99	594.90	4.8	2	39.0603	-89.398538
TA31	UA	10/28/2014	626.55	626.55	Top of PVC	623.89	15.09	19.57	608.80	604.32	20.19	603.70	4.5	2	39.071368	-89.401366
TA32	UA	10/27/2014	621.42	621.42	Top of PVC	618.93	11.31	15.68	607.62	603.25	16.47	602.50	4.4	2	39.074093	-89.402223
TA33	UA	06/02/2015	625.27	625.27	Top of PVC	622.51	12.23	16.89	610.28	605.62	17.44	605.10	4.7	2	39.071556	-89.403506
TA34	UA	06/03/2015	626.52	626.52	Top of PVC	624.10	10.92	15.41	613.18	608.69	16.10	608.00	4.5	2	39.069631	-89.402759
TR32	UA	07/02/2021	621.68	621.68	Top of PVC	619.28	11.00	15.68	608.28	603.60	16.17	603.11	4.68	2	39.074064	-89.397758
NE Riser	S			626.13											39.071111	-89.393889
SG-02	SW			605.87	Top of Prot Casing	605.87									39.059695	-89.391429
SG-03	SW			594.94	Top of Prot Casing	594.94									39.059092	-89.390342
SG-04	SW			599.52	Top of Prot Casing	599.52									39.064146	-89.390504



TABLE 3-1. MONITORING WELL LOCATIONS AND CONSTRUCTION DETAILS

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT GMF GYPSUM STACK POND COFFEEN, ILLINOIS

Well Number HSU	Date Constructed	Top of PVC Elevation (ft)	Point Elevation (ft)	Measuring Point Description	Ground Elevation (ft)	Top Depth (ft BGS)	Bottom Depth (ft BGS)	Screen Top Elevation (ft)	Bottom Elevation (ft)	Well Depth (ft BGS)	Boring Elevation (ft)	Screen Length (ft)	Screen Diameter (inches)	Latitude (Decimal Degrees)	Longitude (Decimal Degrees)
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Notes:

All elevation data are presented relative to the North American Vertical Datum 1988 (NAVD88), GEOID 12A

-- = data not available
BGS = below ground surface
DA = deep aquifer
ft = foot or feet

HSU = Hydrostratigraphic Unit LCU = lower confining unit PVC = polyvinyl chloride S = source water

SW = surface water

UA = uppermost aquifer

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TABLE 3-2. VERTICAL HYDRAULIC GRADIENTS

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT GYPSUM MANAGEMENT FACILITY GYPSUM STACK POND COFFEEN, ILLINOIS

Date	G206 Groundwater Elevation (ft NAVD88) UA	G206D Groundwater Elevation (ft NAVD88) DA (PMP)	Head Change (ft)	Distance Change ¹ (ft)	Vertical H Gradie (dh/	ent ²
3/29/2021		583.94				
4/20/2021	622.07	585.96	36.11	33.51	1.08	down
5/3/2021	622.60	587.42	35.18	33.51	1.05	down
5/17/2021	622.31	587.81	34.50	33.51	1.03	down
6/9/2021	621.71	584.19	37.52	33.51	1.12	down
6/23/2021	620.54	589.66	30.88	33.51	0.92	down
7/12/2021	622.39	590.72	31.67	33.51	0.95	down
7/26/2021	622.00	591.14	30.86	33.51	0.92	down
		_	Middle	of screen elevation	n G206	610.8
			Middle o	f screen elevation	G206D	577.3

[O: KLT 6/4/21, C:YMD 6/7/21][U:KLT 8/25/21, C:EDP 8/31/21]

Notes:

DA = deep aquifer

dh = head change

dl = distance change

ft = foot/feet

NAVD88 = North American Vertical Datum of 1988

PMP = potential migration pathway

UA = uppermost aquifer



¹ Distance change was calculated using the midpoint of the piezometer screen and water table surface. If the water table surface was above the top of the monitoring well screen, then distance change was calculated using the midpoint of both screens.

 $^{^2}$ Vertical gradients between ± 0.0015 are considered flat, and typically have less than 0.02 foot difference in groundwater elevation between wells.

^{-- =} no data collected on date / no vertical gradient calculated

TABLE 3-3. FIELD HYDRAULIC CONDUCTIVITIES

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT GMF GYPSUM STACK POND COFFEEN, ILLINOIS

Well ID	Gradient Position	Bottom of Screen Elevation (ft NAVD88)	Screen Length ¹ (ft)	Field Identified Screened Material	Slug Type	Analysis Method	Falling Head (Slug In) Hydraulic Conductivity (cm/s)	Rising Head (Slug Out) Hydraulic Conductivity (cm/s)	Minimum Hydraulic Conductivity (cm/s)	Maximum Hydraulic Conductivity (cm/s)	Hydraulic Conductivity Geometric Mean (cm/s)
	Aquilei		T	T		1	1	T		T	
G206	D	608.61	4.41	SM, s(CL), CL	solid	Kansas Geological Survey	5.0E-04	4.9E-04			
G209	D	608.29	4.54	CL	solid	Kansas Geological Survey		2.5E-04			
G212	D	609.30	4.55	SM, s(CL), CL	solid	Kansas Geological Survey	2.1E-03	1.8E-03	2.5E-04	4.0E-03	1.4E-03
G215	D	606.68	4.39	SM, s(CL), ML	solid	Kansas Geological Survey	4.0E-03	3.5E-03			
G218	D	605.87	4.44	SM, SC, CL	solid	Kansas Geological Survey	2.6E-03	2.4E-03			
•			•			•				[(): KLT, C:EDP 8/31/21]

Notes:

1. All wells are constructed from 2 inch PVC with 0.01 inch slotted screens.

- - = Test not analyzed/performed

cm/s = centimeters per second

D = downgradient

ft = foot/feet

NAVD88 = North American Vertical Datum of 1988

PVC = polyvinyl chloride

USCS = Unified Soil Classification System

CL = Lean Clay s(CL) = Sandy Lean Clay ML = Silt SC = Clayey Sand SM = Silty Sand



TABLE 3-4. HORIZONTAL HYDRAULIC GRADIENTS AND GROUNDWATER FLOW VELOCITIES

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT GYPSUM MANAGEMENT FACILITY GYPSUM STACK POND COFFEEN, ILLINOIS

 $V = K i / n_e$ V = Groundwater Velocity

K = Hydraulic Conductivity ¹ i = hydraulic gradient $n_e = Effective Porosity$ ²

Across GMF Gypsum Stack Pond (R104 to G215)³

Distance between Wells (ft): 1470
Hydraulic Conductivity (ft/day): 3.96
Effective Porosity (%): 18

Change in Horizontal **R104 Elevation G215 Elevation** Velocity Date **Elevation** Gradient (ft NAVD88) (ft NAVD88) (ft/day) (ft) (ft/ft) 11/16/2015 621.34 4.96 0.07 616.38 0.003 2/8/2016 618.31 5.80 0.004 0.09 624.11 5/9/2016 624.89 619.45 5.44 0.004 0.08 7/25/2016 623.65 617.10 6.55 0.004 0.10 623.49 617.91 5.58 0.004 0.08 11/12/2016 2/4/2017 624.20 618.34 5.86 0.004 0.09 5/13/2017 622.91 618.16 4.75 0.003 0.07 624.09 617.01 7.08 0.005 7/8/2017 0.10 10/21/2017 619.38 615.48 3.90 0.003 0.06 5/8/2018 622.66 617.80 4.86 0.003 0.07 8/2/2018 618.00 3.73 0.003 0.05 621.73 10/23/2018 621.58 616.26 5.32 0.004 0.08 1/15/2019 622.43 618.03 4.40 0.003 0.06 623.34 617.55 5.79 0.004 0.09 8/5/2019 625.63 619.51 6.12 0.004 0.09 1/20/2020 8/10/2020 624.56 617.11 7.45 0.005 0.11 1/20/2021 623.31 617.19 6.12 0.004 0.09 4/20/2021 624.95 618.83 6.12 0.004 0.09 7/26/2021 0.005 625.41 618.79 6.62 0.10 0.004 Average 0.08

Assumes: Sand, Silt, and Clay

[O:KLT 8/13/21, C:EDP 8/31/21]



TABLE 3-4. HORIZONTAL HYDRAULIC GRADIENTS AND GROUNDWATER FLOW VELOCITIES

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT GYPSUM MANAGEMENT FACILITY GYPSUM STACK POND COFFEEN, ILLINOIS

Notes:

- ¹ Hydraulic conductivity values used above are average of the individual wells used in each velocity calculation as derived from slug tests completed in February and March 2021 by Ramboll.
- ² Effective porosity used in these calculations was derived from an average between estimated values of 0.20 for silt materials, 0.267 for gravel, 0.07 for clay, and 0.28 for sand from Morris, D.A. and A.I. Johnson, 1967. Summary of hydrologic and physical properties of rock and soil materials as analyzed by the Hydrologic Laboratory of the U.S. Geological Survey, U.S. Geological Survey Water-Supply Paper 1839-D, 42p. and Heath, R.C., 1983. Basic ground-water hydrology, U.S. Geological Survey Water-Supply Paper 2220, 86p. Effective porosity may be as high as maximum total porosity (50%) calculated in Table 2-1.
- ³ The uppermost aquifer was excavated within the footprint of the GMF GSP during construction. Flow calculated between R104 and G215 may not be representative of actual groundwater flow conditions around the GMF GSP.

-- = not calculated

% = percent ft = foot/feet ft/day = feet per day ft/ft = feet per foot NAVD88 = North American Vertical Datum of 1988 NM = Not Measured



Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C.	Lower	0	0	0	0	0	0		0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
845.600	Upper	0.006	0.010	2.0	0.004	2	0.005	-	200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
G102	01/20/2015																7.6					590
G102	04/08/2015	<0.003	<0.001	0.047	<0.001	<0.01	<0.001	62	28	0.016	<0.002	0.4	<0.001		0.00028		7.1		0.0028	86	<0.001	390
G102	07/23/2015	<0.003	0.0014	0.059	<0.001	<0.01	<0.001	-	30	0.0076	<0.002	0.429	0.0017		<0.0002	0.0013	7.1	-	0.0027	95	<0.001	420
G102	10/06/2015	<0.003	<0.001	0.058	<0.001	<0.01	<0.001		73	0.015	<0.002	0.272	<0.001		<0.0002	0.0016	7.1		0.0038	110	<0.001	590
G102	11/16/2015	<0.003	<0.001	0.069	<0.001	<0.01	<0.001	78	88	0.015	<0.002	0.291	<0.001	<0.01	<0.0002	<0.001	6.9	1.11	0.0025	140	<0.001	600
G102	02/17/2016	<0.003	<0.001	0.038	<0.001	0.014	<0.001	59	19	<0.004	<0.002	0.328	<0.001	<0.01	<0.0002	<0.001	7.5	1.76	0.0012	64	<0.001	400
G102	05/16/2016	<0.003	<0.001	0.029	<0.001	0.01	<0.001	42	3.8	<0.004	<0.002	0.525	<0.001	<0.01	<0.0002	0.0012	7.2	0.0616	0.0012	53	<0.001	310
G102	08/02/2016	<0.003	<0.001	0.044	<0.001	0.014	<0.001	69	42	<0.004	<0.002	0.383	<0.001	<0.01	<0.0002	<0.001	7.2	2.03	0.0032	86	<0.001	400
G102	11/19/2016	<0.003	<0.001	0.052	<0.001	<0.01	<0.001	77	40	<0.004	<0.002	0.389	<0.001	<0.01	<0.0002	<0.001	7.1	0.86	0.0028	95	<0.001	490
G102	02/09/2017	<0.003	0.0063	0.094	<0.001	0.019	<0.001	62	6.4	0.032	0.0057	0.419	0.0079	0.016	<0.0002	0.0017	7.1	1.16	0.0013	55	<0.001	370
G102	05/22/2017	<0.003	<0.001	0.036	<0.001	<0.01	<0.001	40	4	<0.004	<0.002	0.426	<0.001	<0.01	<0.0002	<0.001	7.2	0.773	<0.001	51	<0.001	300
G102	07/09/2017	<0.003	0.0068	0.11	<0.001	0.017	<0.001	83	45	0.029	0.0058	0.27	0.0097	0.016	<0.0002	0.0013	7.2	0.78	0.003	98	<0.001	440
G102	10/25/2017					0.014		56	54			0.366					7.1			130		540
G102	01/26/2018								46			<0.25					7.1					
G102	05/10/2018					<0.01		56	14			0.295					7.2			74		330
G102	10/24/2018					0.018		85	40			0.322					7.0			130		510
G102	01/17/2019					0.015		78	47			0.323					7.2			130		560
G102	08/12/2019					0.022		93	22			0.402					7.2			86		420
G102	01/21/2020					0.015		56	3.4			0.388					7.6			49		330
G102	08/11/2020					<0.01		75	30			0.265					7.3			120		470
G102	01/26/2021					0.017		55	12			0.365					7.1			70		410
G103	01/20/2015																7.6					420



Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C.	Lower	0	0	0	0	0	0		0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
845.600	Upper	0.006	0.010	2.0	0.004	2	0.005		200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
G103	04/08/2015	<0.003	0.0011	0.079	<0.001	<0.01	<0.001	80	56	0.0075	<0.002	0.4	<0.001		<0.0002		7.2		<0.001	54	<0.001	450
G103	07/23/2015	<0.003	0.0017	0.12	<0.001	<0.01	<0.001		55	0.0054	<0.002	0.262	0.0011		<0.0002	0.0029	7.2		<0.001	76	<0.001	390
G103	10/06/2015	<0.003	0.0012	0.1	<0.001	<0.01	<0.001		66	0.019	<0.002	<0.25	<0.001		<0.0002	0.0034	7.3		<0.001	69	<0.001	470
G105	01/20/2015																7.3					620
G105	04/08/2015	<0.003	0.0015	0.081	<0.001	0.098	<0.001	87	35	<0.004	<0.002	0.4	<0.001		<0.0002		7.0		0.0015	130	<0.001	550
G105	07/23/2015	<0.003	<0.001	0.068	<0.001	0.069	<0.001		34	<0.004	<0.002	0.336	<0.001		<0.0002	0.0023	7.1		<0.001	110	<0.001	540
G105	10/06/2015	<0.003	0.0013	0.078	<0.001	0.13	<0.001		37	0.0046	<0.002	0.304	0.0011		<0.0002	0.0028	7.0		0.0011	110	<0.001	560
G106	01/20/2015																7.5					440
G106	04/08/2015	<0.003	<0.001	0.057	<0.001	<0.01	<0.001	84	28	0.012	<0.002	0.47	<0.001		<0.0002		7.1		0.0025	56	<0.001	480
G106	07/23/2015	<0.003	<0.001	0.059	<0.001	<0.01	<0.001		27	<0.004	<0.002	0.378	<0.001		<0.0002	0.0019	7.2		0.0017	55	<0.001	460
G106	10/06/2015	<0.003	<0.001	0.06	<0.001	0.016	<0.001		25	0.013	<0.002	0.343	<0.001		<0.0002	0.0031	7.3		<0.001	55	<0.001	420
G106	11/17/2015	<0.003	<0.001	0.06	<0.001	<0.01	<0.001	80	26	<0.004	<0.002	0.394	<0.001	<0.01	<0.0002	0.0018	7.1	0.815	0.002	56	<0.001	380
G106	02/17/2016	<0.003	<0.001	0.054	<0.001	<0.01	<0.001	82	30	<0.004	<0.002	0.371	<0.001	<0.01	<0.0002	0.0021	7.2	0.701	0.0019	56	<0.001	440
G106	05/16/2016	<0.003	<0.001	0.055	<0.001	<0.01	<0.001	84	25	<0.004	<0.002	0.528	<0.001	<0.01	<0.0002	0.0019	7.1	0.244	0.0019	58	<0.001	380
G106	08/04/2016	<0.003	<0.001	0.056	<0.001	<0.01	<0.001	80	27	<0.004	<0.002	0.504	<0.001	<0.01	<0.0002	0.0025	7.0	0.4	0.0019	58	<0.001	420
G106	11/19/2016	<0.003	0.0013	0.067	<0.001	<0.01	<0.001	86	29	<0.004	<0.002	0.465	<0.001	<0.01	<0.0002	<0.001	7.0	0.923	<0.001	43	<0.001	440
G106	02/09/2017	<0.003	0.0011	0.057	<0.001	0.012	<0.001	73	28	<0.004	<0.002	0.462	<0.001	<0.01	<0.0002	0.0024	7.0	1.2	<0.001	63	<0.001	420
G106	05/22/2017	<0.003	0.0015	0.078	<0.001	<0.01	<0.001	68	30	<0.004	0.0038	0.44	<0.001	<0.01	<0.0002	0.0028	7.1	0.533	<0.001	67	<0.001	380
G106	07/09/2017	<0.003	0.0012	0.052	<0.001	<0.01	<0.001	84	29	<0.004	<0.002	0.366	<0.001	<0.01	<0.0002	0.0018	7.0	1.93	<0.001	60	<0.001	380
G106	10/25/2017					<0.01		72	48			0.47					7.3			45		410
G106	05/10/2018					<0.01		73	38			0.516					7.0			88		420
G106	08/09/2018											0.46					7.0					



Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C.	Lower	0	0	0	0	0	0		0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
845.600	Upper	0.006	0.010	2.0	0.004	2	0.005		200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
G106	10/24/2018					0.014		85	35			0.441					6.8			75		480
G106	01/17/2019					0.058		110	35			0.453					6.9			78		480
G106	08/13/2019					0.029		94	44			0.468					6.9			36		520
G106	01/21/2020					0.04		100	48			0.398					7.3			98		560
G106	08/11/2020					0.016		87	39			0.36					7.0			69		490
G106	01/26/2021					0.045		100	59			0.399					7.5			140		600
G106	06/29/2021																7.2					
G200	01/20/2015		0.0022			<0.01	<0.001	100	80				0.004				7.4			100		570
G200	04/10/2015	<0.003	0.0017	0.1	<0.001	<0.01	<0.001	110	87	<0.004	<0.002	0.303	0.0033		<0.0002		7.0		0.0079	87	<0.001	580
G200	07/22/2015	<0.003	0.0087	0.18	<0.001	<0.01	<0.001		96	0.017	0.0088	0.301	0.017		<0.0002	<0.001	7.0		0.0084	88	<0.001	630
G200	10/05/2015	<0.003	0.038	0.78	0.0038	0.048	0.0012		85	0.086	0.053	0.346	0.082		<0.0002	0.0039	7.1		0.013	90	<0.001	660
G200	11/23/2015	<0.003	0.007	0.17	<0.001	0.39	<0.001	100	75	0.012	0.0068	0.337	0.01	0.019	<0.0002	0.0017	7.2	1.65	0.0041	94	<0.001	520
G200	02/12/2016	<0.003	0.0082	0.24	0.0013	0.014	<0.001	150	93	0.013	0.0074	0.415	0.018	0.021	<0.0002	<0.001	7.2	3.84	0.0097	97	<0.001	540
G200	05/10/2016	<0.003	0.0025	0.13	<0.001	<0.01	<0.001	100	96	0.0041	<0.002	0.389	0.0058	<0.01	<0.0002	<0.001	7.1	0.849	0.0071	100	<0.001	480
G200	07/30/2016	<0.003	<0.001	0.059	<0.001	<0.01	<0.001	88	82	0.0049	<0.002	0.384	0.0012	<0.01	<0.0002	<0.001	7.1	0.662	0.0032	100	<0.001	520
G200	11/18/2016	<0.003	<0.001	0.053	<0.001	0.01	<0.001	88	75	<0.004	<0.002	0.431	<0.001	<0.01	<0.0002	<0.001	7.2	0.29	0.0032	110	<0.001	520
G200	02/10/2017	<0.003	<0.001	0.074	<0.001	<0.01	<0.001	85	82	0.0052	<0.002	0.305	0.0013	<0.01	<0.0002	<0.001	7.1	0.534	0.0067	100	<0.001	700
G200	05/18/2017	<0.003	<0.001	0.063	<0.001	0.01	<0.001	84	96	<0.004	<0.002	0.3	<0.001	<0.01	<0.0002	<0.001	7.0	1.01	0.0062	90	<0.001	620
G200	07/13/2017	<0.003	<0.001	0.057	<0.001	<0.01	<0.001	87	88	<0.004	<0.002	0.299	<0.001	<0.01	<0.0002	<0.001	7.1	0.906	0.0034	110	<0.001	540
G200	10/28/2017					0.34		81	65			0.328					7.2			100		520
G200	01/25/2018								71			0.303					7.2					
G200	05/11/2018					<0.01		90	85			<0.25					7.0			100		460



Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C.	Lower	0	0	0	0	0	0		0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
845.600	Upper	0.006	0.010	2.0	0.004	2	0.005		200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
G200	11/02/2018					0.011		95	61			0.391					7.0			100		480
G200	01/16/2019					0.048		350	54			0.386					7.1			110		700
G200	08/12/2019					<0.01		92	58			0.405					7.0			110		540
G200	01/21/2020					<0.01		110	100			0.302					7.2			120		520
G200	08/11/2020					<0.01		85	63			0.427					7.2			110		530
G200	01/29/2021					0.014		81	53			0.36					7.3			110		580
G200	03/29/2021	<0.003	<0.001	0.044	<0.001	0.04	<0.001	82	50	<0.004	<0.002	0.32	<0.001	<0.02	<0.0002	<0.001	7.3	0.345	0.0019	100	<0.001	460
G200	04/21/2021	<0.003	<0.001	0.047	<0.001	0.035	<0.001	88	63	<0.004	<0.002	0.435	<0.001	<0.02	<0.0002	<0.001	7.2	0.0349	0.0022	95	<0.001	570
G200	05/06/2021	< 0.003	0.0035	0.08	<0.001	0.012	<0.001	140	73	0.0096	0.0029	<0.25	0.0033	<0.02	<0.0002	0.002	7.1	0.0892	0.0035	100	<0.001	570
G200	05/17/2021	<0.003	<0.001	0.055	<0.001	<0.01	<0.001	91	78	<0.004	<0.002	0.342	<0.001	<0.02	<0.0002	<0.001	7.2	0.182	0.0023	110	<0.001	560
G200	06/14/2021																	0				
G200	07/28/2021	<0.003	<0.001	0.044	<0.001	0.11	<0.001	83	51	<0.004	<0.002	0.255	<0.001	<0.02	<0.0002	<0.001	7.0	0.239	0.0028	100	<0.001	500
G206	01/21/2015		<0.001			<0.01	<0.001	84	28				<0.001				7.3			130		480
G206	04/09/2015	<0.003	<0.001	0.052	<0.001	0.026	<0.001	92	29	<0.004	<0.002	0.411	<0.001		<0.0002		7.0		<0.001	130	<0.001	480
G206	07/22/2015	< 0.003	<0.001	0.094	<0.001	<0.01	<0.001		24	<0.004	<0.002	0.39	<0.001		<0.0002	0.0025	7.0		0.0068	32	<0.001	380
G206	10/07/2015	<0.003	0.0028	0.065	<0.001	0.042	<0.001		34	<0.004	<0.002	0.284	<0.001		<0.0002	0.0016	6.8		<0.001	110	<0.001	480
G206	11/18/2015	<0.003	0.0039	0.062	<0.001	<0.01	<0.001	79	32	0.0041	<0.002	0.433	<0.001	<0.01	<0.0002	<0.001	7.1	0.317	<0.001	95	<0.001	460
G206	02/24/2016	<0.003	<0.001	0.056	<0.001	0.033	<0.001	78	26	<0.004	<0.002	0.507	<0.001	<0.01	<0.0002	0.0014	6.7	0.292	<0.001	150	<0.001	500
G206	06/27/2016	<0.003	0.0012	0.062	<0.001	<0.01	<0.001	94	25	<0.004	<0.002	0.469	<0.001	<0.01	<0.0002	0.0025	6.2	0.647	<0.001	130	<0.001	420
G206	08/06/2016	<0.003	0.002	0.064	<0.001	<0.01	<0.001	90	27	0.0042	<0.002	0.449	0.0022	<0.01	<0.0002	0.0024	7.1	0.857	<0.001	130	<0.001	420
G206	11/22/2016	<0.003	<0.001	0.048	<0.001	0.11	<0.001	63	30	<0.004	<0.002	0.463	<0.001	<0.01	<0.0002	0.0015	7.1		<0.001	130	<0.001	480
G206	12/07/2016																	1.62				



Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C.	Lower	0	0	0	0	0	0		0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
845.600	Upper	0.006	0.010	2.0	0.004	2	0.005		200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
G206	02/11/2017	<0.003	<0.001	0.052	<0.001	<0.01	<0.001	70	29	<0.004	<0.002	0.547	<0.001	<0.01	<0.0002	0.0013	7.2	1.2	<0.001	150	<0.001	680
G206	05/18/2017	<0.003	<0.001	0.043	<0.001	<0.01	<0.001	66	29	<0.004	<0.002	<0.25	<0.001	<0.01	<0.0002	0.0011	7.0	0.555	<0.001	120	<0.001	460
G206	07/15/2017	<0.003	0.0019	0.055	<0.001	<0.01	<0.001	61	31	<0.004	<0.002	0.453	<0.001	<0.01	<0.0002	<0.001	7.1	1.33	<0.001	100	<0.001	480
G206	10/30/2017					<0.01		90	30			0.472					7.2			120		460
G206	05/15/2018					0.032		73	26			0.48					7.0			130		450
G206	11/02/2018					<0.01		85	25			0.36					7.0			120		440
G206	01/17/2019					<0.01		81	27			0.458		-			7.1			110		480
G206	08/14/2019					0.013		120	22			0.506		1			7.1			120		470
G206	01/21/2020					<0.01		84	24			0.389		-			7.5	-		120		470
G206	05/05/2020								-			-					7.5	-				
G206	08/13/2020					0.015		81	23			0.391					7.4			130		500
G206	01/27/2021					<0.01		65	22			0.426		1			7.1			130		480
G206D	03/30/2021	<0.003	0.0039	0.14	<0.001	0.13	<0.001	140	70	0.0095	0.003	0.666	0.0031	<0.02	<0.0002	0.033	7.1	0.451	<0.001	590	<0.001	1300
G206D	04/22/2021	<0.003	0.0032	0.089	<0.001	0.12	<0.001	120	67	<0.004	<0.002	0.774	0.0025	<0.02	<0.0002	0.029	7.2	0.14	<0.001	600	<0.001	1300
G206D	05/05/2021	<0.003	0.0039	0.12	<0.001	0.11	<0.001	130	56	<0.004	<0.002	0.766	<0.001	<0.02	<0.0002	0.03	7.1	0.147	<0.001	510	<0.001	1200
G206D	05/18/2021	<0.003	0.0052	0.12	<0.001	0.13	<0.001	130	52	<0.004	<0.002	0.499	<0.001	<0.02	<0.0002	0.029	7.2	0.247	<0.001	250	<0.001	1200
G206D	06/14/2021																	0.142				
G206D	07/27/2021	<0.003	0.0024	0.079	<0.001	0.12	<0.001	110	42	<0.004	<0.002	0.99	<0.001	<0.02	<0.0002	0.027	7.5	0.952	<0.001	400	<0.001	1100
G207	01/21/2015		<0.001			<0.01	<0.001	61	56				<0.001				7.7			72		360
G207	04/09/2015	<0.003	0.0017	0.11	<0.001	0.03	<0.001	65	55	0.0072	<0.002	0.466	0.0019		<0.0002		7.1		0.0012	69	<0.001	370
G207	07/22/2015	<0.003	0.0026	0.11	<0.001	<0.01	<0.001		62	<0.004	<0.002	0.512	0.0032		<0.0002	0.0016	7.2		<0.001	16	<0.001	440
G207	10/07/2015	<0.003	0.004	0.12	<0.001	0.021	<0.001		60	<0.004	<0.002	0.444	0.0011	-	<0.0002	0.0016	7.2	-	<0.001	21	<0.001	



Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C.	Lower	0	0	0	0	0	0		0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
845.600	Upper	0.006	0.010	2.0	0.004	2	0.005		200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
G208	01/21/2015		<0.001			<0.01	<0.001	58	25				<0.001				7.6			36		340
G208	04/09/2015	<0.003	<0.001	0.1	<0.001	<0.01	<0.001	62	24	<0.004	<0.002	0.47	<0.001		<0.0002		7.2		0.0044	35	<0.001	300
G208	07/22/2015	<0.003	<0.001	0.062	<0.001	<0.01	<0.001		27	<0.004	<0.002	0.396	<0.001		<0.0002	<0.001	7.1		<0.001	110	<0.001	530
G208	10/07/2015	<0.003	<0.001	0.091	<0.001	0.022	<0.001		23	<0.004	<0.002	0.387	<0.001		<0.0002	0.0017	7.2		0.0033	33	<0.001	370
G209	01/21/2015		<0.001			0.011	<0.001	150	73				<0.001				7.4			310		940
G209	04/09/2015	<0.003	0.0019	0.1	<0.001	<0.01	<0.001	180	69	0.0063	0.0025	0.409	0.0029		<0.0002		6.9		0.0022	280	<0.001	910
G209	07/22/2015	<0.003	0.0075	0.13	<0.001	<0.01	<0.001		69	0.017	0.0095	0.422	0.0099		<0.0002	0.0035	7.0		0.002	270	<0.001	970
G209	10/07/2015	0.004	0.0085	0.11	0.0042	0.025	0.0041		59	0.035	0.0072	0.328	0.007		0.00027	0.0077	7.1		0.0047	270	0.0035	940
G209	11/18/2015	<0.003	0.0021	0.072	<0.001	<0.01	<0.001	160	67	<0.004	<0.002	0.398	<0.001	<0.01	<0.0002	0.0023	7.0	0.469	0.0036	280	<0.001	810
G209	02/23/2016	<0.003	<0.001	0.066	<0.001	<0.01	<0.001	150	70	<0.004	<0.002	0.475	<0.001	<0.01	<0.0002	0.0015	7.0	0.903	<0.001	280	<0.001	760
G209	05/11/2016	<0.003	<0.001	0.061	<0.001	<0.01	<0.001	160	59	<0.004	<0.002	0.461	<0.001	<0.01	<0.0002	0.0015	7.1	1.48	<0.001	280	<0.001	800
G209	08/06/2016	<0.003	0.0017	0.065	<0.001	0.014	<0.001	160	67	<0.004	<0.002	0.468	<0.001	<0.01	<0.0002	0.0018	7.2	0.673	<0.001	270	<0.001	760
G209	11/22/2016	<0.003	0.0022	0.045	<0.001	0.015	<0.001	100	70	<0.004	<0.002	0.42	<0.001	<0.01	<0.0002	<0.001	7.1	0.832	<0.001	270	<0.001	750
G209	02/11/2017	<0.003	<0.001	0.07	<0.001	<0.01	<0.001	120	60	<0.004	<0.002	0.358	<0.001	<0.01	<0.0002	0.0027	7.0	0.103	<0.001	260	<0.001	960
G209	05/18/2017	<0.003	0.0029	0.077	<0.001	<0.01	<0.001	130	63	<0.004	<0.002	0.263	0.0012	<0.01	<0.0002	0.0019	7.2	1.31	<0.001	240	0.001	820
G209	07/15/2017	<0.003	0.0057	0.063	<0.001	0.012	<0.001	120	72	<0.004	<0.002	0.437	<0.001	<0.01	<0.0002	<0.001	7.3	0.602	<0.001	120	<0.001	780
G209	10/31/2017					0.012		150	63			0.519					7.1			95		730
G209	01/25/2018							120				0.456					7.0					
G209	05/15/2018					0.019		140	65			0.428					7.2			250		760
G209	11/02/2018					0.013		160	59			0.41					7.2			240		740
G209	01/17/2019					0.011		150	68			0.426					7.1			250		860
G209	05/03/2019							150									7.7					



Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C.	Lower	0	0	0	0	0	0		0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
845.600	Upper	0.006	0.010	2.0	0.004	2	0.005		200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
G209	08/14/2019					0.011		160	61			0.586					7.2			240		830
G209	01/22/2020					0.017		150	59			0.406					6.9			250		730
G209	05/05/2020							140									7.2					
G209	08/13/2020					0.018		150	65			0.474					7.2			270		800
G209	01/27/2021					<0.01		120	77			0.401					7.1			250		810
G210	01/21/2015		<0.001			<0.01	<0.001	73	55				<0.001				7.6			99		500
G210	04/09/2015	<0.003	<0.001	0.04	<0.001	0.055	<0.001	78	52	<0.004	<0.002	0.422	<0.001		<0.0002		7.0		0.0024	91	<0.001	490
G210	07/22/2015	<0.003	<0.001	0.037	<0.001	<0.01	<0.001		55	<0.004	<0.002	0.46	<0.001		<0.0002	<0.001	7.1		0.0014	84	<0.001	540
G210	10/07/2015	<0.003	<0.001	0.033	<0.001	0.013	<0.001		49	0.011	<0.002	0.368	<0.001		<0.0002	0.0018	7.2		<0.001	87	<0.001	500
G211	01/21/2015		<0.001			<0.01	<0.001	77	48				<0.001				7.6			80		490
G211	04/09/2015	<0.003	<0.001	0.081	<0.001	<0.01	<0.001	84	54	<0.004	<0.002	0.366	<0.001		<0.0002		7.1		0.0015	87	<0.001	480
G211	07/22/2015	<0.003	<0.001	0.087	<0.001	<0.01	<0.001		44	<0.004	<0.002	0.409	<0.001		<0.0002	<0.001	7.1		0.0025	74	<0.001	540
G211	10/07/2015	<0.003	<0.001	0.092	<0.001	0.014	<0.001		41	<0.004	<0.002	0.31	<0.001		<0.0002	<0.001	7.3		0.0013	78	<0.001	500
G212	01/21/2015		0.0021			0.019	<0.001	62	39				0.0035				7.5			59		400
G212	04/09/2015	<0.003	<0.001	0.056	<0.001	<0.01	<0.001	60	44	<0.004	<0.002	0.346	<0.001		<0.0002		7.0		0.0046	66	<0.001	410
G212	07/22/2015	<0.003	<0.001	0.057	<0.001	<0.01	<0.001		38	<0.004	<0.002	0.396	<0.001		<0.0002	0.0036	7.0		0.0042	56	<0.001	460
G212	10/07/2015	<0.003	<0.001	0.057	<0.001	0.026	<0.001		43	0.0043	<0.002	<0.25	<0.001		<0.0002	0.0035	7.0		0.0044	65	<0.001	410
G212	11/18/2015	<0.003	<0.001	0.052	<0.001	<0.01	<0.001	55	38	<0.004	<0.002	0.34	<0.001	<0.01	<0.0002	0.0015	7.2	0.132	0.0037	54	<0.001	380
G212	02/19/2016	<0.003	<0.001	0.05	<0.001	<0.01	<0.001	58	41	<0.004	<0.002	0.339	<0.001	<0.01	<0.0002	<0.001	7.3	0.582	0.0048	59	<0.001	380
G212	05/11/2016	<0.003	<0.001	0.05	<0.001	<0.01	<0.001	58	37	<0.004	<0.002	0.421	<0.001	<0.01	<0.0002	<0.001	7.3	0.759	0.0041	59	<0.001	400
G212	08/06/2016	<0.003	<0.001	0.059	<0.001	0.016	<0.001	59	37	0.004	<0.002	0.369	0.0016	<0.01	<0.0002	0.0012	7.3	0.992	0.004	55	<0.001	330
G212	11/23/2016	<0.003	<0.001	0.049	<0.001	<0.01	<0.001	51	42	<0.004	<0.002	0.399	<0.001	<0.01	<0.0002	<0.001	7.1		0.0043	54	<0.001	340



Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C.	Lower	0	0	0	0	0	0		0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
845.600	Upper	0.006	0.010	2.0	0.004	2	0.005		200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
G212	12/07/2016																	1.64				
G212	02/15/2017	<0.003	0.001	0.058	<0.001	<0.01	<0.001	53	37	<0.004	<0.002	0.369	0.001	<0.01	<0.0002	<0.001	7.1	0.488	0.0041	55	<0.001	420
G212	05/22/2017	<0.003	<0.001	0.061	<0.001	<0.01	<0.001	46	39	<0.004	<0.002	0.372	<0.001	<0.01	<0.0002	0.0011	7.0	0.729	0.0039	57	<0.001	360
G212	07/15/2017	<0.003	<0.001	0.052	<0.001	<0.01	<0.001	46	44	<0.004	<0.002	0.377	<0.001	<0.01	<0.0002	<0.001	7.6	0.654	0.0046	53	<0.001	430
G212	10/31/2017					<0.01		50	42			0.326					7.3			55		340
G212	05/14/2018					0.014		51	40			0.407					7.2			52		350
G212	11/02/2018					<0.01		53	43			0.289					7.3			49		600
G212	01/16/2019					<0.01		56	43			0.394					7.3			53		440
G212	08/14/2019					<0.01		53	43			0.437					7.3			51		380
G212	01/22/2020					0.012		61	42			0.283					7.2			58		340
G212	08/13/2020					<0.01		54	42			0.323					7.3			53		430
G212	01/26/2021					0.032		56	41			<0.25					6.8			55		400
G212	06/29/2021																7.4					
G213	01/21/2015		<0.001			<0.01	<0.001	66	44				<0.001				7.5			55		390
G213	04/09/2015	<0.003	<0.001	0.064	<0.001	<0.01	<0.001	70	47	<0.004	<0.002	0.35	0.0017		<0.0002		7.0		0.0024	57	<0.001	420
G213	07/22/2015	<0.003	0.0072	0.16	<0.001	<0.01	<0.001		42	0.026	0.01	0.41	0.016		<0.0002	0.0012	7.0		0.0035	50	<0.001	440
G213	10/07/2015	<0.003	0.0045	0.1	0.002	0.014	0.0018		40	0.026	0.0057	0.314	0.008		<0.0002	0.004	7.0		0.0038	51	0.0014	400
G214	01/21/2015		<0.001			<0.01	<0.001	81	100				<0.001				7.5			68		480
G214	04/09/2015	<0.003	<0.001	0.1	<0.001	<0.01	<0.001	88	110	<0.004	<0.002	0.417	<0.001		<0.0002		7.0		0.0011	73	<0.001	500
G214	07/22/2015	<0.003	<0.001	0.092	<0.001	<0.01	<0.001		82	<0.004	<0.002	0.446	<0.001		<0.0002	0.0011	7.1		<0.001	68	<0.001	530
G214	10/07/2015	<0.003	<0.001	0.089	<0.001	0.011	<0.001		67	0.0067	<0.002	0.324	<0.001		<0.0002	0.0019	7.2		<0.001	76	<0.001	540
G215	01/21/2015		0.0064			0.033	<0.001	110	60				<0.001				7.3			170		640



Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C.	Lower	0	0	0	0	0	0		0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
845.600	Upper	0.006	0.010	2.0	0.004	2	0.005		200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
G215	04/09/2015	<0.003	0.046	0.17	<0.001	0.041	<0.001	120	53	<0.004	<0.002	0.418	<0.001		<0.0002		6.8		<0.001	140	<0.001	620
G215	07/22/2015	<0.003	0.053	0.17	<0.001	0.023	<0.001		45	<0.004	<0.002	0.345	0.0017		<0.0002	<0.001	6.9		<0.001	110	<0.001	660
G215	10/07/2015	<0.003	0.04	0.15	<0.001	0.035	<0.001		46	<0.004	<0.002	0.321	0.0015		<0.0002	<0.001	6.8		<0.001	110	<0.001	550
G215	11/24/2015	<0.003	0.11	0.23	<0.001	0.037	<0.001	110	47	<0.004	0.0028	0.34	0.0039	<0.01	<0.0002	0.0011	7.2	2.42	<0.001	110	<0.001	500
G215	02/18/2016	<0.003	0.0034	0.095	<0.001	0.027	<0.001	100	52	<0.004	<0.002	0.359	<0.001	<0.01	<0.0002	<0.001	7.2	0.852	<0.001	130	<0.001	520
G215	05/11/2016	0.0045	0.0068	0.088	<0.001	0.026	<0.001	89	43	<0.004	<0.002	0.463	<0.001	<0.01	<0.0002	<0.001	6.9	0.468	0.0024	110	<0.001	460
G215	07/30/2016	<0.003	0.013	0.096	<0.001	0.015	<0.001	89	47	<0.004	<0.002	0.432	<0.001	<0.01	<0.0002	<0.001	6.9	0.0216	<0.001	110	<0.001	480
G215	11/23/2016	<0.003	0.0086	0.082	<0.001	0.023	<0.001	68	48	<0.004	<0.002	0.429	<0.001	<0.01	<0.0002	<0.001	6.9		<0.001	100	<0.001	500
G215	12/07/2016																	1.58				
G215	02/18/2017	<0.003	0.012	0.095	<0.001	0.021	<0.001	86	46	<0.004	<0.002	0.369	<0.001	<0.01	<0.0002	<0.001	7.3	0.344	<0.001	110	<0.001	510
G215	05/22/2017	<0.003	0.036	0.15	<0.001	0.024	<0.001	82	42	<0.004	<0.002	<0.25	<0.001	<0.01	<0.0002	<0.001	7.4	1.24	<0.001	100	<0.001	470
G215	07/15/2017	<0.003	0.044	0.13	<0.001	0.027	<0.001	79	55	<0.004	<0.002	0.423	<0.001	<0.01	<0.0002	<0.001	7.0	1.01	<0.001	110	<0.001	550
G215	10/31/2017					0.025		90	48		-	0.42					7.2			110		470
G215	05/15/2018					0.063		130	70			0.329					6.9	-		220		660
G215	11/02/2018					0.088		120	55			0.314					6.8			170		480
G215	01/16/2019		-			0.097		120	61		-	0.379					6.9			180		800
G215	08/14/2019					0.085		100	49			0.458					7.0	-		120		520
G215	01/22/2020					0.064		99	48			0.35					7.1	-		130		460
G215	08/13/2020					0.051		110	70			0.366					7.2			210		710
G215	01/26/2021					0.36		180	120			<0.25					6.8			490		1100
G215	06/29/2021							180	110								7.1			470		950
G216	01/21/2015		0.0029			0.014	<0.001	100	78				<0.001				7.3			230		740



Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C.	Lower	0	0	0	0	0	0		0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
845.600	Upper	0.006	0.010	2.0	0.004	2	0.005		200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
G216	04/10/2015	<0.003	0.0064	0.18	<0.001	0.018	<0.001	140	66	<0.004	<0.002	0.385	0.0013		<0.0002		6.9		<0.001	210	<0.001	720
G216	07/22/2015	<0.003	0.0037	0.17	<0.001	0.021	<0.001		67	<0.004	<0.002	0.313	<0.001		<0.0002	0.0026	7.0		<0.001	220	<0.001	800
G216	10/07/2015	<0.003	0.002	0.16	<0.001	0.042	<0.001		85	<0.004	<0.002	0.295	<0.001		<0.0002	0.013	7.0	-	<0.001	210	<0.001	740
G217	01/21/2015		<0.001			<0.01	<0.001	110	82			-	<0.001				7.2			140		670
G217	04/10/2015	<0.003	<0.001	0.088	<0.001	<0.01	<0.001	130	77	<0.004	<0.002	0.401	<0.001		<0.0002		6.8	-	<0.001	130	<0.001	700
G217	07/22/2015	<0.003	0.0053	0.16	<0.001	<0.01	<0.001		78	0.011	0.012	0.31	0.0067		<0.0002	0.0014	7.0		<0.001	130	<0.001	750
G217	10/07/2015	<0.003	0.0019	0.11	<0.001	0.013	<0.001		76	0.0086	0.0032	0.295	0.0019		<0.0002	0.0013	7.1	-	<0.001	130	<0.001	660
G218	01/21/2015		0.0013			<0.01	<0.001	94	100			-	<0.001				7.2			110		660
G218	04/10/2015	<0.003	0.0011	0.14	<0.001	<0.01	<0.001	120	93	<0.004	<0.002	0.381	<0.001		<0.0002		6.8		<0.001	99	<0.001	640
G218	07/22/2015	<0.003	<0.001	0.15	<0.001	<0.01	<0.001		98	<0.004	<0.002	0.308	<0.001		<0.0002	<0.001	6.8		<0.001	97	<0.001	690
G218	10/07/2015	<0.003	<0.001	0.14	<0.001	<0.01	<0.001		96	0.03	<0.002	0.27	<0.001		<0.0002	0.0022	7.0		<0.001	95	<0.001	620
G218	11/24/2015	<0.003	0.0084	0.17	<0.001	<0.01	<0.001	120	99	0.01	<0.002	0.3	<0.001	<0.01	<0.0002	0.0015	7.1	1.23	<0.001	94	<0.001	620
G218	02/19/2016	<0.003	0.0018	0.15	<0.001	<0.01	<0.001	120	100	<0.004	<0.002	0.311	<0.001	<0.01	<0.0002	<0.001	7.0	1.28	<0.001	110	<0.001	560
G218	05/10/2016	<0.003	0.0015	0.14	<0.001	0.011	<0.001	110	97	<0.004	<0.002	0.439	<0.001	<0.01	<0.0002	<0.001	7.0	0.601	<0.001	140	<0.001	600
G218	07/30/2016	<0.003	0.0011	0.15	<0.001	<0.01	<0.001	130	100	<0.004	<0.002	0.382	<0.001	<0.01	<0.0002	<0.001	7.0	0.543	<0.001	120	<0.001	620
G218	11/23/2016	<0.003	0.0014	0.13	<0.001	<0.01	<0.001	92	97	<0.004	<0.002	0.373	<0.001	<0.01	<0.0002	<0.001	7.1		<0.001	130	<0.001	620
G218	12/07/2016																	0.85				
G218	02/18/2017	<0.003	0.0011	0.13	<0.001	<0.01	<0.001	110	88	<0.004	<0.002	0.308	<0.001	<0.01	<0.0002	<0.001	7.2	0.779	<0.001	130	<0.001	630
G218	05/22/2017	<0.003	<0.001	0.15	<0.001	<0.01	<0.001	100	84	<0.004	<0.002	<0.25	<0.001	<0.01	<0.0002	<0.001	7.1	0.975	<0.001	140	<0.001	600
G218	07/17/2017	<0.003	<0.001	0.14	<0.001	<0.01	<0.001	120	81	<0.004	<0.002	0.357	<0.001	<0.01	<0.0002	<0.001	7.1	0.704	<0.001	140	<0.001	720
G218	10/31/2017					<0.01		110	91			0.437					6.9			140		660
G218	01/26/2018	-1					-1		-								6.9					



Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C.	Lower	0	0	0	0	0	0		0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
845.600	Upper	0.006	0.010	2.0	0.004	2	0.005		200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
G218	05/15/2018					0.014		110	91			0.413					7.0			140		640
G218	11/02/2018					<0.01		130	84			0.375					6.9			140		280
G218	01/17/2019					<0.01		120	82			0.361					7.0			140		600
G218	08/14/2019					<0.01		130	81			0.449					7.0			150		660
G218	01/22/2020					0.011		130	83			0.379					7.1			170		560
G218	08/13/2020					<0.01		120	84			0.34					7.1			220		720
G218	01/26/2021					0.017		120	81			0.276					7.0			210		710
MW16S	04/09/2015																7.2					410
R104	01/20/2015																7.8					460
R104	04/08/2015	<0.003	0.0015	0.065	<0.001	<0.01	<0.001	66	47	0.0043	<0.002	0.4	<0.001		<0.0002		7.4		0.0074	72	<0.001	500
R104	07/23/2015	<0.003	<0.001	0.067	<0.001	<0.01	<0.001		45	<0.004	<0.002	0.349	0.0023		<0.0002	0.0044	7.4		0.0072	73	<0.001	450
R104	10/06/2015	<0.003	<0.001	0.063	<0.001	<0.01	<0.001		56	0.0044	<0.002	0.304	<0.001		<0.0002	0.0045	7.6		0.0062	72	<0.001	480
R104	11/17/2015	<0.003	<0.001	0.067	<0.001	<0.01	<0.001	69	51	0.0052	<0.002	0.358	0.0012	<0.01	<0.0002	0.0041	7.4	0.565	0.0063	77	<0.001	420
R104	02/17/2016	<0.003	<0.001	0.06	<0.001	0.012	<0.001	67	58	<0.004	<0.002	0.314	<0.001	<0.01	<0.0002	0.0042	8.1	0.363	0.0064	77	<0.001	440
R104	05/16/2016	<0.003	<0.001	0.062	<0.001	<0.01	<0.001	65	49	<0.004	<0.002	0.473	<0.001	<0.01	<0.0002	0.0038	7.4	0.145	0.006	74	<0.001	420
R104	08/03/2016	<0.003	<0.001	0.064	<0.001	<0.01	<0.001	68	50	<0.004	<0.002	0.372	<0.001	<0.01	<0.0002	0.0042	7.3	0.798	0.006	76	<0.001	460
R201	01/20/2015		<0.001			<0.01	<0.001	110	53				<0.001				7.2			210		750
R201	04/10/2015	<0.003	<0.001	0.084	<0.001	<0.01	<0.001	130	66	<0.004	<0.002	0.353	<0.001		<0.0002		6.8		<0.001	220	<0.001	760
R201	07/22/2015	<0.003	0.0011	0.099	<0.001	<0.01	<0.001		78	<0.004	<0.002	0.28	<0.001		<0.0002	<0.001	6.9		<0.001	250	<0.001	930
R201	10/05/2015	<0.003	<0.001	0.081	<0.001	<0.01	<0.001		74	<0.004	<0.002	0.353	<0.001		<0.0002	<0.001	7.0		<0.001	210	<0.001	840
R201	11/23/2015	<0.003	<0.001	0.078	<0.001	<0.01	0.0012	85	37	<0.004	<0.002	0.377	<0.001	<0.01	<0.0002	0.0069	7.3	0.202	<0.001	150	<0.001	560
R201	02/12/2016	<0.003	0.01	0.084	0.0067	0.014	<0.001	120	75	<0.004	<0.002	0.398	<0.001	<0.01	<0.0002	0.001	7.0	0.543	0.0091	240	<0.001	740



Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C.	Lower	0	0	0	0	0	0		0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
845.600	Upper	0.006	0.010	2.0	0.004	2	0.005		200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
R201	05/10/2016	<0.003	<0.001	0.084	<0.001	<0.01	<0.001	120	85	<0.004	<0.002	0.447	<0.001	<0.01	<0.0002	<0.001	7.0	1.12	<0.001	260	<0.001	840
R201	07/30/2016	<0.003	0.0031	0.092	<0.001	<0.01	<0.001	120	85	<0.004	<0.002	0.368	<0.001	<0.01	<0.0002	<0.001	7.1	0.697	<0.001	260	<0.001	750
R201	11/18/2016	<0.003	0.0013	0.058	<0.001	<0.01	<0.001	81	39	<0.004	<0.002	0.494	<0.001	<0.01	<0.0002	<0.001	7.2	0.055	<0.001	160	<0.001	580
R201	02/11/2017	<0.003	0.0028	0.086	<0.001	<0.01	<0.001	100	79	<0.004	<0.002	0.285	<0.001	<0.01	<0.0002	<0.001	7.1	1.02	<0.001	230	<0.001	900
R201	05/18/2017	<0.003	0.0023	0.087	<0.001	0.011	<0.001	120	74	<0.004	<0.002	0.354	<0.001	<0.01	<0.0002	<0.001	7.2	1.51	<0.001	300	<0.001	820
R201	07/13/2017	<0.003	0.0037	0.17	<0.001	0.01	<0.001	120	81	<0.004	<0.002	0.284	<0.001	<0.01	<0.0002	<0.001	7.0	2.75	<0.001	250	<0.001	780
R201	10/28/2017					0.017		93	30			0.38					7.1			89		660
R201	01/25/2018								31			0.338					7.0					
R201	05/11/2018					< 0.01		87	54			0.306					7.1			190		640
R201	11/02/2018					<0.01		82	24			0.419					7.1			110		470
R201	01/16/2019					<0.01		100	48			0.341					7.1			150		790
R201	08/12/2019					<0.01		120	71			0.466					7.1			220		760
R201	01/21/2020					0.01		130	66			0.309					7.2			210		770
R201	08/11/2020					< 0.01		120	87			0.364					6.9			240		790
R201	01/29/2021					0.01		94	46		-	<0.25					7.0	-		160		710
R201	03/29/2021	<0.003	0.0017	0.073	<0.001	0.019	<0.001	97	55	<0.004	<0.002	0.45	<0.001	<0.02	<0.0002	<0.001	7.0	0.272	<0.001	190	<0.001	660
R201	04/21/2021	<0.003	0.0013	0.071	<0.001	0.019	<0.001	100	63	<0.004	<0.002	0.359	<0.001	<0.02	<0.0002	<0.001	6.9	0.561	<0.001	200	<0.001	810
R201	05/06/2021	<0.003	0.0027	0.078	<0.001	<0.01	<0.001	110	67	0.0043	<0.002	<0.25	<0.001	<0.02	<0.0002	0.0011	7.2	0.742	<0.001	210	<0.001	730
R201	05/17/2021	<0.003	0.0025	0.078	<0.001	<0.01	<0.001	110	76	<0.004	<0.002	0.265	<0.001	<0.02	<0.0002	<0.001	7.0	1.19	<0.001	210	<0.001	770
R201	06/14/2021	<0.003	0.0028	0.082	<0.001	0.016	<0.001	120	68	<0.004	<0.002	0.332	<0.001	<0.02	<0.0002	<0.001	7.0	0.123	<0.001	230	<0.001	820
R201	06/29/2021	<0.003	0.01	0.13	<0.001	0.014	<0.001	140	81	0.0067	0.0037	0.356	0.0059	<0.02	<0.0002	0.002	7.1	1.17	<0.001	220	<0.001	560
R201	07/13/2021	<0.003	0.0022	0.069	<0.001	<0.01	<0.001	97	58	<0.004	<0.002	0.261	<0.001	<0.02	<0.0002	<0.001	7.0	0.581	<0.001	170	<0.001	710



HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT GMF GYPSUM STACK POND COFFEEN, ILLINOIS

Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C.	Lower	0	0	0	0	0	0		0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
845.600	Upper	0.006	0.010	2.0	0.004	2	0.005		200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
R201	07/28/2021	<0.003	0.0019	0.081	<0.001	0.011	<0.001	110	79	<0.004	<0.002	0.504	<0.001	<0.02	0.0011	0.044	7.2	0.744	<0.001	370	<0.001	750

Notes:

Detected at concentration greater than the GWPS

-- = data not available

GWPS = Groundwater protection standard

mg/L = milligrams per liter

pCi/L = picocuries per liter

SU = standard units

35 I.A.C. 845.600 = Residuals in Surface Impoundments: Title 35 of the Illinois Administrative Code § 845



< = concentration is less than the concentration shown, which corresponds to the reporting limit for the method. Estimated concentrations below the reporting limit and associated qualifiers are not provided since they are not utilized in statistics to determine exceedances above Part 845 standards.</p>

TABLE 4-2. GROUNDWATER FIELD PARAMETERS

Sample Location	Sample Date	Dissolved Oxygen (mg/L)	Oxidation Reduction Potential (mV)	pH (field) (SU)	Specific Conductance (micromhos/cm)	Temperature (deg. C)	Turbidity (NTU)
G102	01/20/2015			7.6	691		
G102	04/08/2015			7.1	746		
G102	07/23/2015			7.1	723		
G102	10/06/2015			7.1	857		
G102	11/16/2015	0.42	160	6.9	1040	12.6	3.9
G102	02/17/2016	1.10	152	7.5	682	11.3	
G102	05/16/2016	0	136	7.2	575	16.1	30
G102	08/02/2016	0	96	7.2	648	18.0	30.6
G102	11/19/2016	0	74	7.1	782	13.8	20.5
G102	02/09/2017	0	72	7.1	820	14.5	45.5
G102	05/22/2017	0	61	7.2	1040	15.3	40.5
G102	07/09/2017	0	96	7.2	640	17.0	29.6
G102	10/25/2017	0	94	7.1	665	13.1	28.7
G102	01/26/2018	0	-96	7.1	690	11.1	19.7
G102	05/10/2018	0	94	7.2	661	14.4	16.7
G102	10/24/2018	0	87	7.0	693	14.5	14.8
G102	01/17/2019	0	99	7.2	652	12.3	28.6
G102	08/12/2019	0	98	7.2	700	16.8	27.7
G102	01/21/2020	6.10	148	7.6	599.2	9.9	101
G102	08/11/2020	1.10	129	7.3	949.2	18.3	0
G102	01/26/2021	2.10	141	7.1	685	11.3	3.42
G103	01/20/2015			7.6	555		
G103	04/08/2015			7.2	761		
G103	07/23/2015			7.2	815		
G103	10/06/2015			7.3	896		
G105	01/20/2015			7.3	700		
G105	04/08/2015			7.0	897		
G105	07/23/2015			7.1	912		
G105	10/06/2015			7.0	1037		
G106	01/20/2015			7.5	330		
G106	04/08/2015			7.1	771		
G106	07/23/2015			7.2	790		
G106	10/06/2015			7.3	825		
G106	11/17/2015	0	116	7.1	804	16.6	132
G106	02/17/2016	5.40	167	7.2	800	11.6	
G106	05/16/2016	7.22	162	7.1	758	16.1	46
G106	08/04/2016	0	126	7.0	697	17.3	33.9
G106	11/19/2016	0	105	7.0	812	13.6	24.4
G106	02/09/2017	0	99	7.0	800	14.9	29.5
G106	05/22/2017	0	94	7.1	849	15.2	22.8
G106	07/09/2017	0	121	7.0	705	17.0	45
G106	10/25/2017	0	107	7.3	738	13.3	14.5
G106	05/10/2018	0	106	7.0	728	13.0	24.8
G106	08/09/2018	0	120	7.0	695	14.9	30.4
G106	10/24/2018	0	127	6.8	662	13.9	36.1
G106	01/17/2019	0	123	6.9	695	12.2	20.8



TABLE 4-2. GROUNDWATER FIELD PARAMETERSHYDROGEOLOGIC SITE CHARACTERIZATION REPORT

COFFEEN POWER PLANT GMF GYPSUM STACK POND COFFEEN, ILLINOIS

Sample Location	Sample Date	Dissolved Oxygen (mg/L)	Oxidation Reduction Potential (mV)	pH (field) (SU)	Specific Conductance (micromhos/cm)	Temperature (deg. C)	Turbidity (NTU)
G106	08/13/2019	0	121	6.9	712	17.4	29.8
G106	01/21/2020	1.00	81.4	7.3	951.1	10.8	25.4
G106	08/11/2020	0.41	-145	7.0	915.6	18.1	0
G106	01/26/2021	1.40	49.3	7.5	1223	11.4	4.95
G106	06/29/2021	1.70	-41.3	7.2	869.6	19.8	0
G200	01/20/2015			7.4	382		
G200	04/10/2015			7.0	939		
G200	07/22/2015			7.0	1024		
G200	10/05/2015			7.1	984		
G200	11/23/2015	7.18	29	7.2	948	15.7	844
G200	02/12/2016	10.00	134	7.2	1060	9.3	1000
G200	05/10/2016	0	-33	7.1	1010	18.3	1000
G200	07/30/2016	0	-62	7.1	990	19.2	1000
G200	11/18/2016	0	-76	7.2	874	16.3	1000
G200	02/10/2017	0	-72	7.1	852	15.7	1000
G200	05/18/2017	0	-66	7.0	904	15.4	1000
G200	07/13/2017	0	-63	7.1	940	18.1	1000
G200	10/28/2017	0	-65	7.2	1004	13.0	1000
G200	01/25/2018	0	-77	7.2	890	12.1	99
G200	05/11/2018	0	-55	7.0	1010	13.2	1000
G200	11/02/2018	0	-58	7.0	1020	13.8	1000
G200	01/16/2019	0	-61	7.1	980	12.3	1000
G200	08/12/2019	0	-63	7.0	967	17.0	1000
G200	01/21/2020	4.90	183	7.2	1039	10.4	42.6
G200	08/11/2020	1.80	104	7.2	929	19.1	21.8
G200	01/29/2021	2.70	43.6	7.3	914	9.1	98.3
G200	03/29/2021	1.80	28.1	7.3	886	12.8	115
G200	04/21/2021	0.76	92.7	7.2	898	10.6	13.2
G200	05/06/2021	1.40	10.8	7.1	917	11.9	2.71
G200	05/17/2021	0.22	34	7.2	933	14.2	189
G200	07/28/2021	0.77	-23.5	7.0	829.5	19.3	9.98
G206	01/21/2015			7.3	578		
G206	04/09/2015			7.0	773		
G206	07/22/2015			7.0	758		
G206	10/07/2015			6.8	747		
G206	11/18/2015	0	-165	7.1	830	16.6	65.4
G206	02/24/2016	1.50	22	6.7	769	12.9	19
G206	06/27/2016	0	-31	6.2	838	23.5	
G206	08/06/2016	0	103	7.1	920	19.3	40.8
G206	11/22/2016	0	91	7.1	964	14.9	28.4
G206	02/11/2017	0	123	7.2	905	15.0	47
G206	05/18/2017	0	93	7.0	987	15.7	54.9
G206	07/15/2017	0	85	7.1	931	18.5	58.9
G206	10/30/2017	0	105	7.2	1010	12.8	31.8
G206	05/15/2018	0	91	7.0	938	15.5	43.3
G206	11/02/2018	0	98	7.0	872	15.1	40.6



TABLE 4-2. GROUNDWATER FIELD PARAMETERSHYDROGEOLOGIC SITE CHARACTERIZATION REPORT

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT GMF GYPSUM STACK POND COFFEEN, ILLINOIS

Sample Location	Sample Date	Dissolved Oxygen (mg/L)	Oxidation Reduction Potential (mV)	pH (field) (SU)	Specific Conductance (micromhos/cm)	Temperature (deg. C)	Turbidity (NTU)
G206	01/17/2019	0	101	7.1	922	12.4	47
G206	08/14/2019	0	99	7.1	904	17.0	22.6
G206	01/21/2020	2.90	16.8	7.5	782.6	13.0	2.57
G206	05/05/2020	4.60	176	7.5	781	13.0	3.98
G206	08/13/2020	1.40	202	7.4	819	17.8	0.3
G206	01/27/2021	2.10	39.1	7.1	799	12.0	0.71
G206D	03/30/2021	0.69	-9.6	7.1	2027	15.4	226
G206D	04/22/2021	2.00	44.2	7.2	1892	15.4	54.5
G206D	05/05/2021	4.80	97.6	7.1	1764	18.8	33.5
G206D	05/18/2021	0.23	-48.4	7.2	1843	14.5	2.97
G206D	07/27/2021	2.00	78	7.5	924.3	20.6	20
G207	01/21/2015			7.7	444		
G207	04/09/2015			7.1	607		
G207	07/22/2015			7.2	693		
G207	10/07/2015			7.2	672		
G208	01/21/2015			7.6	396		
G208	04/09/2015			7.2	531		
G208	07/22/2015			7.1	638		
G208	10/07/2015			7.2	743		
G209	01/21/2015			7.4	1004		
G209	04/09/2015			6.9	1280		
G209	07/22/2015			7.0	1030		
G209	10/07/2015			7.1	1086		
G209	11/18/2015	0	-98	7.0	1024	16.5	149
G209	02/23/2016	0.07	76	7.0	1350	12.2	2.6
G209	05/11/2016	0	144	7.1	1400	19.0	30
G209	08/06/2016	0	149	7.2	1478	19.3	31.2
G209	11/22/2016	0	138	7.1	1610	15.1	36.4
G209	02/11/2017	0	-109	7.0	1490	14.5	69.4
G209	05/18/2017	0	-99	7.2	1420	15.5	62.2
G209	07/15/2017	0	161	7.3	1220	17.9	22
G209	10/31/2017	0	140	7.1	1450	12.9	26.4
G209	01/25/2018	0	101	7.0	1410	11.1	30.2
G209	05/15/2018	0	117	7.2	1480	13.5	28.5
G209	11/02/2018	0	148	7.2	1580	13.8	24.1
G209	01/17/2019	0	141	7.1	1490	12.9	30.6
G209	05/03/2019	0	159	7.7	1240	14.5	37
G209	08/14/2019	0	151	7.2	1490	17.1	29.3
G209	01/22/2020	1.10	-69	6.9	1310	11.6	22.6
G209	05/05/2020	0.88	-34.5	7.2	1182	13.0	0.7
G209	08/13/2020	1.50	-60.3	7.2	1285	18.6	5.4
G209	01/27/2021	3.20	47.8	7.1	1315	9.1	4.36
G210	01/21/2015			7.5	611		
G210	04/09/2015			7.0	836		
G210	07/22/2015			7.1	918		
G210	10/07/2015			7.2	813		



TABLE 4-2. GROUNDWATER FIELD PARAMETERSHYDROGEOLOGIC SITE CHARACTERIZATION REPORT

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT GMF GYPSUM STACK POND COFFEEN, ILLINOIS

Sample Location	Sample Date	Dissolved Oxygen (mg/L)	Oxidation Reduction Potential (mV)	pH (field) (SU)	Specific Conductance (micromhos/cm)	Temperature (deg. C)	Turbidity (NTU)
G211	01/21/2015			7.6	604		
G211	04/09/2015			7.1	819		
G211	07/22/2015			7.1	855		
G211	10/07/2015			7.3	926		
G212	01/21/2015			7.5	525		
G212	04/09/2015			7.0	712		
G212	07/22/2015			7.0	744		
G212	10/07/2015			7.0	825		
G212	11/18/2015	0	-63	7.2	706	16.4	38.4
G212	02/19/2016	2.20	128	7.3	721	13.9	
G212	05/11/2016	2.11	141	7.3	732	16.7	25
G212	08/06/2016	0	130	7.3	795	18.0	41.2
G212	11/23/2016	0	126	7.1	924	13.7	50.1
G212	02/15/2017	0	114	7.1	890	15.0	51.2
G212	05/22/2017	0	102	7.0	915	15.6	40.2
G212	07/15/2017	0	130	7.6	838	17.0	33.7
G212	10/31/2017	0	139	7.3	821	12.6	40.3
G212	05/14/2018	0	122	7.2	832	13.8	43.2
G212	11/02/2018	0	121	7.3	890	14.7	30.8
G212	01/16/2019	0	131	7.3	832	12.3	29.8
G212	08/14/2019	0	128	7.3	815	17.0	35.6
G212	01/22/2020	2.80	154	7.2	728.6	11.7	7.72
G212	08/13/2020	3.20	150	7.3	655	18.7	1.7
G212	01/26/2021	2.30	152	6.8	728	11.4	3.14
G212	06/29/2021	1.80	29.9	7.4	703.2	17.9	0
G213	01/21/2015			7.5	504		
G213	04/09/2015			7.0	654		
G213	07/22/2015			7.0	642		
G213	10/07/2015			7.0	679		
G214	01/21/2015			7.5	666		
G214	04/09/2015			7.0	841		
G214	07/22/2015			7.1	938		
G214	10/07/2015			7.2	923		
G215	01/21/2015			7.3	741		
G215	04/09/2015			6.8	948		
G215	07/22/2015			6.9	782		
G215	10/07/2015			6.8	838		
G215	11/24/2015	8.90	-59	7.2	907	16.0	909
G215	02/18/2016	0	-2	7.2	963	12.9	
G215	05/11/2016	0	5	6.9	945	16.9	285
G215	07/30/2016	0	56	6.9	1020	18.3	161
G215	11/23/2016	0	61	6.9	1060	15.9	135
G215	02/18/2017	0	71	7.3	970	14.5	102
G215	05/22/2017	0	84	7.4	1030	15.2	91
G215	07/15/2017	0	61	7.0	992	17.5	1000
G215	10/31/2017	0	129	7.2	868	13.3	36.5



TABLE 4-2. GROUNDWATER FIELD PARAMETERSHYDROGEOLOGIC SITE CHARACTERIZATION REPORT

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT GMF GYPSUM STACK POND COFFEEN, ILLINOIS

Sample Location	Sample Date	Dissolved Oxygen (mg/L)	Oxidation Reduction Potential (mV)	pH (field) (SU)	Specific Conductance (micromhos/cm)	Temperature (deg. C)	Turbidity (NTU)
G215	05/15/2018	0	42	6.9	1020	15.2	132
G215	11/02/2018	0	52	6.8	1050	14.2	179
G215	01/16/2019	0	61	6.9	1022	12.6	123
G215	08/14/2019	0	56	7.0	998	17.2	1000
G215	01/22/2020	0.46	0.5	7.1	938.6	11.1	31.8
G215	08/13/2020	0.20	-35.2	7.2	1086	20.3	13.8
G215	01/26/2021	0.17	-2.9	6.8	1493	12.0	45.9
G215	06/29/2021	0.20	-34.2	7.1	1689	19.1	70.4
G216	01/21/2015			7.3	817		
G216	04/10/2015			6.9	1070		
G216	07/22/2015			7.0	1093		
G216	10/07/2015			7.0	981		
G217	01/21/2015			7.2	802		
G217	04/10/2015			6.8	1049		
G217	07/22/2015			7.0	1135		
G217	10/07/2015			7.1	1219		
G218	01/21/2015			7.2	817		
G218	04/10/2015			6.8	1038		
G218	07/22/2015			6.8	1002		
G218	10/07/2015			7.0	980		
G218	11/24/2015	8.30	-22	7.1	1080	15.8	623
G218	02/19/2016	0	28	7.0	1130	13.6	
G218	05/10/2016	0	-7	7.0	1130	19.8	18
G218	07/30/2016	0	72	7.0	1040	18.5	62.4
G218	11/23/2016	0	76	7.1	945	14.6	40.9
G218	02/18/2017	0	57	7.2	1100	14.7	37.2
G218	05/22/2017	0	62	7.1	1150	15.8	32.4
G218	07/17/2017	0	59	7.1	990	20.0	52.7
G218	10/31/2017	0	69	6.9	1080	13.5	51.9
G218	01/26/2018	0	71	6.9	990	11.9	40.2
G218	05/15/2018	0	81	7.0	1030	15.4	52.1
G218	11/02/2018	0	61	6.9	1080	14.5	51.8
G218	01/17/2019	0	73	7.0	1061	12.5	51.8
G218	08/14/2019	0	69	7.0	1028	16.9	70.6
G218	01/22/2020	1.20	33.9	7.1	1125	13.2	23.2
G218	08/13/2020	0.70	9.6	7.1	1132	20.2	1.3
G218	01/26/2021	1.40	10.5	7.0	1162	12.2	108
MW16S	04/09/2015			7.2	659		
R104	01/20/2015			7.8	591		
R104	04/08/2015			7.4	793		
R104	07/23/2015			7.4	724		
R104	10/06/2015			7.6	659		
R104	11/17/2015	8.20	149	7.4	820	17.1	7.6
R104	02/17/2016	7.30	166	8.1	839	10.9	
R104	05/16/2016	2.67	169	7.4	776	17.6	5.8
R104	08/03/2016	0	119	7.3	695	19.1	15.2



TABLE 4-2. GROUNDWATER FIELD PARAMETERS

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT GMF GYPSUM STACK POND COFFEEN, ILLINOIS

Sample Location	Sample Date	Dissolved Oxygen (mg/L)	Oxidation Reduction Potential (mV)	pH (field) (SU)	Specific Conductance (micromhos/cm)	Temperature (deg. C)	Turbidity (NTU)
R201	01/20/2015			7.2	445		
R201	04/10/2015			6.8	1212		
R201	07/22/2015			6.9	1102		
R201	10/05/2015			7.0	1165		
R201	11/23/2015	0	-69	7.3	985	15.6	80
R201	02/12/2016	11.00	101	7.0	1390	10.3	4.8
R201	05/10/2016	0	-106	7.0	1420	15.5	55
R201	07/30/2016	0	-96	7.1	1520	16.9	57.7
R201	11/18/2016	0	-78	7.2	1350	15.3	45
R201	02/11/2017	0	-57	7.1	1292	15.0	50.2
R201	05/18/2017	0	-59	7.2	1150	14.8	57.5
R201	07/13/2017	0	-98	7.0	1495	17.0	59
R201	10/28/2017	0	-99	7.1	1300	12.6	60.5
R201	01/25/2018	0	56	7.0	1500	11.5	55.9
R201	05/11/2018	0	-88	7.1	1490	13.9	53.9
R201	11/02/2018	0	-91	7.1	1590	14.0	68.7
R201	01/16/2019	0	-94	7.1	1500	12.7	65.8
R201	08/12/2019	0	-93	7.1	1483	17.4	49.6
R201	01/21/2020	0.82	-115	7.2	1198	9.0	5.76
R201	08/11/2020	1.80	-80.1	6.9	1390	18.4	120
R201	01/29/2021	0.27	-32.1	7.0	1106	8.5	65.9
R201	03/29/2021	0.39	-56.5	7.0	1164	13.3	9.32
R201	04/21/2021	0.69	-44.5	6.9	1043	11.0	8.93
R201	05/06/2021	4.80	-42.4	7.2	1236	11.9	119
R201	05/17/2021	0.06	-109	7.0	1278	13.7	4.34
R201	06/14/2021	0.04	-120	7.0	1353	19.9	1.43
R201	06/29/2021	0.06	-116	7.1	1339	23.3	130
R201	07/13/2021	0.38	-27.1	7.0	1145	15.8	14.4
R201	07/28/2021	0.23	-122	7.2	1226	20.0	8.99

Notes:

Field readings are reported with as many significant figures as provided by analytical laboratory. -- = data not available

cm = centimeter

deg. C = degrees Celsius mg/L = milligrams per liter mV = millivolts

NTU = nephelometric turbidity units

SU = standard units

generated 10/22/2021, 11:50:42 AM CDT



6 of 6

FIGURES

PART 845 REGULATED UNIT (SUBJECT UNIT)
PROPERTY BOUNDARY
COFFEEN LAKE STATE FISH AND WILDLIFE AREA

SITE LOCATION MAP

FIGURE 1-1

RAMBOLL AMERICAS ENGINEERING SOLUTIONS, INC.





■ COAL MINE SHAFT

PART 845 REGULATED UNIT (SUBJECT UNIT)

SITE FEATURE

LIMITS OF FINAL COVER

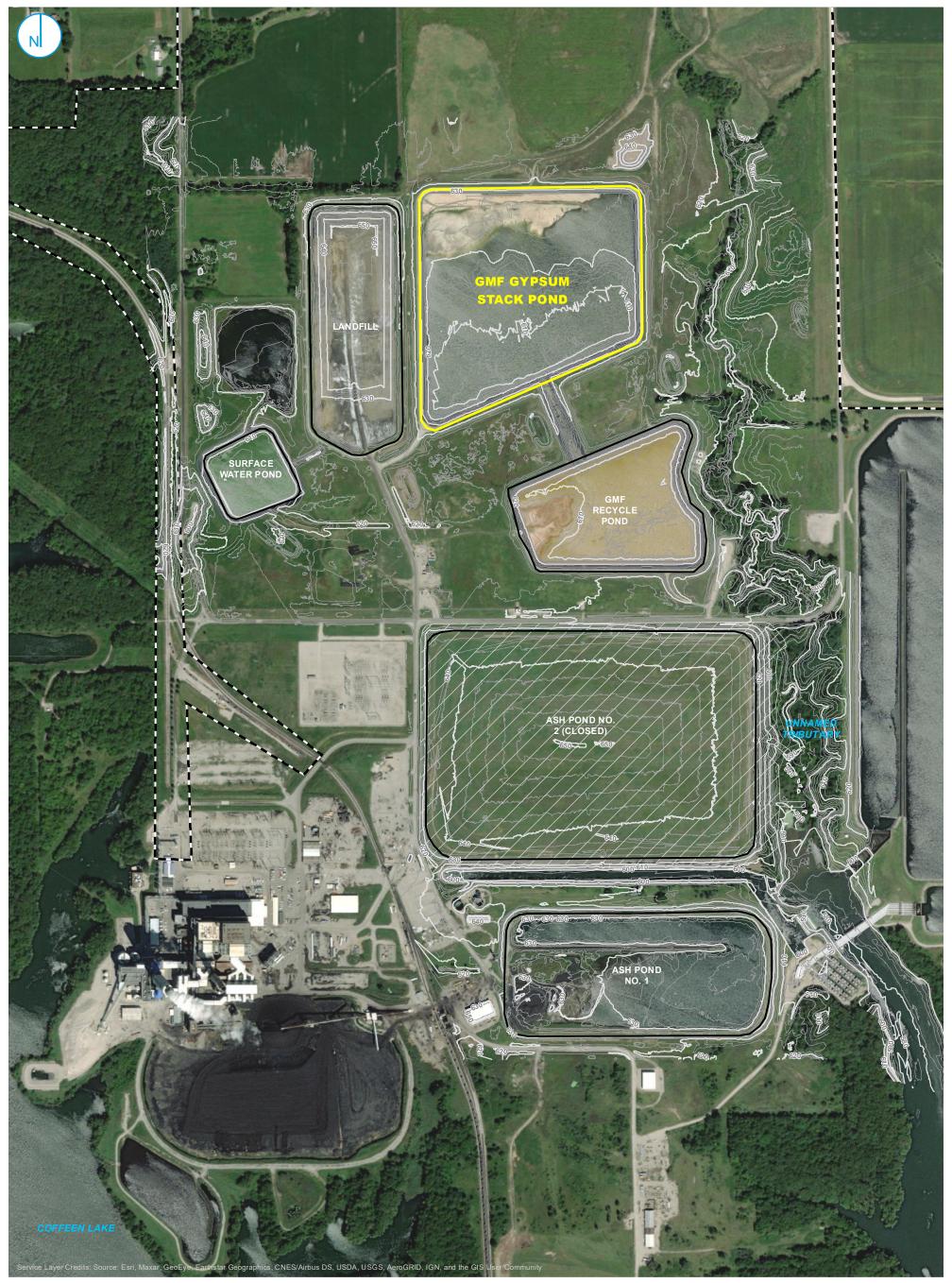
PROPERTY BOUNDARY

SITE MAP

FIGURE 1-2

RAMBOLL AMERICAS ENGINEERING SOLUTIONS, INC.

RAMBOLL



10-FT ELEVATION CONTOUR

NOTE ELEVATION CONTOURS SHOWN IN FEET, 2-FT ELEVATION CONTOUR NORTH AMERICAN VERTICAL DATUM OF PART 845 REGULATED UNIT (SUBJECT UNIT) 1988

SITE TOPOGRAPHIC MAP

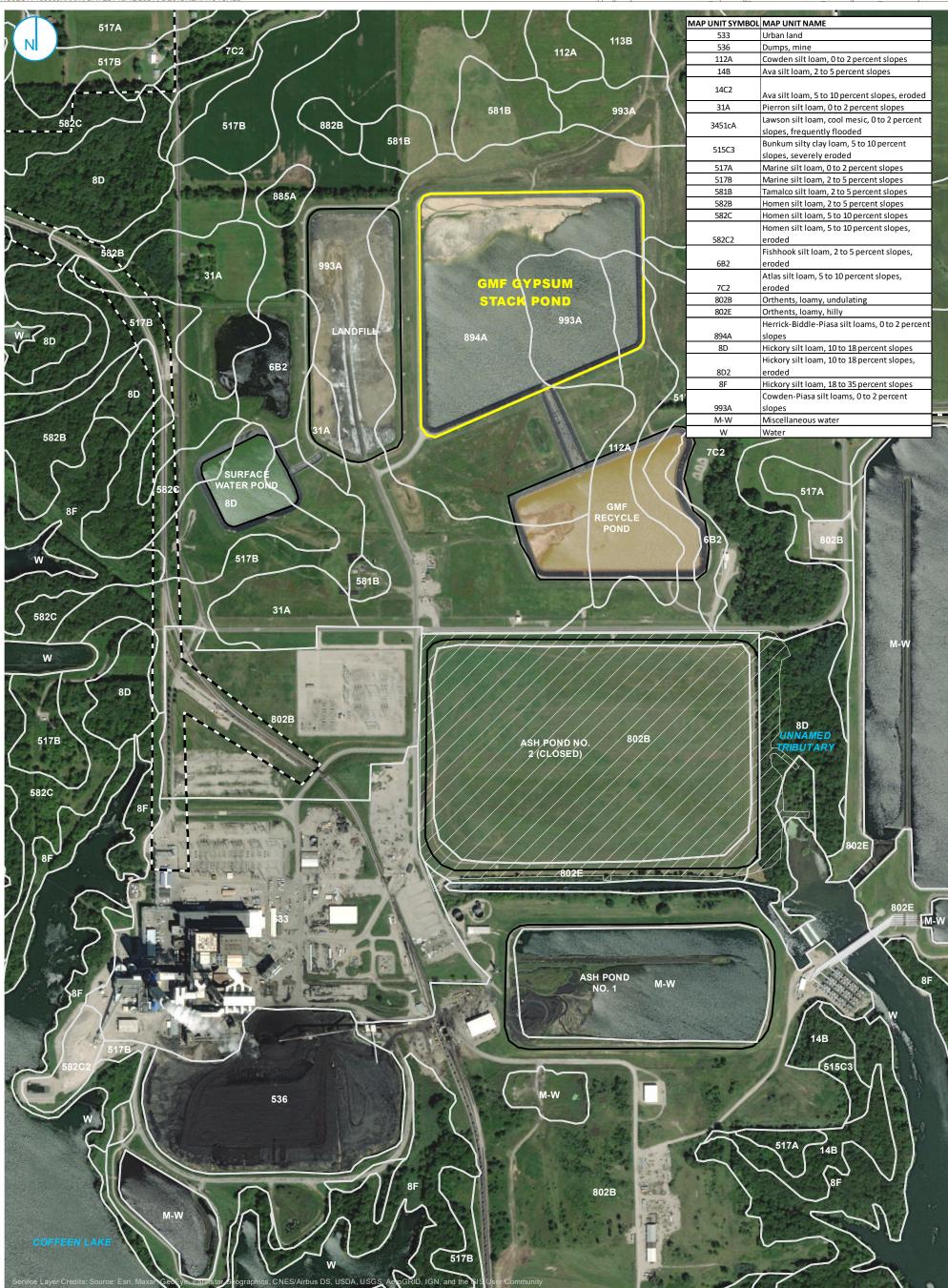
FIGURE 2-1

SITE FEATURE LIMITS OF FINAL COVER PROPERTY BOUNDARY

> HYDROGEOLOGIC SITE CHARACTERIZATION REPORT **GMF GYPSUM STACK POND** COFFEEN POWER PLANT COFFEEN, ILLINOIS

RAMBOLL AMERICAS ENGINEERING SOLUTIONS, INC.

RAMBOLL



PART 845 REGULATED UNIT (SUBJECT UNIT)

SITE FEATURE

LIMITS OF FINAL COVER

PROPERTY BOUNDARY

NRCS SOIL SURVEY MAP UNIT BOUNDARY

SOIL SURVEY MAP

FIGURE 2-2

RAMBOLL AMERICAS ENGINEERING SOLUTIONS, INC.



PART 845 REGULATED UNIT (SUBJECT UNIT)

SITE FEATURE

LIMITS OF FINAL COVER

T PROPERTY BOUNDARY

CAHOKIA ALLUVIUM (INCLUDES ALLUVIAL FAN FACIES)

HAGARSTOWN MEMBER

VANDALIA TILL MEMBER

WATER

750

1,500

___ Feet

ervice Layer Credits: Source: Esri, Maxar, GeoEye, Earthstar Geographics, CNES/Airbus DS, USDA, USOS, AeroGRID, IGN, and the GIS U

ILLINOIS STATE GEOLOGICAL SURVEY (ISGS)

SURFICIAL GEOLOGIC DEPOSITS

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT GMF GYPSUM STACK POND

COFFEEN POWER PLANT COFFEEN, ILLINOIS RAMBOLL AMERICAS ENGINEERING SOLUTIONS, INC.



FIGURE 2-3

GENERALIZED STRATIGRAPHIC COLUMN

FIGURE 2-4

RAMBOLL AMERICAS ENGINEERING SOLUTIONS, INC.

RAMBOLL

MAJOR STRUCTURAL FEATURES OF ILLINOIS

FIGURE 2-5

RAMBOLL AMERICAS ENGINEERING SOLUTIONS, INC.

RAMBOLL



MONITORING WELL

◆ CCR SAMPLE

SOIL BORING

PART 845 REGULATED UNIT (SUBJECT UNIT)

___ Feet

SITE FEATURE

LIMITS OF FINAL COVER
PROPERTY BOUNDARY

0 275 550

FIELD INVESTIGATION LOCATION MAP

GMF GYPSUM STACK POND

COFFEEN POWER PLANT

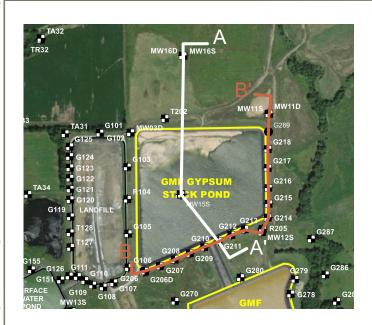
COFFEEN, ILLINOIS

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT

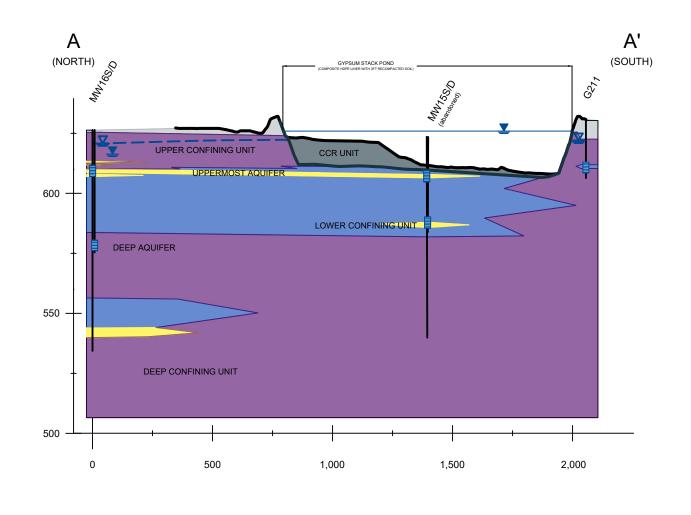
MAP FIGURE 2-6

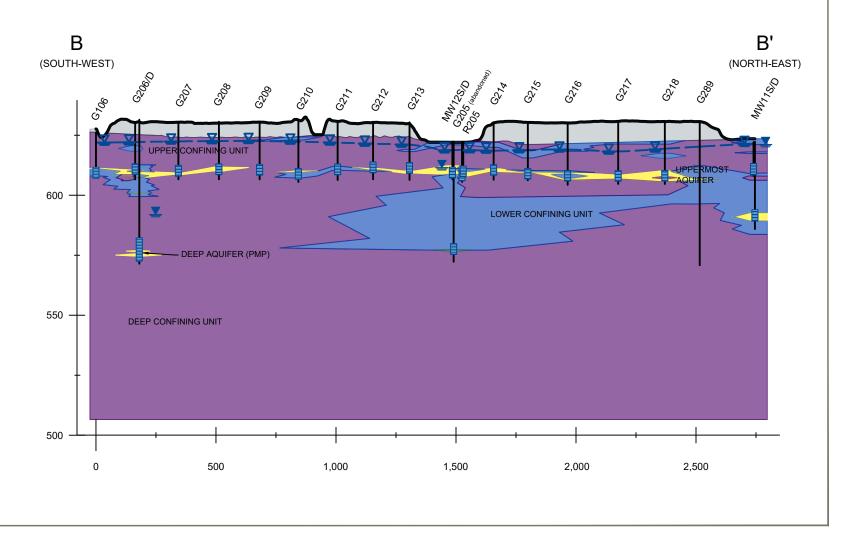
RAMBOLL AMERICAS ENGINEERING SOLUTIONS, INC.





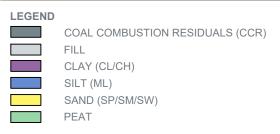
- 1. This profile was developed by interpolation between widely spaced boreholes. Only at the borehole location should it be considered as an approximately accurate representation and then only to the degree implied by the notes on the borehole logs.
- 2. Scale is approximate.
- 3. Vertical scale is exaggerated 10X.
- 4. Groundwater elevations measured on July 26, 2021.





A-A' & B-B'

COFFEEN, ILLINOIS





--- UPPERMOST AQUIFER POTENTIOMETRIC SURFACE

■ UPPERMOST AQUIFER GROUNDWATER ELEVATION

■ BEDROCK GROUNDWATER / OTHER GROUNDWATER / SURFACE WATER ELEVATION(S)

→ SURFACE WATER



FIGURE 2-7

RAMBOLL AMERICAS ENGINEERING SOLUTIONS, INC.





10 FOOT HISTORIC ELEVATION CONTOUR 2 FOOT HISTORIC ELEVATION CONTOUR PART 845 REGULATED UNIT (SUBJECT UNIT

PART 845 REGULATED UNIT (SUBJECT UNIT)
SITE FEATURE

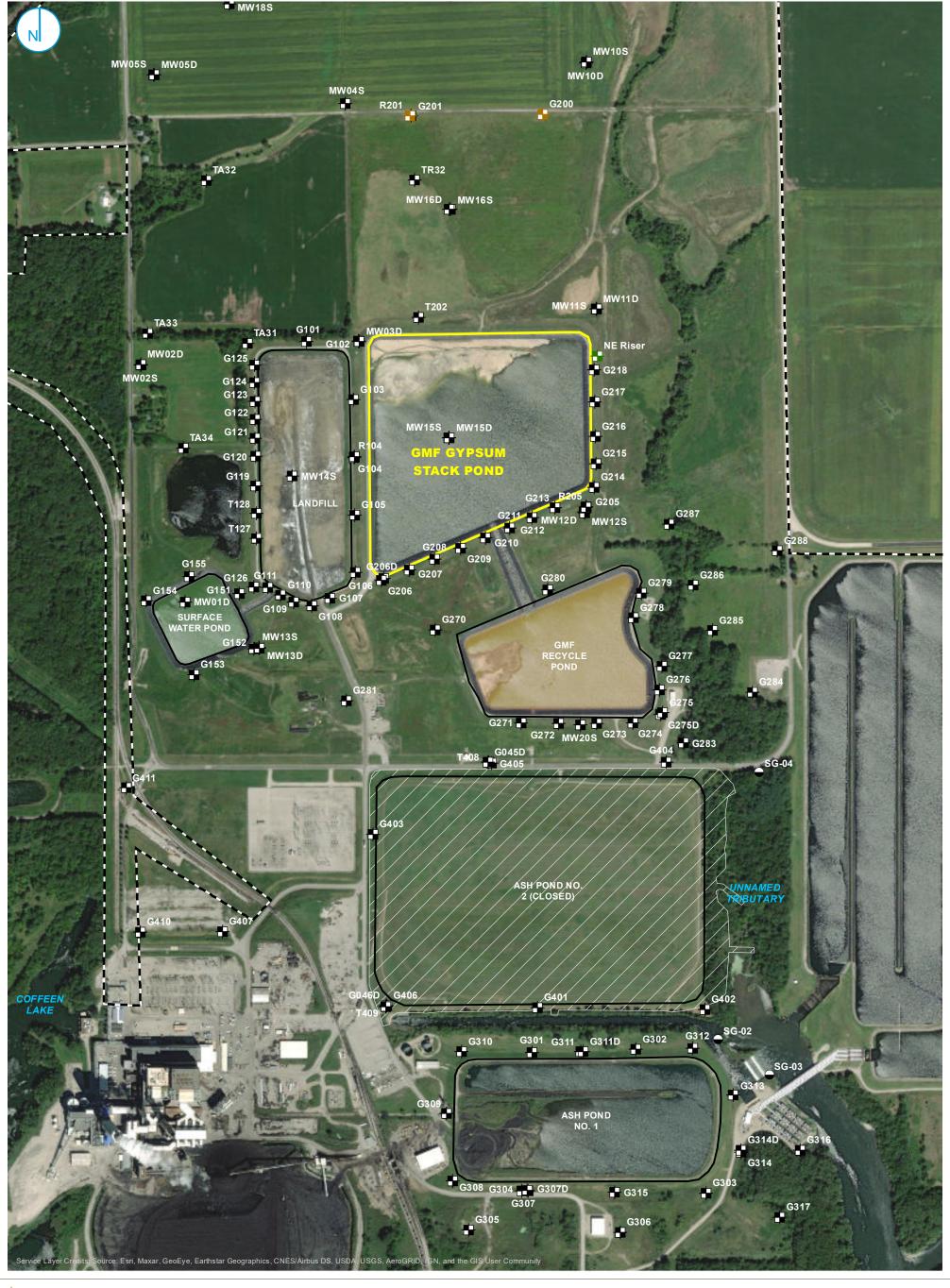
LIMITS OF FINAL COVER
PROPERTY BOUNDARY

BOTTOM OF CCR MAP

FIGURE 2-8

RAMBOLL AMERICAS ENGINEERING SOLUTIONS, INC.

RAMBOLL



BACKGROUND WELL

MONITORING WELL

SOURCE SAMPLE LOCATION

STAFF GAGE

PART 845 REGULATED UNIT (SUBJECT UNIT)

SITE FEATURE

LIMITS OF FINAL COVER

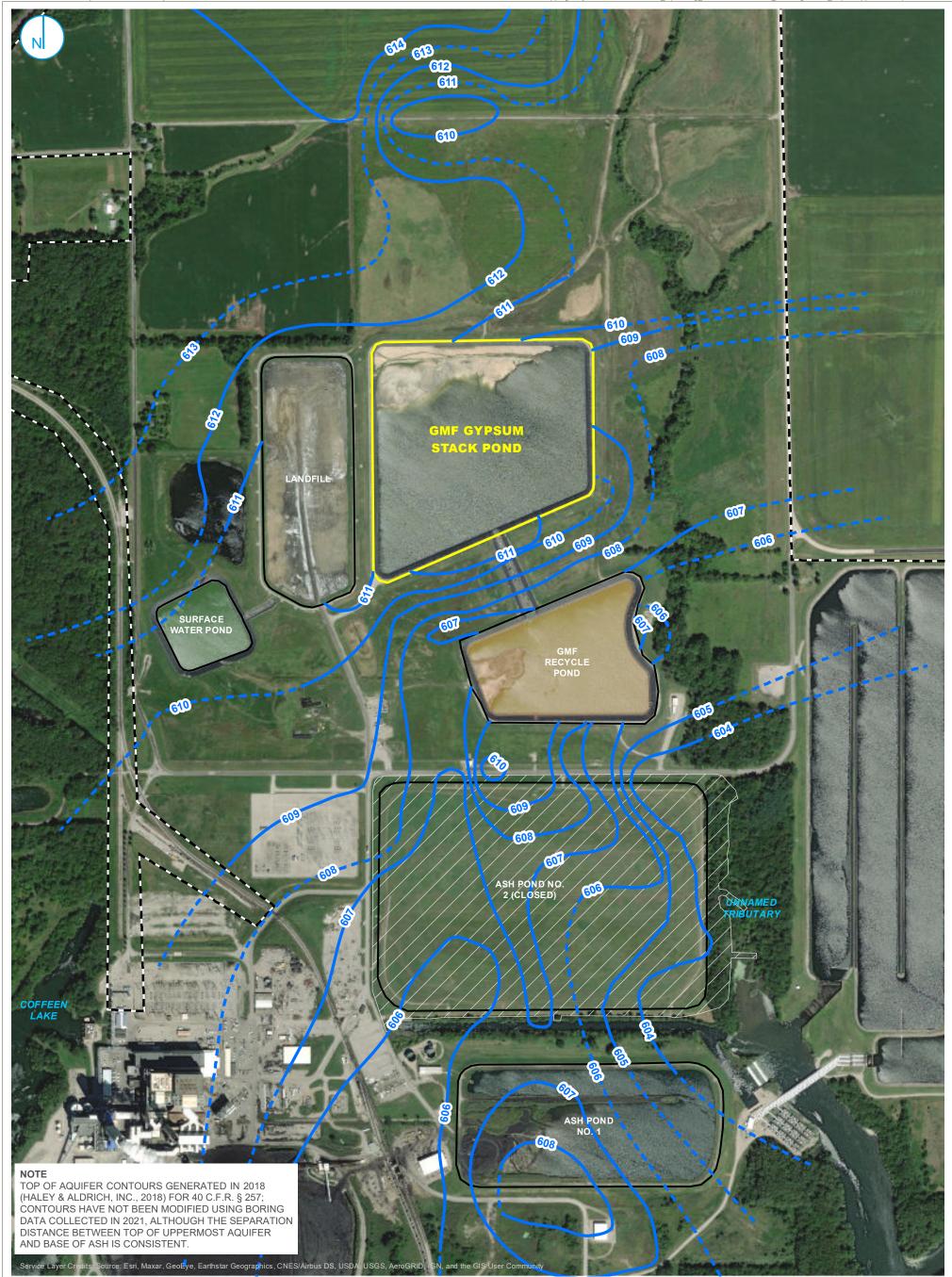
PROPERTY BOUNDARY 550 ☐ Feet

MONITORING WELL LOCATION MAP

FIGURE 3-1

RAMBOLL AMERICAS ENGINEERING SOLUTIONS, INC.

RAMBOLL



HAGARSTOWN MEMBER ELEVATION CONTOUR (1-FT INTERVAL, NAVD88)

■ ■ INFERRED HAGARSTOWN MEMBER ELEVATION CONTOUR

PART 845 REGULATED UNIT (SUBJECT UNIT)

SITE FEATURE

LIMITS OF FINAL COVER

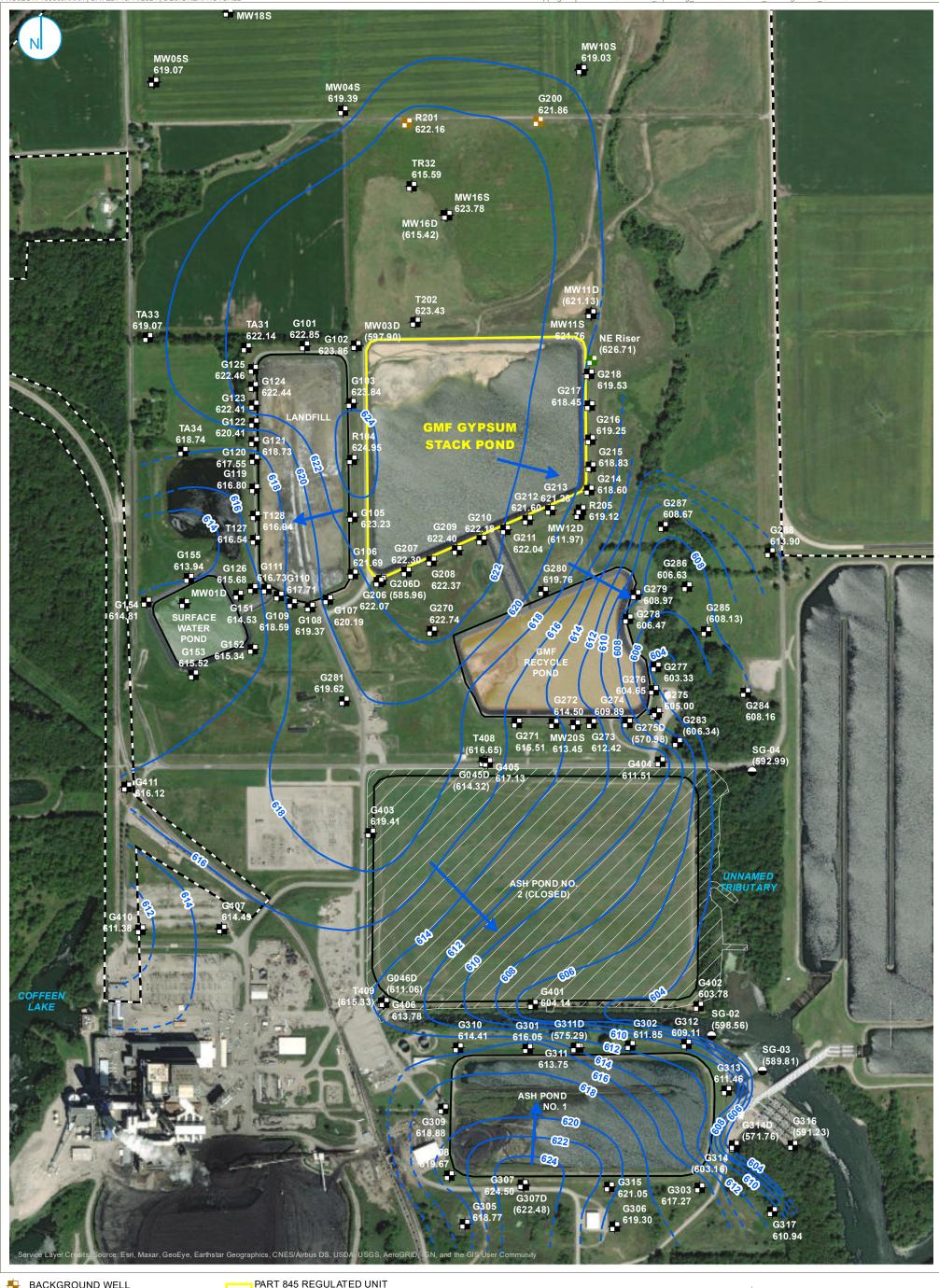
PROPERTY BOUNDARY

TOP OF UPPERMOST AQUIFER

FIGURE 3-2

RAMBOLL AMERICAS ENGINEERING SOLUTIONS, INC.





BACKGROUND WELL

MONITORING WELL

275

SOURCE SAMPLE LOCATION

STAFF GAGE

GROUNDWATER ELEVATION CONTOUR - - -(2-FT CONTOUR INTERVAL, NAVD88)

INFERRED GROUNDWATER ELEVATION CONTOUR

GROUNDWATER FLOW DIRECTION

550

ELEVATIONS IN PARENTHESES WERE NOT USED FOR CONTOURING.

(SUBJECT UNIT)

SITE FEATURE

LIMITS OF FINAL COVER

PROPERTY BOUNDARY

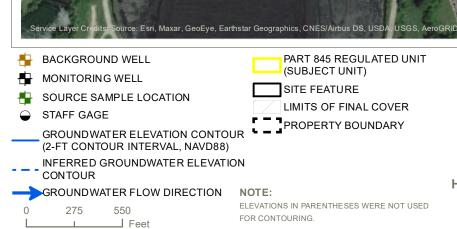
UPPERMOST AQUIFER GROUNDWATER ELEVATION CONTOURS APRIL 20, 2021

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT **GMF GYPSUM STACK POND**

COFFEEN POWER PLANT COFFEEN, ILLINOIS FIGURE 3-3

RAMBOLL AMERICAS ENGINEERING SOLUTIONS, INC.





UPPERMOST AQUIFER GROUNDWATER ELEVATION CONTOURS JULY 26, 2021

G315 620.42

616.44

RAMBOLL AMERICAS ENGINEERING SOLUTIONS, INC.

(606.66)

G317

RAMBOLL

FIGURE 3-4

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT GMF GYPSUM STACK POND

624.60

618.18

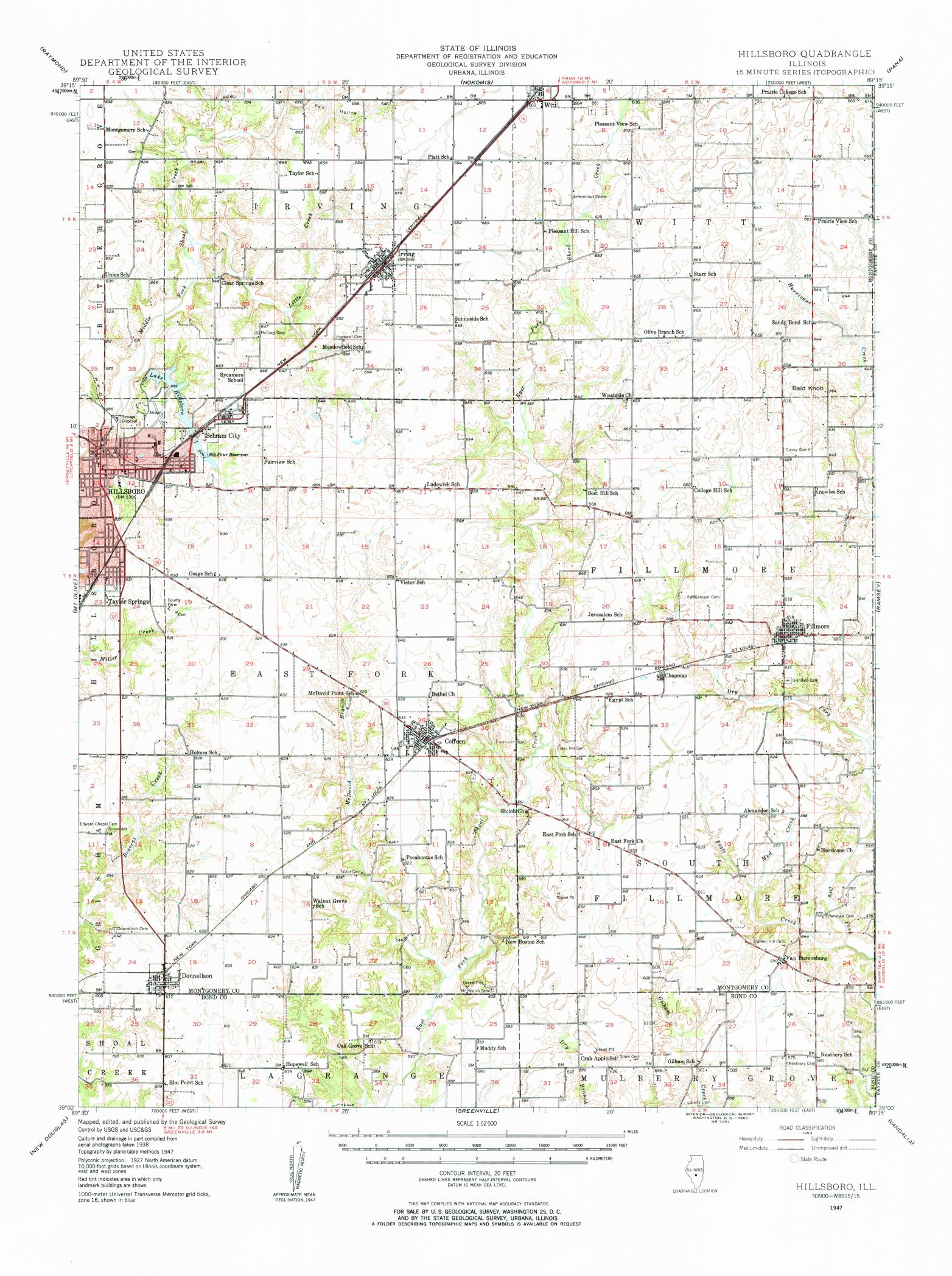
(622.26)

GMF GYPSUM STACK POND

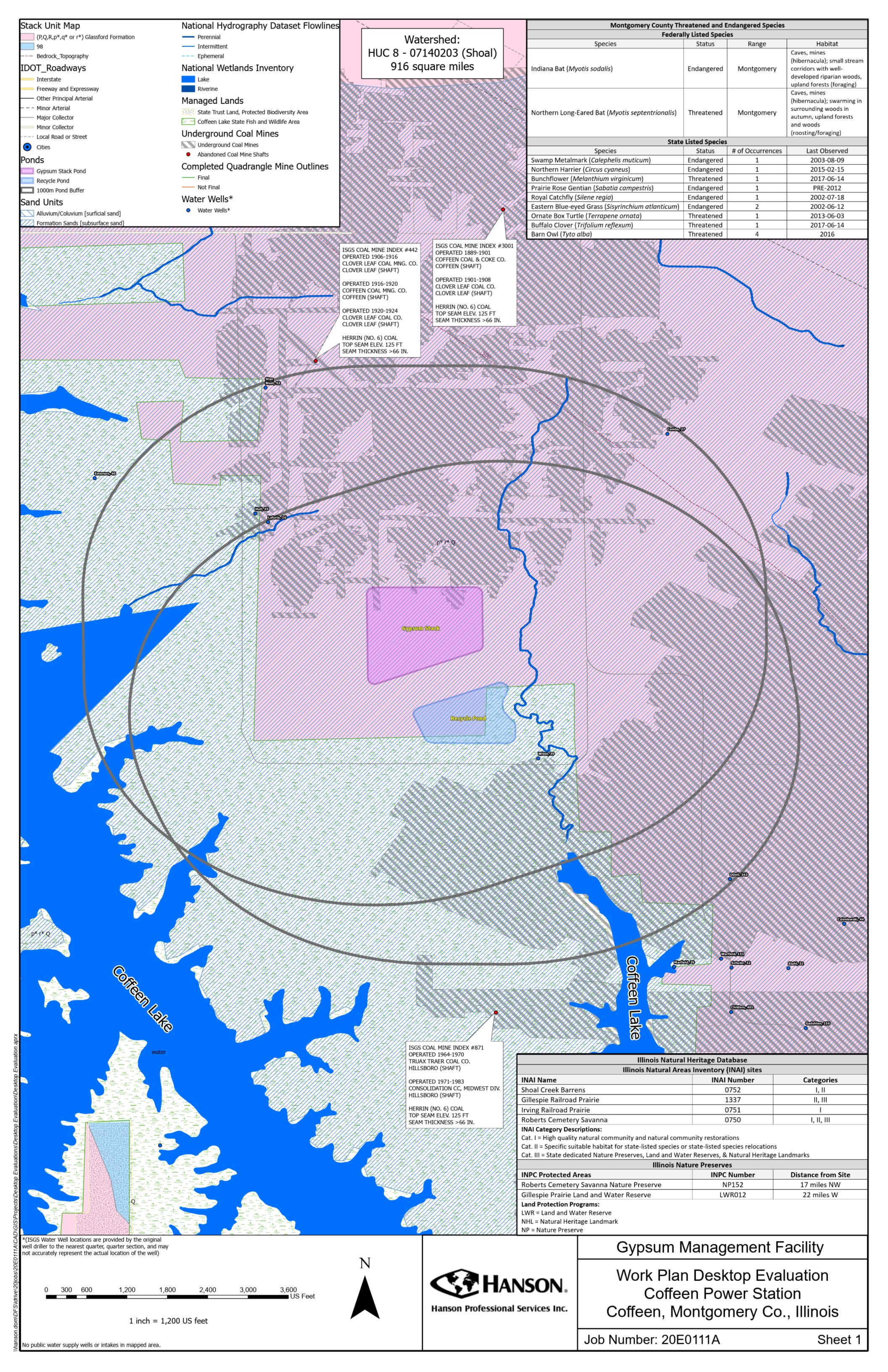
COFFEEN POWER PLANT
COFFEEN, ILLINOIS

APPENDICES

APPENDIX A HISTORIC TOPOGRAPHIC MAP



APPENDIX B INFORMATION PERTINENT TO 35 I.A.C. § 845.220(a)(3)



WATER WELL SURVEY

 $_{\mathtt{Page}-1}$ ILLINOIS STATE GEOLOGICAL SURVEY

Water Well	Тор	Bottom
clay	0	26
Total Depth		26
Casing: 30" CONCRETE from 1' to 26'		
Water from clay at 0' to 0'.		
Driller's Log filed		
Owner Address: ,		
Location source: Location from permit		

Permit Date: Permit #:

COMPANY Bekemeyer, Gust
FARM Lafuria, Mick

DATE DRILLED February 4, 1971 NO.

ELEVATION 0 **COUNTY NO.** 01725

LOCATION NW SE SE

LATITUDE 39.07411 **LONGITUDE** -89.402784

COUNTY Montgomery API 121350172500 3 - 7N - 3W

$_{\mathtt{Page}-1}$ ILLINOIS STATE GEOLOGICAL SURVEY

Private Water Well	Тор	Bottom
brown top soil	0	1
yellow clay	1	7
yellow clay & sand	7	12
yellow gravel & sand	12	14
gray clay & sand	14	35
Total Depth Casing: 36" CONCRETE from -1' to 35' Grout: CONCRETE from 0 to 10. Grout: GRAVEL from 10 to 35.		35
Size hole below casing: 0"		
Water from gravel & sand at 13' to 14'.		
Owner Address: R.R. #1 Box #121 Coffeen, IL Location source: Location from permit		

COMPANY Kohnen, Clarence

FARM Hull, Steve

DATE DRILLED October 17, 1983 NO.

ELEVATION 0 COUNTY NO. 22828

LOCATION 210'N line, 190'W line of SE SE

LATITUDE 39.074433 **LONGITUDE** -89.403283

COUNTY Montgomery API 121352282800 3 - 7N - 3W

$_{\mathtt{Page}-1}$ ILLINOIS STATE GEOLOGICAL SURVEY

Private Water Well	Top	Bottom
brown top soil	0	1
yellow clay	1	8
yellow clay & sand	8	18
yellow gravel & sand	18	30
yellow sand & gravel	30	39
Total Depth Casing: 36" CONCRETE from -1' to 39' " from 0' to 0'		39
Grout: CONCRETE from 0 to 10. Grout: GRAVEL from 10 to 39.		
Size hole below casing: 0"		
Water from at 22' to 39'. Permanent pump installed at 38' on December 1, 1981, with a capacity of 10 gpm		
Owner Address: 109 Laredo Ave. St. Louis, MO Location source: Location from permit		

COMPANY Kohnen, Clarence **FARM** Wibel, William

DATE DRILLED November 30, 1981 NO.

ELEVATION 0 COUNTY NO. 22832

LOCATION 115'S line, 102'W line of SE SE NW **LATITUDE** 39.064493 **LONGITUDE** -89.391801

COUNTY Montgomery API 121352283200 11 - 7N - 3W

 $_{\mathtt{Page}-1}$ ILLINOIS STATE GEOLOGICAL SURVEY

Monitoring	Top	Bottom
clay	0	3
silt	3	33
clay	33	35
Total Depth Casing: 2" SCH 40 PVC from 0' to 23' Screen: 10' of 2" diameter .01 slot Grout: CEMENT/BENT from 0 to 19.		35
Size hole below casing: 7.87"		
Water from at 0' to 10'. Static level 12' below casing top which is 2' above GL		
Owner Address: % White & Brewer Landfill Coffeen, IL Location source: Location from the driller		

COMPANY Fox Drilling **FARM** White & Brewer

DATE DRILLED November 18, 1993 NO. PZ-2

ELEVATION 621GL COUNTY NO. 23615

LOCATION 1470'S line, 300'W line of NE

LATITUDE 39.068212 **LONGITUDE** -89.388821

COUNTY Montgomery API 121352361500 11 - 7N - 3W

$_{\mathtt{Page}-1}$ ILLINOIS STATE GEOLOGICAL SURVEY

Monitoring	Top	Bottom
sandy clay	0	3
silty lcay	3	22
sandy, silty clay	22	23
Total Depth Casing: 2" SCH 40 PVC from 0' to 12' Screen: 10' of 2" diameter .01 slot Grout: CEMENT/BENT 5% from 0 to 12.		23
Size hole below casing: 7.87"		
Water from at 0' to 8'. Static level 10' below casing top which is 2' above GI		
Owner Address: % White & Brewer Landfill Coffeen, IL Location source: Location from the driller		

Permit Date: Permit #: none

COMPANY Fox Drilling
FARM White & Brewer

DATE DRILLED NO. PZ-12

ELEVATION 624GL COUNTY NO. 23618

LOCATION 910'N line, 530'E line of section **LATITUDE** 39.068862 **LONGITUDE** -89.382456

COUNTY Montgomery API 121352361800 11 - 7N - 3W

 $_{\mathtt{Page}-1}$ ILLINOIS STATE GEOLOGICAL SURVEY

0 10 14 29 33	10 14 29
14 29	29
29	
	33
33	1 33
	40
	40

COMPANY Fox Drilling
FARM White & Brewer

DATE DRILLED NO. G-119A

ELEVATION 623GL COUNTY NO. 23619

LOCATION 1060'N line, 730'E line of section **LATITUDE** 39.068452 **LONGITUDE** -89.383158

COUNTY Montgomery API 121352361900 11 - 7N - 3W

 $_{\mathtt{Page}-1}$ ILLINOIS STATE GEOLOGICAL SURVEY

Monitoring	Top	Bottom
clayey silt	0	1
silty clay	1	10
silty sand	10	14
sandy silt	14	20
Total Depth Casing: 2" SCH 40 PVC from 0' to 9' Screen: 10' of 2" diameter .01 slot Grout: CEMENT/BENT 5% from 0 to 4.		20
Size hole below casing: 7.87"		
Water from at 0' to 3'. Static level 9' below casing top which is 2' above GL		
Owner Address: % White & Brewer Landfill Coffeen, IL Location source: Location from the driller		

COMPANY Fox Drilling
FARM White & Brewer

DATE DRILLED NO. G119B

ELEVATION 622GL COUNTY NO. 23620

LOCATION 1080'N line, 750'E line of section **LATITUDE** 39.068397 **LONGITUDE** -89.383229

COUNTY Montgomery API 121352362000 11 - 7N - 3W

 $_{\mathtt{Page}-1}$ ILLINOIS STATE GEOLOGICAL SURVEY

	Top	Bottom
clayey silt	0	4
silty clay	4	18
sandy silt	18	25
sand	25	31
clayey silt	31	33
Total Depth Casing: 2" SCH 5 STAINLESS STL from 0' to 21' Screen: 10' of 2" diameter .01 slot Grout: CEMENT/BENT 5% from 0 to 16.		33
Size hole below casing: 7.87"		
Water from at 0' to 2'. Static level 4' below casing top which is 2' above GL		
Owner Address: % White & Brewer Landfill Coffeen, IL Location source: Location from the driller		

COMPANY Fox Drilling
FARM White & Brewer

DATE DRILLED NO. G-120A

ELEVATION 623GL COUNTY NO. 23621

LOCATION 1990'N line, 730'E line of section **LATITUDE** 39.065884 **LONGITUDE** -89.38313

COUNTY Montgomery API 121352362100 11 - 7N - 3W

 $_{\mathtt{Page}-1}$ ILLINOIS STATE GEOLOGICAL SURVEY

Private Water Well	Top	Bottom
black dirt	0	1
clay	1	3
gravelly clay	3	13
hardpacked gravelly clay & layered gvl	13	19
layered gray gravelly sand & gry gvly cl	19	27
Total Depth Casing: 6" ASTM F480 SDR 21 from 0' to 11' 36" CONCRETE from 11' to 27' Grout: BENTONITE PLUG from 10 to 11. Water from gravelly clay at 11' to 13'.		27
Static level 12' below casing top which is 1' above GL		
Owner Address: 1756 Red Ball Trail Coffeen, IL Address of well: same as above Location source: Location from permit		

Permit Date: June 25, 1998 **Permit #:** 135-001

COMPANY Walters, Steven **FARM** Combs, Jack

DATE DRILLED July 15, 1998 NO.

ELEVATION 0 COUNTY NO. 23873

LOCATION NE NW SE

LATITUDE 39.077695 **LONGITUDE** -89.386518

COUNTY Montgomery API 121352387300 2 - 7N - 3W

 $_{\mathtt{Page}-1}$ ILLINOIS STATE GEOLOGICAL SURVEY

Semi-Private Water Well	Тор	Bottom
coal mine mat coal, concrete, wood, slag	0	5
yellow & gray clay mix	5	7
yellow & gray sandy clay	7	10
gravelly clay	10	17
hardpacked gravelly clay	17	21
gray gravelly clay	21	30
layered gray sand & gray gravelly clay	30	38
gray gravelly clay	38	53
Total Depth Casing: 6" ASTM F480 SDR 21 from 0' to 12' 36" CONCRETE from 0' to 53' Grout: CONCRETE from 9 to 12. Grout: BENT HOLE PLUG from 11 to 12. Water from gravelly clay at 13' to 17'. Static level 16' below casing top which is 3' above GL		53
Owner Address: % Fairmont Minerals 796 CIPS Trail Coffee Address of well: same as above Location source: Location from permit	een, IL	

Permit Date: June 9, 1999 **Permit #:** 135-001

COMPANY Walters, Steven **FARM** Stan Blast Abrasives

DATE DRILLED June 14, 1999 NO. 1

ELEVATION 0 COUNTY NO. 23908

LOCATION SW SE NE

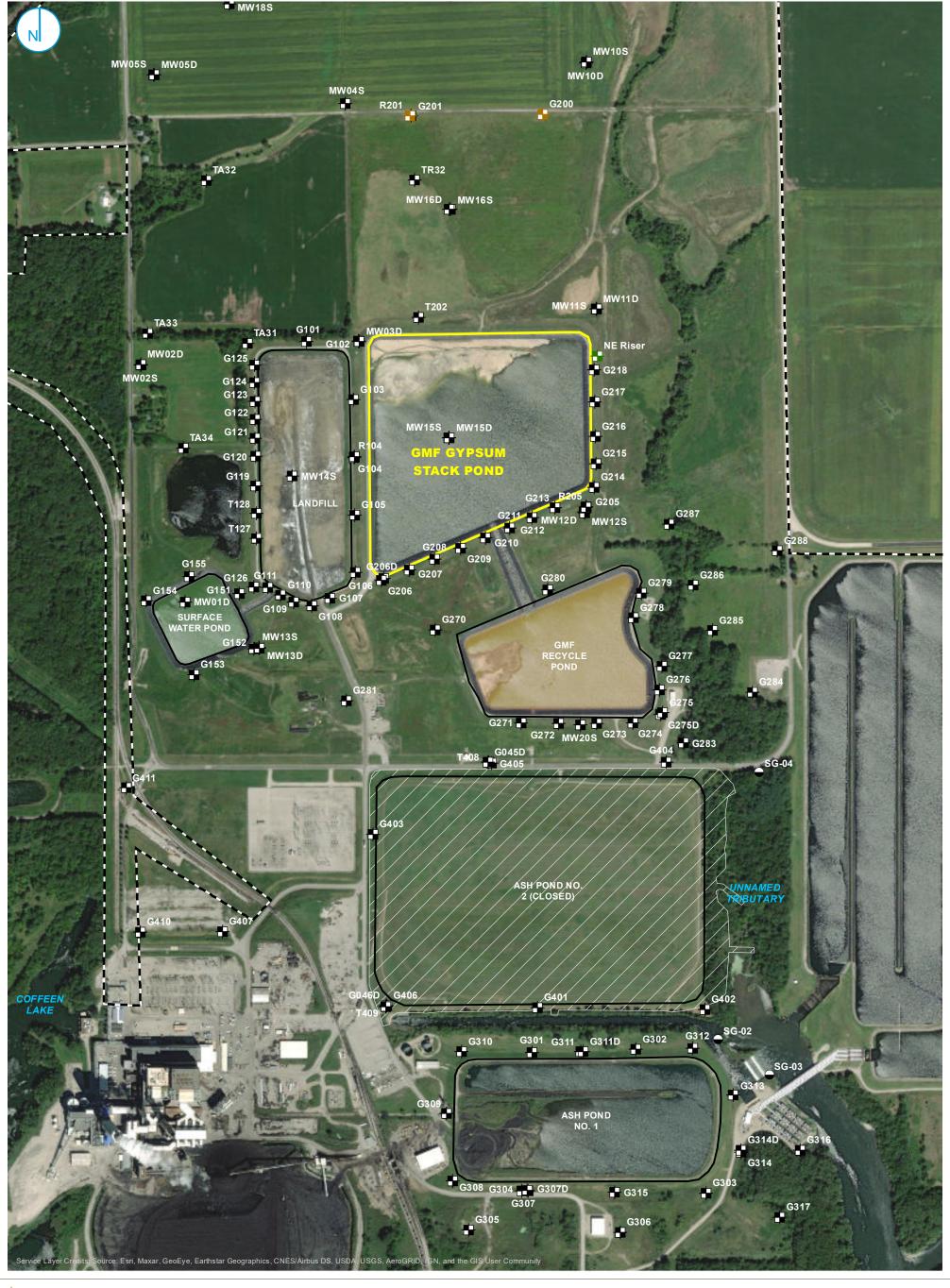
LATITUDE 39.079557 **LONGITUDE** -89.402861

COUNTY Montgomery API 121352390800

3 - 7N - 3W

APPENDIX C BORING AND WELL CONSTRUCTION LOGS

BORING AND WELL LOCATIONS MAP



BACKGROUND WELL

MONITORING WELL

SOURCE SAMPLE LOCATION

STAFF GAGE

PART 845 REGULATED UNIT (SUBJECT UNIT)

SITE FEATURE

LIMITS OF FINAL COVER

PROPERTY BOUNDARY 550 ☐ Feet

MONITORING WELL LOCATION MAP

FIGURE C1

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT **GMF GYPSUM STACK POND**

COFFEEN POWER PLANT COFFEEN, ILLINOIS



ENGINEERING SOLUTIONS, INC.

RAMBOLL AMERICAS

BORING LOGS

CLIENT: Natural Resources Technology, Inc. Site: Coffeen Power Station - Ash Pond 2 Location: 134 CIPS Lane, Coffeen, IL 62017

Project: 16E0080

DATES: Start: 8/16/2016 Finish: 8/17/2016

WEATHER: Cloudy, rain, (hi-70s)

CONTRACTOR: Bulldog Drilling, Inc. Rig mfg/model: CME-750 ATV Drill Drilling Method: 4 1/4" Hollow Stem Auger

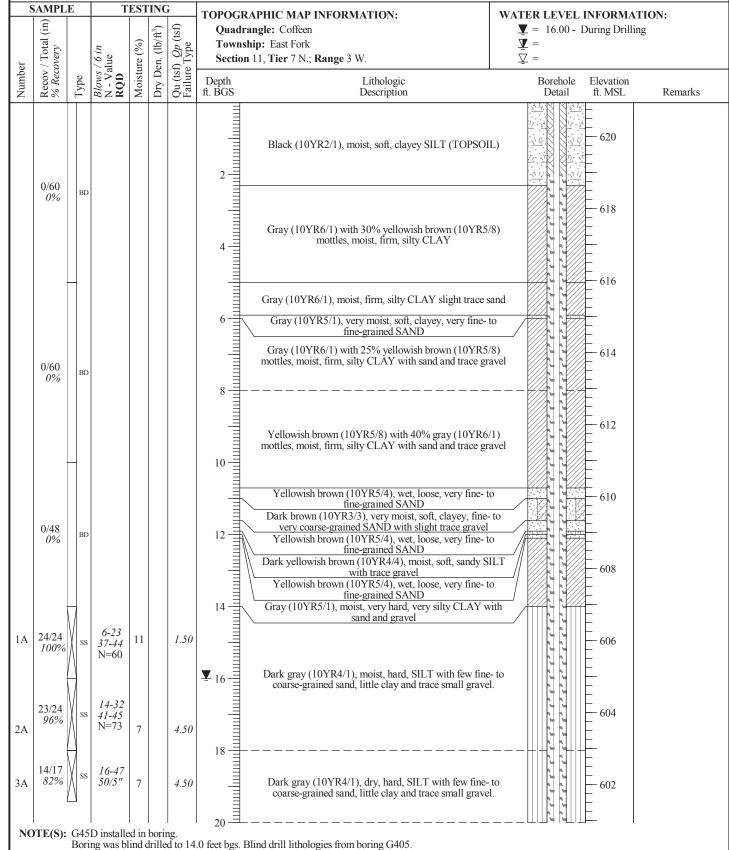
FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill

Eng/Geo: K. Theesfeld



BOREHOLE ID: G405D Well ID: G45D

> Surface Elev: 620.94 ft. MSL Completion: 42.00 ft. BGS **Station:** 873,998.03N 2,515,322.23E



CLIENT: Natural Resources Technology, Inc.
Site: Coffeen Power Station - Ash Pond 2
Location: 134 CIPS Lane, Coffeen, IL 62017

Project: 16E0080

DATES: Start: 8/16/2016 **Finish:** 8/17/2016

WEATHER: Cloudy, rain, (hi-70s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" Hollow Stem Auger

FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill

Helper: M. Hill Eng/Geo: K. Theesfeld **HANSON**

BOREHOLE ID: G405D **Well ID:** G45D

 Surface Elev:
 620.94 ft. MSL

 Completion:
 42.00 ft. BGS

 Station:
 873,998.03N

 2,515,322.23E

	AMPLE TESTING					TOPOGRAPHIC M	AP INFORMATION:	WATER LEVEL INFORMATION:			
Recov / Total (in)		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Cof Township: East F Section 11, Tier 7	ork	$\underline{\Psi}$ = 16.00 - During Drilling $\underline{\Psi}$ = $\underline{\nabla}$ =			
Recov / % Recor	Type	Blows N - V. RQD	Moist	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL Rema	arks		
19/24 79%	ss	15-36 32-36 N=68	8		4.50	coars	ray (10YR4/1), dry, hard, SILT with few fine- e-grained sand, little clay and little small grave by (10YR4/1), moist, dense, fine- to coarse-gra SAND.	el. / [Side]			
A 19/24 79%	ss	10-18 30-33 N=48	8		4.50	Dark g	ray (10YR4/1), dry, hard, SILT with few fine- e-grained sand, little clay and little small grave				
A 11/24 46%	ss	22-42 34-36 N=76	8		4.50	26		596			
7A 11/24 46%	SS	28-26 23-26 N=49	11		4.50	-∃ (10Y	gray (10YR4/1) with 5% light brownish gray (6/2) mottles, dry, hard, SILT with few fine- tegrained sand, little clay and little small grave	to 594			
3A 23/24 96%	ss	7-11 16-25 N=27	10		4.50	Darl (10YR6, hard, SII	gray (10YR4/1) with 5% light brownish gray (2) and dark greenish gray (10YR4/2) mottles, T with few fine- to coarse-grained sand, little and little small gravel.	s, dry, e clay 592			
OA 17/24	ss	7-14 12-12 N=26	16 14 9			Coars Dark gra	ray (10YR4/1), dry, hard, SILT with few fine- e-grained sand, little clay and little small grave by (10YR4/1), dry, very stiff, SILT with some fine-grained SAND. ark gray (10YR3/1), moist, very stiff, CLAY we few silt and little medium-grained sand.	el. very 590			
24/24 100%	SS	2-5 8-13 N=13	15		3.25		ark gray (10YR3/1), moist, very stiff, CLAY w	with 588			
1-1 1-2 22/24 92% 1-3 1-4	SH		15			34 Very da	little silt and little medium-grained sand.	586			
24/24 100%	SS	2-5 8-10 N=13	16		2.00	Very da little silt,	ark gray (10YR3/1), moist, very stiff, CLAY was little medium-grained sand, and trace small gr	with ravel.			
3A 22/24 92%	ss	2-5 7-8 N=12	16		2.00	mott	dark gray (10YR3/1) with 5% black (10YR2/les, moist, very stiff, CLAY with little silt, little edium-grained sand, and trace small gravel.				

CLIENT: Natural Resources Technology, Inc. Site: Coffeen Power Station - Ash Pond 2 Location: 134 CIPS Lane, Coffeen, IL 62017

Project: 16E0080

DATES: Start: 8/16/2016 Finish: 8/17/2016

WEATHER: Cloudy, rain, (hi-70s)

CONTRACTOR: Bulldog Drilling, Inc. Rig mfg/model: CME-750 ATV Drill **Drilling Method:** 4 1/4" Hollow Stem Auger

FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill

Eng/Geo: K. Theesfeld

HANSON

BOREHOLE ID: G405D Well ID: G45D

> Surface Elev: 620.94 ft. MSL **Completion:** 42.00 ft. BGS **Station:** 873,998.03N 2,515,322.23E

	SAMPLE TESTING							TOPOGRA	PHIC MAP INFORMATION:	WATER LEVEL INFORMATION:			
١		1 (in)				/ft³)	(tsf)	Quadran	gle: Coffeen	T = 16.00 - 1	During Drillin		
	er	/ Total overy		/6 in ılue	ture (%)	Den. (lb)	$O_{\rm Type}$		p: East Fork 1, Tier 7 N.; Range 3 W.	$\overline{\Lambda} = \overline{\Lambda}$			
	Number	Recov % Rec	Type	Blows N - Va RQD	Moist	Dry Do	Qu (tsf) Failure	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
	14A	23/24 96%	SS	1-3 7-7 N=10	16		3.50	42	Very dark gray (10YR3/1) with 5% black (10YR2/1 mottles, moist, very stiff, CLAY with little silt, little medium-grained sand, and trace small gravel. [Continued from previous page]		580		
П								72	End of Boring = 42.0 ft. BGS				

CLIENT: Natural Resources Technology, Inc.

Site: Coffeen Energy Center Location: Coffeen, Illinois

Project: 15E0030 **DATES: Start:** 5/1/2007

Finish: 5/1/2007 **WEATHER:** Partly sunny, warm

CONTRACTOR: Reynolds Drilling Corp. **Rig mfg/model:** CME-550 ATV Drill

Drilling Method: 41/4" Hollow stem auger with split spoon

sampler

FIELD STAFF: Driller: A. Rachford Helper: M. Brown

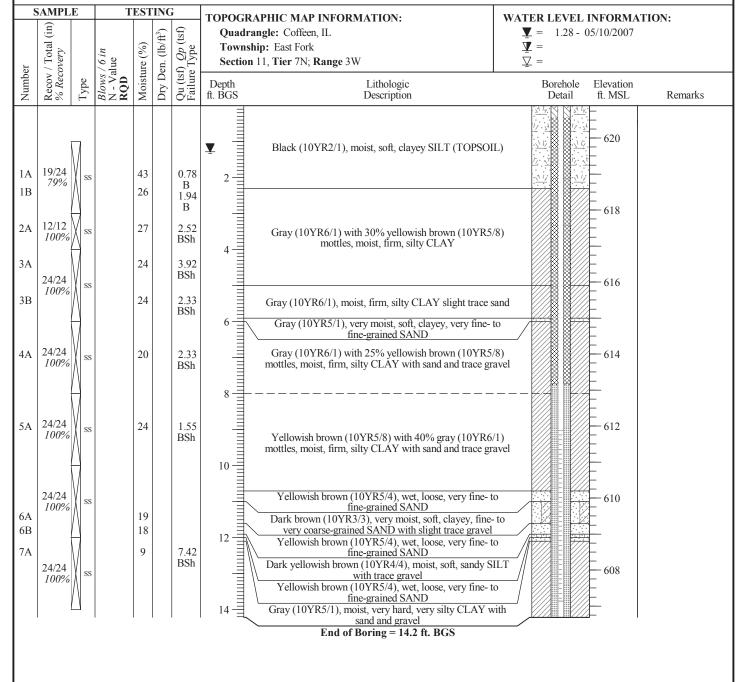
Eng/Geo: R. Hasenyager



BOREHOLE ID: SB21 Formerly MW21S

Well ID: G405

Surface Elev: 620.90 ft. MSL Completion: 14.21 ft. BGS Station: 873,996.79N 2,515,335.70E



CLIENT: Natural Resources Technology, Inc.
Site: Coffeen Power Station - Ash Pond 2
Location: 134 CIPS Lane, Coffeen, IL 62017

Project: 16E0080

DATES: Start: 8/19/2016

Finish: 8/19/2016 WEATHER: Sunny, (mid-70s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" Hollow Stem Auger

FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill

Eng/Geo: K. Theesfeld



BOREHOLE ID: G406D **Well ID:** G46D

 Surface Elev:
 621.91 ft. MSL

 Completion:
 52.00 ft. BGS

 Station:
 872,519.70N

 2,514,697.78E

- 1					ING		TOPOGRAPHIC MAP INFORMATION: WATER	WATER LEVEL INFORMATION:				
	Recov / Total (in) % Recovery		/ 6 in lue	re (%)	Dry Den. (lb/ft³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen $\underline{\underline{\mathbf{Y}}}$ =Township: East Fork $\underline{\underline{\mathbf{Y}}}$ =Section 11, Tier 7 N.; Range 3 W. $\underline{\underline{\mathbf{Y}}}$ =	Dry - 1	Ouring Drilling			
	Recov % Recc	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry De	Qu (tsf Failure	Depth Lithologic I ft. BGS Description	Borehole Detail	Elevation ft. MSL	Remarks		
A				11		1.50	Dark brown (10YR3/3), moist, stiff, SILT with few clay and trace organics.					
В	18/24 75%	SS	4-3 4-6 N=7	15		3.00	□ □ □		620			
A	12/24 50%	ss	4-6 3-3 N=9	12		2.50	mottles, SILT with some clay and trace small gravel.	,,,,,	_ _ _ _			
A A	3/24 13%	ss	3-3 4-7 N=7	18 24		2.00	Brown (10YR5/3) with 10% dark brown (10YR3/3) mottles, SILT with some clay. Grayish brown (10YR5/2) with 5% dark yellowish brown (10YR4/6) mottles, moist, stiff, CLAY with few silt and little fine-grained sand. Very pale brown (10YR7/4) with 25% yellowish brown (10YR5/6) mottles, moist, medium, CLAY with trace silt.		618			
A	20/24 83%	ss	2-3 4-5 N=7	21		1.25	Brown (10YR5/3) with 10% dark brown (10YR3/3) mottles, SILT with some clay and trace small gravel. Brown (10YR5/3) with 10% dark brown (10YR3/3) mottles, SILT with some clay. Grayish brown (10YR5/2) with 5% dark yellowish brown (10YR4/6) mottles, moist, stiff, CLAY with few silt and little fine-grained sand. Very pale brown (10YR7/4) with 25% yellowish brown (10YR5/6) mottles, moist, medium, CLAY with trace silt. Gray (10YR5/1) with 10% yellowish brown (10YR5/6) and 5% very dark gray (10YR3/1) mottles, moist, stiff, CLAY with little silt, little fine- to medium-grained sand and trace small gravel. Gray (10YR5/1) with 10% yellowish brown (10YR5/6) and 5% very dark gray (10YR3/1) mottles, moist, stiff, CLAY with little silt, little fine- to medium-grained sand and trace small gravel. Gray (10YR5/1) with 10% yellowish brown (10YR5/6) and 5% very dark gray (10YR3/1) mottles, moist, stiff, CLAY with few fine- to medium-grained sand, little silt, and trace small gravel. Gray (10YR6/1) with 25% brownish yellow (10YR6/8) and 5% strong brown (7.5YR4/6) mottles, moist, stiff, CLAY with few fine- to medium-grained sand, little silt, and trace small gravel.		616			
A	19/24 79%	ss	1-3 4-6 N=7	18		1.75	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) and 5% very dark gray (10YR3/1) mottles, moist, stiff, CLAY with little silt, little fine- to medium-grained sand and trace small gravel.		612			
δA	23/24 96%	ss	2-2 4-5 N=6	18		2.50	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) and 5% very dark gray (10YR3/1) mottles, moist, stiff, CLAY with few fine- to medium-grained sand, little silt, and trace small gravel.		- 012			
'A	21/24 88%	ss	1-3 4-5 N=7	16		1.00	\exists		610			
SA	23/24 96%	ss	1-2 2-2 N=4	18		0.75	Gray (10YR6/1) with 10% brownish yellow (10YR6/8) and 5% strong brown (7.5YR4/6) mottles, moist, stiff, CLAY with some fine- to medium-grained sand, little silt, and trace small gravel. Gray (10YR6/1) with 5% brownish yellow (10YR6/6) mottles, wet, loose, fine-grained SAND with some clay.		608			
ВВ				17		0.75	Gray (10YR6/1) with 5% brownish yellow (10YR6/6) mottles, wet, loose, fine-grained SAND with some clay		606			
A	22/24 92%	ss	4-13 27-23 N=40	8			Gray (10YR6/1) with 5% brownish yellow (10YR6/6) mottles, moist, loose, fine-grained SAND with some clay.					
В				8			18		604			
)A	17/24 71%	ss	13-31 33-42 N=64	7		4.50	Gray (10YR6/1) with 5% brownish yellow (10YR6/6) mottles, moist, loose, fine-grained SAND with some clay. Gray (10YR5/1), dry, hard, SILT with few clay, few fine-to coarse-grained sand and trace small gravel.	,,,,,,,	- - - - - - - - - - - - - -			

CLIENT: Natural Resources Technology, Inc. Site: Coffeen Power Station - Ash Pond 2 Location: 134 CIPS Lane, Coffeen, IL 62017

Project: 16E0080

DATES: Start: 8/19/2016 **Finish:** 8/19/2016

WEATHER: Sunny, (mid-70s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" Hollow Stem Auger

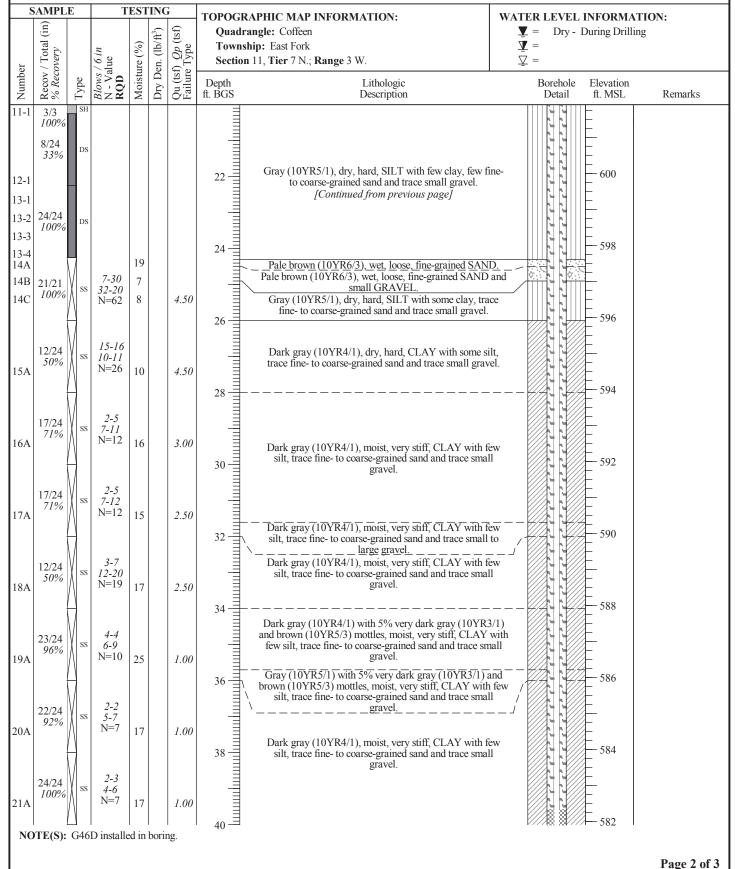
FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill

Eng/Geo: K. Theesfeld



BOREHOLE ID: G406D **Well ID:** G46D

Surface Elev: 621.91 ft. MSL Completion: 52.00 ft. BGS Station: 872,519.70N 2,514.697.78E



CLIENT: Natural Resources Technology, Inc. Site: Coffeen Power Station - Ash Pond 2 Location: 134 CIPS Lane, Coffeen, IL 62017

Project: 16E0080

DATES: Start: 8/19/2016 **Finish:** 8/19/2016

WEATHER: Sunny, (mid-70s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" Hollow Stem Auger

FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill

Eng/Geo: K. Theesfeld

HANSON

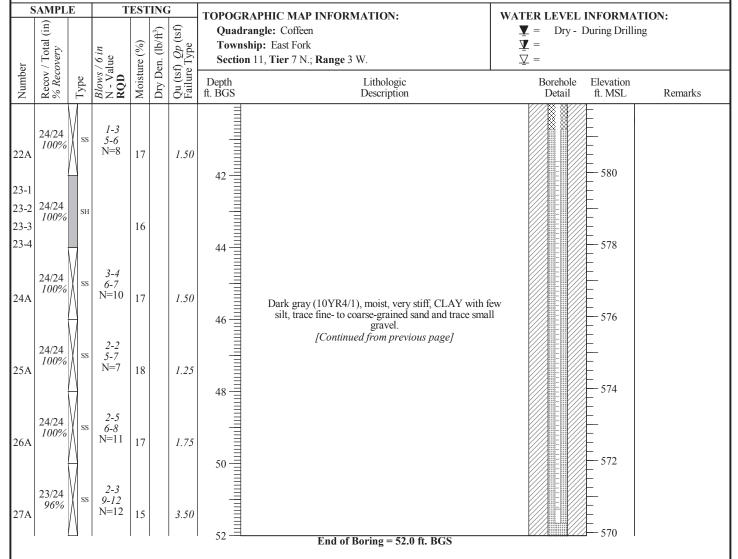
BOREHOLE ID: G406D **Well ID:** G46D

 Surface Elev:
 621.91 ft. MSL

 Completion:
 52.00 ft. BGS

 Station:
 872,519.70N

 2,514,697.78E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A **DATES: Start:** 2/2/2010

Finish: 2/2/2010

WEATHER: Overcast, cold (lo-30's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill **Drilling Method:** 41/4" HSA w/SS samplers

FIELD STAFF: Driller: T. List

Helper: M. Herbst/S. Hamby Eng/Geo: S. Suzanna Simpson

HANSON

BOREHOLE ID: G101 Well ID: G101

Surface Elev: 625.27 ft. MSL Completion: 21.92 ft. BGS **Station:** 876,551.76N 2,514,214.31E

S	SAMPLE TESTING				INC		TOPOGRAPHIC MAP INFORMATION:	WATER LEVEL INFORMATION:				
5	Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 10, Tier 7N; Range 3W	$\underline{\nabla}$ = 15.50 - While drilling $\underline{\nabla}$ = 12.38 - Upon Completion $\underline{\nabla}$ = 7.31 - 3/1/2010				
Indilibel	Recov % Re	Type	Blows N - V RQD	Moist	Dry D	Qu (ts Failur	Depth Lithologic ft. BGS Description	Borehole Elevation Detail ft. MSL Remark				
A	18/24 75%	ss	1-1 2-3 N=3	24			TOPSOIL - Brown (10YR5/3), moist, soft, silty CL with slight trace sand and gravel, roots. Dark grayish brown (10YR4/2), moist, soft, silty CL with slight trace sand, trace roots.					
A	18/24 75%	ss	1-3 3-5 N=6	30			Dark grayish brown (10YR4/2) with 15% yellowish to (10YR5/4) mottles, moist, medium, silty CLAY, slitrace roots. Grayish brown (10YR5/2) with 15% yellowish brown (10YR5/4) mottles, moist, medium, silty CLAY, slitrace roots.	light / Day of the control of the co				
A	19/24 79%	ss	2-3 4-4 N=7	26			Gray (10YR5/1) with 25% yellowish brown (10YR mottles, moist, medium, silty CLAY, slight trace ro	25/6) 620				
A	19/24 79%	ss	1-3 4-3 N=7	21			Gray (10YR5/1) with 10% yellowish brown (10YR mottles, moist, medium, silty CLAY with slight trace	25/6) 618				
5A	22/24 92%	ss	1-3 3-4 N=6	23			Gray (10YR5/1) with 10% yellowish brown (10YR mottles, moist, medium, silty CLAY with trace sand slight trace gravel.	25/6) 1 and				
5A	20/24 83%	ss	1-2 2-3 N=4	24			Gray (10YR6/1) with 35% yellowish brown (10YR mottles, moist, soft, silty CLAY with trace sand and strace gravel.	85/6) slight				
7A	22/24 92%	ss	1-2 3-2 N=5	17			Gray (10YR5/1) with 25% yellowish brown (10YR mottles, very moist, medium, silty, sandy CLAY with	13/6) n trace 612				
BA BB	14/24 58%	SS	1-2 5-8 N=7	15 13			Brown (10YR5/3), very moist, medium, silty, clayey, fine- to coarse-grained SAND with slight trace grav Brown (10YR5/3), very moist, loose, silty, very fine coarse-grained SAND with slight trace gravel.	vel610				
9A	16/24 67%	ss	2-5 15-25 N=20	16			Brown (10YR5/3), very moist, medium dense, silty,	very 608				
0A 0B	17/24 71%	SS	19-20 22-18 N=42	14			Brown (10YR5/3), very moist, dense, silty, very fine coarse-grained SAND with slight trace gravel.	le- to				

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 2/2/2010 **Finish:** 2/2/2010

WEATHER: Overcast, cold (lo-30's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 41/4" HSA w/SS samplers

FIELD STAFF: Driller: T. List

Helper: M. Herbst/S. Hamby **Eng/Geo:** S. Suzanna Simpson

BOREHOLE ID: G101 **Well ID:** G101

Surface Elev: 625.27 ft. MSL Completion: 21.92 ft. BGS Station: 876,551.76N

HANSON

2,514,214.31E

	SAMPL	E	T	EST	INC	j	TOPOGRAF	PHIC MAP INFORMATION:	WATER LEVEL INFORMATION:
er	/ Total (in)		/ 6 in Ilue	ture (%)	en. (lb/ft³)	f) Qp (tsf) a Type	Quadrang Township	gle: Coffeen, IL : East Fork), Tier 7N; Range 3W	$\underline{\Psi}$ = 15.50 - While drilling $\underline{\Psi}$ = 12.38 - Upon Completion $\underline{\nabla}$ = 7.31 - 3/1/2010
Number	Recov % Rec	Type	Blows N - Va RQD	Moist	Dry D	Qu (tsf) Failure	Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL Remarks
11A	16/23 70%	ss	2-16 42-60/5" N=58	8				Brown (10YR5/3), slightly moist, hard, clayey SILT v slight trace sand and gravel. Dark gray (10YR4/1), slightly moist, hard, clayey SII with slight trace sand and slight trace gravel. End of Boring = 21.9 ft, BGS	

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A **DATES: Start:** 4/28/2006

Finish: 4/28/2006 WEATHER: Partly cloudy, mild (mid-60's) CONTRACTOR: Testing Service Corp. Rig mfg/model: CME-650 Track Drill Drilling Method: 4½" HSA (blind drill)

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

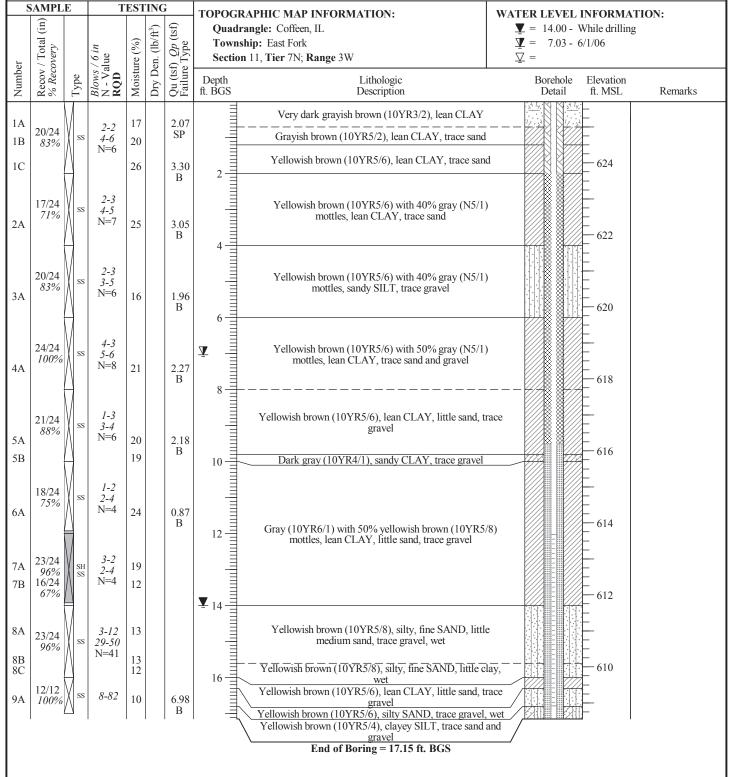
Eng/Geo: R. Hasenyager



BOREHOLE ID: SB-03a

Well ID: G102 (MW3S)
Surface Elev: 625.70 ft. MSL
Completion: 17.15 ft. BGS
Station: 876.554.77N

2,514,531.48E



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 4/27/2006 Finish: 4/27/2006

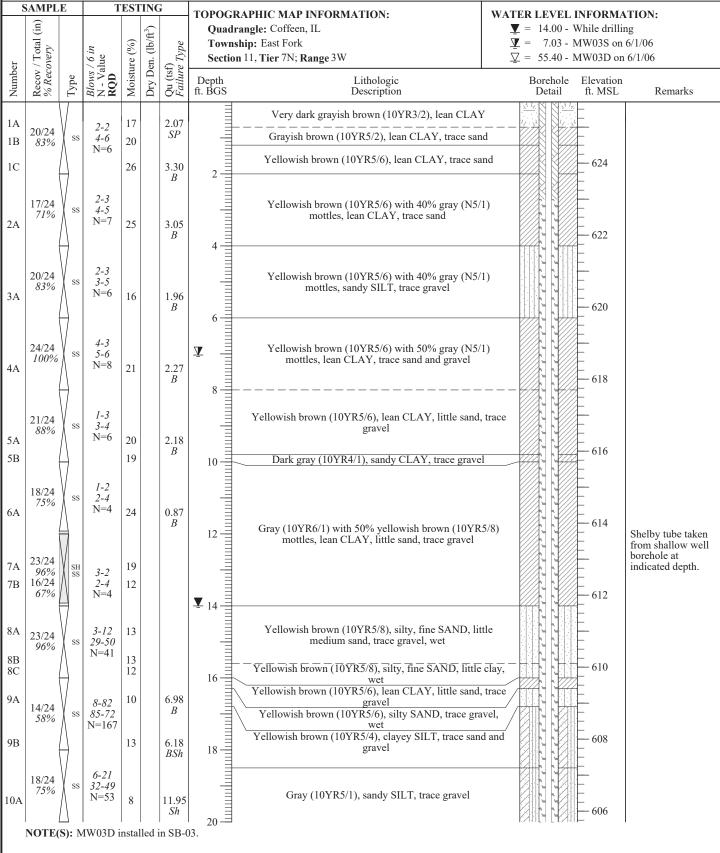
WEATHER: Sunny, mild (high-50's)

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS sampler & 41/4" HSA overdrill

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager



HANSON

626 ft. MSL

58 ft. BGS

2,514,535.3E

876,554.5N

BOREHOLE ID: SB-03

Surface Elev:

Completion:

Station:

Well ID: MW3D

CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois
Project: 05S3004A

DATES: Start: 4/27/2006

Finish: 4/27/2006

Finish: 4/27/2006

WEATHER: Sunny, mild (high-50's)

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS sampler & 41/4" HSA overdrill

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

DODEWOLE ID. CD. CO.

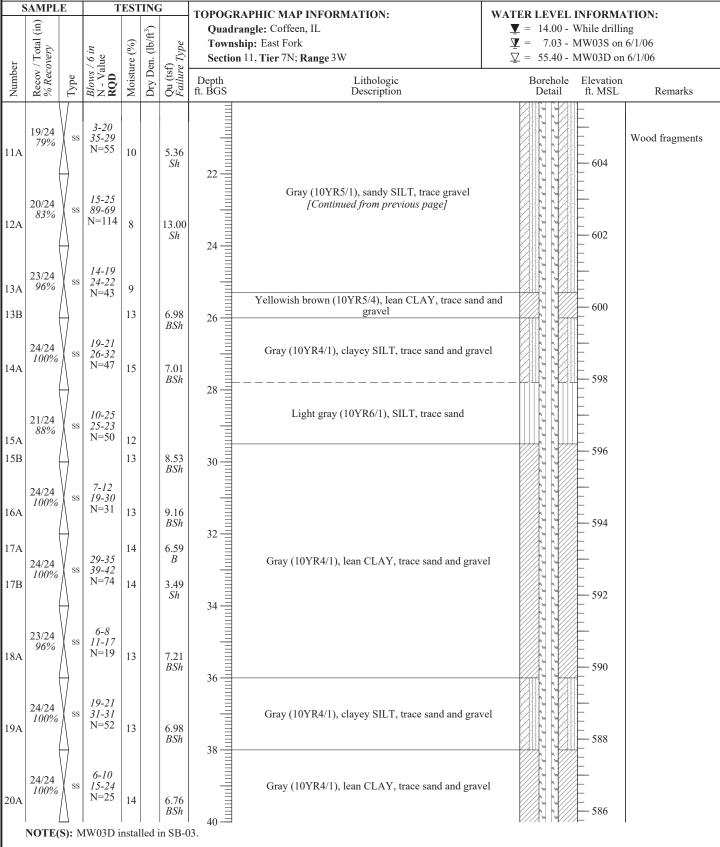
BOREHOLE ID: SB-03 Well ID: MW3D

 Surface Elev:
 626 ft. MSL

 Completion:
 58 ft. BGS

 Station:
 876,554.5N

 2,514,535.3E



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois
Project: 05S3004A

DATES: Start: 4/27/2006
Finish: 4/27/2006

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS sampler & 41/4" HSA overdrill

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy Eng/Geo: R. Hasenyager **HANSON**

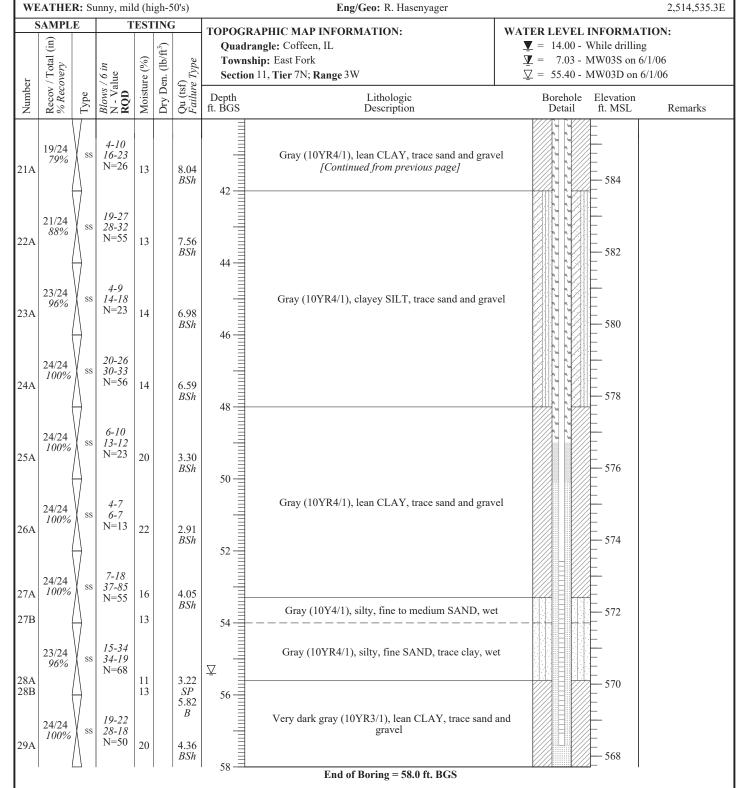
BOREHOLE ID: SB-03 Well ID: MW3D

 Surface Elev:
 626 ft. MSL

 Completion:
 58 ft. BGS

 Station:
 876,554.5N

 2,514,535.3E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois

Project:

DATES: Start: 2/15/2010

Finish: 2/15/2010

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill **Drilling Method:** 41/4" HSA w/SS samplers

FIELD STAFF: Driller: T. List

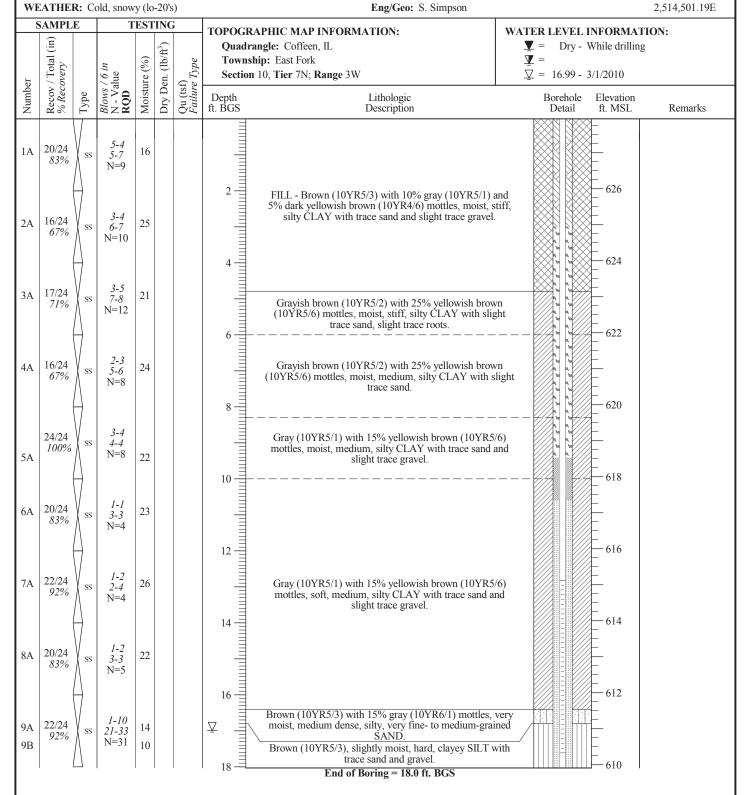
Helper: M. Herbst/S. Hamby

BOREHOLE ID: G103

Well ID: G103

Surface Elev: 627.94 ft. MSL Completion: 18.03 ft. BGS **Station:** 876,199.48N

HANSON



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A **DATES:** Start: 2/15/2010

Finish: 2/15/2010
WEATHER: Overcast, cold, windy (lo-20's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 41/4" HSA w/SS samplers

FIELD STAFF: Driller: T. List

Helper: M. Herbst/S. Hamby **Eng/Geo:** S. Suzanna Simpson

HANSON

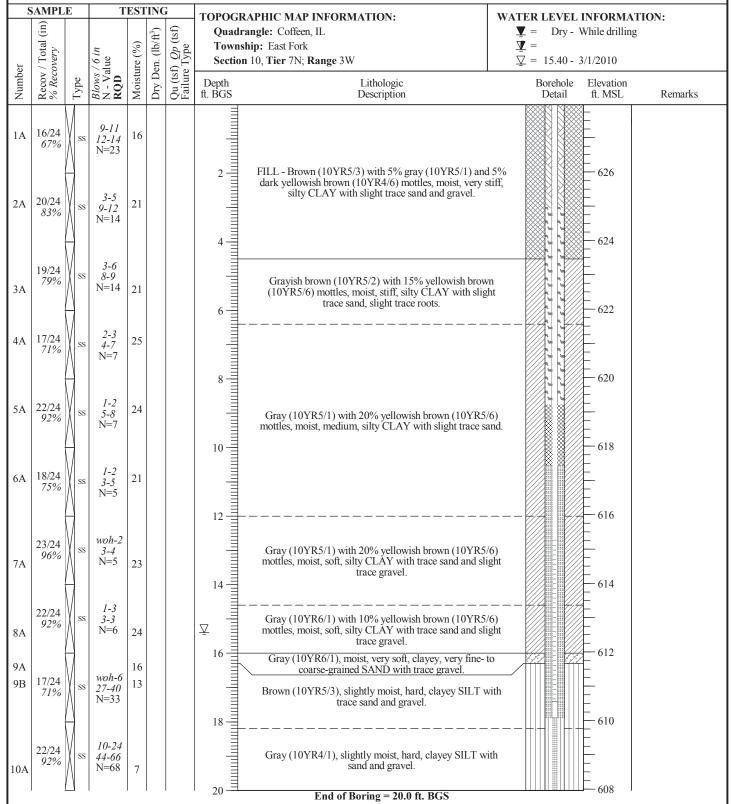
BOREHOLE ID: G104 **Well ID:** G104

 Surface Elev:
 627.96 ft. MSL

 Completion:
 20.00 ft. BGS

 Station:
 875,849.26N

 2.514,504.98E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois

Project:

DATES: Start: 2/16/2010

Finish: 2/16/2010

WEATHER: Sunny, cold, windy (mid-20's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill

Drilling Method: 41/4" HSA w/SS samplers

FIELD STAFF: Driller: T. List

Helper: M. Herbst/S. Hamby

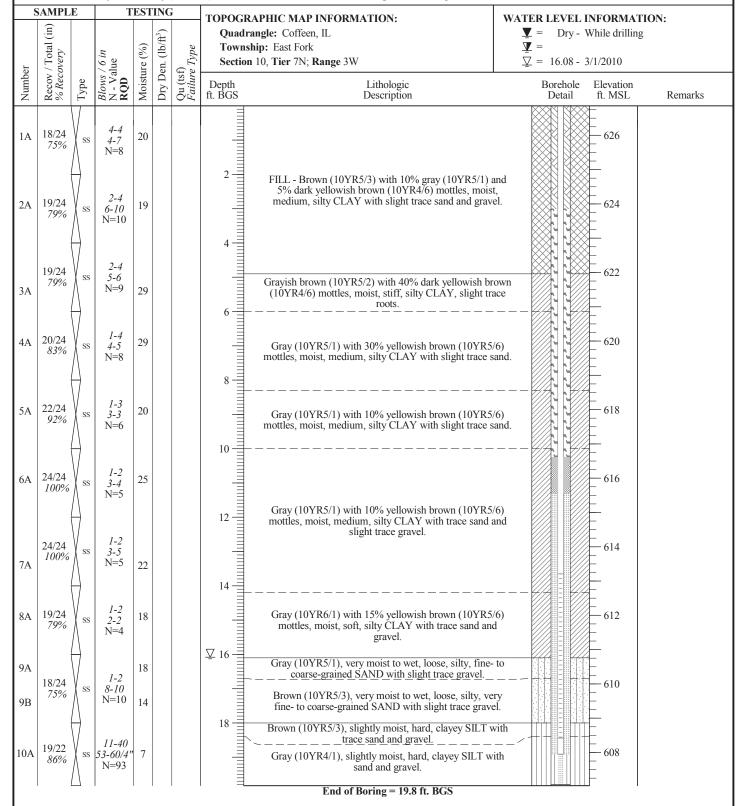
Eng/Geo: S. Simpson

HANSON

BOREHOLE ID: G105 Well ID: G105

Surface Elev: 626.86 ft. MSL Completion: 19.83 ft. BGS

Station: 875,499.70N 2,514,509.15E



NOTE(S): Well completed prior to construction of berm road. Boring surface elevation is as of the well install date and not the final constructed elevation.

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A **DATES: Start:** 2/16/2010

Finish: 2/16/2010

WEATHER: Overcast, cold, windy (mid-20's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 4½" HSA w/SS samplers

FIELD STAFF: Driller: T. List

Helper: M. Herbst/S. Hamby **Eng/Geo:** S. Suzanna Simpson

HANSON

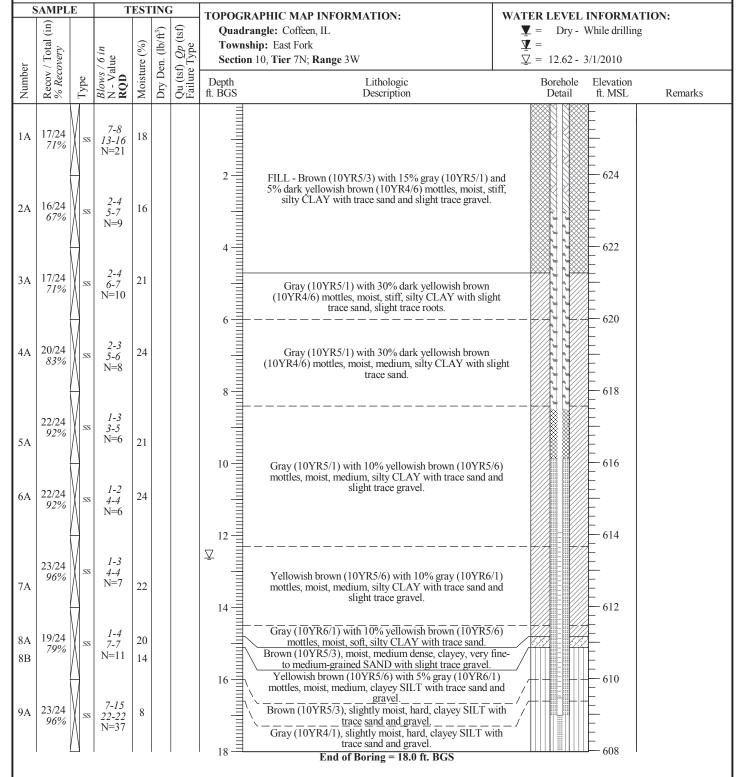
BOREHOLE ID: G106 **Well ID:** G106

 Surface Elev:
 625.96 ft. MSL

 Completion:
 18.00 ft. BGS

 Station:
 875,149.76N

 2.514.512.79E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois

Project:

DATES: Start: 2/17/2010

Finish: 2/17/2010

WEATHER: Overcast, cold, windy (mid-20's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill

Drilling Method: 41/4" HSA w/SS samplers

FIELD STAFF: Driller: T. List

Helper: M. Herbst/S. Hamby

Eng/Geo: S. Simpson

HANSON

BOREHOLE ID: G107 Well ID: G107

> Surface Elev: 627.11 ft. MSL **Completion:** 20.00 ft. BGS **Station:** 874,994.33N

> > 2,514,358.25E

Remarks
Remarks

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois

Project:

DATES: Start: 2/12/2010

WEATHER: Overcast, cold ~25F

Finish: 2/12/2010

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 4'/" HSA w/SS samplers

FIELD STAFF: Driller: T. List

Helper: M. Herbst/S. Hamby

Eng/Geo: D. Lamb

HANSON

BOREHOLE ID: G108 **Well ID:** G108

 Surface Elev:
 625.58 ft. MSL

 Completion:
 20.00 ft. BGS

 Station:
 874,948.81N

2,514,248.25E

SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Œ, Quadrangle: Coffeen, IL $\mathbf{Y} = 14.50$ - While drilling Dry Den. (lb/ft3 Recov / Total (% Recovery Township: East Fork Moisture (%) TypeBlows / 6 in N - Value RQD Section 10, Tier 7N; Range 3W ∇ = 8.93 - 3/1/2010 Qu (tsf) Failure 1 Number Elevation Lithologic Borehole Description Detail ft. MSL Remarks 24-25 23/24 SS 13-13 96% FILL - Grayish brown (10YR5/2) with 10% yellowish N = 381A 16 brown (10YR5/6) mottles, moist, hard, silty CLAY with 624 slight trace gravel. 13/24 2A 26 8-11 54% N=13 FILL - Grayish brown (10YR5/2) with 10% yellowish 622 brown (10YR5/6) mottles, moist, stiff, silty CLAY with trace sand and slight trace gravel. 2-2 5-7 20/24 Grayish brown (10YR5/2) with 25% yellowish brown 83% 3A 28 (10YR5/8) and 5% very dark brown (10YR2/2) mottles, 620 moist, medium, clayey SILT with trace sand and slight trace gravel. 24/24 4A 18 5-6 100% N=8618 ∇ 23/24 5A 20 Gray (10YR5/1) with 10% brownish yellow (10YR6/8) 3-4 96% mottles, moist, medium, silty CLAY with trace sand and slight trace gravel. 616 10 24/24 3-5 100% N=6 19 614 Grayish brown (10YR5/2) with brownish yellow 12 (10YR6/8) mottles, moist, soft, sandy CLAY with trace __ _gr<u>avel.</u> __ 19/24 Brownish yellow (10YR6/8), very moist, soft, sandy 1-2 79% CLAY with trace gravel. 19 7A 612 Brownish yellow (10YR6/8), very moist, soft, sandy SILT 14 with trace gravel. \blacksquare Brownish yellow (10YR6/6), wet, medium, SILT. 23/24 19 8A 7-10 96% N = 118B13 610 Light yellowish brown (10YR6/4) with 10% yellowish brown (10YR5/6) mottles, moist, stiff, clayey SILT with 10-24 trace sand and gravel. 22/24 9A 11 25-10 N = 49608 9B 8 Gray (10YR5/1), slightly moist, hard, SILT with gravel. 10-25 Gray (10YR5/1), wet, hard, SILT with sand and gravel. 24/24 V 10 40-40 100% N = 65Gray (10YR5/1), very moist, hard, SILT with gravel. 20 End of Boring = 20.0 ft. BGS

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois

Project:

DATES: Start: 2/11/2010

Finish: 2/11/2010 **WEATHER:** Sunny, cold ~32F

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 4'4" HSA w/SS samplers

Drilling Method: 4¹/₄" HSA w/SS sar

FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby

Eng/Geo: D. Lamb

HANSON

BOREHOLE ID: G109 **Well ID:** G109

 Surface Elev:
 624.79 ft. MSL

 Completion:
 18.00 ft. BGS

 Station:
 874,970.10N

 2,514,137.84E

	SAMPLE		T	EST	ING	ř	TOPOGR	APHIC MAP INFORMATION:	WATER LEVEL INFORMATION:			
er	Recov / Total (in) % Recovery		/ 6 in alue	Moisture (%)	Dry Den. (lb/ft³)	f) e Type	Quadra Townsl	angle: Coffeen, IL hip: East Fork 10, Tier 7N; Range 3W	Ψ = 14.20 - While drilling Ψ = 11.50 - Upon completion Ψ = 8.85 - 3/1/2010			
Number	Recov % Rec	Type	Blows / 6 ii N - Value RQD	Moist	Dry D	Qu (tsf) Failure	Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL Remarks			
1A	24/24 100%	ss	6-7 7-8 N=14	22			2	FILL - Grayish brown (10YR5/2) with 40% yellowis brown (10YR5/6) mottles, moist, stiff, silty CLAY w slight trace gravel.	h ith			
2A	19/24 79%	SS	3-5 5-6 N=10	27					622			
3A	20/24 83%	SS	2-5 6-8 N=11	24				Light yellowish brown (10YR6/4) with 50% brownis yellow (10YR6/8) mottles, moist, medium, silty CLA slight trace roots.				
4A	24/24 100%	ss	2-4 5-6 N=9	19			6 ————————————————————————————————————	Light brownish gray (10YR6/2) with 10% brownish ye (10YR6/6) mottles, moist, medium, silty CLAY with sl trace sand and gravel.	llow ight 618			
5A	22/24 92%	ss	2-3 4-5 N=7	20			10	Light brownish gray (10YR6/2) with 10% brownish yel (10YR6/6) and 2% very dark gray (10YR3/1) mottle moist, medium, silty CLAY with slight trace sand an	s, ///			
6A	24/24 100%	SS	1-3 3-4 N=6	19				gravel.	614			
7A	23/24 96%	SS	1-1 2-2 N=3	19			12 —	Light brownish gray (10YR6/2) with 30% brownish ye (10YR6/8) mottles, moist, medium, sandy CLAY wi slight trace gravel.	llow th 612			
8A	22/24 92%	SS	8-15 15-21 N=30	14			¥ 14 =	Brownish yellow (10YR6/6), wet, dense, silty SAND v trace gravel.	vith = 610			
9A	24/24 100%	SS	12-29 44-45 N=73	7			16	Brownish yellow (10YR6/8), wet, dense, SAND with to gravel.	608			
				′			18	Grayish brown (10YR5/2), slightly moist, hard, grave SILT with sand. End of Boring = 18.0 ft. BGS				

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 2/11/2010

Finish: 2/11/2010 WEATHER: Sunny, cold 10-20F CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 4½" HSA w/SS samplers

FIELD STAFF: Driller: T. List

Helper: M. Herbst/S. Hamby

Eng/Geo: .

BOREHOLE ID: G110

Well ID: G110

Surface Elev: 624.81 ft. MSL Completion: 18.00 ft. BGS Station: 875,015.42N 2,514,057.73E

SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: (iii Qu (tsf) *Qp* (tsf) Failure Type $\mathbf{V} = 15.00$ - While drilling Dry Den. (lb/ft³) Quadrangle: Coffeen, IL Recov / Total (% Recovery Township: East Fork Moisture (%) Blows / 6 in N - Value RQD Section 10, Tier 7N; Range 3W ∇ = 9.50 - 3/1/2010 Number Lithologic Borehole Elevation ft. BGS Description ft. MSL Remarks 8-6 24/24 624 6-8 100% N = 121 A 17 FILL - Yellowish brown (10YR5/6) with 20% light 2-4 5-7 brownish gray (10YR6/2) mottles, moist, stiff, silty CLAY 14/24 622 25 2A with slight trace sand. 58% N=9620 22/24 5-9 92% 22 3A Grayish brown (10YR5/2) with 20% yellowish brown 3-6 (10YR5/6) and 5% very dark brown (10YR2/2) mottles, 24/24 618 8-9 moist, medium, silty CLAY. 100% N=1418 24/24 616 4-6 Grayish brown (10YR5/2) with 20% yellowish brown 100% ∇ 21 (10YR5/6) and 5% very dark brown (10YR2/2) mottles, 5A moist, medium, silty CLAY with slight trace sand and 10 24/24 4-6 100% N=821 6A Light brownish gray (10YR6/2) with 30% brownish yellow (10YR6/8) mottles, moist, medium, clayey SILT with trace sand and gravel. 24/24 Light brownish gray (10YR6/2) with 30% brownish yellow (10YR6/8) mottles, very moist, medium, sandy CLAY with 3-3 100% 7A 22 trace gravel. Light brownish gray (10YR6/2) with 30% brownish yellow (10YR6/8) mottles, moist, soft, clayey SILT with trace sand and slight trace gravel. 610 19/24 \blacksquare 8A 24 2-1 79% N=4Gray (10YR6/1) with 30% brownish yellow (10YR6/8) mottles, wet, soft, sandy CLAY. 7-26 608 24/24 SS 49-60 Grayish brown (10YR5/2), slightly moist, hard, clayey 100% N=75 9A 6 SILT with trace sand and gravel. End of Boring = 18.0 ft. BGS

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois

Project:

DATES: Start: 2/10/2010

WEATHER: Sunny, breezy ~25F

Finish: 2/11/2010

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill

Drilling Method: 41/4" HSA w/SS samplers

FIELD STAFF: Driller: T. List

Helper: M. Herbst/S. Hamby

Eng/Geo: D. Lamb

HANSON

BOREHOLE ID: G111 Well ID: G111

> Surface Elev: 625.28 ft. MSL Completion: 18.00 ft. BGS **Station:** 875,058.70N

> > 2,513,981.72E

SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Œ, Quadrangle: Coffeen, IL $\mathbf{Y} = 14.50$ - While drilling Dry Den. (lb/ft3 Recov / Total (% Recovery Township: East Fork **T** = Moisture (%) Blows / 6 in N - Value RQD Section 10, Tier 7N; Range 3W ∇ = 10.50 - 3/1/2010 Qu (tsf) Failure 1 Number Lithologic Borehole Elevation Description ft. MSL Remarks 20/24 6-9 SS 83% 624 N = 131 A 18 FILL - Grayish brown (10YR5/2) with 10% yellowish 13/24 2A 20 brown (10YR5/6) mottles, moist, stiff, silty CLAY with 7-8 54% slight trace sand and gravel. 622 N=1218/24 6-8 75% N=10 620 20 3A Grayish brown (10YR5/2) with 20% yellowish brown (10YR5/6) and 5% dark brown (10YR3/3) mottles, moist, medium, clayey SILT with slight trace sand, slight trace 4-12 16/24 20-17 67% roots. 618 N=3218 Grayish brown (10YR5/2) with 5% yellowish brown (10YR5/6) mottles, moist, medium, clayey SILT with trace 22/24 21 4-5 sand 92% 616 $\bar{\Delta}$ 2-3 24/24 23 6A 6-6 100% 614 N=9Grayish brown (10YR5/2) with 5% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY with slight trace sand. 24/24 20 5-6 N=9 100% 612 Gravish brown (10YR5/2) with 30% yellowish brown (10YR5/8) mottles, moist, stiff, clayey SILT with trace 14 sand and gravel. 24/24 17 8A 2-2 100% Yellowish brown (10YR5/8), wet, soft, clayey SAND with N=4 610 trace gravel. 12-50 18/18 Grayish brown (10YR5/2) with 20% brownish yellow 66 9A 100% (10YR6/6) mottles, slightly moist, hard, clayey SILT with N=116608 gravel. 0/6 0% End of Boring = 18.0 ft. BGS

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois

Project:

DATES: Start: 2/9/2010

Finish: 2/9/2010

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill

Drilling Method: 41/4" HSA w/SS samplers

FIELD STAFF: Driller: T. List

Helper: M. Herbst/S. Hamby

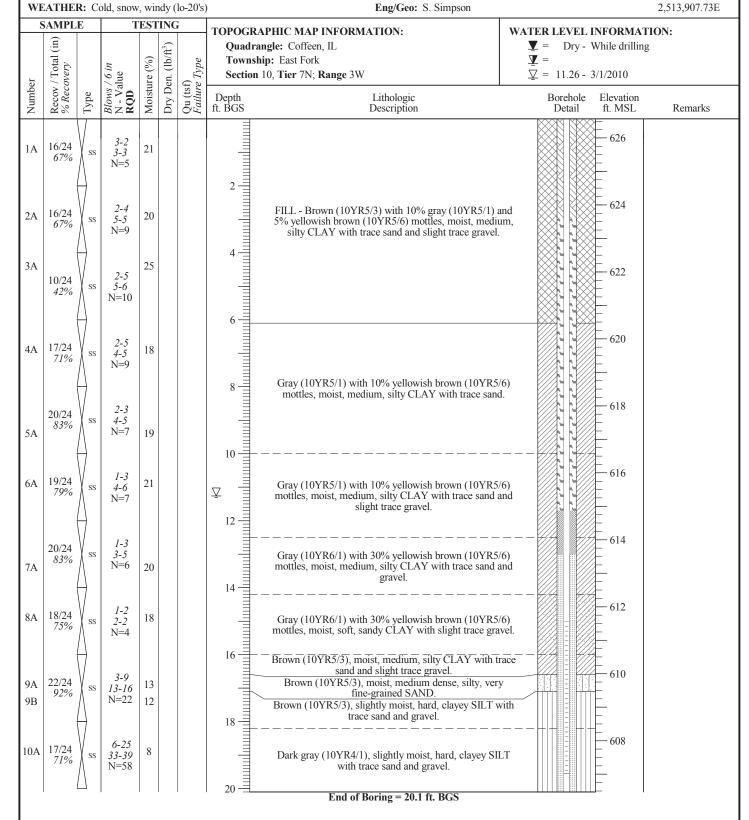
Eng/Geo: S. Simpson

HANSON

BOREHOLE ID: G119 Well ID: G119

> Surface Elev: 626.57 ft. MSL **Completion:** 20.10 ft. BGS **Station:** 875,675.04N

> > 2,513,907.73E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A **DATES: Start: 2/8/2010**

Finish: 2/8/2010

WEATHER: Cold, snow, windy (mid-20's)

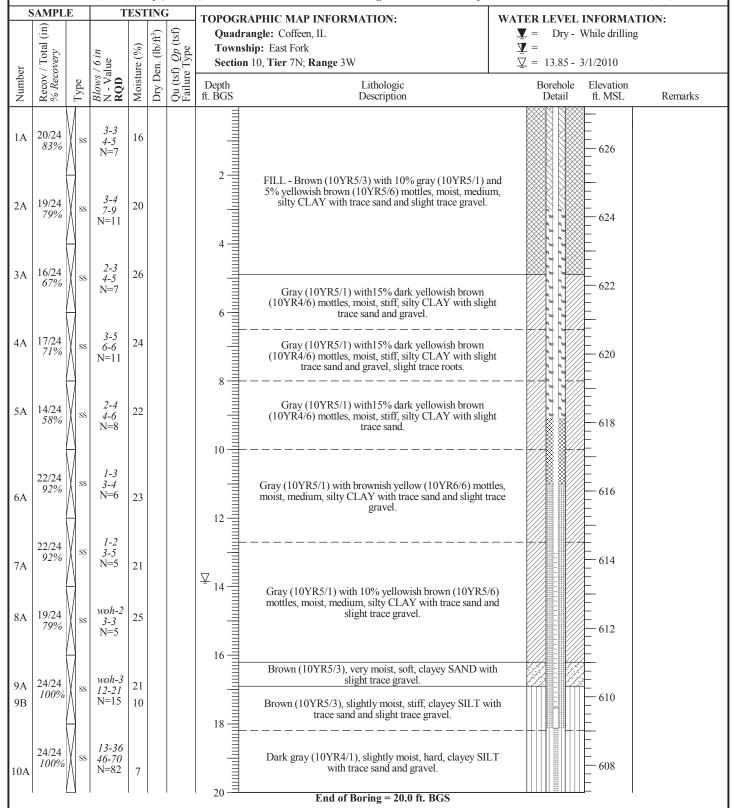
CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill **Drilling Method:** 41/4" HSA w/SS samplers

FIELD STAFF: Driller: T. List

Helper: M. Herbst/S. Hamby Eng/Geo: S. Suzanna Simpson

BOREHOLE ID: G120 Well ID: G120

> Surface Elev: 627.21 ft. MSL Completion: 20.00 ft. BGS **Station:** 875,854.43N 2,513,905.84E



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois

Project:

DATES: Start: 2/4/2010

Finish: 2/4/2010 WEATHER: Overcast, cold (lo-30's) CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 4'4" HSA w/SS samplers

FIELD STAFF: Driller: T. List

Helper: M. Herbst/S. Hamby

Eng/Geo: S. Simpson

HANSON

BOREHOLE ID: G121 **Well ID:** G121

 Surface Elev:
 627.94 ft. MSL

 Completion:
 22.00 ft. BGS

 Station:
 875,964.59N

2,513,904.35E

S	SAMPL	E	TESTING		TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
ber	Recov / Total (in) % Recovery	% Kecovery Type Blows / 6 in N - Value RQD Moisture (%) Dry Den. (Ib/ft³) Qu (tsf) Failure Type				st) tre Type	Quadrar Townshi Section 1	ngle: Coffeen, IL p: East Fork 0, Tier 7N; Range 3W	$\underline{\underline{\mathbf{Y}}}$ = Dry - While drilling $\underline{\underline{\mathbf{Y}}}$ = $\underline{\underline{\mathbf{Y}}}$ = 14.44 - 3/1/2010			
Number	Reco % Re	Type	Blow. N - V RQD	Mois	Dry I	Qu (tsf) Failure	Depth ft. BGS	Lithologic Description		evation MSL Remarks		
1A	22/24 92%	SS	4-4 7-8 N=11	15			2			626		
2A	20/24 83%	SS	3-5 8-12 N=13	16				FILL - Brown (10YR4/3) with 10% gray (10YR5/1) a 5% dark yellowish brown (10YR4/6) mottles, moist, s silty CLAY with slight trace sand and gravel.	iff,	624		
3A	18/24 75%	SS	1-4 5-6 N=9	27						024		
	18/24	SS	2-3 5-6				6-	Dark yellowish brown (10YR4/4), moist, medium, si CLAY with slight trace sand.	////h h ////	622		
4A	75%	$\stackrel{\wedge}{\rightarrow}$	N=8	25			8	Gray (10YR5/1) with 25% dark yellowish brown (10YR4/6) mottles, moist, medium, clayey SILT with t sand.	race	620		
5A	18/24 75%	SS	2-2 3-4 N=5	24			8	Gray (10YR6/1) with 10% yellowish brown (10YR5/	6)	618		
6A	13/24 54%	SS	2-2 4-4 N=6	23			10 =	mottles, moist, medium, silty CLAY with slight trace s	and.			
7A	19/24 79%	SS	woh-2 3-4 N=5	23			12 = -			616		
8A	18/24 75%	SS	1-2 2-2 N=4	23			14 — \(\sum_{} \)	Gray (10YR6/1) with 10% yellowish brown (10YR5/mottles, moist, medium, silty CLAY with trace sand a slight trace gravel.		614		
	23/24	<u>/ </u>	N=4 woh-woh	i			16 —			612		
9A	96%	SS	1-2	21			18 —	Brown, (10YR5/3), very moist, very soft, clayey SAN with slight trace gravel.		610		
10A	22/24 92%	SS	4-12 26-30 N=38	8			20	Brown (10YR5/3), slightly moist, hard, very silty CL/with trace sand and gravel.		608		

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois

Project:

DATES: Start: 2/4/2010

WEATHER: Overcast, cold (lo-30's)

Finish: 2/4/2010

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill **Drilling Method:** 4¹/₄" HSA w/SS samplers

FIELD STAFF: Driller: T. List

Helper: M. Herbst/S. Hamby

Eng/Geo: S. Simpson

HANSON

BOREHOLE ID: G121 Well ID: G121

> Surface Elev: 627.94 ft. MSL **Completion:** 22.00 ft. BGS **Station:** 875,964.59N

> > 2,513,904.35E

	SAMPL	E	Т	EST	ING	ř	TOPOGRA	PHIC MAP INFORMATION:	WATER LEVEL INFORMATION:			
er	/ Total (in)		ws / 6 in Value D	Moisture (%)	en. (lb/ft³)	f) e Type	Quadran Townshi	gle: Coffeen, IL p: East Fork 0, Tier 7N; Range 3W	$\underline{\Psi}$ = Dry - While drilling $\underline{\Psi}$ = $\underline{\nabla}$ = 14.44 - 3/1/2010			
Number	Recov % Rec	Type	Blows N - Va RQD	Moist	Dry Den.	Qu (tsf) Failure	Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL Remarks			
11A	24/24 100%	SS	13-23 31-48 N=54	7			22	Dark gray (10YR4/1), slightly moist, hard, clayey SII with sand and gravel. End of Boring = 22.0 ft, BGS	LT			

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois

Project:

DATES: Start: 2/4/2010

Finish: 2/4/2010

WEATHER: Overcast, cold (lo-30's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill

Drilling Method: 41/4" HSA w/SS samplers

FIELD STAFF: Driller: T. List

Helper: M. Herbst/S. Hamby

Eng/Geo: D. Lamb

BOREHOLE ID: G122

Well ID: G122

Surface Elev: 628.05 ft. MSL **Completion:** 20.00 ft. BGS **Station:** 876,080.14N

2,513,902.82E

HANSON

	SAMPLE		Т	EST	INC	j	TOPOGR	APHIC MAP INFORMATION:	WATER LEVEL INFORMATION:			
er	Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) Failure Type	Quadra Townsl	angle: Coffeen, IL nip: East Fork 10, Tier 7N; Range 3W	$\underline{\underline{V}}$ = 17.00 - While drilling $\underline{\underline{V}}$ = $\underline{\underline{\nabla}}$ = 12.84 - 3/1/2010			
Number	Recov % Rec	Type	Blows N - VS RQD	Moist	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL Remarks			
1A	23/24 96%	ss	5-5 6-11 N=11	16			2	FILL - Grayish brown (10YR5/2) with 15% gray	626			
2A	20/24 83%	ss	3-3 5-5 N=8	22			4-	(10YR5/1) and 5% yellowish brown (10YR5/6) mottle moist, stiff, silty CLAY with trace sand and slight tra- gravel.	es, KXXIX IXXXIIII I			
3A	20/24 83%	ss	3-4 5-6 N=9	25			6-	Dark yellowish brown (10YR4/4), moist, medium, sil	ty 622			
4A	19/24 79%	ss	1-5 5-6 N=10	29				CLAY.				
5A	20/24 83%	SS	<i>I-3</i> 3-3 N=6	21			8 =	Gray (10YR5/1) with 20% dark yellowish brown (10YR4/6) mottles, moist, medium, silty CLAY.	620			
6A	19/24 79%	ss	1-2 3-3 N=5	22			10 -	Grayish brown (10YR5/2), moist, medium, clayey SII with sand.	T			
7A	16/24	SS	1-2 3-4	21			12 -	Yellowish brown (10YR5/6) mottles, moist, medium, s CLAY with trace gravel. Gray (10YR5/1), moist, medium, clayey SILT with fi				
	67%		N=5				14	sand. Brown (10YR4/3), moist, medium, silty CLAY with tr				
8A	19/24 79%	ss	1-1 2-2 N=3	21			16	Grayish brown (10YR5/2), moist, soft, sandy SILT.				
	20/24	ss	1-1 4-16				₹ -	Yellowish brown (10YR5/4) with 40% dark yellowish brown (10YR4/6) mottles, moist, medium, silty CLA with slight trace sand.	h Y = 612			
9A	83%		N=5	14			18	Dark yellowish brown (10YR4/6), wet, medium dens silty SAND.	e, 610			
10A	24/24 100%	SS	10-38 51-58 N=89	6			20	Brownish yellow (10YR6/6), slightly moist, hard, clay SILT with trace sand and gravel. Gray (10YR5/1), slightly moist, hard, clayey SILT wigravel.	/			

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois

Project:

DATES: Start: 2/3/2010

WEATHER: Overcast, cold (lo-30's)

Finish: 2/4/2010

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 4¹/₄" HSA w/SS samplers

FIELD STAFF: Driller: T. List

Helper: M. Herbst/S. Hamby

Eng/Geo: D. Lamb

HANSON

BOREHOLE ID: G123 **Well ID:** G123

 Surface Elev:
 628.12 ft. MSL

 Completion:
 24.00 ft. BGS

 Station:
 876,189.60N

2,513,901.46E

5	SAMPLE		Т	TESTING			TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
ber	Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) Failure Type	Quadra Townsl	angle: Coffeen, IL hip: East Fork 10, Tier 7N; Range 3W	$ \mathbf{Y} = 16.50 $ While drilling $ \mathbf{Y} = $ $ \mathbf{Y} = $ $ \mathbf{Y} = 15.98 - 3/1/2010 $			
Number	Recov % Rea	Type	Blows N - V RQD	Moist	Dry I	Qu (ts Failu	Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL Remarks			
1A	22/24 92%	SS	6-8 9-11 N=17	16			2		628			
2A	19/24 79%	SS	4-5 6-7 N=11	18				FILL - Grayish brown (10YR5/2) with 10% gray (10YR5/1) and 5% yellowish brown (10YR5/6) mottl moist, stiff, silty CLAY with trace sand and slight tra gravel.	es, ce			
3A	17/24 71%	SS	2-3 5-6 N=8	24			6-	Dark grayish brown (10YR4/2) with 20% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with sl	own ight			
4A	18/24 75%	SS	1-3 5-8 N=8	24			6-	trace sand, slight trace roots. Gray (10YR5/1) with 30% yellowish brown (10YR5/mottles, moist, medium, silty CLAY with slight trace sand, slight trace sand, slight trace roots.	60			
5A	20/24 83%	ss	2-3 4-5 N=7	18			10 —	Gray (10YR5/1) with 10% yellowish brown (10YR5/mottles, moist, medium, silty CLAY with trace sand a	6) nd 618			
6A	19/24 79%	SS	1-2 3-5 N=5	21				slight trace gravel.				
7A	23/24 96%	SS	1-3 4-4 N=7	19			12	Gray (10YR5/1) with 10% yellowish brown (10YR5/mottles, moist, medium, silty CLAY with sand and slitrace gravel.	6) ght			
0.4	22/24 92%	SS	1-2 3-3 N=5	1.7			14	Dark yellowish brown (10YR3/6) with dark yellowis brown (10YR4/6) mottles, moist, medium, silty CLA with trace gravel.	h Y 614			
8A	22/24	\bigvee	1-1	17			∑ 16 — ▼	Gray (10YR6/1) with 30% yellowish brown (10YR5/mottles, moist, medium, sandy SILT.	6) 612			
9A	92%	SS	2-2 N=3	19			18	Dark yellowish brown (10YR4/6), wet, soft, clayey SA	ND. 610			
10A	12/24 50%	SS	3-3 3-4 N=6	16			20	Yellowish brown (10YR5/6), wet, medium, sandy SII with trace gravel.	T			

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois

Project:

DATES: Start: 2/3/2010

Finish: 2/4/2010

WEATHER: Overcast, cold (lo-30's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill

Drilling Method: 41/4" HSA w/SS samplers

FIELD STAFF: Driller: T. List

Helper: M. Herbst/S. Hamby

Eng/Geo: D. Lamb



BOREHOLE ID: G123 Well ID: G123

Surface Elev: 628.12 ft. MSL **Completion:** 24.00 ft. BGS **Station:** 876,189.60N

2,513,901.46E

	SAMPL	E	T	EST	INC	ř	TOPOGRAP	HIC MAP INFORMATION:	WATER LEVEL IN	FORMATION:
l l	/ Total (in)		/ 6 in alue	ıre (%)	Den. (lb/ft³)	f) e Type	Quadrang Township	de: Coffeen, IL : East Fork , Tier 7N; Range 3W	$\underline{\underline{V}} = 16.50 - WI$ $\underline{\underline{V}} = $ $\underline{\underline{V}} = 15.98 - 3/1$	hile drilling
Number	Recov / Tota % Recovery	Type	Blows N - Va RQD	Moisture	Dry D	Qu (tsf) Failure	Depth ft. BGS	Lithologic Description		Elevation ft. MSL Remarks
11A	20/24 83%	SS	6-12 12-12 N=24	11			22	Yellowish brown (10YR5/6), wet, medium, sandy SII with trace gravel. [Continued from previous page] Gray (10YR5/1), moist, very stiff, clayey SILT with tragravel.		- 608 606
12A	19/24 79%	SS	3-8 13-9 N=21	9			24	Dark gray (10YR4/1), moist, very stiff, sandy SILT w trace clay and gravel. End of Boring = 24.0 ft. BGS	ith	

CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois

Project:

DATES: Start: 2/3/2010

Finish: 2/3/2010 WEATHER: Sunny, cold (mid-30's) **Drilling Method:** 41/4" HSA w/SS samplers

Rig mfg/model: CME-750 ATV Drill

CONTRACTOR: Layne-Western Co

FIELD STAFF: Driller: T. List

Helper: M. Herbst/S. Hamby

Eng/Geo: S. Simpson

HANSON

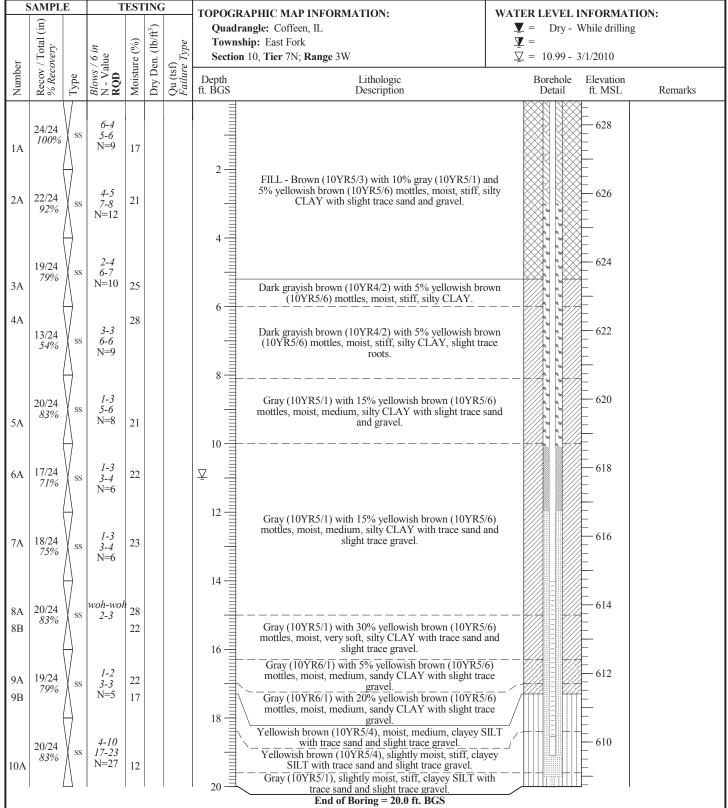
BOREHOLE ID: G124 **Well ID:** G124

 Surface Elev:
 628.70 ft. MSL

 Completion:
 20.00 ft. BGS

 Station:
 876,304.85N

2,513,900.34E



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 2/2/2010 **Finish:** 2/3/2010

WEATHER: Sunny, cold (mid-30's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 41/4" HSA w/SS samplers

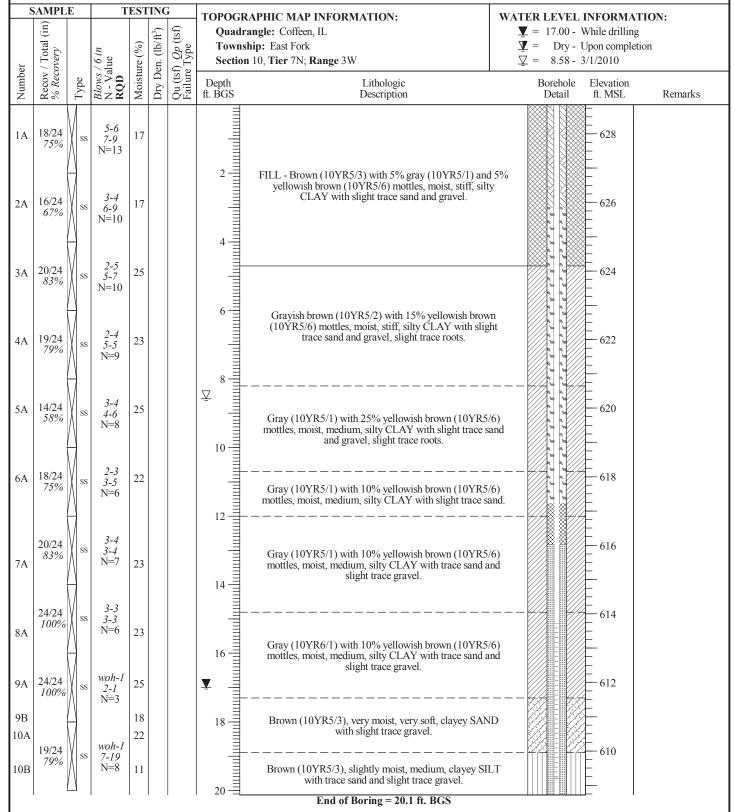
FIELD STAFF: Driller: T. List

Helper: M. Herbst/S. Hamby **Eng/Geo:** S. Suzanna Simpson

HANSON

BOREHOLE ID: G125 Well ID: G125

> Surface Elev: 628.85 ft. MSL Completion: 20.13 ft. BGS Station: 876,409.47N 2,513,899.12E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois

Project:

DATES: Start: 2/10/2010

Finish: 2/10/2010

WEATHER: Partly cloudy, wind 10 mph, ~25F

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill

Drilling Method: 41/4" HSA w/SS samplers

FIELD STAFF: Driller: T. List

Helper: M. Herbst/S. Hamby

Eng/Geo: D. Lamb

BOREHOLE ID: G126

Well ID: G126

Surface Elev: 622.96 ft. MSL Completion: 18.00 ft. BGS **Station:** 875,062.44N

2,513,895.37E

SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Œ, Quadrangle: Coffeen, IL $\mathbf{Y} = 13.20$ - While drilling Dry Den. (lb/ft3 Recov / Total (% Recovery Township: East Fork Moisture (%) Blows / 6 in N - Value RQD Section 10, Tier 7N; Range 3W ∇ = 8.82 - 3/1/2010 Qu (tsf) Failure 1 Number Lithologic Borehole Elevation Description Detail ft. MSL Remarks FILL - Dark yellowish brown (10YR4/6), moist, very stiff, 20/24 sandy CLAY with silt and trace gravel. 12-10 622 SS 83% N=19 1 A 15 Light grayish brown (10YR6/2), slightly moist, stiff, silty CLAY. 19/24 2A 26 6-9 620 N=10Light grayish brown (10YR6/2) with 20% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY, slight trace roots. 2-4 5-7 22/24 618 92% 3A 20 24/24 616 4-5 Dark brown (10YR3/3) with 10% dark yellowish brown 100% (10YR4/6) mottles, moist, stiff, silty CLAY. 22 $\bar{\Delta}$ 24/24 4-5 614 100% 22 5A 10 Light grayish brown (10YR6/2) with 30% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with trace sand and slight trace gravel. 22/24 4-5 612 92% 18 6A Grayish brown (10YR5/2) with dark yellowish brown (10YR4/6) mottles, moist, medium, clayey SILT with sand and slight trace gravel. 20/24 19 3-4 610 83% Gray (10YR6/1). very moist, medium, sandy CLAY with trace gravel. 4-15 12-29 N=27 24/24 20 8A 608 Yellowish brown (10YR5/8) with 40% dark yellowish 100% brown (10YR4/6) mottles, slightly moist, very stiff, clayey SILT with trace gravel. Brownish yellow (10YR6/8), moist, very stiff, sandy SILT with gravel. 29-39 23/24 9A 8 Grayish brown (10YR5/2), moist, hard, sandy SILT with 606 39-29 96% N = 78gravel. End of Boring = 18.0 ft. BGS

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 12/19/2011

Finish: 12/19/2011 WEATHER: Cloudy, rain (mid-60's)

CONTRACTOR: Testing Service Corp.
Rig mfg/model: CME-550 ATV Drill
Drilling Method: 4½" HSA w/SS samplers

FIELD STAFF: Driller: B. Williamson Helper: R. McCuan

Eng/Geo: R. Fiorito

HANSON

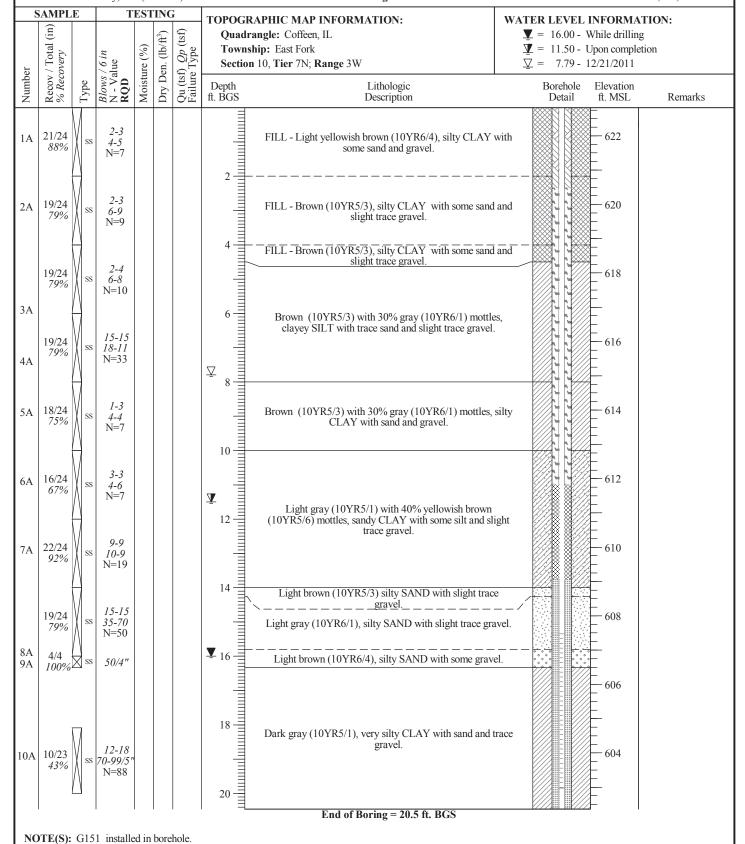
BOREHOLE ID: G151 **Well ID:** G151

 Surface Elev:
 622.82 ft. MSL

 Completion:
 20.46 ft. BGS

 Station:
 875,023.67N

 2,513,805.93E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 12/20/2011 **Finish:** 12/20/2011

WEATHER: Cloudy, (mid-40's), rainy later.

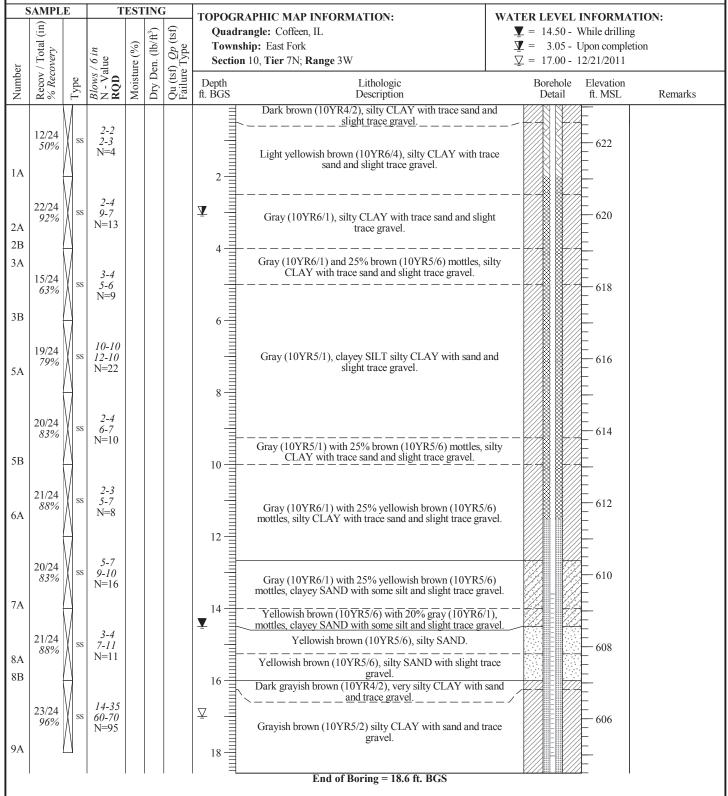
CONTRACTOR: Testing Service Corp.
Rig mfg/model: CME-550 ATV Drill
Drilling Method: 4½" HSA w/SS samplers

FIELD STAFF: Driller: B. Williamson Helper: R. McCuan

Eng/Geo: R. Fiorito

BOREHOLE ID: G152 **Well ID:** G152

Surface Elev: 623.06 ft. MSL Completion: 18.57 ft. BGS Station: 874,687.53N 2,513,894.46E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A

SAMPLE

DATES: Start: 12/15/2011 Finish: 12/15/2011

WEATHER: Cloudy, windy, (mid-high 30's)

TESTING

CONTRACTOR: Testing Service Corp. Rig mfg/model: CME-550 ATV Drill **Drilling Method:** 41/4" HSA w/SS samplers

FIELD STAFF: Driller: B. Williamson Helper: R. McCuan

TOPOGRAPHIC MAP INFORMATION:

Eng/Geo: R. Fiorito

2,513,532.68E WATER LEVEL INFORMATION:

> $\mathbf{V} = 16.00$ - While drilling $\mathbf{V} = 15.70$ - Upon completion

BOREHOLE ID: G153

Well ID: G153

Surface Elev: 623.30 ft. MSL

Completion: 20.76 ft. BGS

Station: 874,532.71N

Œ. Qu (tsf) *Qp* (tsf) Failure Type Dry Den. (lb/ft³) Quadrangle: Coffeen, IL Recov / Total (% Recovery Moisture (%) Township: East Fork Blows / 6 in N - Value RQD Section 10, Tier 7N; Range 3W ∇ = 17.55 - 12/21/2011 Number Lithologic Borehole Elevation ft. BGS Description Detail ft. MSL Remarks FILL - Light yellowish brown (10YR6/4), silty CLAY with sand and slight trace gravel. 16/24 1A SS 67% 622 1B FILL - Gray (10YR6/1), silty CLAY with sand and slight trace gravel. woh-3 17/24 4-6 71% N=7620 2A Gray (10YR6/1) with 50% brownish yellow (10YR6/8) mottles, silty CLAY with trace sand. 18/24 3A 5-5 75% 618 4-5 21/24 4A 6-8 Gray (10YR5/1), silty CLAY with trace sand. 88% N=11616 20/24 4-5 83% N=85A Gray (10YR6/1), silty CLAY with sand and slight trace gravel. Gray (10YR6/1), silty CLAY with sand and slight trace gravel. 1-4 20/24 4-6 83% Gray (10YR6/1) with 50% brownish yellow (10YR6/8) N=8612 mottles, silty CLAY with sand and slight trace gravel. 6A 23/24 Gray (10YR6/1) with 25% brownish yellow (10YR6/8) 6-15 96% mottles, clayey SAND with trace silt. N=12610 7A 15-23 19/24 84 37-50 79% 608 N = 60Yellowish brown (10YR5/8), silty SAND with slight trace 9/12 9A 50-99 606 ∇ Brownish yellow (10YR6/8), silty CLAY with sand and trace gravel. 18 23-50 20/24 58-109 83% Dark gray (10YR4/1), silty CLAY with sand and trace N=108604 gravel. 20 11A End of Boring = 20.8 ft. BGS

NOTE(S): G153 installed in borehole.

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 12/16/2011

Finish: 12/16/2011 **WEATHER:** Ptly. cloudy (mid-30's)

CONTRACTOR: Testing Service Corp.

Rig mfg/model: CME-550 ATV Drill

Drilling Method: 4½" HSA w/SS samplers

FIELD STAFF: Driller: B. Williamson Helper: R. McCuan

Eng/Geo: R. Fiorito

HANSON

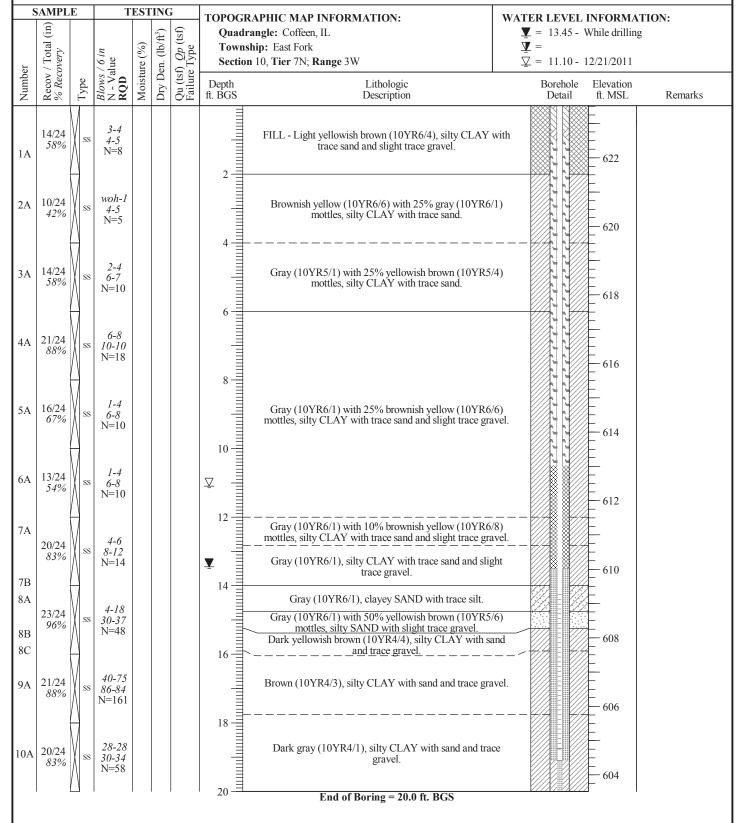
BOREHOLE ID: G154 **Well ID:** G154

 Surface Elev:
 623.52 ft. MSL

 Completion:
 20.00 ft. BGS

 Station:
 874,978.38N

 2.513.243.10E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A

SAMPLE

(iii

DATES: Start: 12/19/2011 Finish: 12/19/2011

WEATHER: Cloudy, rainy, (mid-40's)

TESTING

CONTRACTOR: Testing Service Corp. Rig mfg/model: CME-550 ATV Drill **Drilling Method:** 41/4" HSA w/SS samplers

FIELD STAFF: Driller: B. Williamson Helper: R. McCuan

Eng/Geo: R. Fiorito

2,513,501.75E

BOREHOLE ID: G155

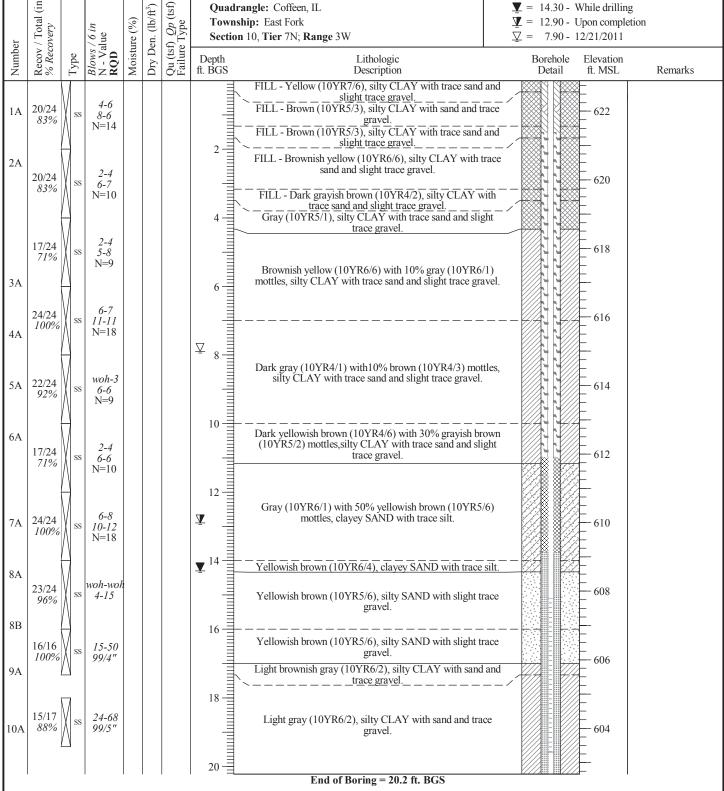
Well ID: G155

Surface Elev: 622.89 ft. MSL

Completion: 20.23 ft. BGS

Station: 875,127.65N

TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Quadrangle: Coffeen, IL $\mathbf{V} = 14.30$ - While drilling Township: East Fork $\mathbf{V} = 12.90$ - Upon completion Section 10, Tier 7N; Range 3W ∇ = 7.90 - 12/21/2011



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 2/25/2008 **Finish:** 2/25/2008

WEATHER: Overcast, cold

CONTRACTOR: Testing Service Corp.

Rig mfg/model: CME-650 Track Drill

Drilling Method: 31/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

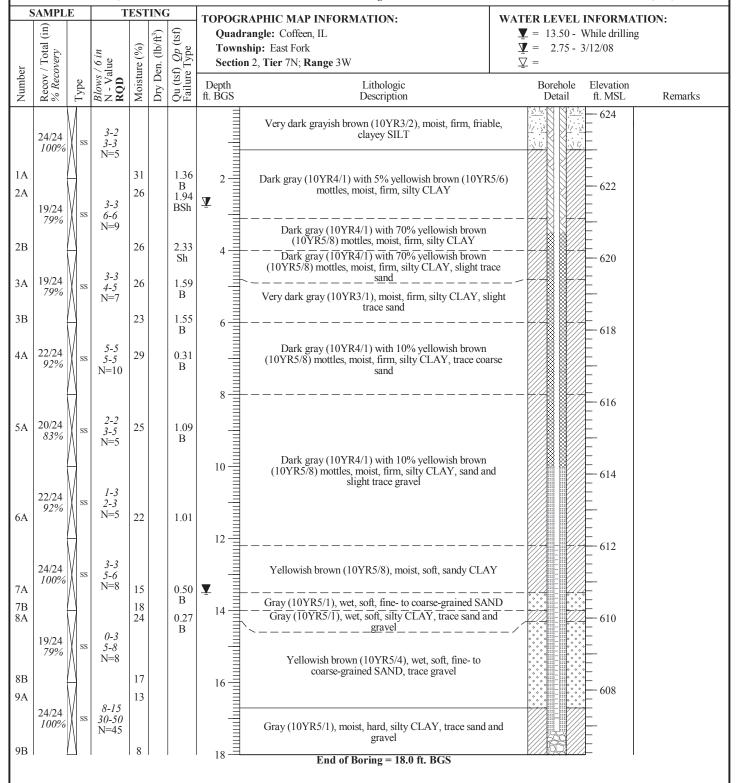
Eng/Geo: .

HANSO

BOREHOLE ID: G200 **Well ID:** G200

Surface Elev: 624.20 ft. MSL Completion: 18.00 ft. BGS Station: 877.930.59N

2,515,649.96E



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 2/25/2008

Finish: 2/25/2008
WEATHER: Overcast, cold

CONTRACTOR: Testing Service Corp. **Rig mfg/model:** CME-650 Track Drill

Drilling Method: 31/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: .



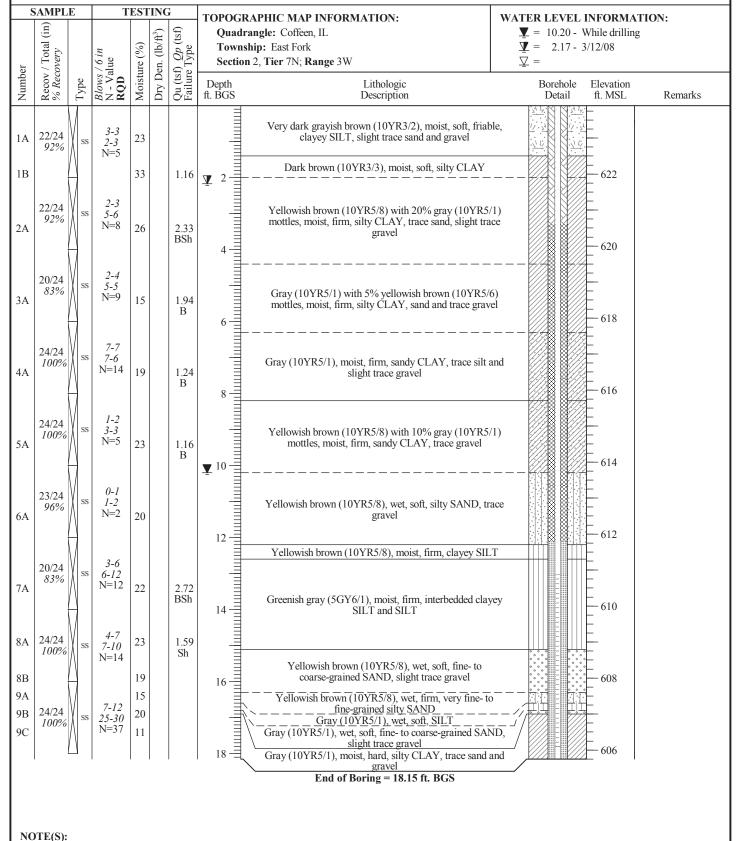
BOREHOLE ID: G201 **Well ID:** G201

 Surface Elev:
 623.90 ft. MSL

 Completion:
 18.15 ft. BGS

 Station:
 877,924.94N

 2.514,849.47E



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 2/21/2008 **Finish:** 2/21/2008

WEATHER: Overcast, cold

CONTRACTOR: Testing Service Corp.
Rig mfg/model: CME-650 Track Drill

Drilling Method: 31/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: .

BOREHOLE ID: G205

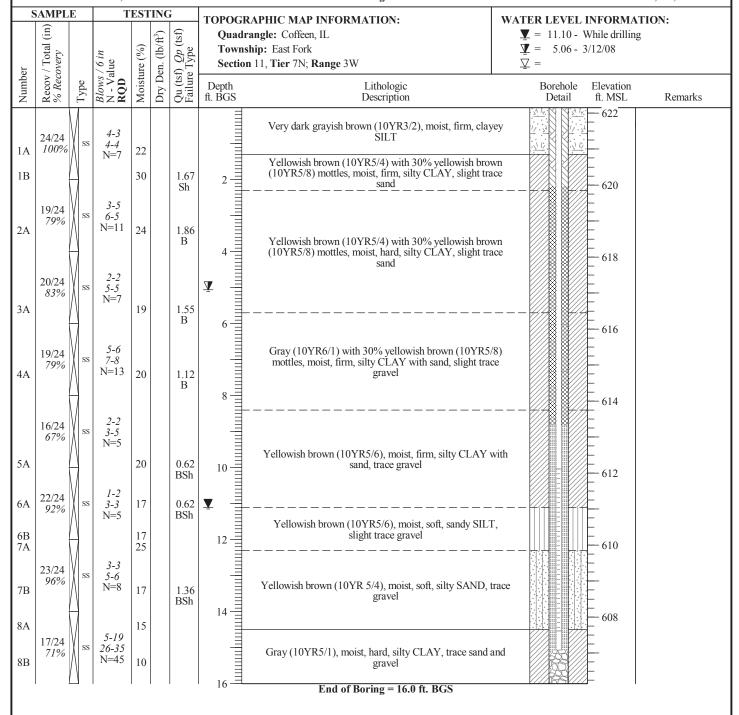
Well ID: G205

 Surface Elev:
 622.15 ft. MSL

 Completion:
 16.00 ft. BGS

 Station:
 875,550.19N

 2,515,914.87E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 10/14/2010

Finish: 10/14/2010

WEATHER: Sunny, warm, breezy (lo-70's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 41/4" HSA w/SS samplers

FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: .

HANSON PARENCE ID. COOK

BOREHOLE ID: G206 **Well ID:** G206

Surface Elev: 630.54 ft. MSL Completion: 24.00 ft. BGS Station: 875,103.91N 2,514,669.16E

	MPLI	C	Т	EST	INC		TOPOGRAPHIC MAP INFORMATION:	WATER LEVEL INFORMATION:			
oer v / Total (in)	% Recovery		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	$\underline{\underline{\mathbf{V}}}$ = 22.00 - While drilling $\underline{\underline{\mathbf{V}}}$ = 21.54 - Upon completion $\underline{\underline{\mathbf{V}}}$ =			
Number Recov /	% Re	Type	Blow. N - V RQD	Mois	Dry I	Qu (t Failu	Depth Lithologic ft. BGS Description	Borehole Elevation Detail ft. MSL Remark			
1A 12 5	2/24	ss	2-2 3-2 N=5	18			FILL - Grayish brown (10YR5/2), moist, firm, silty of with trace sand and gravel. FILL - Dark gray (10YR4/1) with 30% dark yellow brown (10YR4/6) mottles, moist, firm, silty CLAY trace sand and gravel. Very dark gray (10YR3/1) with 20% dark yellowish (10YR4/6) mottles, moist, firm, silty CLAY with trace sand, trace roots. Dark grayish brown (10YR4/2) with 35% dark yellow brown (10YR4/6) mottles, moist, firm, silty CLAY trace sand. Gray (10YR5/1) with 20% dark yellowish brown (10YR4/6) mottles, moist, firm, clayey SILT with the sand and gravel. Gray (10YR5/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand.	CLAY = 630			
2A 20 8	0/24 83%	ss	2-2 3-5 N=5	16			4	628			
3A 20	0/24	SS	4-9 6-8 N=15	19			FILL - Dark gray (10YR4/1) with 30% dark yellov brown (10YR4/6) mottles, moist, firm, silty CLAY trace sand and gravel.				
4A 19	9/24 79%	ss	2-4 5-6 N=9	20				624			
5A 17	7/24 71%	ss	2-3 4-5 N=7	30			Very dark gray (10YR3/1) with 20% dark yellowish (10YR4/6) mottles, moist, firm, silty CLAY with the sand, trace roots. Dark grayish brown (10YR4/2) with 35% dark yellow brown (10YR4/6) mottles, moist, firm, silty CLAY trace sand.	race 622			
	2/24 92%	SS	2-3 4-6 N=7	19			Gray (10YR5/1) with 20% dark yellowish brow (10YR4/6) mottles, moist, firm, clayey SILT with t sand and gravel.				
7A 23	3/24	SS	1-2 3-4 N=5	23			Gray (10YR5/1) with 15% dark yellowish brow (10YR4/6) mottles, moist, firm, silty CLAY with trac and gravel.				
	2/24	SS	1-1 3-3 N=4	22			Gray (10YR5/1) with 15% dark yellowish brow (10YR4/6) mottles, moist, soft, silty CLAY with trac and gravel.	e sand			
	4/24	ss	1-1 2-2 N=3	21			Dark yellowish brown (10YR4/6) with 30% gravel. Gray (10YR5/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with trace and gravel. Gray (10YR5/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with trace and gravel. Gray (10YR5/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, very soft, silty CLAY with sand and gravel. Gray (10YR5/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, very soft, silty CLAY with sand and gravel.	e sand 614 n e sand			
10A 24	4/24	ss	woh-wol 1-5	25			Gray (10YR5/1) with 15% dark yellowish brow (10YR4/6) mottles, moist, very soft, silty CLAY with sand and gravel. Gray (10YR5/1), moist, very soft, very fine- to fine-g sandy CLAY with trace gravel.	n trace 612			

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A

SAMPLE

DATES: Start: 10/14/2010

Finish: 10/14/2010

WEATHER: Sunny, warm, breezy (lo-70's)

TESTING

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill **Drilling Method:** 41/4" HSA w/SS samplers

FIELD STAFF: Driller: D. Mahurin

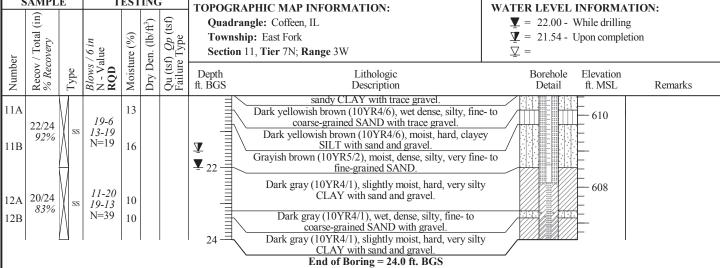
Helper: J. Litsch/D. Smail Eng/Geo: .

HANSON

BOREHOLE ID: G206 Well ID: G206

> Surface Elev: 630.54 ft. MSL Completion: 24.00 ft. BGS **Station:** 875,103.91N 2,514,669.16E

WATER LEVEL INFORMATION:



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A

DATES: Start: 10/8/2010

Finish: 10/8/2010 WEATHER: Sunny, mild

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill **Drilling Method:** 41/4" HSA w/SS samplers

FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: R. Hasenyager

HANSON

BOREHOLE ID: G207 Well ID: G207

> Surface Elev: 630.61 ft. MSL **Completion:** 24.00 ft. BGS **Station:** 875,166.36N 2,514,837.94E

SAMPI	Æ	T	EST	INC		TOPOGR	APHIC MAP INFORMATION:	WATER LEVEL INFORMATION:			
Number Recov / Total (in) % Recovery		/ 6 in Jue	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Quadra Townsl	angle: Coffeen, IL hip: East Fork 11, Tier 7N; Range 3W	$\bar{\Lambda} = \bar{\Lambda} = $	Dry - '	While drilling Upon completion	
Number Recov / % Recov	Type	Blows / 6 in N - Value RQD	Moistu	Dry De	Qu (tsf Failure	Depth ft. BGS	Lithologic Description		orehole Detail	Elevation ft. MSL	Remarks
23/24 96%	ss	6-5 5-8 N=10	15			2	FILL - Yellowish brown (10YR5/6) with 10% gray (10YR6/1) and 5% black (10YR2/1) mottles, slightl moist, hard, silty CLAY with trace sand and slight tra gravel.	у 💹		630	
A 24/24 100%	ss	4-4 7-8 N=11	15				FILL - Gray (10YR5/1) with 20% yellowish brown (10YR5/8) mottles, moist, hard, silty CLAY with sand trace gravel.	and			
23/24 96%	ss	3-6 7-9 N=13	17			4	FILL - Yellowish brown (10YR5/8) with 20% gray (10YR6/1) mottles, moist, hard, silty CLAY with sand trace gravel.	and		626	
4A 24/24 100%	ss	3-4 6-7 N=10	16				FILL - Dark yellowish brown (10YR4/4), moist, hard, CLAY with slight trace sand and gravel. FILL - Gray (10YR5/1) with 20% yellowish brown (10YR5/8) mottles, moist, hard, silty CLAY with slig trace sand and gravel.				
5A 24/24 100%	ss	2-2 3-4 N=5	22			* =	Yellowish brown (10YR5/6) with 25% gray (10YR5/mottles, moist, firm, silty CLAY with slight trace sand gravel. Gray (10YR5/1) with 15% yellowish brown (10YR5/1)	and (8) $-$		622	
5B 22/24 92%	ss	2-2 2-5 N=4	27			8 10 10 12 12 12 12 12 12 12 12 12 12 12 12 12	mottles, moist, firm, silty CLAY with slight trace sand gravel. Gray (10YR5/1) with 40% yellowish brown (10YR5, mottles, very moist, firm, silty CLAY with slight trace s and gravel.	(8)	7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	620	
7A 24/24 100%	ss	2-2 2-3 N=4	27			12 - 12 - 14 - 14 - 14 - 14 - 14 - 14 -	Yellowish brown (10YR5/8) with 30% gray (10YR6/mottles, very moist, soft, silty CLAY with trace sand a gravel.	(1) nd		618	
3A 24/24 100%	ss	woh-1 2-3 N=3	25				Gray (10YR6/1) with 30% yellowish brown (10YR5/mottles, very moist, soft, silty CLAY with sand and sli	(8) — — — — — — — — — — — — — — — — — — —		616	
9A 23/24 96%	ss	woh-2 2-3 N=4	22			16 = 18 = 18	Yellowish brown (10YR5/6) with 25% gray (10YR6/mottles, very moist, very soft, sandy, silty CLAY with t gravel.	(1) race		614	
0A 24/24 100%	ss	woh-woh 2-3	19			20	Yellowish brown (10YR5/6) with 30% gray (10YR6/mottles, very moist, very soft, sandy CLAY with silt a trace gravel.	 (1) nd		612	

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 10/8/2010 **Finish:** 10/8/2010

WEATHER: Sunny, mild

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 41/4" HSA w/SS samplers

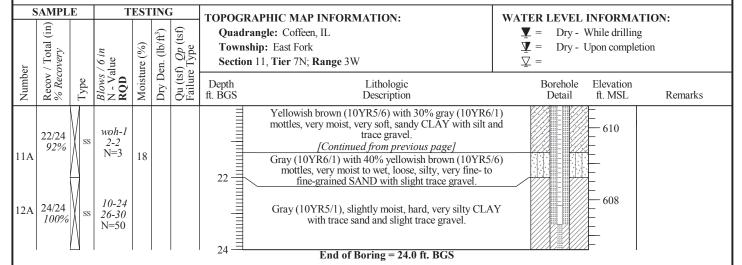
FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: R. Hasenyager

HANSON

BOREHOLE ID: G207 **Well ID:** G207

> Surface Elev: 630.61 ft. MSL Completion: 24.00 ft. BGS Station: 875,166.36N 2.514.837.94E



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 10/7/2010 **Finish:** 10/7/2010

WEATHER: Sunny, warm (lo-70's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 41/4" HSA w/SS samplers

FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: .



BOREHOLE ID: G208 **Well ID:** G208

 Surface Elev:
 630.57 ft. MSL

 Completion:
 24.00 ft. BGS

 Station:
 875,231.46N

 2,514,993.57E

	SAMPLE TESTING			EST	ING		TOPOGRAPHIC MAP INFORMATION:	WATER LEVEL INFORMATION:			
)er	Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	$ \mathbf{Y} = \text{Dry - While drilling} $ $ \mathbf{Y} = 23.92 - \text{Upon completion} $ $ \mathbf{Y} = Y$			
Numper	Recov % Re	Type	Blows N - V RQD	Moist	Dry L	Qu (ts Failur	Depth Lithologic ft. BGS Description	Borehole Elevation Detail ft. MSL Remarks			
Α	19/24 79%	ss	4-3 3-5 N=6	23			FILL - Brown (10YR4/3) with 5% dark gray (10Y and 5% dark yellowish brown (10YR4/6) mottles, firm, silty CLAY with trace sand and gravel.	moist,			
A	24/24 100%	ss	2-3 4-6 N=7	14			FILL - Brown (10YR4/3) with 15% dark gray (10Y and 5% dark yellowish brown (10YR4/6) mottles, firm, silty CLAY with trace sand and gravel. Very dark gray (10YR3/1), moist, firm, silty CLAY trace sand and gravel, trace roots. Dark gray (10YR4/1) with 10% dark yellowish b (10YR4/6) mottles, moist, firm, silty CLAY with sand, trace roots. Dark gray (10YR4/1) with 15% dark yellowish b (10YR4/6) mottles, moist, firm, silty CLAY with sand, trace roots.	628			
A	24/24 100%	ss	2-4 4-7 N=8	21			FILL - Brown (10YR4/3) with 15% dark gray (10 and 5% dark yellowish brown (10YR4/6) mottles, firm, silty CLAY with trace sand and gravel.	moist,			
A	24/24 100%	ss	2-4 6-8 N=10	17			8	624			
SΑ	20/24 83%	ss	2-2 4-5 N=6	24			Very dark gray (10YR3/1), moist, firm, silty CLA' trace sand and gravel, trace roots. Dark gray (10YR4/1) with 10% dark yellowish b (10YR4/6) mottles, moist, firm, silty CLAY with sand, trace roots.	rown			
δA	23/24 96%	ss	1-2 4-4 N=6	26			Dark gray (10YR4/1) with 15% dark yellowish b (10YR4/6) mottles, moist, firm, silty CLAY with sand, trace roots.	rown trace 620			
'A	19/24 79%	ss	1-2 2-3 N=4	23			Dark gray (10YR4/1) with 15% dark yellowish b (10YR4/6) mottles, moist, soft, silty CLAY with tra-	rown ce sand,			
3A	22/24 92%	ss	1-1 2-3 N=3	24			trace roots.	-616			
θA	24/24 100%	SS	1-1 2-3 N=3	24			Dark gray (10YR4/1) with 15% dark yellowish b (10YR4/6) mottles, moist, soft, silty CLAY with tra and gravel, trace roots.				
В			woh-wol	20			Dark gray (10YR4/1) with 15% dark yellowish b (10YR4/6) mottles, moist, soft, silty CLAY with tra and gravel, trace roots. Gray (10YR5/1) with 20% dark yellowish bro (10YR4/6) mottles, very moist, soft, silty, very fir fine-grained sandy CLAY with trace gravel.	wn ne- to			
0A 0B	22/24 92%	ss	1-2	20 17			Dark yellowish brown (10YR4/6) with 15% gr (10YR5/1) mottles, very moist to wet, soft, clayey fine- to medium-grained SAND with trace grav	, very			

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 10/7/2010 **Finish:** 10/7/2010

WEATHER: Sunny, warm (lo-70's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 41/4" HSA w/SS samplers

FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: .

HANSON
BOREHOLE ID: G208

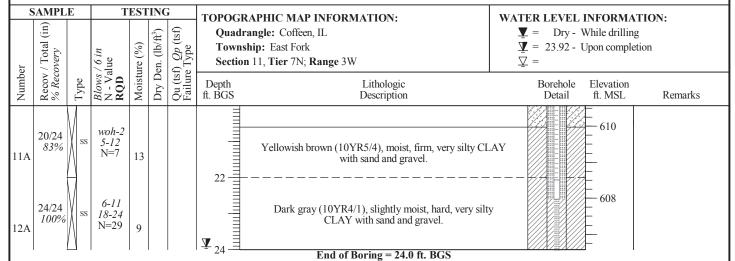
Well ID: G208

 Surface Elev:
 630.57 ft. MSL

 Completion:
 24.00 ft. BGS

 Station:
 875,231.46N

 2,514,993.57E



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 10/7/2010 **Finish:** 10/7/2010

WEATHER: Sunny, cool (lo-50's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 4½" HSA w/SS samplers

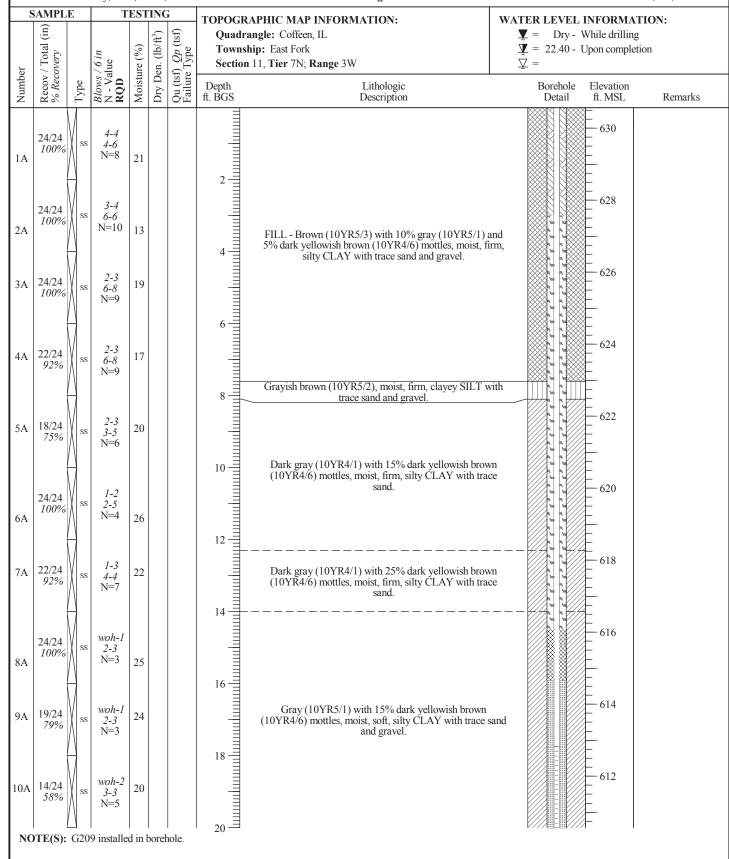
FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: .



BOREHOLE ID: G209 **Well ID:** G209

Surface Elev: 630.57 ft. MSL Completion: 24.00 ft. BGS Station: 875,298.23N 2,515,149.56E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 10/7/2010

Finish: 10/7/2010 **WEATHER:** Sunny, cool (lo-50's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 4½" HSA w/SS samplers

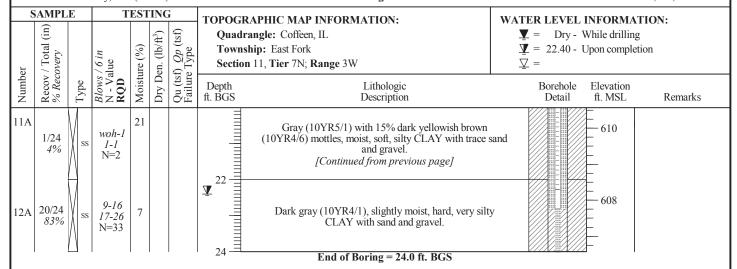
FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: .

HANSON

BOREHOLE ID: G209 **Well ID:** G209

Surface Elev: 630.57 ft. MSL Completion: 24.00 ft. BGS Station: 875,298.23N 2,515,149.56E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 10/6/2010 **Finish:** 10/6/2010

WEATHER: Sunny, warm (mid-70's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 41/4" HSA w/SS samplers

FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: .



BOREHOLE ID: G210 **Well ID:** G210

 Surface Elev:
 630.48 ft. MSL

 Completion:
 25.00 ft. BGS

 Station:
 875,359.71N

 2,515,298.97E

S	AMPL	Ε	Т	EST	ING		TOPOGRA	APHIC MAP INFORMATION:	WATER LEVEL INFORMATION:			
oer	Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Townsh	ngle: Coffeen, IL ip: East Fork 11, Tier 7N; Range 3W	$\underline{\underline{\mathbf{Y}}}$ = 20.00 - While drilling $\underline{\underline{\mathbf{Y}}}$ = 19.90 - Upon completion $\underline{\underline{\mathbf{Y}}}$ =			
Number	Recov % Re	Type	Blows N - V RQD	Moist	Dry I	Qu (t Failu	Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL Remarks			
A	20/24 83%	SS	4-3 5-6 N=8	16			4		630			
A	24/24 100%	ss	3-4 7-7 N=11	15			4-=	FILL - Brown (10YR4/3) with 20% dark gray (N5/1 mottles, moist, firm, silty CLAY with trace sand and gra) 628 			
A	24/24 100%	SS	3-5 10-9 N=15	19			6-		626			
A	24/24 100%	ss	3-6 9-11 N=15	17				FILL - Dark grayish brown (10YR4/2), slightly moist, f clayey SILT with trace sand and gravel.				
)					8	Gray (10YR5/1) with 10% dark grayish brown (10YR- mottles, moist, firm, silty CLAY with trace sand.	4/2)			
iΑ	24/24 100%	SS	3-4 5-7 N=9	15				Dark gray (10YR4/1), moist, firm, silty CLAY with trasand, trace roots. Gray (10YR5/1) with 15% yellowish brown (10YR5/mottles, moist, firm, silty CLAY with trace sand.				
δA	22/24 92%	ss	2-2 4-6 N=6	26			10 = -	Gray (10YR5/1) with 15% yellowish brown (10YR5/mottles, moist, firm, silty CLAY with trace sand and gra	ivel.			
7A	19/24 79%	ss	1-3 3-5 N=6	23			14	Gray (10YR5/1) with 5% dark yellowish brown (10YR mottles, moist to very moist, firm, silty CLAY with sar	4/6) ad			
3A	24/24 100%	ss	2-2 2-4 N=4	26			16	Gray (10YR5/1) with 5% dark yellowish brown (10YR mottles, moist, soft, silty CLAY with sand and trace gra				
ЭА	24/24 100%	ss	1-1 2-3 N=3	24			16 = 18 = 18	Gray (10YR5/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with sand a trace gravel.	and			
0A	20/24 83%	ss	1-1 2-2 N=3	24			¥ ₂₀	Gray (N6/1), very moist, very soft, silty, very fine-to fine-grained sandy CLAY with trace gravel.	612			

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A

DATES: Start: 10/6/2010

Finish: 10/6/2010

SAMPLE

WEATHER: Sunny, warm (mid-70's)

TESTING

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill **Drilling Method:** 41/4" HSA w/SS samplers

FIELD STAFF: Driller: D. Mahurin

Helper: J. Litsch/D. Smail Eng/Geo: .

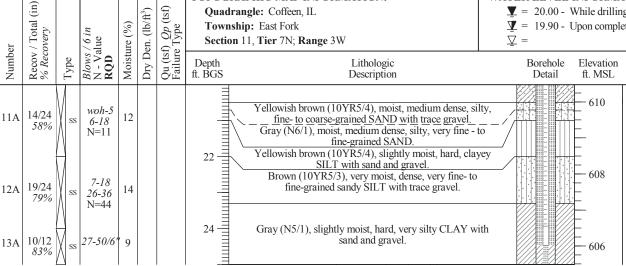


BOREHOLE ID: G210 Well ID: G210

Surface Elev: 630.48 ft. MSL Completion: 25.00 ft. BGS **Station:** 875,359.71N 2,515,298.97E

Remarks





Quadrangle: Coffeen, IL

Township: East Fork

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 10/11/2010

Finish: 10/11/2010 **WEATHER:** Sunny, warm (lo-70's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 41/4" HSA w/SS samplers

FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: .



BOREHOLE ID: G211 **Well ID:** G211

 Surface Elev:
 630.31 ft. MSL

 Completion:
 24.00 ft. BGS

 Station:
 875,424.49N

 2,515,449.06E

- 2	SAMPL	Ľ	Т	EST	INC	j	TOPOGR	APHIC MAP INFORMATION:	WATER LEVEL INFORMATION:
er	Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Towns	angle: Coffeen, IL hip: East Fork 11, Tier 7N; Range 3W	$\underline{\underline{\mathbf{Y}}}$ = 20.00 - While drilling $\underline{\underline{\mathbf{Y}}}$ = 20.60 - Upon completion $\underline{\underline{\mathbf{Y}}}$ =
Number	Recov % Rec	Type	Blows N - V RQD	Moist	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL Remarks
lΑ	20/24 83%	ss	5-3 4-5 N=7	23			2-	FILL - Brown (10YR4/3) with 20% dark gray (10YR4 mottles, moist, firm, silty CLAY with trace sand and gray	avel.
2A	20/24 83%	ss	3-5 5-8 N=10	17					628
3A	24/24 100%	ss	2-3 5-7 N=8	24			6	FILL - Dark gray (10YR4/1) with 20% brown (10YR4 and 5% dark yellowish brown (10YR4/6) mottles, mo firm, silty CLAY with trace sand and gravel.	ist,
4A	24/24 100%	ss	3-5 7-9 N=12	29					624
5A	24/24 100%	ss	2-2 3-5 N=5	31			8 = = = = = = = = = = = = = = = = = = =	Dark gray (10YR4/1) with 20% dark yellowish brow (10YR4/6) mottles, moist, firm, silty CLAY, slight traroots.	n 622
6A	17/24 71%	ss	1-2 4-4 N=6	19			10 = 12 = 12	Dark gray (10YR4/1) with 20% dark yellowish brow (10YR4/6) mottles, moist, firm, silty CLAY with trac sand.	620 m
7A	24/24 100%	ss	1-2 2-4 N=4	22				Dark gray (10YR4/1) with 20% dark yellowish brow	n 618
8A	24/24 100%	ss	1-1 3-2 N=4	28			14 = 16 = 16 = 1	(10YR4/6) mottles, moist, soft, silty CLAY with trace s and gravel.	and 616
9A	22/24 92%	ss	1-1 1-2 N=2	19			18	Dark yellowish brown (10YR4/6) with 30% gray (10YR5/1) mottles, very moist, soft, very fine-to fine-grained sandy CLAY with trace gravel.	614
0A	19/24 79%	ss	1-4 5-11 N=9	13			18 ————————————————————————————————————	Dark yellowish brown (10YR4/4) with 15% grayish brown (10YR5/2) mottles, moist, firm, clayey SILT with sand gravel.	own and 612

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A

DATES: Start: 10/11/2010

Finish: 10/11/2010

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill **Drilling Method:** 41/4" HSA w/SS samplers

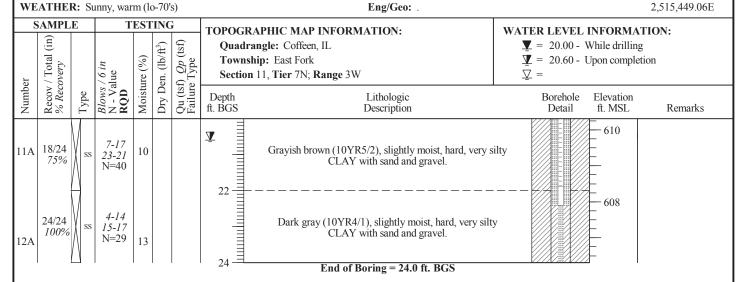
FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: .



BOREHOLE ID: G211 Well ID: G211

> Surface Elev: 630.31 ft. MSL Completion: 24.00 ft. BGS **Station:** 875,424.49N 2,515,449.06E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 10/11/2010

Finish: 10/11/2010 WEATHER: Sunny, warm (lo-80's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 41/4" HSA w/SS samplers

FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: .



BOREHOLE ID: G212 **Well ID:** G212

 Surface Elev:
 630.59 ft. MSL

 Completion:
 24.00 ft. BGS

 Station:
 875,486.50N

 2,515,583.03E

SAMPL	Ł	1	ESI	INC		TOPOGR	APHIC MAP INFORMATION:	WATER LEVEL INFORMATION:			
Recov / Total (in)		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Quadra Townsl	angle: Coffeen, IL nip: East Fork 11, Tier 7N; Range 3W	$ \mathbf{V} = 19.00 - 10 $ $ \mathbf{V} = 20.72 - 10 $ $ \mathbf{V} = 20.72 - 10 $	_	on	
Recov / %	Type	Blows N - V RQD	Moist	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
A 24/24 100%	SS	4-3 3-6 N=6	17				FILL - Brown (10YR4/3), slightly moist, firm, silty CL with trace sand and gravel.	AY	630		
A 24/24 100%	SS	2-3 4-5 N=7	21			2 =	FILL - Dark gray (10YR4/1) with 20% brown (10YR4 and 10% dark yellowish brown (10YR4/6) mottles, mother firm, silty CLAY with trace sand.	1/3) ist,	628		
24/24 100%	ss	2-5 6-7 N=11	13			4-	FILL - Brown (10YR4/3) with 15% dark gray (10YR4)	1/1)	626		
A 24/24 100%	ss	2-5 7-10 N=12	15			6	and 10% dark yellowish brown (10YR4/6) mottles, mot firm, silty CLAY with trace sand and gravel.		- - - - - - - - - - -		
5A 24/24 100%	SS	2-2 4-7 N=6	29				Dark gray (10YR4/1) with 10% dark yellowish brow (10YR4/6) mottles, moist, firm, silty CLAY, slight tra	m ce	622		
5A 18/24 75%	ss	2-3 4-6 N=7	23			10 =	Dark gray (10YR4/1) with 10% dark yellowish brow (10YR4/6) mottles, moist, firm, silty CLAY with traces and.	m se	620		
'A 17/24 71%	SS	1-2 2-2 N=4	25			12 =			618		
3A 24/24 100%	SS	woh-1 2-3 N=3	27			16	Dark gray (10YR4/1) with 15% dark yellowish brow (10YR4/6) mottles, moist, soft, silty CLAY with trace s and gravel.	n and	616		
9A 22/24 92%	ss	1-1 2-2 N=3	25			16 =			614		
0A 24/24 100%	SS	woh-woh 1-2	19			18 − −	Gray (10YR5/1) with 30% dark yellowish brown (10YR4/6) mottles, very moist, very soft, silty, very fine fine-grained sandy CLAY with trace gravel. Gray (10YR5/1), loose, wet, silty, very fine- to	e- to	612		

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A

DATES: Start: 10/11/2010

Finish: 10/11/2010

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill **Drilling Method:** 41/4" HSA w/SS samplers

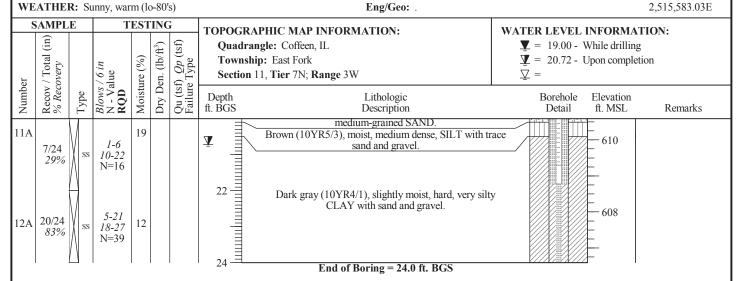
FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: .



BOREHOLE ID: G212 Well ID: G212

> Surface Elev: 630.59 ft. MSL Completion: 24.00 ft. BGS **Station:** 875,486.50N 2,515,583.03E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 10/12/2010

Finish: 10/12/2010
WEATHER: Partly cloudy, mild (mid-50's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 4½" HSA w/SS samplers

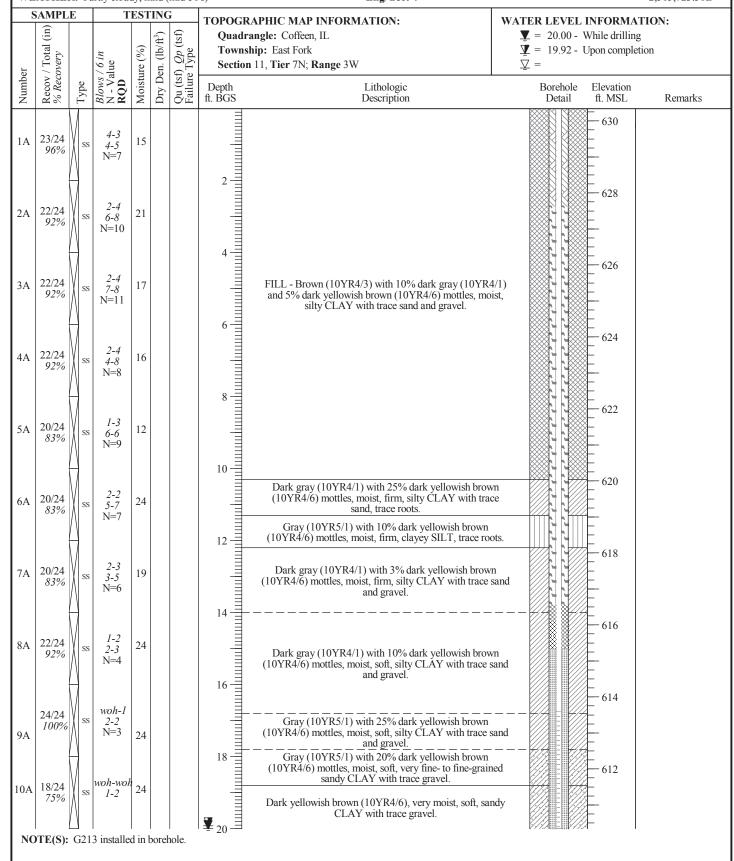
FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: .



BOREHOLE ID: G213 Well ID: G213

> Surface Elev: 630.34 ft. MSL Completion: 24.00 ft. BGS Station: 875,544.37N 2,515,723.51E



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 10/12/2010

Finish: 10/12/2010 **WEATHER:** Partly cloudy, mild (mid-50's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 4½" HSA w/SS samplers

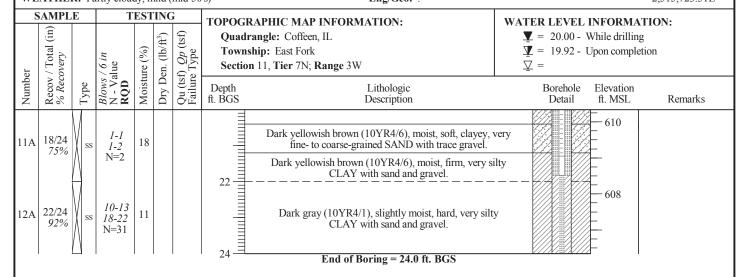
FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: .

HANSON
BOREHOLE ID: G213

Well ID: G213

Surface Elev: 630.34 ft. MSL Completion: 24.00 ft. BGS Station: 875,544.37N 2,515,723.51E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 10/14/2010

Finish: 10/14/2010 WEATHER: Sunny, cool (lo-40's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 41/4" HSA w/SS samplers

FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: .



BOREHOLE ID: G214 **Well ID:** G214

 Surface Elev:
 630.39 ft. MSL

 Completion:
 24.00 ft. BGS

 Station:
 875,668.02N

 2,515,960.84E

	SAMPLE TESTING		Т	EST	INC	2	Eng/Geo: .	2,515,900.841			
	l (in)				Dry Den. (lb/ft³)	Qu (tsf) Qp (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION: Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	WATER LEVEL INFORMATION: $ \Psi = \text{Dry - While drilling} $ $ \Psi = \text{Dry - Upon completion} $ $ \nabla = $			
Number	Recov / Tota % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry De	Qu (tsf) Failure	Depth Lithologic fl. BGS Description	Borehole Elevation Detail ft. MSL Remarks			
1A	24/24 100%	SS	6-7 7-9 N=14	15			FILL - Brown (10YR4/3) with 10% dark gray (10Y and 10% dark yellowish brown (10YR4/6) mottles, firm, silty CLAY with trace sand and gravel. FILL - Dark gray (10YR4/1) with 15% dark yellow brown (10YR4/6) mottles, moist, firm, silty CLAY trace sand and gravel.	moist,			
2A	24/24 100%	SS	3-3 6-5 N=9	22			FILL - Dark gray (10YR4/1) with 15% dark yello brown (10YR4/6) mottles, moist, firm, silty CLAY trace sand and gravel.	owish — 628			
3A	24/24 100%	SS	3-4 6-8 N=10	18				- 626			
4A	24/24 100%	SS	3-4 7-10 N=11	17			FILL - Brown (10YR4/3) with 10% dark yellowish (10YR4/6) and 5% dark gray (10YR4/1) mottles, if firm, silty CLAY with trace sand and gravel.	brown moist, — 624			
5A	24/24 100%	ss	3-2 4-5 N=6	19			FILL - Brown (10YR4/3) with 10% dark yellowish (10YR4/6) and 5% dark gray (10YR4/1) mottles, ifirm, silty CLAY with trace sand and gravel. Grayish brown (10YR5/2) with 15% dark gray (10Yand 10% dark yellowish brown (10YR4/6) mottles, firm, clayey SILT with trace sand and gravel. Brown (10YR4/3) with 15% dark gray (10YR4/110% dark yellowish brown (10YR4/6) mottles, mois silty CLAY with trace sand and gravel.	moist,			
6A	24/24 100%	SS	2-3 4-7 N=7	24			Brown (10YR4/3) with 15% dark gray (10YR4/1 10% dark yellowish brown (10YR4/6) mottles, mois silty CLAY with trace sand and gravel.) and st, firm,			
7A 7B	24/24 100%	SS	2-3 4-6 N=7	22 16			Gray (10YR6/1) with 30% dark yellowish brov (10YR4/6) mottles, moist, soft, silty CLAY.	618			
8A	24/24 100%	SS	woh-2 3-4 N=5	22			Dark gray (10YR4/1) with 10% dark yellowish by (10YR4/6) mottles, moist, firm, silty CLAY with tra	rown ce sand 616			
9A	22/24 92%	SS	1-2 2-3 N=4	21			Dark gray (10YR4/1) with 10% dark yellowish by (10YR4/6) mottles, moist, soft, silty CLAY with transport and gravel. Gray (10YR5/1) with 30% dark yellowish brow (10YR4/6) mottles, moist, soft, silty CLAY with transport and gravel.	rown ce sand 614			
10A	22/24 92%	SS	woh-2 2-2 N=4	15			and gravel. Gray (10YR5/1), wet, loose, silty, very fine-to-medium-grained SAND with trace gravel and clayey	612			
10B				21			Gray (10 Y R5/1), wet, loose, silty, very fine-to- medium-grained SAND with trace gravel and clayey	seams.			

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A

DATES: Start: 10/14/2010

Finish: 10/14/2010

WEATHER: Sunny, cool (lo-40's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill **Drilling Method:** 41/4" HSA w/SS samplers

FIELD STAFF: Driller: D. Mahurin

Eng/Geo: .

Helper: J. Litsch/D. Smail



BOREHOLE ID: G214 Well ID: G214

> Surface Elev: 630.39 ft. MSL **Completion:** 24.00 ft. BGS **Station:** 875,668.02N 2,515,960.84E

	SAMPL	E	T	EST	INC	j	TOPOGR	APHIC MAP INFORMATION:	WATER LEVEL INFORMATION:
er	/ Total (in)		/ 6 in Ilue	ıre (%)	Den. (lb/ft³)	f) <i>Qp</i> (tsf) Type	Quadra Townsh Section	ngle: Coffeen, IL ip: East Fork 11, Tier 7N; Range 3W	$\underline{\Psi}$ = Dry - While drilling $\underline{\Psi}$ = Dry - Upon completion $\underline{\nabla}$ =
Number	Recov / Tota % Recovery	Type	Blows N - Vz RQD	Moisture	Dry D	Qu (tsf) Failure	Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL Remarks
11A 11B	83%	SS	woh-woh 3-12	24 14			22	Gray (10YR5/1), wet, loose, silty, very fine- to medium-grained SAND with trace gravel and clayey sea [Continued from previous page] Dark yellowish brown (10YR4/6), moist, firm, very si CLAY with trace sand and gravel.	
12A	24/24 100%	SS	12-28 32-28 N=60	7				Dark gray (10YR4/1), slightly moist, hard, very silty CLAY with trace sand and gravel.	y 608
1							24 ===	End of Boring = 24.0 ft. BGS	,

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 10/13/2010 **Finish:** 10/13/2010

WEATHER: Sunny, warm, windy (hi-60's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 41/4" HSA w/SS samplers

FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: .



BOREHOLE ID: G215 **Well ID:** G215

 Surface Elev:
 630.48 ft. MSL

 Completion:
 24.31 ft. BGS

 Station:
 875,810.19N

 2,515,971.55E

S	AMPL	E	Т	EST	INC		TOPOGR	APHIC MAP INFORMATION:	WATER LEVEL INFORMATION:	
ber	Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Quadra Townsh Section	angle: Coffeen, IL nip: East Fork 11, Tier 7N; Range 3W	$\underline{\Psi}$ = Dry - While drilling $\underline{\Psi}$ = Dry - Upon completion $\underline{\nabla}$ = 22.52 - 10/14/10	
Number	Reco'	Type	Blow, N - V RQD	Mois	Dry I	Qu (t Failu	Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL Ren	narks
1A	23/24 96%	ss	5-3 3-5 N=6	18					630	
2A	19/24 79%	ss	3-3 5-6 N=8	17			4-	FILL - Brown (10YR4/3) with 30% dark gray (10YR4 and 10% dark yellowish brown (10YR4/6) mottles, motifirm, silty CLAY with trace sand and gravel.	4/1) bist, = 628	
3A	20/24 83%	SS	2-3 7-7 N=10	13					626	
4A	23/24 96%	SS	3-6 6-7 N=12	16			6 -	FILL - Dark grayish brown (10YR4/2), moist, firm, si CLAY with trace sand and gravel.		
4B				27			8	FILL - Gray (10YR5/1) with 15% dark yellowish brov (10YR4/6) mottles, moist, firm, silty CLAY with traces and.	wn ce	
5A	20/24 83%	SS	3-3 3-5 N=6	20				Very dark gray (10YR3/1), moist, firm, silty CLAY w trace sand, trace roots.	rith 622	
6A	13/24 54%	ss	2-2 3-5 N=5	24			8 10 11 12 12 12 12 12 12 12 12 12 12 12 12	Dark gray (10YR4/1) with 30% dark yellowish brow (10YR4/6) moist, firm, silty CLAY with trace sand.		
7A	19/24 79%	SS	2-3 4-6 N=7	17				Gray (10YR5/1) with 30% dark yellowish brown (10YR4/6) mottles, moist, firm, clayey SILT with transand.	ce618	
8A	20/24 83%	SS	2-3 4-5 N=7	19			14 -	Dark gray (10YR4/1), moist, firm, clayey SILT with tr sand.	race	
9A	22/24 92%	ss	1-3 3-4 N=6	19			18	Dark gray (10YR4/1) with 30% Dark yellowish brow (10YR4/6) mottles, moist, firm, silty CLAY with trace s and gravel.	vn sand	
10A	24/24 100%	SS	woh-1 2-2 N=3	17				Dark gray (10YR4/1) with 30% Dark yellowish brow (10YR4/6) mottles, moist, soft, sandy CLAY with tragravel.	vn ce =	

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A

DATES: Start: 10/13/2010 Finish: 10/13/2010

WEATHER: Sunny, warm, windy (hi-60's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill **Drilling Method:** 41/4" HSA w/SS samplers

FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: .

HANSON

BOREHOLE ID: G215 Well ID: G215

> Surface Elev: 630.48 ft. MSL Completion: 24.31 ft. BGS **Station:** 875,810.19N 2,515,971.55E

SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Recov / Total (in) % Recovery Qu (tsf) *Qp* (tsf) Failure Type Dry Den. (lb/ft³) Quadrangle: Coffeen, IL ▼ = Dry - While drilling Moisture (%) **Township:** East Fork $\underline{\Psi}$ = Dry - Upon completion Blows / 6 in N - Value RQD Section 11, Tier 7N; Range 3W ∇ = 22.52 - 10/14/10 Depth Lithologic Borehole Elevation ft. BGS Detail Remarks

Number Description ft. MSL Dark yellowish brown (10YR4/6), moist, medium dense, 610 clayey SILT with sand and trace gravel. 2-4 20/24 17 11A 4-4 Yellowish brown (10YR5/6), moist, medium dense, silty, 83% very fine- to fine-grained SAND.

Dark yellowish brown (10YR4/6) with 30% dark gray (10YR4/1) mottles, moist, firm, sandy CLAY with trace N=822 gravel. ∇ 608 Gravish brown (10YR5/2), slightly moist, very firm, very 7-11 17-19 24/24 silty CLAY with sand and gravel. 12A 11 SS 100% N = 28Dark gray (10YR4/1), slightly moist, hard, very silty 12B 9 0/4 CLAY with sand and gravel. BD 0%

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 10/13/2010 **Finish:** 10/13/2010

WEATHER: Partly cloudy, mild, windy (lo-60's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 41/4" HSA w/SS samplers

FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: .



BOREHOLE ID: G216 **Well ID:** G216

 Surface Elev:
 630.28 ft. MSL

 Completion:
 26.00 ft. BGS

 Station:
 875,976.05N

 2,515,968.53E

SS 2-4 N=4	3-2 2-4	Moisture (%) Dry Den. (lb/ft³)	Qu (tsf) Qp (tsf) Failure Type	Quadra Townsh Section Depth	ngle: Coffeen, IL ip: East Fork 11, Tier 7N; Range 3W	$\underline{\underline{\mathbf{Y}}} = 21.00 - \underline{\underline{\mathbf{Y}}} = \text{Dry} - \underline{\underline{\mathbf{Y}}} = \mathbf{V}$	While drilling Upon completion	on
3-2 SS 2-4 N=4	3-2 2-4	Moist Dry E	Qu (ts Failur	Denth				
SS 2-4 N=4	2-4		1	ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
		22		2 —			630	
N='	J-7	28		2	FILL - Brown (10YR4/3) with 20% dark gray (10YR4 and 5% dark yellowish brown (10YR4/6) mottles, moi firm, silty CLAY with trace sand and gravel.	/6) st,	628	
SS 2-2 N=0		19			FILL - Dark gray (10YR4/1) with 15% brown (10YR4 and 10% dark yellowish brown (10YR4/6) mottles, mo firm, clayey SILT with trace sand and gravel.	/3) ist,	626	
ss 6-6	3-4 6-6 [=10	19		8	FILL - Dark grayish brown (10YR4/2) with 10% dar yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		624	
SS 3-6 N=0	J-0	18		10 —	Dark gray (10YR4/1) with 40% gray (10YR6/1) and 10	7, 7, 7, 7, 7, 7, 7, 7, 7, 7, 7, 7, 7, 7	622	
	J=4	17		10 =	dark yellowish brown (10YR4/6) mottles, moist, firm clayey SILT with trace sand.	, , , , , , , , , , , , , , , , , , , ,	620	
SS 3-4 N=:	J-7	20		12	Gray (10YR5/1) with 20% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with trace stand gravel.	and	618	
SS 2-2 5-5 N=	J-J	20		I <u>∃</u>	Very dark gray (10YR3/1), moist, firm, silty CLAY witrace sand.	th	616	
ss 3-3	3-3	18		16	Gray (10YR5/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace s and gravel.	and	614	
		17			gravel.	ee , , ,	612	
ss		N=5	3-3 N=5	3-3 N=5	woh-2 3-3 N=5 N=5 18 18 18 18-11 17	Oh-woh 1-2 Gray (10YR5/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, firm, sandy CLAY with trace gravel. Dark yellowish brown (10YR4/6) and 40% gray	Oh-woh 17 Gray (10YR5/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, firm, sandy CLAY with trace gravel. Dark yellowish brown (10YR4/6) and 40% gray	Gray (10YR5/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, firm, sandy CLAY with trace gravel. Dark yellowish brown (10YR4/6) and 40% gray

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 10/13/2010 **Finish:** 10/13/2010

WEATHER: Partly cloudy, mild, windy (lo-60's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 41/4" HSA w/SS samplers

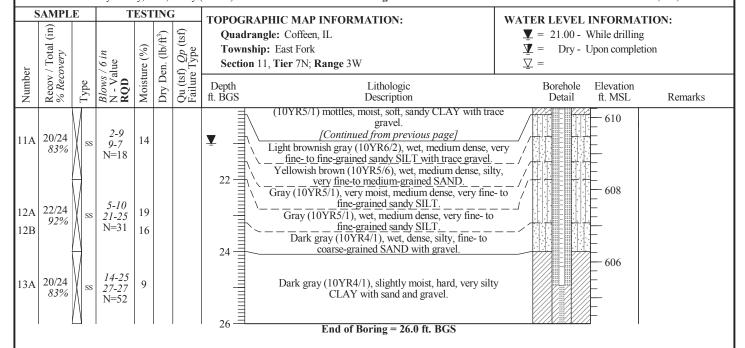
FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: .



BOREHOLE ID: G216 **Well ID:** G216

Surface Elev: 630.28 ft. MSL Completion: 26.00 ft. BGS Station: 875,976.05N 2,515,968.53E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 10/12/2010

Finish: 10/12/2010 WEATHER: Sunny, warm (lo-80's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 41/4" HSA w/SS samplers

FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: .



BOREHOLE ID: G217 **Well ID:** G217

 Surface Elev:
 630.67 ft. MSL

 Completion:
 26.00 ft. BGS

 Station:
 876,185.57N

 2,515,963.02E

SAMPL	E	Т	EST	INC		TOPOGRA	APHIC MAP INFORMATION:	WATER LEVEL INFORMATION:			
Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Townsh	ingle: Coffeen, IL nip: East Fork 11, Tier 7N; Range 3W	$ \mathbf{Y} = 23.00 - 10 $ $ \mathbf{Y} = 24.82 - 10 $ $ \mathbf{Y} = 23.98 - 10 $	Upon completion	on	
Recov / '% Recov	Type	Blows N - V RQD	Moist	Dry I	Qu (ts Failu	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
A 20/24 83%	SS	5-2 3-4 N=5	21				FILL - Brown (10YR4/3) with 10% dark gray (10YR4	//)	630		
19/24 79%	ss	2-3 5-6 N=8	28			2 = 4 = 4	and 5% dark yellowish brown (10YR4/6) mottles, mor firm, silty CLAY with trace sand and gravel.		628		
3A 19/24 79%	SS	2-3 6-7 N=9	14				FILL - Dark gray (10YR4/1) with 25% dark yellowis brown (10YR4/6) mottles, moist, firm, silty CLAY witrace sand and gravel.	 sh th	— 626 —		
1A 23/24 96%	SS	5-6 7-8 N=13	15			8 =	FILL - Brown (10YR4/3) with 10% dark gray (10YR4 and 10% dark yellowish brown (10YR4/6) mottles, motifirm, silty CLAY with trace sand and gravel.		— 624 — 624		
5A 20/24 83%	SS	3-5 7-6 N=12	13				FILL - Dark grayish brown (10YR4/2) with 5% darl yellowish brown (10YR4/6) slightly moist, firm, clayer SILT with trace sand and gravel.	ey	622		
5A 19/24 79%	SS	3-3 4-5 N=7	27			10	FILL - Very dark gray (10YR3/1), moist, firm, silty CL with trace sand and gravel. Dark gray (10YR4/1), moist, firm, silty CLAY with traces and and gravel. Gray (10YR5/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with traces and, trace roots.	ace	620		
7A 18/24 75%	SS	3-4 6-8 N=10	28			12 ==	Dark grayish brown (10YR4/2) with 10% dark yellow brown (10YR4/6) mottles, moist, firm, silty CLAY transand, trace roots.	ish ce	618		
3A 20/24 83%	SS	2-4 6-8 N=10	16			14 =	Dark gray (10YR4/1), moist, firm, silty CLAY with transand and gravel.	ace	616		
9A 19/24 79%	SS	2-3 4-5 N=7	26			16 =	Dark gray (10YR4/1) with 10% yellowish brown (10YR5/6) mottles, moist, firm, silty CLAY with trace s and gravel.	sand	614		
0A 19/24 79%	SS	1-2 2-3 N=4	18			18	Gray (10YR5/1) with 5% dark yellowish brown (10YR mottles, moist, soft, silty CLAY with sand and trace gra		612		

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 10/12/2010

Finish: 10/12/2010 **WEATHER:** Sunny, warm (lo-80's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 4½" HSA w/SS samplers

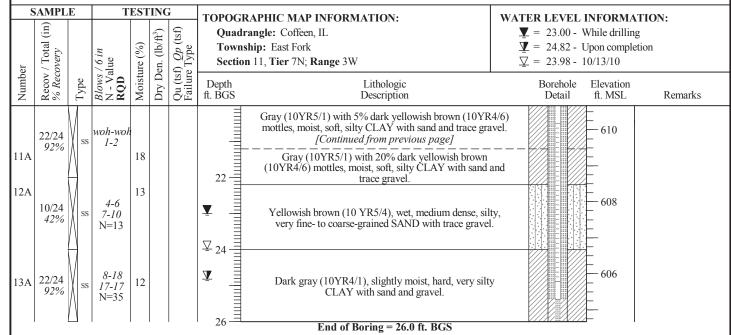
FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: .



BOREHOLE ID: G217 **Well ID:** G217

Surface Elev: 630.67 ft. MSL Completion: 26.00 ft. BGS Station: 876,185.57N 2,515,963.02E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 10/12/2010

Finish: 10/12/2010 **WEATHER:** Partly cloudy, warm (lo-70's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 4¼" HSA w/SS samplers

FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: .



BOREHOLE ID: G218 **Well ID:** G218

 Surface Elev:
 630.64 ft. MSL

 Completion:
 26.00 ft. BGS

 Station:
 876,380.92N

 2,515,962.16E

s 2-2 N=3 S N=5 N=5	(%) Woistnre (%) 20	Dry Den. (lb/tf) Qu (tsf) Qp (tsf) Failure Type	Quadra Townsh Section Depth ft. BGS	APHIC MAP INFORMATION: angle: Coffeen, IL aip: East Fork 11, Tier 7N; Range 3W Lithologic Description FILL - Brown (10YR4/3) with 15% dark gray (10YR4 and 5% dark yellowish brown (10YR4/6) mottles, moi	
s 4-1 2-1 N=3	20	Dry D Qu (ts Failur		Description FILL - Brown (10YR4/3) with 15% dark gray (10YR4 and 5% dark yellowish brown (10YR4/6) mottles, moi	Detail ft. MSL Remarks 630
s 2-1 N=3 s 2-2 s 3-5			2 —	and 5% dark yellowish brown (10YR4/6) mottles, moi	/1)
s 3-5	20		. ⊢	soft, silfy CLAY with trace sand and gravel.	st,
			4-3	FILL - Dark gray (10YR4/1) with 30% brown (10YR4	628
s 2-3 4-8 N=7	17			and 10% dark yellowish brown (10YR4/6) mottles, mo firm, silty CLAY with trace sand and gravel.	ist, = 626
s 2-5 6-8 N=11	14		8-	FILL - Brown (10YR5/3) with 10% dark gray (10YR4 mottles, slightly moist, firm, clayey SILT with trace sa	71) = 624
s 3-4 8-7 N=12	17				622
s 2-2 3-5 N=5	19		10 = 12 = 12	brown (10YR4/6) mottles, moist, firm, clayey ŠILT w trace sand. Gray (10YR5/1) with 30% dark yellowish brown	620
s 2-3 5-7 N=8	22			sand, slight trace roots.	618
s 2-3 4-5 N=7	19		14 -	Dark gray (10YR4/1) with 15% dark yellowish brow (10YR4/6) mottles, moist, firm, clayey SILT with trac sand.	n xe = 616
s 2-2 2-4 N=4	19		16 = 18 = 18	Gray (10YR5/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with trace s and gravel.	and 614
s 1-2 2-3 N=4	18				612
s s s s s s s s s s s s s s s s s s s	N=7 2-5 6-8 N=11 3-4 8-7 N=12 2-2 3-5 N=5 2-3 5-7 N=8 2-3 4-5 N=7 2-2 2-4 N=4	2-5 6-8 N=11 3-4 8-7 N=12 19 2-2 3-5 N=5 25 2-3 5-7 N=8 22 N=8 19 19 2-2 3-5 N=7 19 19 19 19 19 19 19 19 19 19	2-5 6-8 N=11 3-4 8-7 N=12 17 N=12 19 2-2 3-5 N=5 25 2-3 5-7 N=8 22 N=8 19 19 1-2 2-3 2-3 19 1-2 2-3 2-3 19	2-5 6-8 14 N=11	Dark gray (10YR4/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, firm, clayey SILT with traces and. 2-3 4-5 N=7 16 Gray (10YR5/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with trace s and gravel. 1-2 2-3 N=4 18

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A

DATES: Start: 10/12/2010

Finish: 10/12/2010

WEATHER: Partly cloudy, warm (lo-70's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill **Drilling Method:** 41/4" HSA w/SS samplers

FIELD STAFF: Driller: D. Mahurin

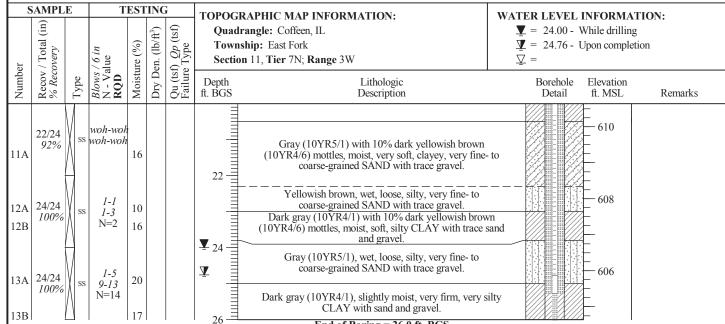
Helper: J. Litsch/D. Smail

Eng/Geo: .



BOREHOLE ID: G218 Well ID: G218

> Surface Elev: 630.64 ft. MSL Completion: 26.00 ft. BGS **Station:** 876,380.92N 2,515,962.16E



End of Boring = 26.0 ft. BGS

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 2/26/2008 **Finish:** 2/26/2008

CONTRACTOR: Testing Service Corp. **Rig mfg/model:** CME-650 Track Drill

Drilling Method: 31/4" HSA w/SS & CME samplers

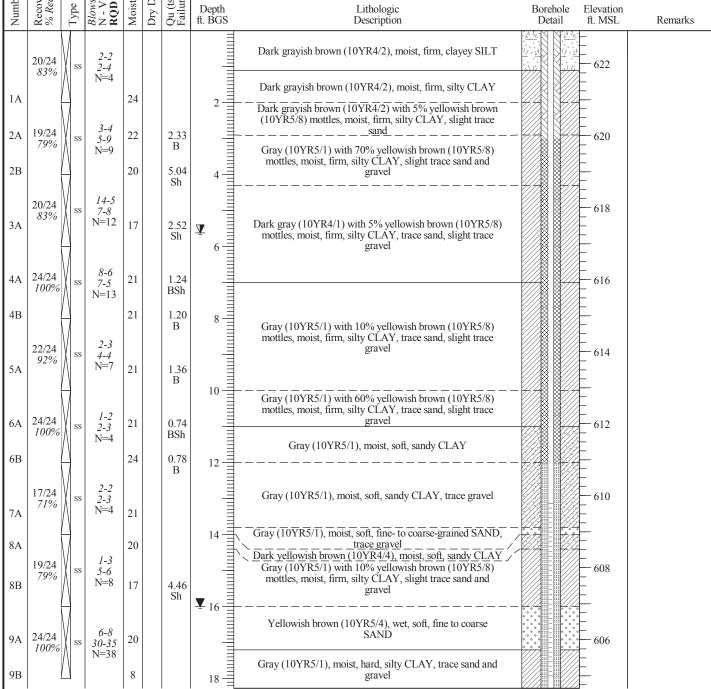
FIELD STAFF: Driller: B. Williamson

BOREHOLE ID: G270

Well ID: G270

Surface Elev: 622.92 ft. MSL Completion: 18.27 ft. BGS Station: 874,801.92N

Helper: R. Keedy WEATHER: Overcast, cold Eng/Geo: 2,514,996.84E SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Qu (tsf) *Qp* (tsf) Failure Type $\mathbf{V} = 16.00$ - While drilling Dry Den. (lb/ft³) Quadrangle: Coffeen, IL Recov / Total (% Recovery Moisture (%) Township: East Fork Ψ = 5.62 - 3/12/08 Blows / 6 in N - Value RQD Section 11, Tier 7N; Range 3W Number Depth Lithologic Borehole Elevation ft. BGS Description Detail ft. MSL Remarks Dark grayish brown (10YR4/2), moist, firm, clayey SILT 2-2 20/24 622 2-4 83%



End of Boring = 18.27 ft. BGS

NOTE(S):

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 9/9/2009 Finish: 9/10/2009

WEATHER: Sunny, warm (70's)

CONTRACTOR: Layne-Western Co **Rig mfg/model:** CME-750 ATV Drill

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: G. Mills Helper: J. Twellman

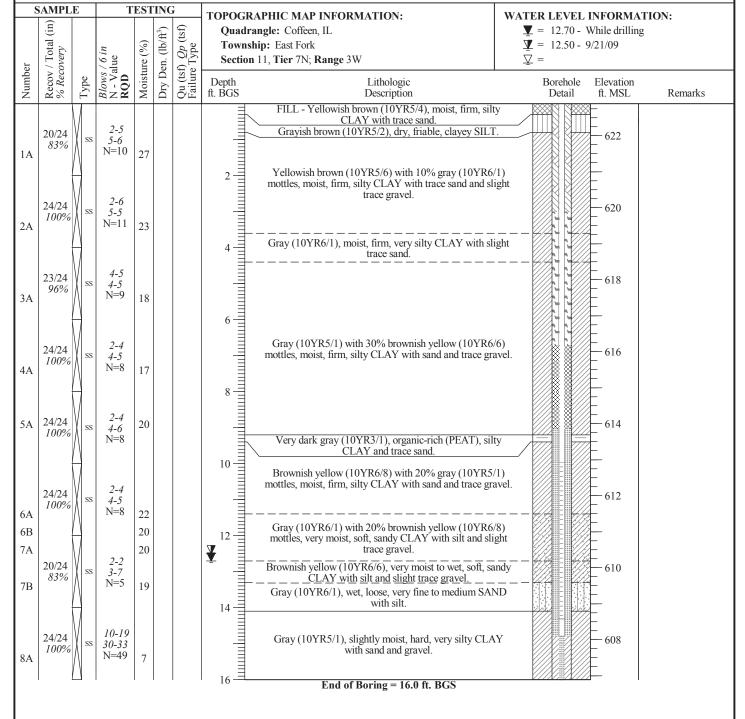
Eng/Geo: R. Hasenyager

BOREHOLE ID: G271

Well ID: G271

Surface Elev: 622.89 ft. MSL Completion: 16.00 ft. BGS Station: 874,239.38N

2,515,517.12E



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 9/10/2009 **Finish:** 9/10/2009

WEATHER: Sunny, warm (70's)

CONTRACTOR: Layne-Western Co **Rig mfg/model:** CME-750 ATV Drill

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: G. Mills Helper: J. Twellman

Eng/Geo: R. Hasenyager

BOREHOLE ID: G272

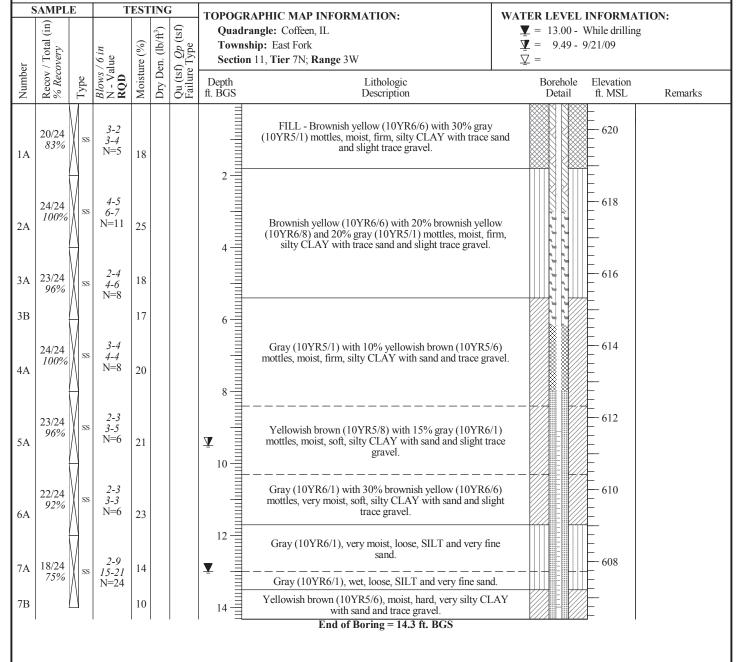
Well ID: G272

 Surface Elev:
 620.72 ft. MSL

 Completion:
 14.32 ft. BGS

 Station:
 874,234.83N

 2.515,744.99E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A

DATES: Start: 9/10/2009 Finish: 9/10/2009 CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill

Drilling Method: 41/4" HSA w/SS & CME samplers

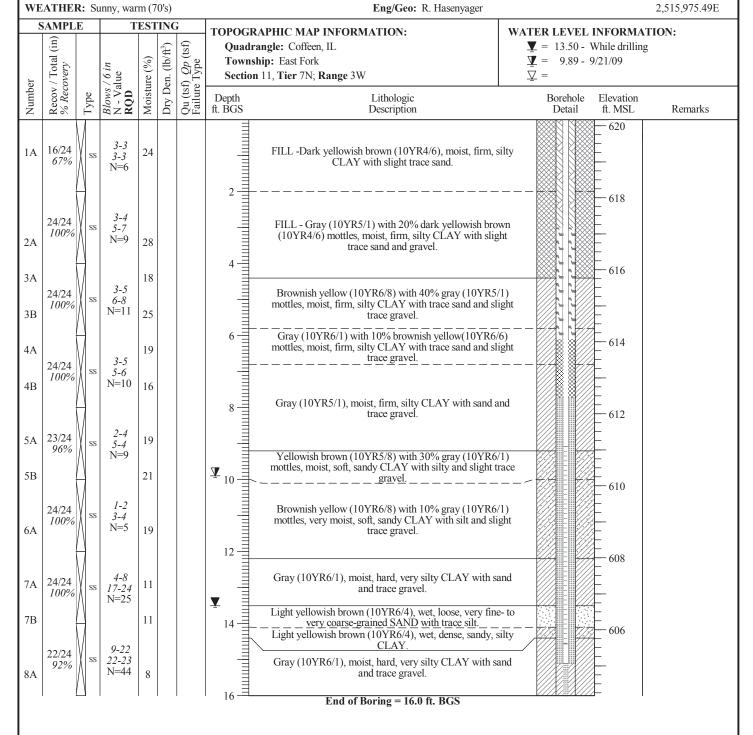
FIELD STAFF: Driller: G. Mills Helper: J. Twellman

Eng/Geo: R. Hasenyager



BOREHOLE ID: G273 Well ID: G273

> Surface Elev: 620.17 ft. MSL **Completion:** 16.00 ft. BGS **Station:** 874,235.24N 2,515,975.49E



NOTE(S):

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 9/16/2009 **Finish:** 9/16/2009

WEATHER: Sunny, warm (80's)

NOTE(S):

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: G. Mills Helper: J. Twellman

Eng/Geo: R. Hasenyager

BOREHOLE ID: G274 Well ID: G274

Surface Elev: 621.67 ft. MSL Completion: 18.06 ft. BGS Station: 874,239.25N 2,516,195.60E

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SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Qu (tsf) *Qp* (tsf) Failure Type Quadrangle: Coffeen, IL $\mathbf{V} = 16.00$ - While drilling Dry Den. (lb/ft³) Recov / Total (% Recovery Moisture (%) Township: East Fork $\Psi = 13.12 - 9/21/09$ Blows / 6 in N - Value RQD Section 11, Tier 7N; Range 3W Number Borehole Lithologic Elevation ft. BGS Description ft. MSL Remarks 2-3 20/24 3-4 83% N=61 A 17 620 $FILL\ -\ Gray\ (10YR5/1)\ with\ 30\%\ yellowish\ brown\ (10YR5/6)\ mottles,\ moist,\ firm,\ silty\ CLAY\ with\ trace\ sand$ 4-6 24/24 7-9 and slight trace gravel. 100% N = 1325 2A 618 24/24 26 3A 6-9 100% N=103B 21 3-6 Gray (10YR5/1) with 20% yellowish brown (10YR5/6) 16/24 6-8 mottles, moist, firm, silty CLAY with slight trace sand and 67% N=12gravel. 24 4A 614 24/24 4-6 Dark gray (10YR4/1) with 15% yellowish brown (10YR5/8) mottles, moist, soft, silty CLAY with trace sand 100% N=8 20 5A and slight trace gravel. 1-3 22/24 4-6 92% 19 6A 610 Gray (10YR5/1) with 30% yellowish brown (10YR5/8) 23/24 **T** 4-4 mottles, moist, soft, silty CLAY with sand and trace gravel. 96% N=6 7A 21 608 1-3 22/24 3-6 92% N=68A 17 Yellowish brown (10YR5/8), very moist, soft, silty CLAY 606 **▼**16 with sand and trace gravel Brownish yellow (10YR6/6), wet, loose, very fine- to very coarse-grained SAND. wor-4 14/24 9A 13 SS 9-11 58% Brownish yellow (10YR6/6), moist, firm, very silty CLAY N=13with sand and gravel. Gray (10YR6/1), moist, hard, very silty CLAY with sand 9В and gravel. End of Boring = 18.1 ft. BGS

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 9/16/2009 **Finish:** 9/16/2009

WEATHER: Sunny, warm (80's)

CONTRACTOR: Layne-Western Co **Rig mfg/model:** CME-750 ATV Drill

Drilling Method: 41/4" HSA w/SS & CME samplers

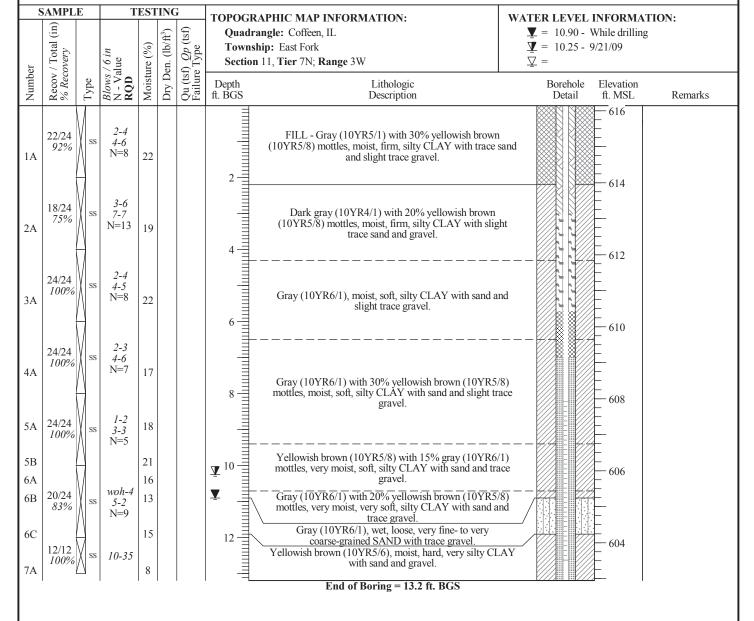
FIELD STAFF: Driller: G. Mills Helper: J. Twellman

Eng/Geo: R. Hasenyager

BOREHOLE ID: G275

Well ID: G275

Surface Elev: 616.14 ft. MSL Completion: 13.19 ft. BGS Station: 874,298.94N 2,516,375.86E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 9/16/2009

Finish: 9/16/2009
WEATHER: Sunny, mild (70'S)

CONTRACTOR: Layne-Western Co **Rig mfg/model:** CME-750 ATV Drill

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: G. Mills Helper: J. Twellman

Eng/Geo: R. Hasenyager



BOREHOLE ID: G276 **Well ID:** G276

 Surface Elev:
 629.14 ft. MSL

 Completion:
 28.00 ft. BGS

 Station:
 874,438.60N

 2,516,358.83E

Page 1 of 2

_	SAMPL		ınny, mile		INC	2		Eng/Geo: R. Hasenyager			2,516,358.83E
	Recov / Total (in)				Dry Den. (lb/ft³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	TOPOGR Quadra Towns Section	APHIC MAP INFORMATION: angle: Coffeen, IL hip: East Fork 11, Tier 7N; Range 3W	WATER LEVEL 1 $\underline{\Psi} = \text{Dry - V}$ $\underline{\Psi} = 25.55 - 9$ $\underline{\nabla} = $	While drillin	
Number	Recov % Reco	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry De	Qu (tsf Failure	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	17/24 71%	ss	5-8 9-10 N=17	10			2				
2A	19/24 79%	SS	7-7 10-14 N=17	15				FILL - Yellowish brown (10YR5/4) with 20% gray (10YR5/1) mottles, moist, hard, silty CLAY with trace s and slight trace gravel.	and	626 626	
3A	11/24 46%	ss	5-10 14-27 N=24	14			4 = = = = = = = = = = = = = = = = = = =				Rock fragment in split spoon shoe
4A	24/24 100%	ss	5-9 10-14	8				FILL - Yellowish brown (10YR5/4) with 20% gray (10YR5/1) mottles, slightly moist, hard, silty CLAY w trace sand and slight trace gravel.	th	622	
4B			N=19	5			8	FILL - Yellowish brown (10YR5/4) with 10% gray (10YR5/1) mottles, slightly moist, hard, friable, claye SILT with sand and trace gravel.	y	_	
5A	17/24 71%	ss	4-4 8-19 N=12	22			10-				
6A	17/24 71%	ss	4-5 8-14 N=13	14			10 ====================================	FILL - Yellowish brown (10YR5/4) with 25% gray (10YR5/1) mottles, slightly moist, firm, silty CLAY wis slight trace sand and gravel.	th	- - - - - - - - -	
7A	16/24 67%	SS	6-7 2-4 N=9	20					1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1	616	
8A	20/24 83%	SS	2-4 6-6 N=10	21			14	Gray (10YR6/1) with 10% yellowish brown (10YR5/mottles, moist, firm, silty CLAY with slight trace sand a gravel.	3) nd		
9A	22/24		1-4	17			16-			<u></u>	
9B	22/24 92%	ss	5-7 N=9	13			18	Gray (10YR6/1) with 20% yellowish brown (10YR5/mottles, moist, soft, sandy CLAY with silt and slight tragravel.	6) ce	612	
10A	23/24 96%	ss	2-3 8-12 N=11	20			20	Gray (10YR6/1) with 30% yellowish brown (10YR5/5) mottles, moist, firm, silty CLAY with sand and slight tragravel.		610	

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 9/16/2009

Finish: 9/16/2009 **WEATHER:** Sunny, mild (70'S)

CONTRACTOR: Layne-Western Co **Rig mfg/model:** CME-750 ATV Drill

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: G. Mills

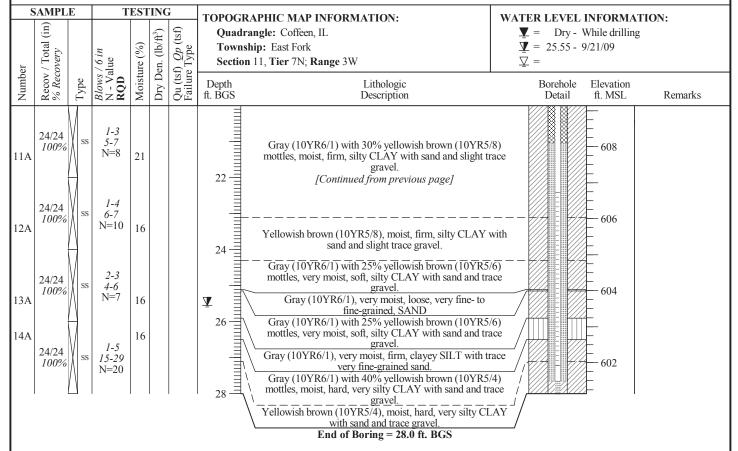
Helper: J. Twellman **Eng/Geo:** R. Hasenyager



BOREHOLE ID: G276 Well ID: G276

> Surface Elev: 629.14 ft. MSL Completion: 28.00 ft. BGS Station: 874,438.60N

> > 2,516,358.83E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 9/14/2009 **Finish:** 9/14/2009

WEATHER: Sunny, mild (70'S)

CONTRACTOR: Layne-Western Co **Rig mfg/model:** CME-750 ATV Drill

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: G. Mills Helper: J. Twellman

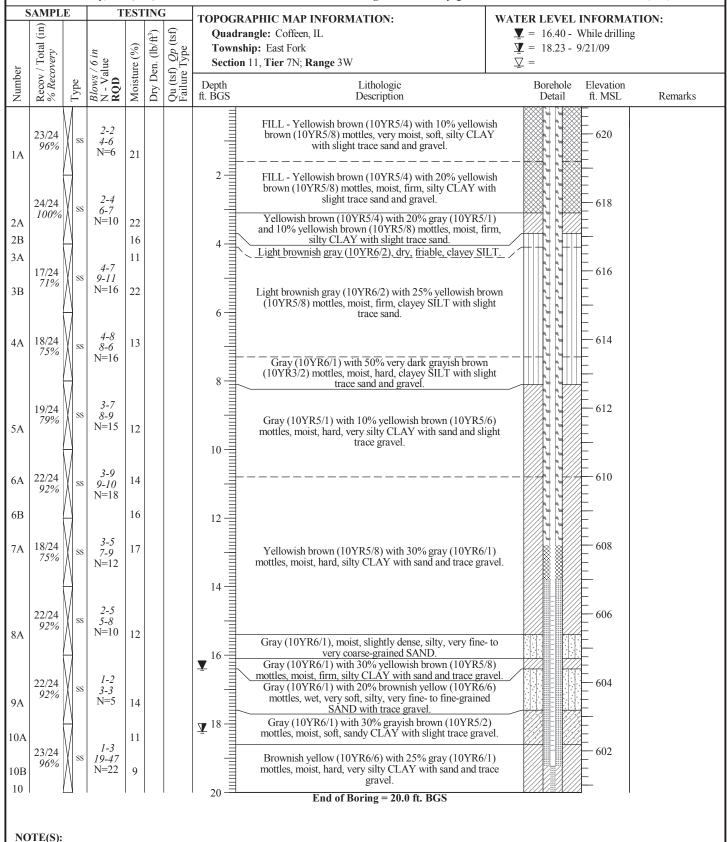
Eng/Geo: R. Hasenyager



BOREHOLE ID: G277 Well ID: G277

> Surface Elev: 620.79 ft. MSL Completion: 20.00 ft. BGS Station: 874,581.80N 2.516,370.51E

> > Page 1 of 1



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 9/11/2009

Finish: 9/11/2009 **WEATHER:** Sunny, warm (70's)

CONTRACTOR: Layne-Western Co **Rig mfg/model:** CME-750 ATV Drill

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: G. Mills Helper: J. Twellman

 $\pmb{Eng/Geo\colon}\ R.\ Hasenyager$

BOREHOLE ID: G278 Well ID: G278

Surface Elev: 628.85 ft. MSL Completion: 24.06 ft. BGS

HANSON

Station: 874,875.37N 2,516,200.66E

	LE	T	EST	INC		TOPOGR	APHIC MAP INFORMATION:	WATER LEVEI	INFORMA	ATION:
Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Quadra Townsl Section	nngle: Coffeen, IL nip: East Fork 11, Tier 7N; Range 3W	$ \underline{\nabla} = \text{Dry} - \frac{\nabla}{2} = 23.98 - \frac{\nabla}{2} = 0 $		ng
Recov / 7	Type	Blows N - V RQD	Moist	Dry I	Qu (t Failu		Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
A 16/24	SS	3-4 6-7 N=10	19			2 —			628	
A 24/24 100%	SS	4-7 8-11 N=15	21			4-			626	
22/24 92%	SS	9-10 9-35 N=19	10			6		1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,	624	Rock fragment in split spoon shoe
A 4/24 17%	SS	20-7 10-8 N=17					FILL - Yellowish brown (10YR5/4) with 15% yellow brown(10YR5/8) mottles, moist, hard, silty CLAY w trace sand and slight trace gravel.	ish ith	622	Rock fragment in split spoon shoe
A 14/24 58%	SS	11-6 8-8 N=14	15			2			620	Rock fragment in split spoon shoe
A 20/24 83%	SS	6-4 8-9 N=12	26			12 —			618	
A 24/24 100%	SS	2-4 8-11 N=12	18				Gray (10YR6/1) with 30% brownish yellow (10YR6/	/6)	616	
A 20/24	SS	4-7 10-11 N=17	12			14	mottles, moist, firm, clayey SILT with slight trace sar Very dark gray (10YR3/1), moist, firm, clayey SILT v slight trace sand and trace roots.		614	
В	$\left\langle \cdot \right\rangle$		22			16	Gray (10YR5/1) with 30% yellowish brown (10YR5, mottles, moist, firm, silty CLAY with sand and trace gra	/8) avel.		
A 22/24 92%		4-6 6-9 N=12	17			18 -	Gray (10YR6/1) with 10% yellowish brown (10YR5. mottles, moist, firm, silty CLAY with sand and trace gray.		612	
20/24 83%	SS	2-4 5-8 N=9	21			20	Yellowish brown (10YR5/8) with 20% gray (10YR5, mottles, moist, firm, silty CLAY with sand and trace grays.		610	

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

TESTING

Moisture (%)

19

16

10

▼ 24

Blows / 6 in N - Value RQD

N=11

1-5

10-18 N=15 SS

Dry Den. (lb/ft³)

Location: Coffeen, Illinois Project: 05S3004A

DATES: Start: 9/11/2009 Finish: 9/11/2009

WEATHER: Sunny, warm (70's)

SAMPLE

Recov / Total (in) % Recovery

18/24

20/24

83%

75%

Number

11A

11B

12A

12B

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: G. Mills Helper: J. Twellman

TOPOGRAPHIC MAP INFORMATION:

Quadrangle: Coffeen, IL

Township: East Fork

Eng/Geo: R. Hasenyager

2,516,200.66E WATER LEVEL INFORMATION:

BOREHOLE ID: G278

Well ID: G278

Surface Elev: 628.85 ft. MSL

Completion: 24.06 ft. BGS

Station: 874,875.37N

Remarks

▼ = Dry - While drilling $\mathbf{V} = 23.98 - 9/21/09$

 $\nabla =$

Qu (tsf) *Qp* (tsf) Failure Type Section 11, Tier 7N; Range 3W Depth Borehole Elevation Lithologic ft. BGS Description Detail ft. MSL Yellowish brown (10YR5/8) with 20% gray (10YR5/1) mottles, moist, firm, silty CLAY with sand and trace gravel. 608 [Continued from previous page] Yellowish brown (10YR5/6), very moist, soft, silty, very fine- to medium-grained SAND. 606 Yellowish brown (10YR5/4), moist, hard, very silty CLAY with sand and trace gravel.

End of Boring = 24.1 ft. BGS

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 9/10/2009

Finish: 9/10/2009 WEATHER: Sunny, warm (80's) **CONTRACTOR:** Layne-Western Co **Rig mfg/model:** CME-750 ATV Drill

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: G. Mills Helper: J. Twellman

Eng/Geo: R. Hasenyager



BOREHOLE ID: G279 **Well ID:** G279

 Surface Elev:
 629.19 ft. MSL

 Completion:
 28.00 ft. BGS

 Station:
 875,028.06N

 2,516,245.60E

SAMPLE		T	EST	INC		TOPOGR	APHIC MAP INFORMATION:	WATER LEVEL	INFORMAT	ION:
Number Recov / Total (in) % Recovery	е	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Quadra Townsh Section	ingle: Coffeen, IL iip: East Fork 11, Tier 7N; Range 3W	▼ = 23.60 - ▼ = 24.68 - ▽ =	9/21/09	
Number Recov // % Recov	Type	Blov N - N RQJ	Moi	Dry	Qu (Failt		Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
A 24/24 100%	SS	3-3 5-6 N=8	18			2	FILL - Brown (10YR4/3) with 30% yellowish brown (10YR5/6) mottles, moist, firm, silty CLAY with sand trace gravel.	n and	628	
24/24 100%	SS	5-9 10-11 N=19	14			4-			626	
3A 24/24 100%	ss	5-9 9-10 N=18	17			6-		7,7,7,7,7	624	
1A 24/24 100%	ss	4-5 7-6 N=12	21			8-		7,7,7,7	622	
5A 24/24 100%	SS	3-3 5-7 N=8	19			4 ————————————————————————————————————	FILL - dark gray (10YR4/1) with 10% brownish yello (10YR6/6) mottles, moist, hard, silty CLAY with sand trace gravel.	ow and	620	
5A 24/24 100%	SS	3-4 6-9 N=10	17			12		7,2,2,2,2,2	618	
7A 23/24 96%	SS	2-5 5-6 N=10	23					7,7,7,7	616	
24/24 100%	SS	2-3 7-6 N=10	23			16.3	Brownish yellow (10YR6/8) with 30% gray (10YR5/mottles, moist, firm, silty CLAY with slight trace sand gravel.	1) and	614	
18/24 75%	SS	4-7 8-9 N=15	25			16	Yellowish brown (10YR5/8) with 20% gray (10YR6/mottles, moist, firm, silty CLAY with slight trace sand gravel.	and	612	
0A 24/24 100%	SS	3-6 7-10 N=13	17			20	Gray (10YR6/1) with 25% yellowish brown (10YR5/mottles, moist, firm, silty CLAY with sand and trace gra	8) ivel.	610	

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

TESTING

Location: Coffeen, Illinois Project: 05S3004A

DATES: Start: 9/10/2009 Finish: 9/10/2009

WEATHER: Sunny, warm (80's)

SAMPLE

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: G. Mills Helper: J. Twellman

TOPOGRAPHIC MAP INFORMATION:

Quadrangle: Coffeen, IL

Eng/Geo: R. Hasenyager

2,516,245.60E WATER LEVEL INFORMATION:

BOREHOLE ID: G279

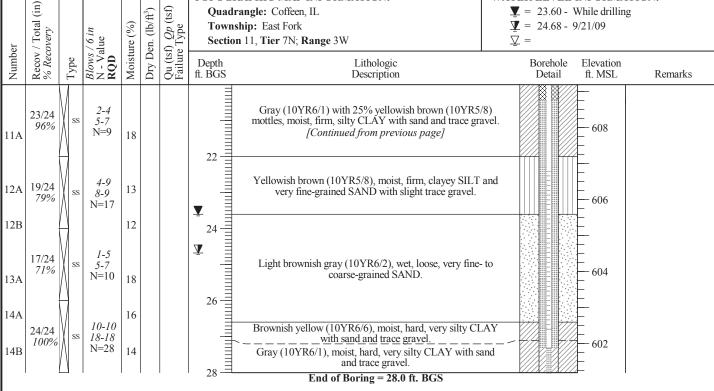
Well ID: G279

Surface Elev: 629.19 ft. MSL

Completion: 28.00 ft. BGS

Station: 875,028.06N

 $\mathbf{V} = 23.60$ - While drilling



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 2/26/2008 **Finish:** 2/26/2008

WEATHER: Overcast, cold

NOTE(S):

CONTRACTOR: Testing Service Corp. **Rig mfg/model:** CME-650 Track Drill

Drilling Method: 31/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: .

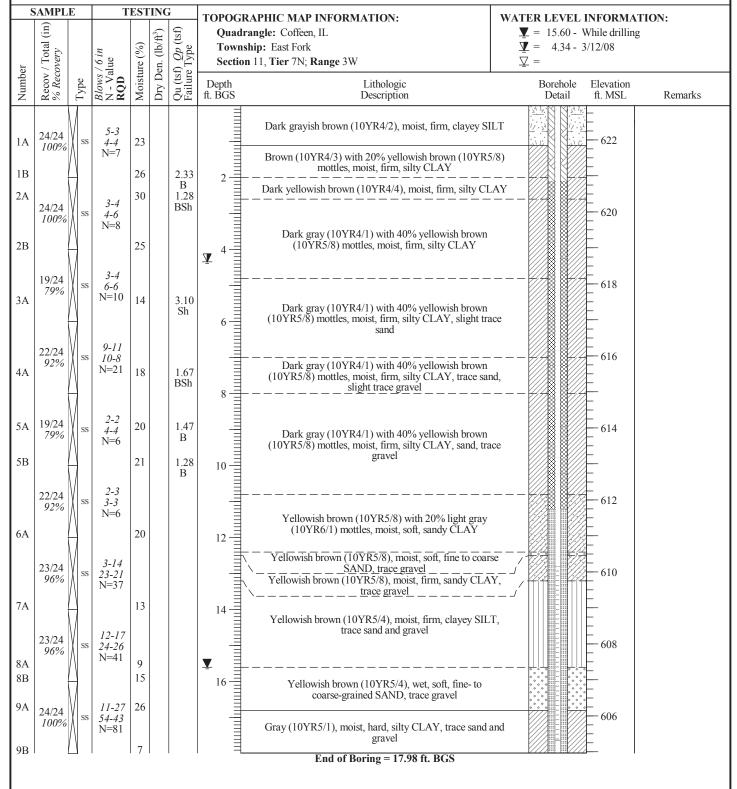
BOREHOLE ID: G280

Well ID: G280

Surface Elev: 622.95 ft. MSL Completion: 17.98 ft. BGS Station: 875,045.11N

2,515,679.48E

Page 1 of 1



CLIENT: Natural Resource Technology, Inc.

Site: Coffeen Energy Center Location: Coffeen, Illinois Project: 15E0030

DATES: Start: 9/8/2015

NOTE(S): G281 installed in borehole.

Finish: 9/8/2015 **WEATHER:** Sunny, hi 70's

CONTRACTOR: Ramsey Geotechnical Engineering, LLC

Rig mfg/model: D-50 Turbo Tracked MST 800ATV Drilling Method: Hollow Stem Auger (3¼"overdrill / 4¼")

FIELD STAFF: Driller: D. Crump Helper: D. Groves

Eng/Geo: K. Theesfeld



BOREHOLE ID: G281 **Well ID:** G281

 Surface Elev:
 623.82 ft. MSL

 Completion:
 20.29 ft. BGS

 Station:
 2,514,455.48N

874,375.37E

S	SAMPL	E	Т	EST	TINC	j	TOPOGR	APHIC MAP INFORMATION:	WATER LEVEL INFORMATION:
er	Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Quadra Townsh	ngle: Coffeen, IL lip: East Fork 10, Tier 7N; Range 3W	$\underline{\underline{\mathbf{V}}} = 14.00$ - During Drilling $\underline{\underline{\mathbf{V}}} = \underline{\underline{\mathbf{V}}} = \underline{\underline{\mathbf{V}}} = \underline{\underline{\mathbf{V}}}$
Number	Recov % Rec	Type	Blows N - Va RQD	Moist	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL Remarks
1A	17/24 71%	ss	15-10 7-6 N=17	14			2	Light gray (10YR7/2), dry, very stiff, SILT with little and trace gravel. Yellowish brown (10YR5/4) with 5% dark brown (10YR3/3) mottles, dry, very stiff, SILT with few clay trace gravel.	
2A	19/24 79%	SS	2-4 5-5 N=9	25		1.50	4-3	Yellowish brown (10YR5/4) with 15% dark yellowish brown (10YR4/6) and 5% dark brown (10YR3/3) mot moist, stiff, SILT with few clay.	sh ttles,
3A	22/24 92%	ss	2-2 3-4 N=5	23		0.40	6	Yellowish brown (10YR5/4) with 15% dark yellowish brown (10YR4/6) mottles, moist, medium, CLAY wisome silt and trace fine-grained sand and small grave	ith ////
4A	24/24 100%	ss	5-5 6-6 N=11	19		1.20	4 ————————————————————————————————————	Yellowish brown (10YR5/4) with 15% dark yellowish brown (10YR4/6) and 5% dark brown (10YR3/3) mot moist, stiff, CLAY with some silt and trace fine-grain sand and small gravel.	tles.
5A	20/24 83%	ss	2-2 3-4 N=5	21		1.40	10	Yellowish brown (10YR5/4) with 30% dark yellowish brown (10YR4/6) and 5% dark brown (10YR3/3) mot moist, stiff, SILT with some clay and trace very fine-fine-grained sand and small gravel.	tles,
6A	22/24 92%	ss	2-2 3-3 N=5	18		0.50	12	Dark yellowish brown (01YR4/6) with 30% yellowish brown (10YR5/4) mottles, moist, soft, SILT with few and little fine- to coarse-grained sand and small gravel, wood fragments.	clay 😹 🖼 🗀
7A	17/24 71%	ss	3-4 5-5 N=9	19		0.30	¥ 14 -	Dark yellowish brown (01YR4/6) with 15% yellowish brown (10YR5/4) mottles, moist, soft, SILT with few and very fine- to fine-grained sand and trace small gra	clay
	19/24 79%	ss	3-11 21-28 N=32					Dark yellowish brown (10YR4/4), wet, dense, very fine fine-grained SAND with some silt, few clay and trace signavel.	e- to mall — — — 608
	24/24 100%	ss	21-36 39-50 N=75					Dark yellowish brown (10YR4/4), wet, dense, very fine fine-grained SAND with few silt, little clay and trace st gravel. Yellowish brown (10YR5/6) with 5% strong brown	mall control of the second of
8A	11/24 46%	SS	16-9 30-50 N=39	7		4.50	16	(7.5YR5/6) mottles, moist, hard, SILT with few clay a little fine-grained sand and small gravel. Dark grayish brown (10YR4/2) with 5% strong brow (7.5YR5/6) mottles, moist, hard, SILT with few clay a little fine-grained sand and small gravel.	and
	0/3	BD					20 =	End of boring = 20.29 feet	

CLIENT: Natural Resource Technology, Inc.

Site: Coffeen Energy Center Location: Coffeen, Illinois

DATES: Start: 9/4/2015 Finish: 9/4/2015

WEATHER: Sunny, hi 70's

Drilling Method: Hollow Stem Auger (31/4"overdrill / 41/4") Project: 15E0030

FIELD STAFF: Driller: D. Crump Helper: D. Groves

Eng/Geo: K. Theesfeld

CONTRACTOR: Ramsey Geotechnical Engineering, LLC

Rig mfg/model: D-50 Turbo Tracked MST 800ATV

HANSON

BOREHOLE ID: G301 Well ID: G301

> Surface Elev: 620.27 ft. MSL **Completion:** 16.21 ft. BGS **Station:** 2,515,582.97N 872,234.82E

5	SAMPL	E	T	EST	TIN(j	TOPOGRA	APHIC MAP INFORMATION:	WATER LEVEL INFORMATION:
er	Recov / Total (in) % Recovery		/6 in alue	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Quadra Townsh	ngle: Coffeen, IL ip: East Fork 11, Tier 7N; Range 3W	$\underline{\underline{\mathbf{V}}} = 12.00 - \text{During Drilling}$ $\underline{\underline{\mathbf{V}}} = \underline{\underline{\mathbf{V}}} = \underline{\underline{\mathbf{V}}} = \underline{\underline{\mathbf{V}}}$
Number	Recov % Rec	Type	Blows / 6 in N - Value RQD	Moist	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL Remarks
1A	19/24 79%	ss	4-4 3-5 N=7	18		1.80		Brown (10YR4/3), moist, stiff, SILT with few clay, tronganics.	
2A	22/24 92%	SS	2-4 4-6 N=8	26		2.40	2 4 4 4	Brown (10YR5/3) with 30% yellowish brown (10YR5/2) mottles, moist, stiff, SILT with few clay, trace organic Brown (10YR5/3) with 30% yellowish brown (10YR5/2) mottles, moist, stiff, SILT with few clay, trace very fine medium-grained sand.	cs 618 5/6) e- to
3A	21/24 88%	SS	2-2 3-4 N=5	22		1.30		Brown (10YR5/3) with 15% yellowish brown (10YR5 mottles, moist, stiff, CLAY with some silt and trace verifine- to medium-grained sand.	5/6) ery
4A	24/24 100%	SS	6-4 6-6 N=10	19		1.10	6	Brown (10YR5/3) with 10% yellowish brown (10YR5 mottles, moist, medium to stiff, CLAY with some silt a little very fine- to coarse-grained sand and small grave	and ///& &///C
5A	21/24 88%	ss	1-2 3-4 N=5	21				Brown (10YR5/3) with 5% yellowish brown (10YR5 mottles, moist, stiff, SILT with some clay and little ve fine- to coarse-grained sand and small gravel.	/6) — 612
6A	24/24 100%	SS	3-2 3-3 N=5	19		0.80	10 - 12 - 12 - 12 - 12 - 12 - 12 - 12 - 12 - 12 - 12	Brown (10YR5/3) with 20% yellowish brown (10YR5 mottles, moist, stiff, SILT with some clay and little ve fine- to coarse-grained sand and small gravel.	5/6)
7A	24/24 100%	ss	2-4 6-21 N=10	13		1.60		Yellowish brown (10YR5/6) with 10% grayish brown (10YR5/2) and 5% yellowish brown (10YR5/4) mottl wet, stiff, SILT with few clay and little fine- to coarse-grained sand and small gravel.	vn les,
8A	21/24 88%	SS	20-27 50 N=77	7		4.50	14 -	Grayish brown (10YR5/2) with 5% brown (10YR5/2) mottles, dry, hard, SILT with few clay, very fine-to coarse-grained sand and small gravel.	3)
	0%							End of boring = 16.21 feet	

CLIENT: Natural Resource Technology, Inc.

Site: Coffeen Energy Center Location: Coffeen, Illinois Project: 15E0030

DATES: Start: 9/3/2015 **Finish:** 9/4/2015

WEATHER: Sunny, hi 70's

CONTRACTOR: Ramsey Geotechnical Engineering, LLC

Rig mfg/model: D-50 Turbo Tracked MST 800ATV **Drilling Method:** Hollow Stem Auger (3¹/₄"overdrill / 4¹/₄")

FIELD STAFF: Driller: D. Crump Helper: D. Groves

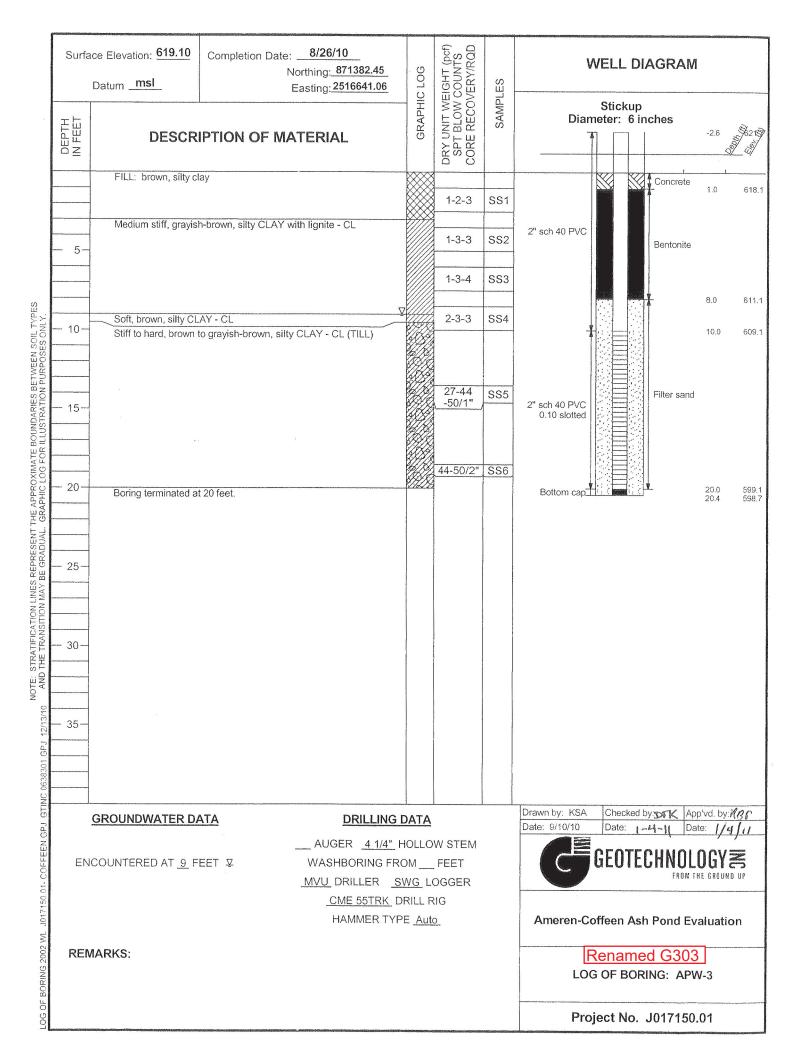
Eng/Geo: K. Theesfeld

HANSON

BOREHOLE ID: G302 **Well ID:** G302

> Surface Elev: 617.95 ft. MSL Completion: 18.39 ft. BGS Station: 2,516,214.19N 872,252.95E

SAMPLE **TESTING** TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: (iii Qu (tsf) *Qp* (tsf) Failure Type $\mathbf{V} = 14.00$ - During Drilling Dry Den. (lb/ft³) Quadrangle: Coffeen, IL Recov / Total (% Recovery Moisture (%) Township: East Fork **T** = Blows / 6 in N - Value RQD $\nabla =$ Section 11, Tier 7N; Range 3W Number Lithologic Borehole Elevation ft. BGS Description ft. MSL Remarks Yellowish brown (10YR5/6) to dark yellowish brown 5-5 23/24 (10YR4/6), moist, hard, SILT with some clay and trace 5-6 96% fine-grained sand and small gravel. N=101 A 17 1.50 Very dark brown (10YR3/1), moist, hard, SILT with some clay and trace fine-grained sand and small gravel. 2-3 20/24 2A 27 2.60 4-4 83% N=7Dark grayish brown (10YR4/2) with very dark gray (10YR3/1) mottles, moist, stiff, CLAY with some silt and 614 trace sand. 21/24 3-4 Brown (10YR5/3) with dark yellowish brown (10YR4/6) 88% mottles, moist, stiff, CLAY with some silt and trace sand. 3A 26 1.80 24/24 18 1.60 8-8 100% N=15Brown (10YR5/3) with dark yellowish brown (10YR4/6) 610 mottles, moist, stiff, CLAY with some silt and few very fine- to fine-grained sand. 24/24 17 1.80 5-5 100% 608 Brown (10YR4/3) with dark yellowish brown (10YR4/6) 2-2 18/24 mottles, moist, stiff, SILT with some clay and few sand. 4-5 75% N=619 6A Dark gray (10YR4/1), moist, stiff, SILT with some clay 606 and few sand. Yellowish brown (10YR5/8) with reddish brown (5YR4/4) inclusions, moist, stiff, SILT with some clay and few sand. 24/24 8-8 100% N=157A 16 1.70 Dark gray (10YR4/1), moist, stiff, SILT with some clay, few sand and trace small gravel. **▼** 14 604 8A 18 0.80 8B 22/24 12 4.50 Grayish brown (10YR5/2), wet, hard, SILT with little clay 25-32 92% and very fine-grained sand and trace small gravel. N = 30602 Brown (10YR5/3), wet, hard, SILT with little clay and very fine-grained sand and trace small gravel. 7-24 24/24 48-38 100% N=72 4.50 9A 8 Brown (10YR5/3) grading to yellowish brown (10YR5/4), moist, hard, SILT with some clay, few small gravel. 0/50% End of boring = 18.39 feet



DRY UNIT WEIGHT (pcf) SPT BLOW COUNTS CORE RECOVERY/RQD 8/26/10 Surface Elevation: 623.46 Completion Date: WELL DIAGRAM Northing: 871397.48 GRAPHIC LOG Datum msl SAMPLES Easting: 2515520.23 Stickup 626,8 Diameter: 6 inches DEPTH IN FEET **DESCRIPTION OF MATERIAL** Medium stiff, grayish-brown, silty CLAY - CL Concrete 1.0 622.5 0-1-1 SS1 2" sch 40 PVC 1-2-5 SS2 Bentonite 5-1-3-4 SS3 8.0 615.5 NOTE: STRATHICATION LINES REPRESENT THE APPROXIMATE BOUNDARIES BETWEEN SOIL TYPES AND THE TRANSITION MAY BE GRADUAL. GRAPHIC LOG FOR ILLUSTRATION PURPOSES ONLY. 1-2-3 SS4 10-10.0 613.5 Filter sand 1-2-1 SS5 Medium to fine SAND - SP 2" sch 40 PVC 15 Hard, gray, silty CLAY - CL (TILL) 0.10 slotted 14-47 SS6 20 20.0 20.4 603.5 603.1 Bottom cap 11: Boring terminated at 20 feet. 25 - 30 35 0638301.GPJ GTINC Drawn by: KSA Checked by: Drk App'vd. by: RS/ **DRILLING DATA GROUNDWATER DATA** Date: 9/10/10 COFFEEN.GP. AUGER 4 1/4" HOLLOW STEM ENCOUNTERED AT 14 FEET ▼ WASHBORING FROM ___ FEET MVU DRILLER SWG LOGGER CME 55TRK DRILL RIG HAMMER TYPE Auto Ameren-Coffeen Ash Pond Evaluation **REMARKS:** Renamed G304 LOG OF BORING: APW-4 Project No. J017150.01

CLIENT: Natural Resource Technology, Inc.

Site: Coffeen Power Station

Location: Coffeen, Illinois

Project: 15E0030

DATES: Start: 5/3/2016 **Finish:** 5/3/2016

NOTE(S): G305 installed in borehole.

WEATHER: Cloudy, breezy, warm, lo 60s

CONTRACTOR: Ramsey Geotechnical Engineering LLC

Rig mfg/model: D-50 Turbo Tracked MST 800ATV **Drilling Method:** 4 1/4" HSA, split spoon sampler

FIELD STAFF: Driller: B. Williamson Helper: D. Crump

Eng/Geo: S. Keim

BOREHOLE ID: G305 Well ID: G305

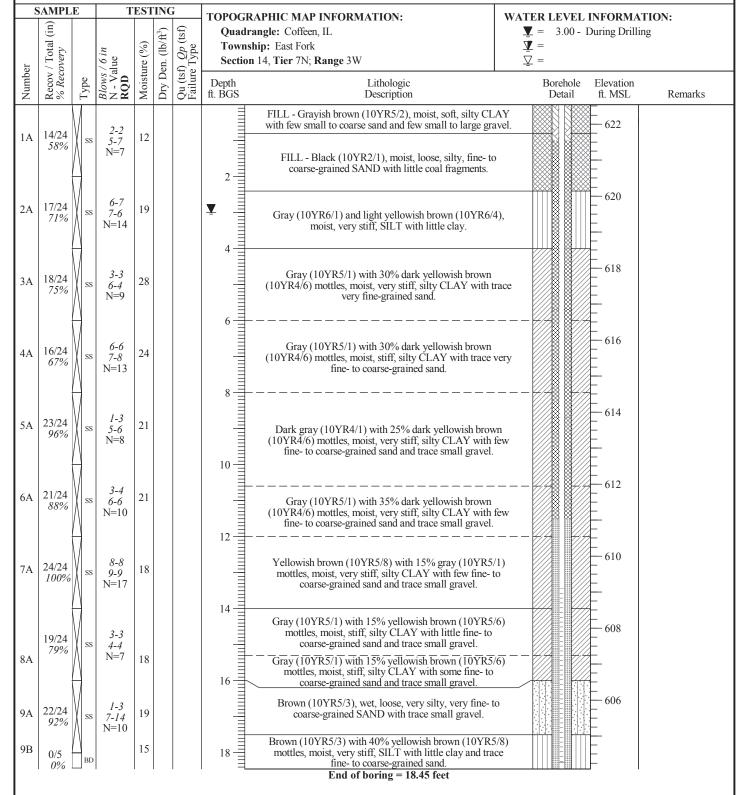
 Surface Elev:
 622.54 ft. MSL

 Completion:
 18.45 ft. BGS

 Station:
 2,515,199.36N

871,156.33E

Page 1 of 1



CLIENT: Natural Resource Technology, Inc.

Site: Coffeen Power Station
Location: Coffeen, Illinois
Project: 15E0030

DATES: Start: 5/3/2016 Finish: 5/3/2016 WEATHER: Sunny, calm, warm, lo 60s CONTRACTOR: Ramsey Geotechnical Engineering LLC

Rig mfg/model: D-50 Turbo Tracked MST 800ATV **Drilling Method:** 4 ½" HSA, split spoon sampler

FIELD STAFF: Driller: B. Williamson Helper: D. Crump

Eng/Geo: S. Keim

HANSON

BOREHOLE ID: G306 Well ID: G306

> Surface Elev: 622.84 ft. MSL Completion: 18.00 ft. BGS Station: 2,516,120.41N

871,140.98E

	SAMPL		T		INC		mon	Eng/Geo. 5. Kenn	0/1,140.70E
							Quadra	APHIC MAP INFORMATION: Ingle: Coffeen, IL	WATER LEVEL INFORMATION: $\underline{\Psi} = 5.50$ - During Drilling
	Total very		6 in ue	e (%)	1. (lb/	Qp (Γ ype	l	iip: East Fork 14, Tier 7N; Range 3W	$ \underline{\underline{Y}} = $ $ \underline{\nabla} = $
Number	Recov / Total (in) % Recovery	Type	Blows / 6 i. N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL Remarks
1A	12/24 50%	SS	1-3 3-4 N=6	14				Very dark brown (10YR2/2), moist, medium, SILT w little clay and few very fine- to medium-grained sand, rotrace coal fragments.	1
2A	24/24		5-4	21			2 -	Dark gray (10YR4/1) with 5% dark yellowish brown (10YR3/6) mottles, moist, stiff, SILT with little clay a trace very fine- to medium-grained sand	nn — — — — — — — — — — — — — — — — — —
2B	100%	ss	5-4 N=9	19			4-	Gray (10YR6/1) with 10% yellowish brown (10YR5/mottles, moist, very stiff, SILT with little clay and travery fine-grained sand.	/6)
3A	22/24 92%	ss	2-2 3-3 N=5	30			Ĭ.	Gray (10YR6/1) with 20% yellowish brown (10YR5/	/6) ————————————————————————————————————
4A	20/24 83%	SS	3-4 6-6 N=10	26				mottles, moist, very stiff, SILT with some clay and tra very fine-grained sand.	500s,
5A	24/24 100%	ss	2-2 3-3 N=5	23			10	Gray (10YR5/1) with 30% dark yellowish brown	-614
6A	22/24 92%	ss	1-2 3-4 N=5	20			10 -	(10YR4/6) mottles, moist, very stiff, silty CLAY with to very fine- to coarse-grained sand.	612
7A	20/24 83%	SS	5-6 6-6 N=12	21				Gray (10YR5/1) with 30% dark yellowish brown (10YR4/6) mottles, moist, stiff, silty CLAY with few v fine- to coarse-grained sand.	very — 610
8A	20/24 83%	SS	2-2 8-14	15			14 =	Yellowish brown (10YR5/6), wet, soft, very fine-to coarse-grained sandy CLAY with little silt. Yellowish brown (10YR5/6), wet, medium dense, silt	608
8B		1	N=10	12			16	very fine- to medium-grained SAND with trace coarse-grained sand. Yellowish brown (10YR5/6), moist, dense, fine- to	
9A	23/24 96%	ss	14-17 28-50/5" N=45	10				coarse-grained SAND with little silt, little very fine-grains sand, and trace small gravel. Brown (10YR5/3) with 20% dark yellowish brown	ined
9B			11 -43	13			18	(10YR4/6) mottles, moist, hard, SILT with little clay, the very fine- to coarse-grained sand, and trace small grave. End of boring = 18.0 feet	few = = = = = = =

CLIENT: Illinois Power Generating Co.

Site: Coffeen Power Station Ash Pond 1

Location: Coffeen, Illinois **Project:** 16E0108

NOTE(S):

DATES: Start: 07/26/2016 **Finish:** 07/27/2016

WEATHER: Overcast, warm & humid (mid-80s)

CONTRACTOR: Bulldog Drilling, Inc.

Rig mfg/model: CME 55LC Track Drill **Drilling Method:** 41/4" Hollow Stem Auger w/Continuous Split

Spoon

FIELD STAFF: Driller: J. Gates

Helper: C. Clines

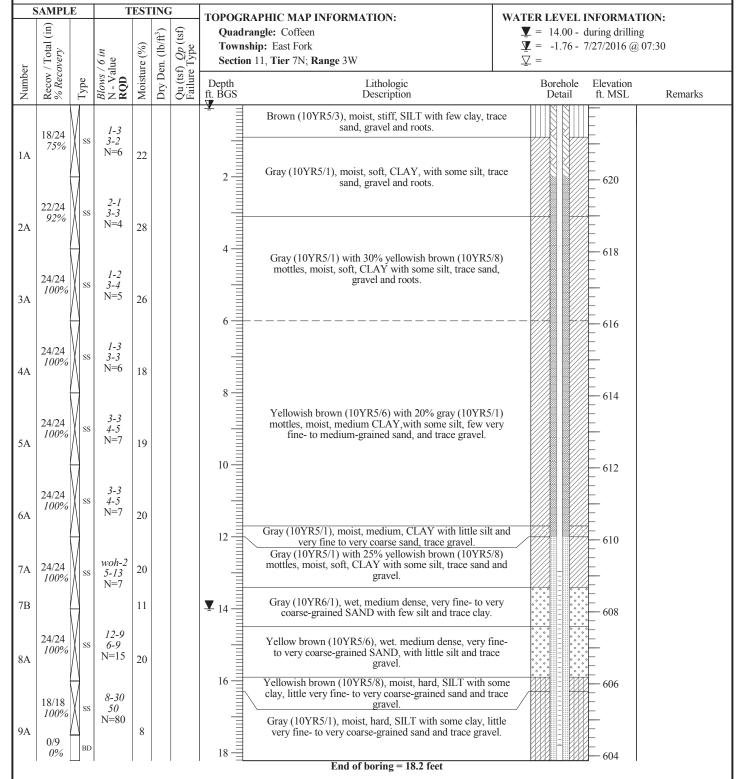
Eng/Geo: R. Hasenyager

BOREHOLE ID: G307

Well ID: G307 Surface Elev: 622.08 ft. MSL

Completion: 18.22 ft. BGS **Station:** 871,398.55N

2,515,553.26E



CLIENT: Natural Resource Technology, Inc.

Site: Coffeen Energy Center Location: Coffeen, Illinois Project: 15E0030

DATES: Start: 9/14/2015 **Finish:** 9/14/2015

NOTE(S): G401 installed in borehole.

WEATHER: Sunny, hi 60's

CONTRACTOR: Ramsey Geotechnical Engineering, LLC

Rig mfg/model: D-50 Turbo Tracked MST 800ATV Drilling Method: Hollow Stem Auger (3¼"overdrill / 4¼")

FIELD STAFF: Driller: D. Crump Helper: D. Groves

Eng/Geo: R. Hasenyager

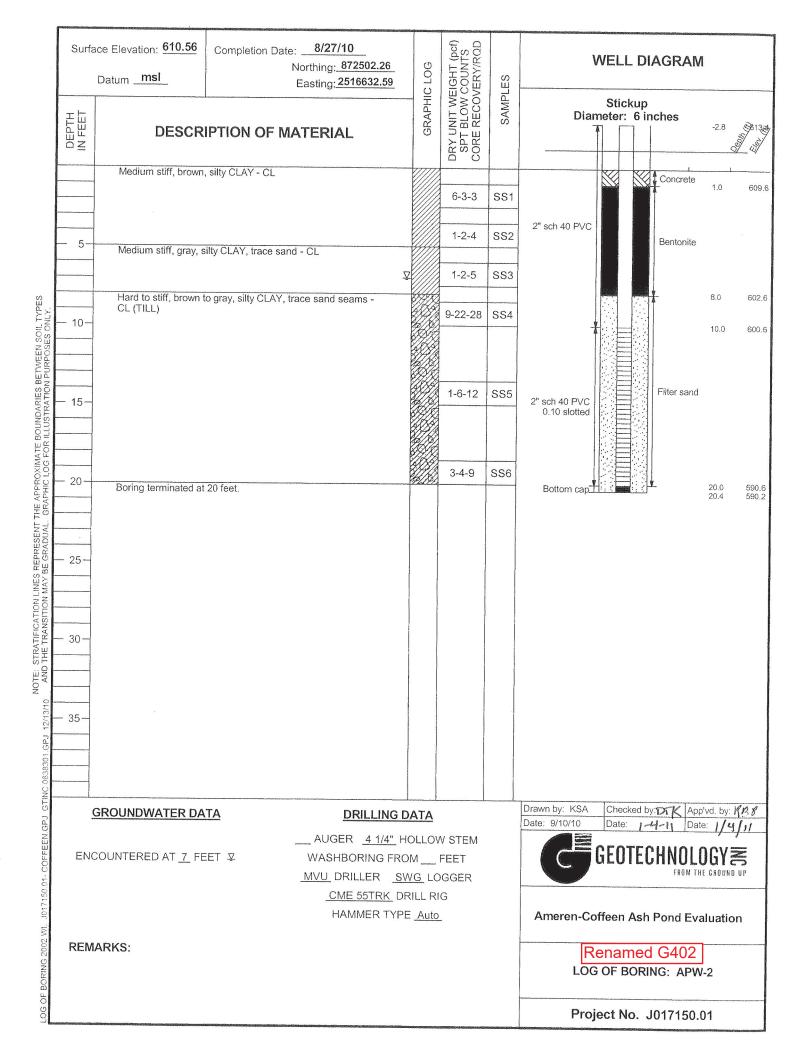
HANSON

BOREHOLE ID: G401 **Well ID:** G401

Surface Elev: 623.03 ft. MSL Completion: 19.30 ft. BGS Station: 2,515,614.84N

872,510.57E

S	SAMPL	E	Т	EST	ΓINC	Ţ	TOPOGR	APHIC MAP INFORMATION:	WATER	LEVE	L INFORMA	LIUN.
)er	Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Quadr Towns	angle: Coffeen, IL hip: East Fork 111, Tier 7N; Range 3W	▼ =	Dry -	During Drillin	
Number	Recov % Rec	Type	Blows N - V: RQD	Moist	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description]	Borehole Detail	Elevation ft. MSL	Remarks
1A	16/24 67%	ss	2-2 3-7 N=5	17			2 =	Dark grayish brown (10YR4/2), moist, soft, CLAY w little silt and trace very fine- to fine-grained sand - FII Yellowish brown (10YR5/6) moist, medium, CLAY w some silt and trace very fine- to coarse-grained sand - F.	L. ———		622	
2A 2B	21/24 88%	SS	8-11 8-9 N=19	17		1.80	2	Dark gray (10YR4/1), moist, stiff, SILT with little clay trace very fine-grained sand. Yellowish brown (10YR5/6), moist, stiff, CLAY with s silt and trace very fine- to fine-grained sand.			620	
3A	23/24 96%	ss	3-4 7-8 N=11	23		2.50	10 12 12 12 12 12 12 12 12 12 12 12 12 12	Gray (10YR5/1) with 20% yellowish brown (10YR5, mottles, moist, medium, CLAY with some silt and travery fine- to fine-grained sand.	/6) ce		618	
4A	24/24 100%	SS	8-9 12-14 N=21	21		3.30		Gray (10YR5/1) with 30% yellowish brown (10YR5.	/Q) It I		616	
4B				19		2.80	8	mottles, moist, stiff, SILT and very fine-grained SAND trace clay.				
5A	24/24 100%	ss	2-3 4-5 N=7	21		1.30	10-				614	
6A	24/24 100%	ss	2-4 5-6 N=9	17		2.50	12	Gray (10YR5/1) with 30% yellowish brown (10YR5. mottles, moist, medium, CLAY with some silt and travery fine- to fine-grained sand.	/6) ce		612	
7A	24/24 100%	ss	9-7 8-9 N=15	21		1.40	14 —				610	
8A 8B	24/24 100%	SS	2-3 2-4 N=5	17 19		1.30	16	Gray (10YR6/1), moist soft, CLAY with very fine-t fine-grained sand and little silt.			608	
8В 9А	20/24 83%	SS	5-4 5-10 N=9	21				Yellowish brown (10YR5/6), wet, loose, very fine-transfer fine-grained SAND with trace silt. Yellowish brown (10YR5/6), wet medium, SILT with severy fine-grained sand and little clay. Yellowish brown (10YR5/6), wet, loose, very fine-transfer fine-trans	ome		606	
9B 10A	12/16 75%	ss	23-41 50/4"	16		4.50	18	medium-grained SAND with trace silt. Gray (10YR5/1), moist, very hard, SILT with few clay little very fine- to very coarse sand.	and			
. 011		/ \		"				End of boring = 19.3 feet			604	



CLIENT: Natural Resource Technology, Inc.

Site: Coffeen Energy Center Location: Coffeen, Illinois Project: 15E0030

DATES: Start: 9/11/2015

Finish: 9/11/2015 WEATHER: Raining, hi 60's

CONTRACTOR: Ramsey Geotechnical Engineering, LLC

Rig mfg/model: D-50 Turbo Tracked MST 800ATV **Drilling Method:** Hollow Stem Auger (31/4"overdrill / 41/4")

Eng/Geo: K. Theesfeld

FIELD STAFF: Driller: D. Crump

Helper: D. Groves

BOREHOLE ID: G403

Well ID: G403 Surface Elev: 623.81 ft. MSL

Completion: 18.15 ft. BGS **Station:** 2,514,616.63N 873,561.34E

HANSON

The content of the	Section 11, Tier 78: Range 3W Section 11, Tier 78: Range	SAMPLE	TE	EST	ING	Ť		IATION.	WATER LEVEL INFORMATION:
19/24 18 2-2 2-3	19/24 79% 78 2-2 79% 78 2-2 79%	/ Total (in)	lue	ıre (%)	en. (lb/ft³)	$\int_{\text{Type}} Qp \text{ (tsf)}$	Quadrangle: Coffeen, IL Township: East Fork		$\underline{\underline{\mathbf{Y}}} = 15.00 - \text{During Drilling}$ $\underline{\underline{\mathbf{Y}}} =$
2.	2.A. 22.24 SS 3-4 29 1.50 Very dark brown (10/RS/2) grading to dark grayish brown (10/RS/2) mottles, most, stiff, SLT. with some clay, trace wood. Yellowish brown (10/RS/2) mottles, most, stiff, CLAY with little silt, trace very fine-grained sand sand sand scans; (=1/16 thick). Yellowish brown (10/RS/2) mottles, most, stiff, CLAY with little silt, trace very fine-grained sand. Yellowish brown (10/RS/2) with 15% yellowish brown (10/RS/2) mottles, most, stiff, CLAY with little silt, and trace very fine-grained sand. 618 Grayish brown (10/RS/2) with 15% yellowish brown (10/RS/2) mottles, most, stiff, CLAY with little silt and trace very fine-grained sand. 619 Yellowish brown (10/RS/2) with 15% yellowish brown (10/RS/2) mottles, most, stiff, CLAY with little silt and trace very fine-grained sand. 619 Grayish brown (10/RS/2) with 15% yellowish brown (10/RS/3) mottles, most, stiff, CLAY with little silt and trace very fine-grained sand. 610 Grayish brown (10/RS/2) with 30% yellowish brown (10/RS/6) mottles, most, very stiff, CLAY with little silt and trace very fine-grained sand. 619 Grayish brown (10/RS/2) with 45% yellowish brown (10/RS/6) mottles, most, very stiff, CLAY with little silt and trace very fine-grained sand. 610 Grayish brown (10/RS/3) with 45% yellowish brown (10/RS/6) mottles, most, very stiff, CLAY with little silt, few very fine-to necdum-grained sand, and trace gravel. 610 Grayish brown (10/RS/6) with 30% spayish brown (10/RS/6) with 30% spayish brown (10/RS/6) with 30% spayish brown (10/RS/6), with 30%	Numbe Recov % Rec Type	N - Va RQD	Moistu	Dry De	Qu (tsf Failure	Depth t. BGS	Lithologic Description	
2.	2.A. 22.24 SS 3-4 29 1.50 Very dark brown (10/RS/2) grading to dark grayish brown (10/RS/2) mottles, most, stiff, SLT. with some clay, trace wood. Yellowish brown (10/RS/2) mottles, most, stiff, CLAY with little silt, trace very fine-grained sand sand sand scans; (=1/16 thick). Yellowish brown (10/RS/2) mottles, most, stiff, CLAY with little silt, trace very fine-grained sand. Yellowish brown (10/RS/2) with 15% yellowish brown (10/RS/2) mottles, most, stiff, CLAY with little silt, and trace very fine-grained sand. 618 Grayish brown (10/RS/2) with 15% yellowish brown (10/RS/2) mottles, most, stiff, CLAY with little silt and trace very fine-grained sand. 619 Yellowish brown (10/RS/2) with 15% yellowish brown (10/RS/2) mottles, most, stiff, CLAY with little silt and trace very fine-grained sand. 619 Grayish brown (10/RS/2) with 15% yellowish brown (10/RS/3) mottles, most, stiff, CLAY with little silt and trace very fine-grained sand. 610 Grayish brown (10/RS/2) with 30% yellowish brown (10/RS/6) mottles, most, very stiff, CLAY with little silt and trace very fine-grained sand. 619 Grayish brown (10/RS/2) with 45% yellowish brown (10/RS/6) mottles, most, very stiff, CLAY with little silt and trace very fine-grained sand. 610 Grayish brown (10/RS/3) with 45% yellowish brown (10/RS/6) mottles, most, very stiff, CLAY with little silt, few very fine-to necdum-grained sand, and trace gravel. 610 Grayish brown (10/RS/6) with 30% spayish brown (10/RS/6) with 30% spayish brown (10/RS/6) with 30% spayish brown (10/RS/6), with 30%	79% SS 2	2-2	25		0.80	Very dark brown (10Y (10YR4/2), moist, m	edium, SILT with some clay, tra-	ce
3A 8.24 33%	3.4			20			Very dark brown (10Y (10YR4/2), moist, stit	R2/2) grading to dark grayish broff, SILT with some clay, trace wo	own ood.
3A 8.24 33%	3.4	211 92% \ SS 3	J-7			1.50	Yellowish brown (10 (10YR2/2) mottles, movery fine-graine	oist, stiff, CLAY with little silt, tr	wn race 620
12	6A 100% N=5 23	8/24 SS 4	2-3 4-4	25		1.50	4-1	R5/4), moist, stiff, CLAY with live very fine-grained sand.	ittle
12— 1.40 12— 12— 12— 12— 12— 12— 12— 13— 14— 12— 12— 12— 12— 12— 13— 14— 109785/6) and 5% dark brown (10YR5/3) mottles, moist, stiff, SILT with some clay, few very fine- to coarse-grained sand, and trace gravel. 14— 15— 17	6A 100% N=5 23	88% ss 8	8-7	20		1.30	6	ery dark grayish brown (10YR3/ CLAY with little silt and trace ve	72)
12 12 12 12 12 12 13 14 15 15 15 15 15 15 16 16	6A 100% N=5 23	83% ss 3	3-3	22		0.70		noist, medium, CLAY with little s	n silt — 614
8A 24/24 100% SS 3-6 N=5 17 SS 21-25 N=33 8 4.50 18 18 18 18 18 18 18 1	8A 24/24	100% SS 3	3-4	23		1.40	-	oist, very stiff, CLAY with little s	
SS 3-2 17	SS 3-2 17	88% ss 6	6-5	20		0.90	Grayish brown (10Y (10YR5/6) and 5% da stiff, SILT with some c	clay, few very fine- to coarse-grai	ined
0% Very dark grayish brown (10YR3/2), dry, hard, SILT with	Very dark grayish brown (10Y R3/2), dry, hard, SILT with little clay and few very fine- to coarse-grained sand and gravel.	$8A \begin{vmatrix} \frac{24}{24} \\ \frac{100\%}{100\%} \end{vmatrix}$ ss 3	3-6	17			Grayish brown (10YR5/6) mottles, m few very fine- to coa Yellowish brown (10YR5/2) mottles, m few very fine- to coa Yellowish brown (10YR5/2) mottles, m	loist, medium, SILT with little clarse-grained sand, and trace grave DYR5/6) with 30% grayish brown oist, medium, SILT with little clarse-grained sand, and trace grave	ay,
intile day and lew very line- to coarse-grained sand and /	gravel.	9A $\begin{vmatrix} 19/24 \\ 79\% \\ 0/2 \end{vmatrix}$ ss $\begin{vmatrix} 2I \\ N=1 \end{vmatrix}$	1-25	8		4.50	\ Very dark grayish brov	ay and few silt. DYR5/6) with 30% grayish brow loist, stiff, SILT with few clay, vocygranied sand, and gravel. YR5/6), moist, very stiff, SILT wand few sand and gravel. wn (10YR3/2), dry, hard, SILT v	ery / - 606
	6						\	gravel.	

CLIENT: Natural Resources Technology, Inc.

Site: Coffeen Energy Center

Location: Coffeen, Illinois **Project:** 15E0030

DATES: Start: 5/1/2007 **Finish:** 5/1/2007

WEATHER: Partly sunny, warm

CONTRACTOR: Reynolds Drilling Corp. **Rig mfg/model:** CME-550 ATV Drill

Drilling Method: 41/4" Hollow stem auger with split spoon

sampler

FIELD STAFF: Driller: A. Rachford

Helper: M. Brown Eng/Geo: R. Hasenyager

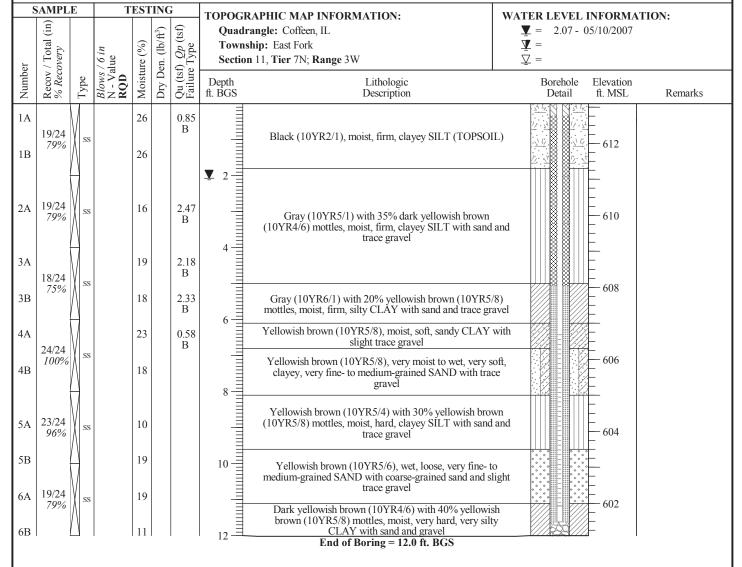
HANSON

BOREHOLE ID: SB22 Formerly MW22S

Well ID: G404

Surface Elev: 613.10 ft. MSL Completion: 12.00 ft. BGS Station: 873.999.77N

2,516,397.85E



CLIENT: Natural Resources Technology, Inc.

Site: Coffeen Energy Center Location: Coffeen, Illinois

Project: 15E0030 **DATES: Start:** 5/1/2007

Finish: 5/1/2007 WEATHER: Partly sunny, warm CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-550 ATV Drill

Drilling Method: 41/4" Hollow stem auger with split spoon

sampler

FIELD STAFF: Driller: A. Rachford Helper: M. Brown

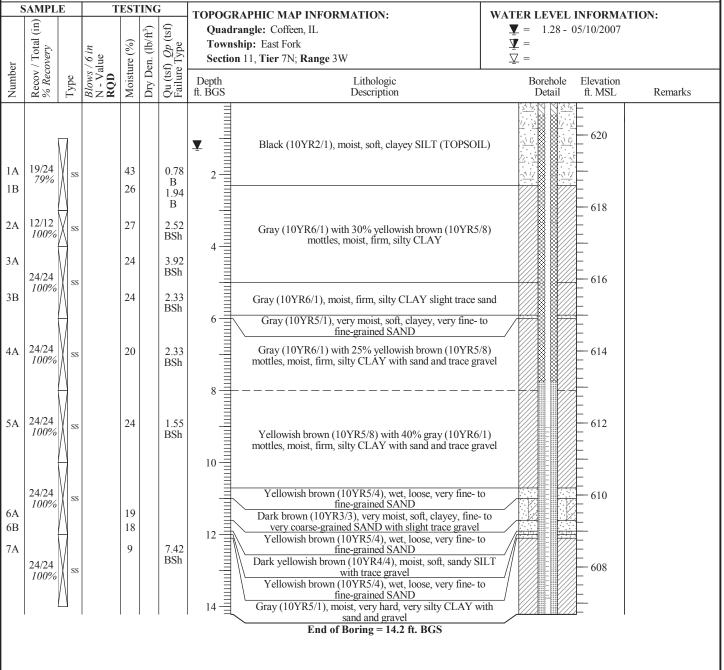
Eng/Geo: R. Hasenyager



BOREHOLE ID: SB21 Formerly MW21S

Well ID: G405

Surface Elev: 620.90 ft. MSL Completion: 14.21 ft. BGS **Station:** 873,996.79N 2,515,335.70E



CLIENT: Natural Resources Technology, Inc.
Site: Coffeen Power Station - Ash Pond 2
Location: 134 CIPS Lane, Coffeen, IL 62017

Project: 16E0080

DATES: Start: 8/19/2016 **Finish:** 8/19/2016

WEATHER: Sunny, (mid-70s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" Hollow Stem Auger

FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill

Eng/Geo: K. Theesfeld



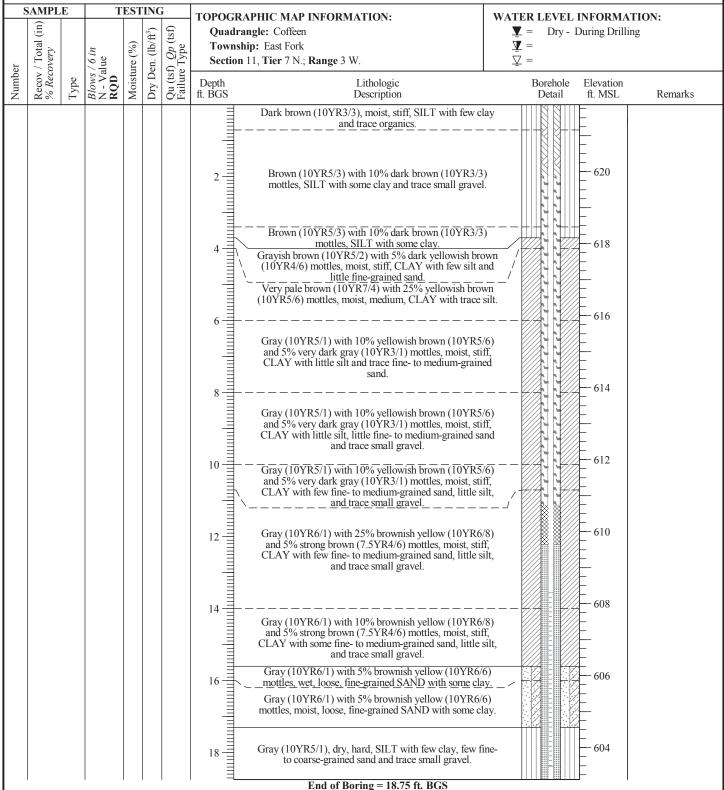
BOREHOLE ID: G406 **Well ID:** G406

 Surface Elev:
 621.86 ft. MSL

 Completion:
 18.75 ft. BGS

 Station:
 872,521.34N

 2.514,702.38E



NOTE(S): G406 installed in boring.

Boring was blind drilled adjacent to G406D.

CLIENT: Natural Resources Technology, Inc.
Site: Coffeen Power Station - Ash Pond 2
Location: 134 CIPS Lane, Coffeen, IL 62017

Project: 16E0080

DATES: Start: 8/19/2016

Finish: 8/19/2016 WEATHER: Sunny, (mid-70s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" Hollow Stem Auger

FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill

Eng/Geo: K. Theesfeld



BOREHOLE ID: G406D **Well ID:** G46D

 Surface Elev:
 621.91 ft. MSL

 Completion:
 52.00 ft. BGS

 Station:
 872,519.70N

 2,514,697.78E

	AMPL	E	T	EST	INC	j	TOPOGRAPHIC MAP INFORMATION: WA	TER LEVEL	INFORMATIO	ON.
r	Recov / Total (in) % Recovery	_	<i>' 6 in</i> ue	re (%)	Dry Den. (lb/ft³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Quadrangle: Coffeen Township: East Fork		During Drilling	v-1•
Number	Recov / % Reco	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry De	Qu (tsf) Failure	Depth Lithologic ft. BGS Description	Borehole Detail	Elevation ft. MSL	Remarks
1A 1B	18/24 75%	ss	4-3 4-6 N=7	11		1.50 3.00	Dark brown (10YR3/3), moist, stiff, SILT with few clay and trace organics. Brown (10YR5/3) with 10% dark brown (10YR3/3) mottles, SILT with some clay and trace small gravel.		- - - - - - - - - - - - - - - - - - -	
2A	12/24 50%	SS	4-6 3-3 N=9	12		2.50				
BA	3/24 13%	SS	3-3 4-7 N=7	18 24		2.00	Brown (10YR5/3) with 10% dark brown (10YR3/3) mottles, SILT with some clay. Grayish brown (10YR5/2) with 5% dark yellowish brown (10YR4/6) mottles, moist, stiff, CLAY with few silt and little fine-grained sand. Very pale brown (10YR7/4) with 25% yellowish brown (10YR5/6) mottles, moist, medium, CLAY with trace silt.		618 	
A	20/24 83%	SS	2-3 4-5 N=7	21		1.25	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) and 5% very dark gray (10YR3/1) mottles, moist, stiff, CLAY with little silt and trace fine- to medium-grained sand.		614	
5A	19/24 79%	ss	1-3 4-6 N=7	18		1.75	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) and 5% very dark gray (10YR3/1) mottles, moist, stiff, CLAY with little silt, little fine- to medium-grained sand and trace small gravel.		- - - - - - - - - - - - - - - - - - -	
A	23/24 96%	SS	2-2 4-5 N=6	18		2.50	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) and 5% very dark gray (10YR3/1) mottles, moist, stiff, CLAY with little silt, little fine- to medium-grained sand and trace small gravel. Gray (10YR5/1) with 10% yellowish brown (10YR5/6) and 5% very dark gray (10YR3/1) mottles, moist, stiff, CLAY with few fine- to medium-grained sand, little silt, and trace small gravel. Gray (10YR6/1) with 25% brownish yellow (10YR6/8) and 5% strong brown (7.5YR4/6) mottles, moist, stiff, CLAY with few fine- to medium-grained sand, little silt, and trace small gravel.		610	
'A	21/24 88%	SS	1-3 4-5 N=7	16		1.00	7			
A	23/24 96%	SS	1-2 2-2 N=4	18		0.75	Gray (10YR6/1) with 10% brownish yellow (10YR6/8) and 5% strong brown (7.5YR4/6) mottles, moist, stiff, CLAY with some fine- to medium-grained sand, little silt, and trace small gravel. Gray (10YR6/1) with 5% brownish yellow (10YR6/6) mottles, wet, loose, fine-grained SAND with some clay.			
В	/			17		0.75	Gray (10YR6/1) with 5% brownish yellow (10YR6/6) mottles, wet, loose, fine-grained SAND with some clay.		606	
Α	22/24 92%	ss	4-13 27-23 N=40	8			Gray (10YR6/1) with 5% brownish yellow (10YR6/6) mottles, moist, loose, fine-grained SAND with some clay.			
В	17/24 71%	ss	13-31 33-42 N=64	8		4.50	Gray (10YR6/1) with 5% brownish yellow (10YR6/6) mottles, moist, loose, fine-grained SAND with some clay. Gray (10YR5/1), dry, hard, SILT with few clay, few fine-to coarse-grained sand and trace small gravel.	,,,,,,,,		

CLIENT: Natural Resources Technology, Inc. Site: Coffeen Power Station - Ash Pond 2 Location: 134 CIPS Lane, Coffeen, IL 62017

Project: 16E0080

DATES: Start: 8/19/2016 **Finish:** 8/19/2016

WEATHER: Sunny, (mid-70s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" Hollow Stem Auger

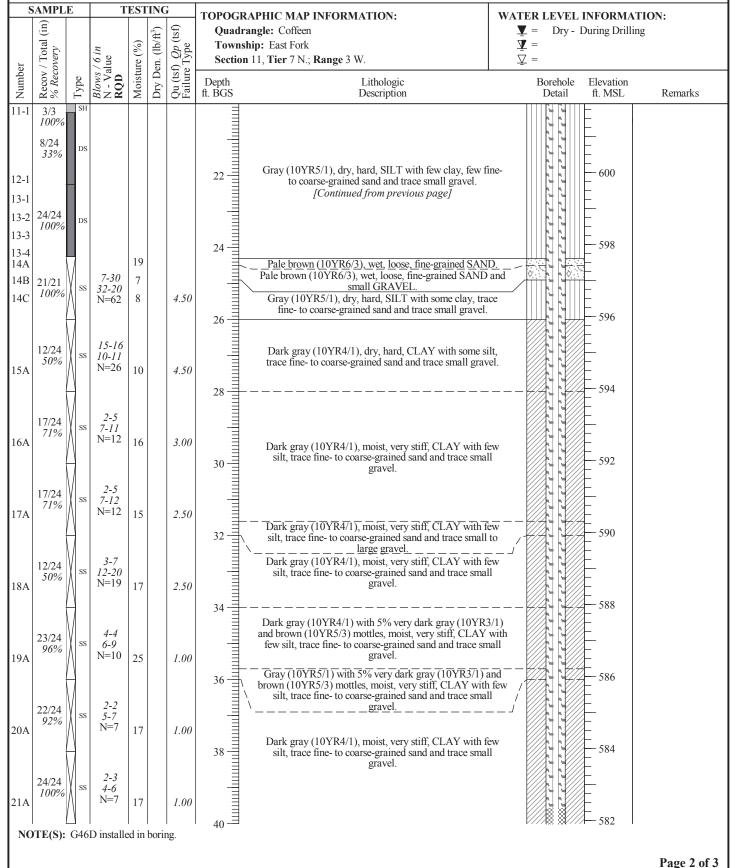
FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill

Eng/Geo: K. Theesfeld



BOREHOLE ID: G406D **Well ID:** G46D

Surface Elev: 621.91 ft. MSL Completion: 52.00 ft. BGS Station: 872,519.70N 2,514.697.78E



CLIENT: Natural Resources Technology, Inc. Site: Coffeen Power Station - Ash Pond 2 Location: 134 CIPS Lane, Coffeen, IL 62017

Project: 16E0080

DATES: Start: 8/19/2016 Finish: 8/19/2016

WEATHER: Sunny, (mid-70s)

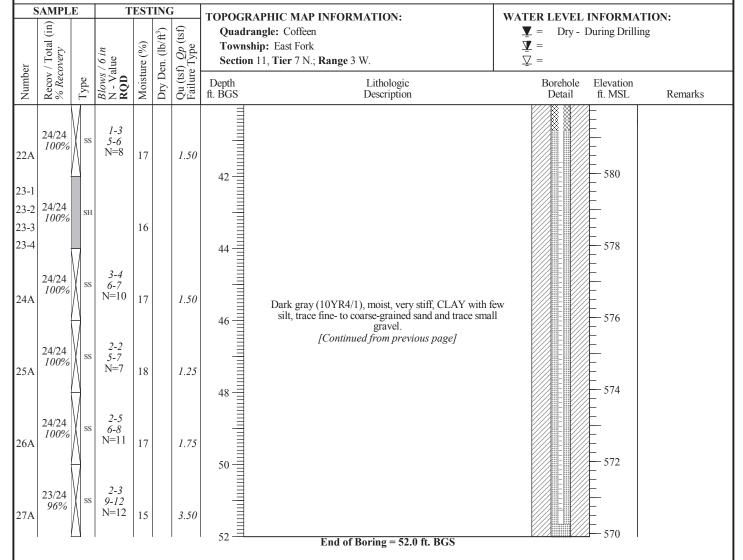
CONTRACTOR: Bulldog Drilling, Inc. Rig mfg/model: CME-750 ATV Drill

Drilling Method: 4 1/4" Hollow Stem Auger

FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill Eng/Geo: K. Theesfeld

BOREHOLE ID: G406D Well ID: G46D

> Surface Elev: 621.91 ft. MSL Completion: 52.00 ft. BGS **Station:** 872,519.70N 2,514,697.78E



CLIENT: Natural Resources Technology, Inc.
Site: Coffeen Power Station - Ash Pond 2
Location: 134 CIPS Lane, Coffeen, IL 62017

Project: 16E0080

DATES: Start: 8/16/2016 **Finish:** 8/16/2016

WEATHER: Rain, (mid-70s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" Hollow Stem Auger

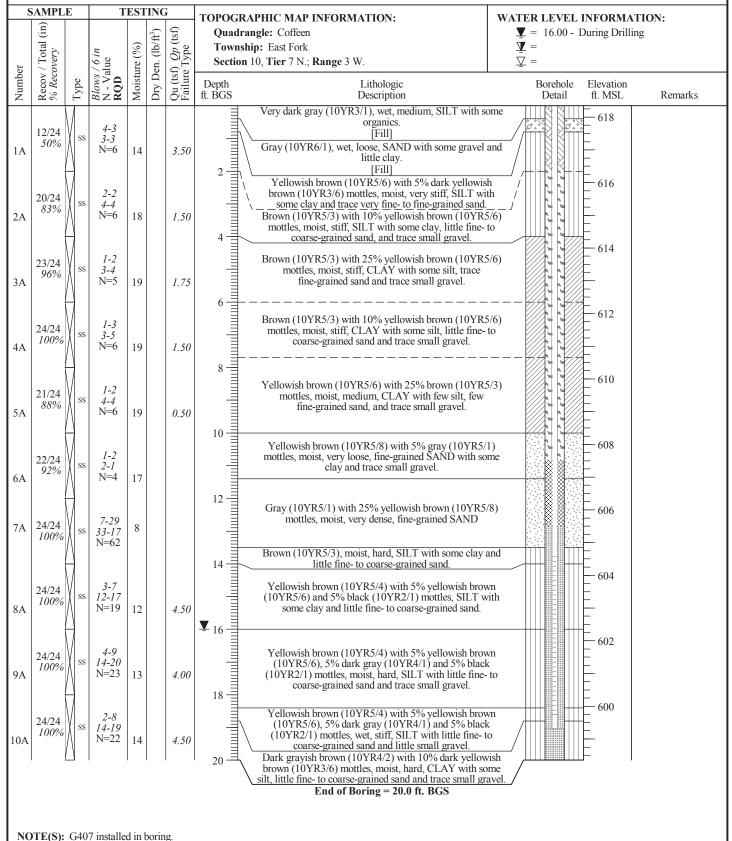
FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill

Eng/Geo: K. Theesfeld



BOREHOLE ID: G407 **Well ID:** G407

> Surface Elev: 618.35 ft. MSL Completion: 20.00 ft. BGS Station: 2,513,705.87N 2,513,705.87E



CLIENT: Illinois Power Generating Company **Site:** Coffeen Power Station - Ash Pond 2

Location: Coffeen, Illinois

Project: 16E0031A

DATES: Start: 2/23/2018 **Finish:** 2/23/2018

WEATHER: Overcast, mild (mid-40s)

CONTRACTOR: Bulldog Drilling, Inc. **Rig mfg/model:** CME-750 ATV Drill

Drilling Method: 41/4" HSA with continuous split spoon

FIELD STAFF: Driller: C. Dutton Helper: M. Baetje

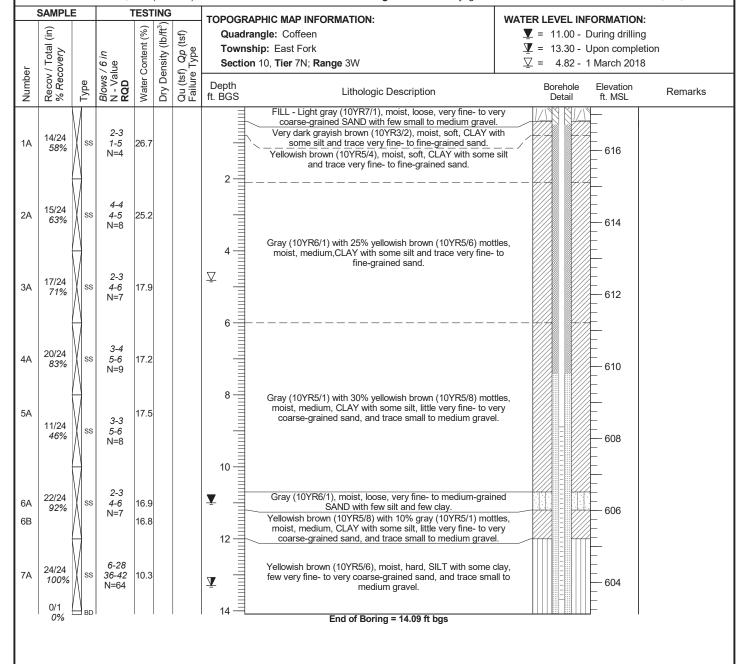
Eng/Geo: R. Hasenyager

BOREHOLE ID: G410

Well ID: G410

Surface Elev: 617.21 ft. MSL Completion: 14.09 ft. BGS

Station: 872,968.54N 2.513.206.33E



CLIENT: Illinois Power Generating Company **Site:** Coffeen Power Station - Ash Pond 2

Location: Coffeen, Illinois

Project: 16E0031A

DATES: Start: 2/22/2018 Finish: 2/22/2018 WEATHER: Overcast, cool (lo-40s) CONTRACTOR: Bulldog Drilling, Inc. Rig mfg/model: CME-750 ATV Drill

Drilling Method: 41/4" HSA with continuous split spoon

FIELD STAFF: Driller: C. Dutton Helper: M. Baetje

Eng/Geo: R. Hasenyager

HANSON
BOREHOLE ID: G411

Well ID: G411

 Surface Elev:
 620.49 ft. MSL

 Completion:
 16.47 ft. BGS

 Station:
 873,844.81N

2,513,122.38E

;	SAMPLE		٦	rest	ING		TOPOGR/	APHIC MAP INFORMATION:	WATER LEVEL INFORMATION:
	(ii)			(%))/ff ³ ,	Qu (tsf) Qp (tsf) Failure Type		ingle: Coffeen	▼ = 13.00 - During drilling
	zal 🗡		~	jt	€,	e (ts	Towns	hip: East Fork	▼ = 8.00 - Upon completion
	Tot		6 <i>in</i> e	onte	sity	호호	Section	n 10, Tier 7N; Range 3W	$\nabla = 2.65 - 1 \text{ March } 2018$
per	> 00		s/ /alu	ŭ)en	(st)		, , , , , , , , , , , , , , , , , , ,	
Number	Recov / Total (% Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft³	on (Depth ft. BGS	Lithologic Description	Borehole Elevation Remark Detail ft. MSL
_	15.00	-	₩ ∠ №	>		ОШ	II. BGS	Very dark grayish brown (10YR3/2), moist, soft, CLAY wi	
	1	1						some silt and trace sand.	620
4.0	17/24	$\ \ _{\infty} \ $	5-2	45.0					
1A	71%	ss	<i>4-4</i> N=6	15.6				Dark yellowish brown (10YR4/4), moist, soft, CLAY with so silt.	ome
	/							Siit.	
	+	+					2 =		
	1	./					\(\bar{\bar{\bar{\bar{\bar{\bar{\bar{		618
2A	17/24	ss	4-4 4-6	28.9			∄		
	71%	Λ	N=8						
	/						4		
	Ţ						4 =		
		$/\!\!/$	2-3					Gray (10YR6/1) with 25% yellowish brown (10YR5/6) mott	iles, 616
3A	20/24 83%	ss	4-5	18.8			-	moist, medium,CLAY with some silt.	
	00,0	^\	N=7						
	1						6 =		
	١						"		
	24/24	/	2-3						614
4A	100%	ss	<i>4-4</i> N=7	19.0					
	/	\	14-7				}		<i>H</i>
	H	-					₹ 8 🖺		
	1	./							612
5A	20/24	ss	2-3 4-4	21.0					
<i>></i> A	83%	M	N=7	21.0				Gray (10YR5/1) with 30% yellowish brown (10YR5/8) mott	
	/						10 —	moist, medium, CLAY with some silt, few very fine- to ve coarse-grained sand, and trace gravel.	ry
	t	1					10 =	g, g	
	1	./	2-4						610
	24/24 100%	ss	5-6				=		
ŝΑ	100%	/\	N=9	14.4			1		
<i>,</i> ,	/	1					12	Dark grayish brown (10YR4/2), moist, medium, CLAY wi	th
	Ţ	7					12	some silt, few very fine- to very coarse-grained sand, an	nd ///::::::::::::::::::::::::::::::::::
	24/24	/	1-3				_	trace small gravel.	608
7A	24/24 100%	ss	2-4	18.9			₹ 🛔		
	/	'\	N=5					Brown (10YR4/3), wet, loose, very fine- to very coarse-grain	ined
	ļ	1					14	SAND with little silt and few clay.	
	1								606
	23/24		3-17				=	Vollouigh brown (10VRF/R) maint hand CILT with	
	96%	ss	20-20 N=37]	Yellowish brown (10YR5/6), moist, hard, SILT with some c little very fine- to very coarse-grained sand, and trace sma	
]	medium gravel.	
	0/6	BD					16 —		
	0%					l	=_	End of Boring = 16.47 ft bgs	

CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/3/2006

Finish: 5/3/2006
WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 31/4" HSA w/SS sampler

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager



BOREHOLE ID: SB-01 **Well ID:** MW1D

 Surface Elev:
 607 ft. MSL

 Completion:
 40 ft. BGS

 Station:
 874,972.6N

 2,513,478.0E

Page 1 of 2

SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Ξ $\mathbf{Y} = 34.00$ - While drilling Quadrangle: Coffeen, IL Dry Den. (lb/ft3 Recov / Total % Recovery Township: East Fork Qu (tsf) Failure Type Moisture (%) Blows / 6 in N - Value RQD Section 10, Tier 7N; Range 3W ∇ = 36.28 - MW01D on 6/1/06 Number Depth ft. BGS Lithologic Borehole Elevation Description ft. MSL Remarks 16/24 Very dark gray (10YR3/1), clayey SILT, trace sand SS 1-2 67% N=21 A 22 0.78 606 B 2-3 5-5 Light gray (10YR7/1) with 40% yellowish brown 20/24 SS (10YR5/8) mottles, clayey SILT, trace sand and gravel 83% N=82A 13 3.71 604 BSh2-2 Gray (10YR5/1) with 25% yellowish brown (10YR5/6) 22/24 3-6 92% mottles, clayey SILT, trace sand and gravel N=53A 14 2.62 602 BSh8-12 23/24 Yellowish brown (10YR5/6) with 20% black (10YR2/1) 19-19 96% mottles, clayey SILT, little sand and gravel N = 3113 600 4A 3.30 BSh4-9 24/24 13-19 100% N = 2213 5A 4.80 598 BShYellowish brown (10YR5/6) with 40% gray (N5/1) mottles, clayey SILT, trace sand and gravel 3-6 22/24 12-15 92% N=18 12 8.73 596 6A B14-19 24/24 23-30 100% N=42 7A 12 594 7.86 Dark gray (N4/1) with 25% yellowish brown (10YR5/6) mottles, clayey SILT, trace sand and gravel 4-8 24/24 12-14 N=20 100% 8A 13 7.56 592 16-16 24/24 20-21 100% N = 369A 14 590 7.01 Dark gray (N4/1), clayey SILT, trace sand and gravel 3-5 24/24 8-11 SS 100% N = 1310A 14 5.24 588 BNOTE(S): MW01D installed in SB-01.

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility Location: Coffeen, Illinois

Project: 05S3004A **DATES: Start:** 5/3/2006

Finish: 5/3/2006

WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig **Drilling Method:** 31/4" HSA w/SS sampler

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager



BOREHOLE ID: SB-01 Well ID: MW1D

607 ft. MSL **Surface Elev: Completion:** 40 ft. BGS Station: 874,972.6N 2,513,478.0E

_	SAMPL		Т	EST	`		TOROGE	A DITIC MAD INFORMATION.	NAT A PRINTER	I DVDI	INIEOD34:	2,313,170.0E
	Recov / Total (in) % Recovery			Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) Failure Type	Quadra	APHIC MAP INFORMATION: ngle: Coffeen, IL lip: East Fork 10, Tier 7N; Range 3W	<u>Ā</u> = <u>Ā</u> =	34.00 - 1	INFORMA While drillin MW01D on	ng
Number	Recov % Reco	Type	Blows / 6 in N - Value RQD	Moistu	Dry De	Qu (tsf Failure	Depth ft. BGS	Lithologic Description]	Borehole Detail	Elevation ft. MSL	Remarks
11A	24/24 100%			15		3.69 B	22				586	Shelby tube taken from shallow well borehole at indicated depth.
12A	24/24 100%	SS	10-11 12-16 N=23	14		5.24 B	24 —				584	Shelby tube taken
13A	6/24 25%	SH		14		3.69 B	26 —				582	from shallow well borehole at indicated depth.
14A	24/24 100%	SS	10-12 18-18 N=30	15		4.27 B	28 —	Dark gray (N4/1), clayey SILT, trace sand and grav [Continued from previous page]	vel		580	
15A	24/24 100%	ss	5-9 11-16 N=20	14		4.27 B	30 —				578	
16A	24/24 100%	ss	5-8 10-12 N=18	17		2.72 BSh	32 —				576	
17A	24/24 100%	ss	8-14 14-16 N=28	14		5.62 B	₹ 34				574	
18A	23/24 96%	SS	8-28 40-65 N=68	14			_ 36	Gray (N4/1), silty, fine to medium SAND, little coasand, trace gravel, wet	arse		572	
19A 19B	24/24 100%	SS	24-14 17-16 N=31	13		5.43 B	36 38 38	Very dark gray (10YR3/1), silty CLAY			570	
20A	24/24 100%	SS	3-5 6-10 N=11	24		3.50 BSh	40	Dark gray (N4/1) with 30% dark yellowish brown (10YR4/6) mottles, silty CLAY End of Boring = 40.0 ft. BGS	n		568	
	NOTE(S): N	1W01D i	nstal	lled	in SB-(01.					
												Page 2 of 2

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/5/2006

Finish: 5/5/2006 **WEATHER:** Partly cloudy, mild (high-50's)

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 41/4" HSA (blind drill)

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager



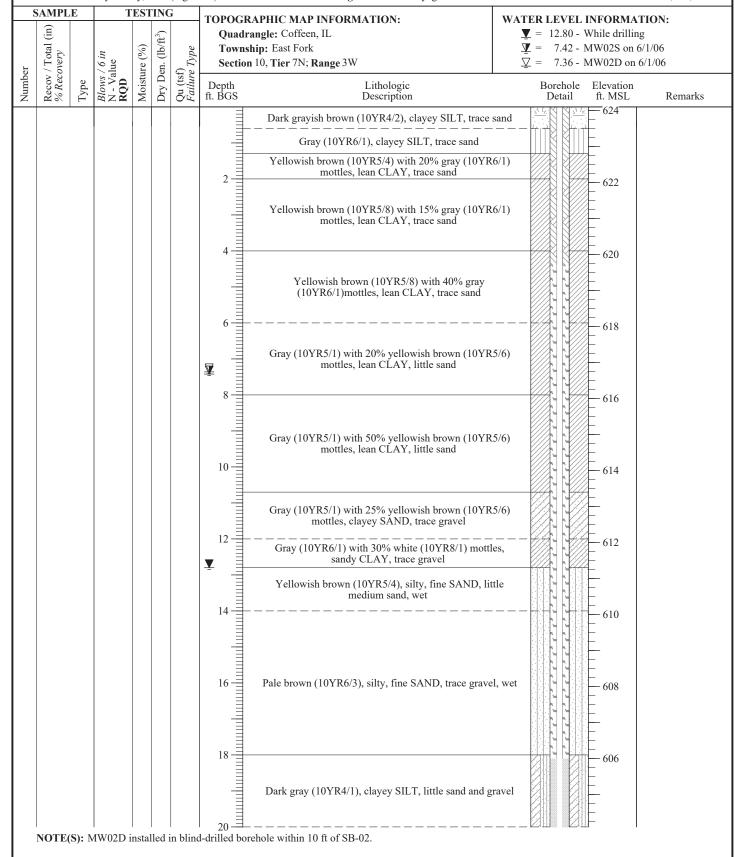
BOREHOLE ID: SB-02b **Well ID:** MW2D

 Surface Elev:
 624 ft. MSL

 Completion:
 27 ft. BGS

 Station:
 876,414.0N

 2,513,209.7E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A **DATES: Start:** 5/5/2006

Finish: 5/5/2006

WEATHER: Partly cloudy, mild (high-50's)

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 41/4" HSA (blind drill)

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

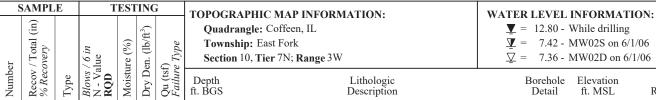
Eng/Geo: R. Hasenyager

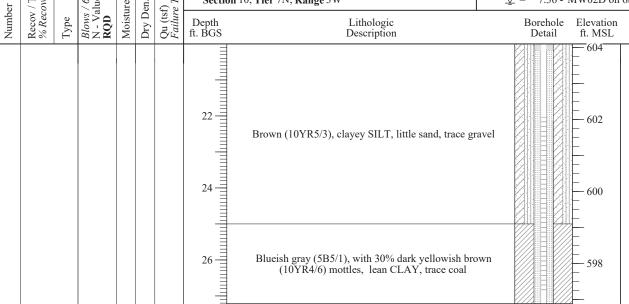
HANSON

BOREHOLE ID: SB-02b Well ID: MW2D

Surface Elev: 624 ft. MSL **Completion:** 27 ft. BGS **Station:** 876,414.0N 2,513,209.7E

Remarks





End of Boring = 27.22 ft. BGS See SB-02 for sample & testing details

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois
Project: 05S3004A

DATES: Start: 5/5/2006

Finish: 5/5/2006

WEATHER: Partly cloudy, mild (high-50's)

CONTRACTOR: Testing Service Corporation **Rig mfg/model:** CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Helper: R. Keedy
Eng/Geo: R. Hasenyager

HANSON

BOREHOLE ID: SB-02 **Well ID:** n/a

 Surface Elev:
 624 ft. MSL

 Completion:
 50 ft. BGS

 Station:
 876,410.0N

 2,513,210.0E

5	SAMPL	E	T	EST	TING	j	TOPOGR	APHIC MAP INFORMATION:	WA	TER	LEV	EL	INFORMAT	TION:
er	Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	sf) re Type	Quadra Towns	angle: Coffeen, IL hip: East Fork 10, Tier 7N; Range 3W	-	<u>Ā</u> =	12.8 7.4	30 - 1 2 - 1	While drilling MW02S on 6 MW02D on 6	g /1/06
Number	Recov % Rec	Type	Blows N - V RQD	Moist	Dry D	Qu (tsf) Failure I	Depth ft. BGS	Lithologic Description			oreh Deta		Elevation ft. MSL	Remarks
A	24/24		3-3	23				Dark grayish brown (10YR4/2), clayey SILT, trace	sand	- Hi		<u> </u>	624	
В	100%	SS	4-5 N=7	18		1.96		Gray (10YR6/1), clayey SILT, trace sand						
lC			11 /	29		B 1.94 B	2 =	Yellowish brown (10YR5/4) with 20% gray (10YR mottles, lean CLAY, trace sand	6/1)				622	
	\	$\langle $	2 1										1 22	
	24/24 100%	ss	3-4 4-6				4	Yellowish brown (10YR5/8) with 15% gray (10YR mottles, lean CLAY, trace sand	6/1)		1-: -		1	
2A		$^{\prime}$	N=8	25		2.89 B					# - -		‡	
	{	-					4						620	
	24/24	\bigvee	3-5					Yellowish brown (10YR5/8) with 40% gray			1-1		E	
3A	100%	SS	5-7 N=10	20		2.91		(10YR6/1)mottles, lean CLAY, trace sand						
ЗA		$^{\prime}\setminus$		20		B B					1-1		1	
							. =			_ ///			618	
	24/24	ss	10-8 8-10				8	Gray (10YR5/1) with 20% yellowish brown (10YR	5/6)				‡	
4A	100%	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	N=16	17		2.91	X	mottles, lean CLAY, little sand			1-1			
	[В	8 =						F (16	
	\												616	
	24/24 100%	SS	3-3 4-5						-10		1-1		E	
5A	100%	Λ	N=7	19		1.94		Gray (10YR5/1) with 50% yellowish brown (10YR mottles, lean CLAY, little sand	5/6)				1	
	{	-				В	10 -						614	
6A	\	$\langle $	2-3	18		2.13 B					1-1		1	
	24/24 100%	ss	6-5			Б		Gray (10YR5/1) with 25% yellowish brown (10YR	5/6)		43		1	
6B		$^{\prime}$	N=9	17			=	mottles, clayey SAND, trace gravel	5/0)				1	
	{	-)					12 =	Gray (10YR6/1) with 30% white (10YR8/1) mottl		- ///	1-		612	
7A	24/24	\bigvee	4-4	14		2.06		sandy CLAY, trace gravel	-0,				F	
	100%	SS	7-10 N=11			В		Yellowish brown (10YR5/4), silty, fine SAND, lit	tle		1		E	
7B		/\	1, 11	17				medium sand, wet			1-7		E	
)					14			-			610	
	24/24	$V \mid$	15-23											
8A	100%	SS	33-68 N=56	10							[-7		<u> </u>	
J1 1	/	\					16 —	Pola brown (10VP6/2) silty fine CAND to	1 11114		12		<u> </u>	
9A	10/10	\sqrt{ss}	48-62/4'	10		3.92	10	Pale brown (10YR6/3), silty, fine SAND, trace grave	ı, wei				608	
<i>)</i> / 1.	100%	\triangle		10		3.92 Sh					1-7		F	
											1		F	
	-	_,					18					Ш	E 606	
	12/12 100%	\bigvee_{ss}	15-45			0.0-							606	
0A	100%	\triangle		9		8.07 <i>BSh</i>		Dark gray (10YR4/1), clayey SILT, little sand and g	ravel		12-			
	_	_					l ₂₀ <u>∃</u>	grout pumped from bottom of borehole.			1		<u> </u>	

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/5/2006

DATES: Start: 5/5/2006 Finish: 5/5/2006 WEATHER: Partly cloudy, mild (high-50's) CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

HANSON

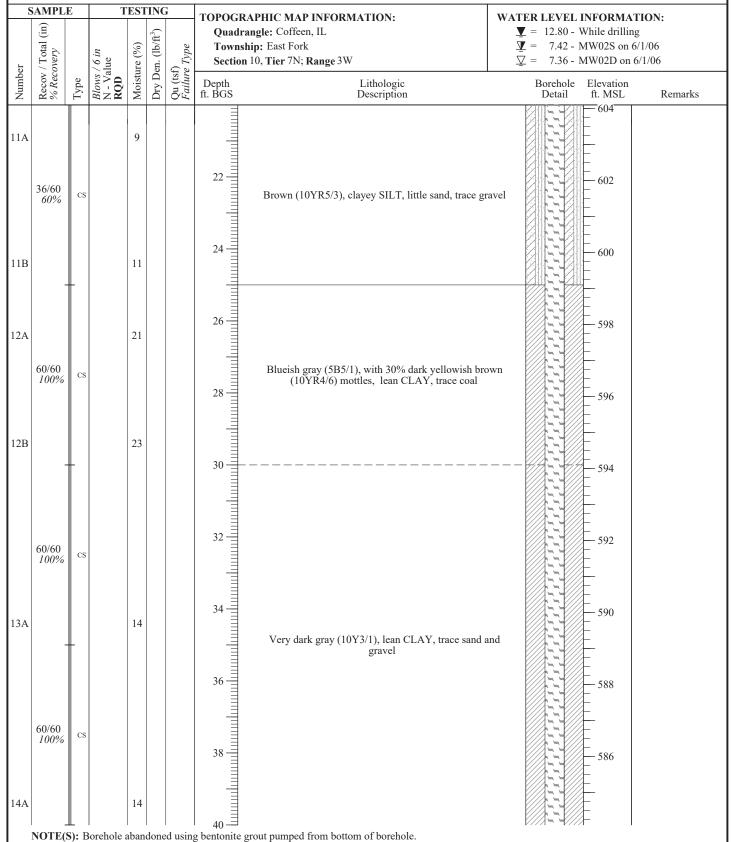
BOREHOLE ID: SB-02 Well **ID:** n/a

 Surface Elev:
 624 ft. MSL

 Completion:
 50 ft. BGS

 Station:
 876,410.0N

 2,513,210.0E



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/5/2006

DATES: Start: 5/5/2006 Finish: 5/5/2006 WEATHER: Partly cloudy, mild (high-50's) CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Helper: R. Keedy Eng/Geo: R. Hasenyager HANSON
BOREHOLE ID: SB-02

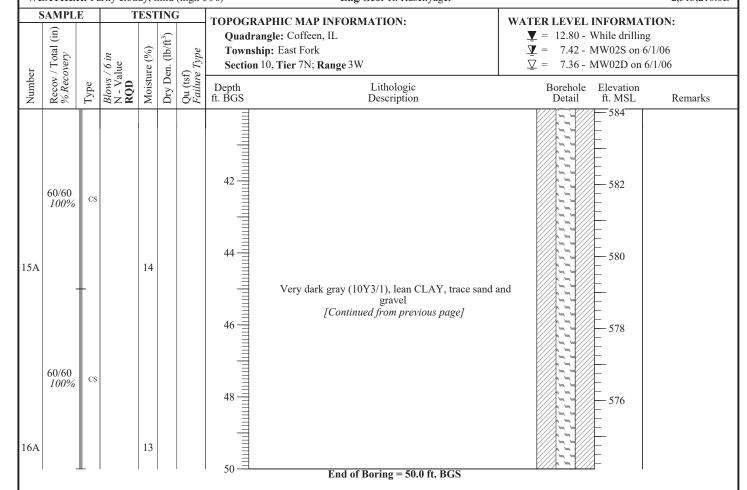
Well ID: n/a

 Surface Elev:
 624 ft. MSL

 Completion:
 50 ft. BGS

 Station:
 876,410.0N

 2,513,210.0E



CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/5/2006

Finish: 5/5/2006 WEATHER: Partly cloudy, mild (high-50's) CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 31/4" HSA (blind drill)

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

HANSON

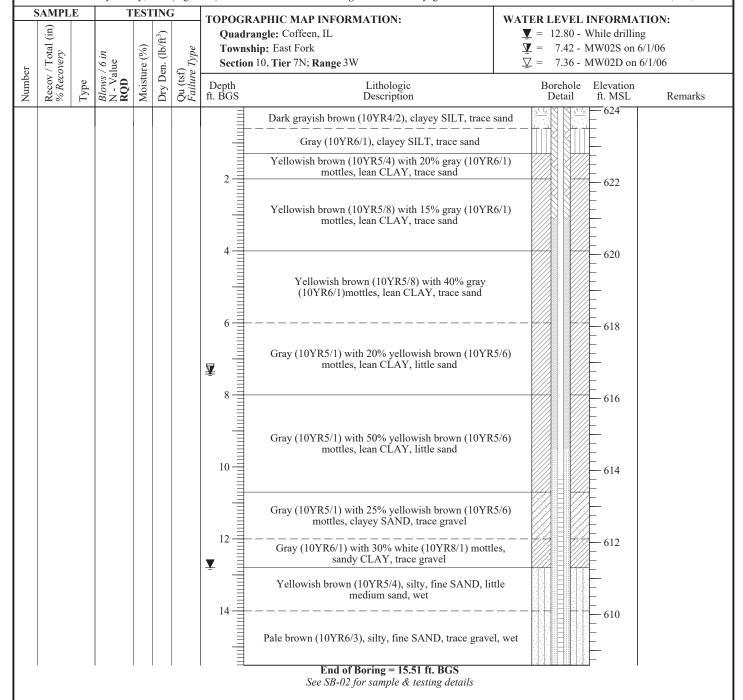
BOREHOLE ID: SB-02a Well ID: MW2S

 Surface Elev:
 624 ft. MSL

 Completion:
 16 ft. BGS

 Station:
 876,408.9N

 2,513,210.0E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois
Project: 05S3004A

DATES: Start: 5/5/2006

Finish: 5/5/2006

WEATHER: Partly cloudy, mild (high-50's)

CONTRACTOR: Testing Service Corporation **Rig mfg/model:** CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Helper: R. Keedy
Eng/Geo: R. Hasenyager

HANSON

BOREHOLE ID: SB-02 **Well ID:** n/a

 Surface Elev:
 624 ft. MSL

 Completion:
 50 ft. BGS

 Station:
 876,410.0N

 2,513,210.0E

SAMPLE TESTING					ING	j	TOPOGRAPHIC MAP INFORMATION: W				VATER LEVEL INFORMATION:										
er	Recov / Total (in) % Recovery		/ 6 in ılue	Moisture (%)	Dry Den. (lb/ft³)	sf) re Type	Quadrangle: Coffeen, IL Township: East Fork Section 10, Tier 7N; Range 3W			$ \mathbf{Y} = 12.80 $ - While drilling $ \mathbf{Y} = 7.42 $ - MW02S on 6/1/06 $ \mathbf{Y} = 7.36 $ - MW02D on 6/1/06			g /1/06								
Number	Recov % Rec	Type	Blows / 6 in N - Value RQD	Moist	Dry D	Qu (tsf) Failure I	Depth ft. BGS	Lithologic Description			oreh Deta		Elevation ft. MSL	Remarks							
A	24/24		3-3	23				Dark grayish brown (10YR4/2), clayey SILT, trace	sand	- Hi		<u> </u>	624								
В	100%	SS	4-5 N=7	18		1.96		Gray (10YR6/1), clayey SILT, trace sand													
lC			11 /	29		B 1.94 B	2 =	Yellowish brown (10YR5/4) with 20% gray (10YR mottles, lean CLAY, trace sand	6/1)				622								
	\		2 1										1 22								
	24/24 100%	ss	3-4 4-6					Yellowish brown (10YR5/8) with 15% gray (10YR6 mottles, lean CLAY, trace sand Yellowish brown (10YR5/8) with 40% gray (10YR6/1)mottles, lean CLAY, trace sand	6/1)		1-: -		1								
2A		$^{\prime}$	N=8	25		2.89 B					# <u>-</u> -		‡								
	{	-					4						620								
	24/24	\bigvee	3-5					Yellowish brown (10YR5/8) with 40% gray			1-1		E								
3A	100%	SS	5-7 N=10	20		2.91		(10YR6/1)mottles, lean CLAY, trace sand													
ЗA		$^{\prime}$		20		B B					1-1		1								
							. =			_ ///			618								
	24/24	ss	10-8 8-10)			8	Gray (10YR5/1) with 20% yellowish brown (10YR mottles, lean CLAY, little sand	5/6)				‡								
4A	100%		N=16			2.91					1-1										
						В	8 =						_ 								
	\												616								
	24/24 100%	ss	3-3 4-5						-10		1-1		E								
5A	100%	Λ	N=7	19		1.94		Gray (10YR5/1) with 50% yellowish brown (10YR mottles, lean CLAY, little sand	5/6)				1								
	{	-				В	10 -						614								
6A	\	$\langle $	2-3	18		2.13 B					1-1		1								
	24/24 100%	SS	6-5 N=9	17		Б		Gray (10YR5/1) with 25% yellowish brown (10YR	5/6)		43		1								
6B								mottles, clayey SAND, trace gravel	5/0)				1								
	{	\vdash					12 =	Gray (10YR6/1) with 30% white (10YR8/1) mottl		- ///	1-		612								
7A	24/24	\bigvee	4-4	14		2.06		sandy CLAY, trace gravel	-0,				F								
	100%	SS	7-10 N=11			В		Yellowish brown (10YR5/4), silty, fine SAND, litt	tle		1		E								
7B		/\	11 11	11 11	14 .11	14-11	11-11	11-11	19-11	1, 11	=11 17				medium sand, wet			1-7		E	
)					14			-			610								
	24/24	$V \mid$	15-23																		
8A	100%	SS	33-68 N=56	10							[-7		<u> </u>								
J1 1	/	\					16 —	Pola brown (10VP6/2) silty fine CAND to	1 11104		12		<u> </u>								
9A	10/10	\sqrt{ss}	48-62/4'	10		3.92	10	Pale brown (10YR6/3), silty, fine SAND, trace grave	ı, wei				608								
<i>)</i> / 1	100%	Δ -		10	10		3.92 Sh					1-7		F							
											1		F								
	-	_,					18					Ш	E 606								
	12/12 100%	ss	15-45			0.0-							606								
0A				9		8.07 <i>BSh</i>		Dark gray (10YR4/1), clayey SILT, little sand and gra	ravel		12-										
	_	_					₂₀ <u> </u>	grout pumped from bottom of borehole.			1		<u> </u>								

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/5/2006

DATES: Start: 5/5/2006 Finish: 5/5/2006 WEATHER: Partly cloudy, mild (high-50's) CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

HANSON

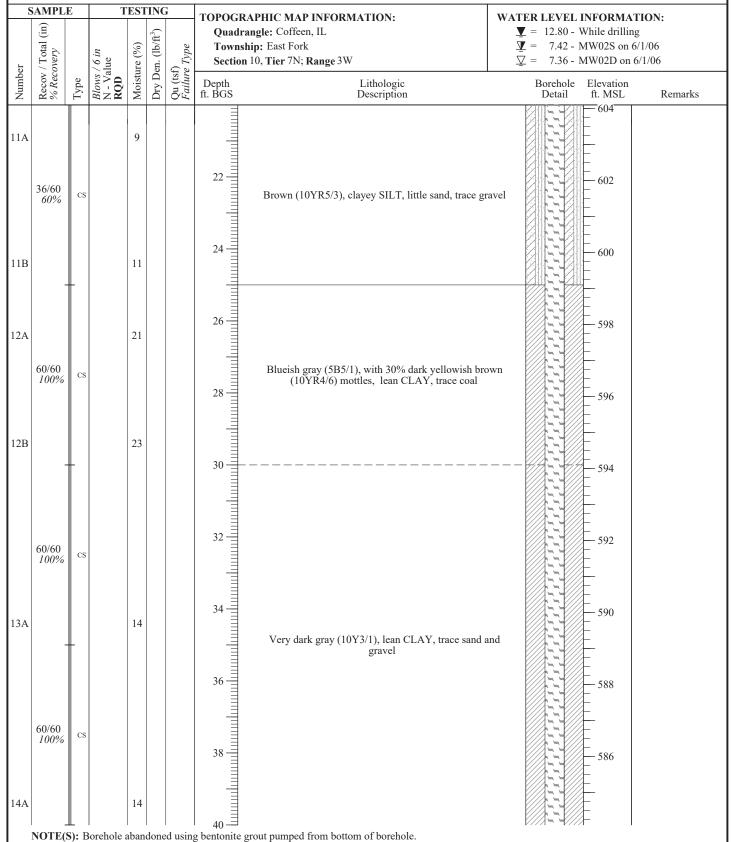
BOREHOLE ID: SB-02 Well **ID:** n/a

 Surface Elev:
 624 ft. MSL

 Completion:
 50 ft. BGS

 Station:
 876,410.0N

 2,513,210.0E



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/5/2006

DATES: Start: 5/5/2006 Finish: 5/5/2006 WEATHER: Partly cloudy, mild (high-50's) CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Helper: R. Keedy Eng/Geo: R. Hasenyager HANSON
BOREHOLE ID: SB-02

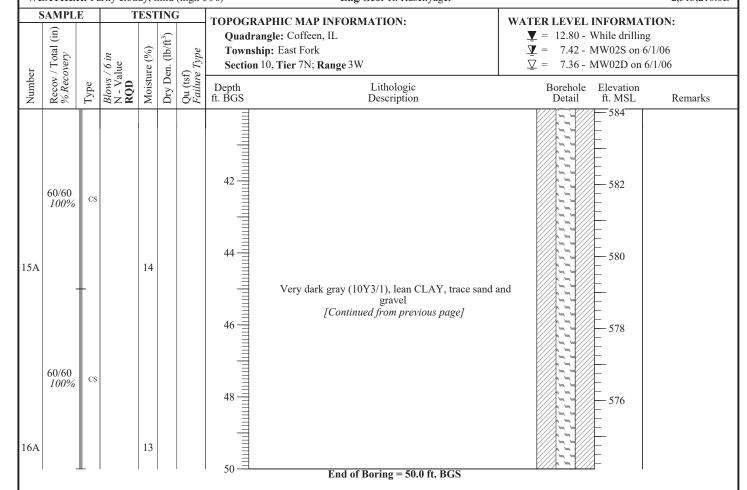
Well ID: n/a

 Surface Elev:
 624 ft. MSL

 Completion:
 50 ft. BGS

 Station:
 876,410.0N

 2,513,210.0E



CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 4/27/2006

Finish: 4/27/2006 WEATHER: Sunny, mild (high-50's) CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS sampler & 41/4" HSA overdrill

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

HANSON

BOREHOLE ID: SB-03 Well ID: MW3D

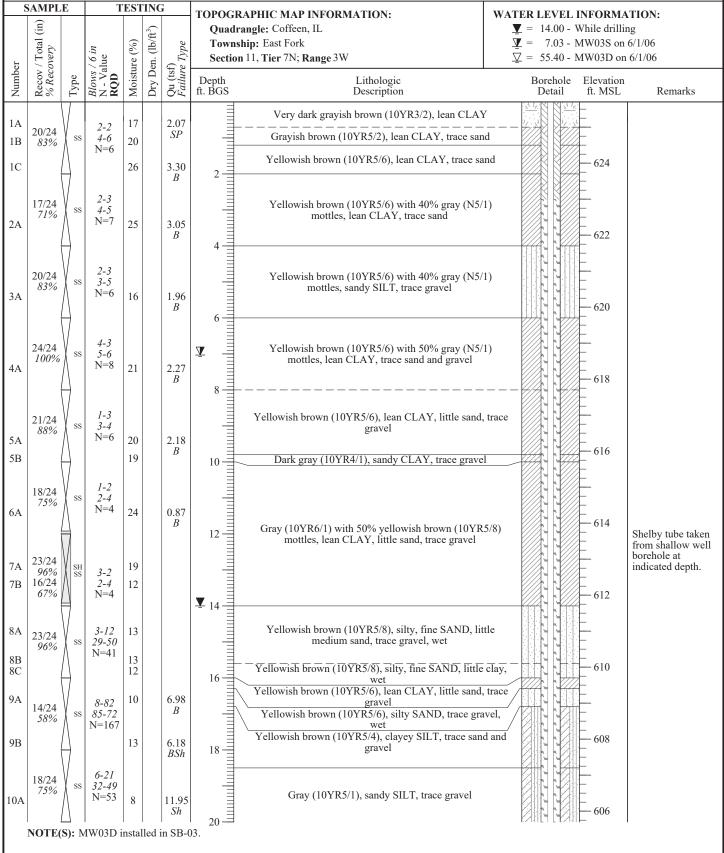
 Surface Elev:
 626 ft. MSL

 Completion:
 58 ft. BGS

 Station:
 876,554.5N

 2,514,535.3E

Page 1 of 3



CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois
Project: 05S3004A

DATES: Start: 4/27/2006

Finish: 4/27/2006

Finish: 4/27/2006
WEATHER: Sunny, mild (high-50's)

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS sampler & 41/4" HSA overdrill

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Helper: R. Keedy Eng/Geo: R. Hasenyager **HANSON**

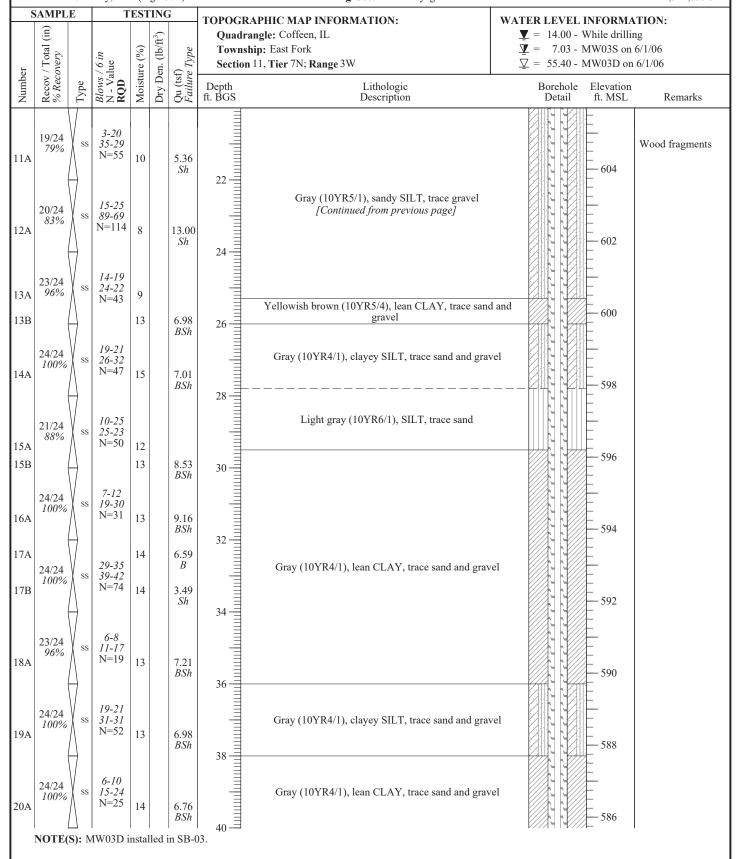
BOREHOLE ID: SB-03 Well ID: MW3D

 Surface Elev:
 626 ft. MSL

 Completion:
 58 ft. BGS

 Station:
 876,554.5N

 2,514,535.3E



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 4/27/2006

Finish: 4/27/2006

WEATHER: Sunny, mild (high-50's)

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS sampler & 41/4" HSA overdrill

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy Eng/Geo: R. Hasenyager BOREHOLE ID: SB-03 Well ID: MW3D

 Surface Elev:
 626 ft. MSL

 Completion:
 58 ft. BGS

 Station:
 876,554.5N

 2,514,535.3E

HANSON

SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Œ, $\mathbf{Y} = 14.00$ - While drilling Quadrangle: Coffeen, IL Dry Den. (lb/ft³) Recov / Total (% Recovery Qu (tsf) Failure Type Township: East Fork $\nabla = 7.03 - MW03S \text{ on } 6/1/06$ Moisture (%) Blows / 6 in N - Value RQD Section 11, Tier 7N; Range 3W ∇ = 55.40 - MW03D on 6/1/06 Number Lithologic Borehole Elevation Description Detail ft. MSL Remarks 4-10 19/24 SS 16-23 Gray (10YR4/1), lean CLAY, trace sand and gravel 79% N = 26[Continued from previous page] 21A 13 8.04 BSh 584 19-27 21/24 SS 28-32 88% N = 5522A 13 7.56 BSh582 4-9 23/24 14-18 Gray (10YR4/1), clayey SILT, trace sand and gravel 96% N = 2314 23A 6.98 BSh580 20-26 24/24 30-33 100% N = 5614 24A 6.59 BSh578 6-10 24/24 13-12 100% N=2320 25A 3.30 BSh576 4-7 6-7 Gray (10YR4/1), lean CLAY, trace sand and gravel 24/24 100% N = 1322 26A 2 91 BSh574 7-18 24/24 37-85 100% 16 4.05 N=55 BSh572 Gray (10Y4/1), silty, fine to medium SAND, wet 27B 13 15-34 23/24 Gray (10YR4/1), silty, fine SAND, trace clay, wet 34-19 96% $\bar{\Delta}$ N = 683.22 11 570 SP 28B 13 5.82 B19-22 Very dark gray (10YR3/1), lean CLAY, trace sand and 24/24 SS 28-18 gravel 100% N = 5029A 20 BShEnd of Boring = 58.0 ft. BGS

NOTE(S): MW03D installed in SB-03.

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/11/2006

Finish: 5/11/2006

WEATHER: Partly sunny, cool (mid-50's)

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 41/4" HSA (blind drill)

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

HANSON

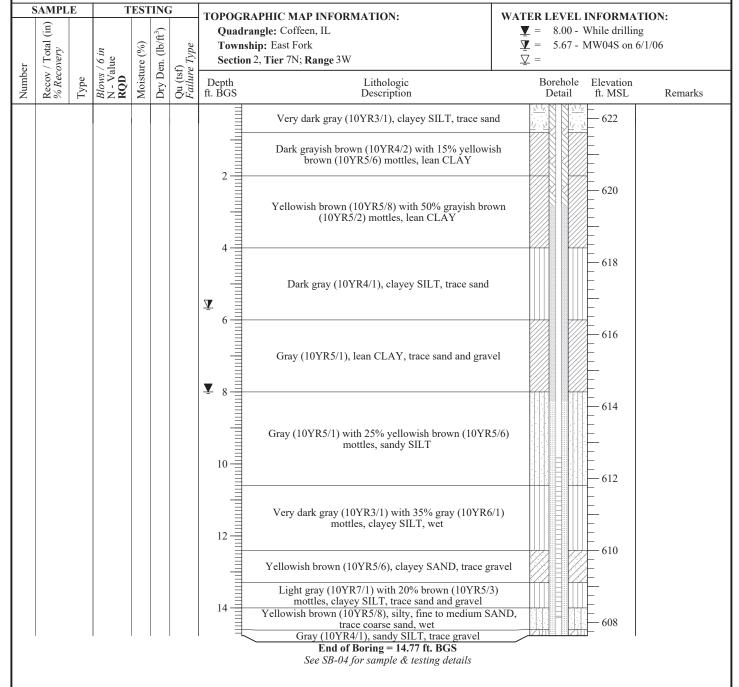
BOREHOLE ID: SB-04a Well ID: MW4S

 Surface Elev:
 622 ft. MSL

 Completion:
 15 ft. BGS

 Station:
 877,999.7N

 2,514,450.6E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A **DATES: Start: 5/11/2006**

Finish: 5/11/2006

WEATHER: Partly sunny, cool (mid-50's) SAMPLE

TESTING

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager TOPOGRAPHIC MAP INFORMATION:

WATER LEVEL INFORMATION:

 Ψ = 8.00 - While drilling

HANSON

622 ft. MSL

55 ft. BGS

2,514,445.0E

878,000.0N

BOREHOLE ID: SB-04

Surface Elev:

Completion:

Station:

Well ID: n/a

 Ξ Quadrangle: Coffeen, IL Dry Den. (lb/ft3 Recov / Total % Recovery Township: East Fork $\Psi = 5.67 - MW04S \text{ on } 6/1/06$ Qu (tsf) Failure Type Moisture (%) Blows / 6 in N - Value RQD Section 2, Tier 7N; Range 3W Number Lithologic Borehole Elevation Description ft. MSL Remarks Very dark gray (10YR3/1), clayey SILT, trace sand 1A 1-2 24 1.09 21/24 SS 2-4 88% Dark grayish brown (10YR4/2) with 15% yellowish N=41B 29 brown (10YR5/6) mottles, lean CLAY 620 Yellowish brown (10YR5/8) with 50% grayish brown 5-6 (10YR5/2) mottles, lean CLAY N=92A 27 2.72 BSh618 2-3 16/24 67% 4-6 Dark gray (10YR4/1), clayey SILT, trace sand N=73A 23 1.71 \mathbf{V} B 616 *4-5 5-7* 24/24 Gray (10YR5/1), lean CLAY, trace sand and gravel 100% N=1020 4A 1.40 **BSP** 1-2 2-2 24/24 100% Gray (10YR5/1) with 25% yellowish brown (10YR5/6) 23 5A 0.70 mottles, sandy SILT 612 24 0.31 6A 0 - 124/24 BSh 2-3 100% Very dark gray (10YR3/1) with 35% gray (10YR6/1) 6B 28 mottles, clayey SILT, wet 12 7A 20 0.08 610 22/24 Yellowish brown (10YR5/6), clayey SAND, trace gravel 3-7 92% 7В 16 Light gray (10YR7/1) with 20% brown (10YR5/3) mottles, clayey SÍLT, trace sand and gravel 7C 12 Yellowish brown (10YR5/8), silty, fine to medium SAND, 608 trace coarse sand, wet 14 4.36 8A 4-9 23/24 22-35 N=31 96% 8B11 606 27-38 24/24 54-50 100% N=92 Gray (10YR4/1), sandy SILT, trace gravel 9A 7 604 24-29 24/24 39-34 SS 100% N = 689 10A 2.18 BSh20

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A

DATES: Start: 5/11/2006 Finish: 5/11/2006

WEATHER: Partly sunny, cool (mid-50's) SAMPLE TESTING

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

TOPOGRAPHIC MAP INFORMATION:

Eng/Geo: R. Hasenyager

WATER LEVEL INFORMATION:

 Ψ = 8.00 - While drilling

BOREHOLE ID: SB-04

Surface Elev:

Completion:

Station:

Well ID: n/a

622 ft. MSL

55 ft. BGS

878,000.0N

2,514,445.0E

Œ, Quadrangle: Coffeen, IL Dry Den. (lb/ft3) Recov / Total (% Recovery Qu (tsf) Failure Type Township: East Fork Ψ = 5.67 - MW04S on 6/1/06 Moisture (%) Section 2, Tier 7N; Range 3W Number Elevation Lithologic Borehole Description Remarks 60/60 600 CS 100% 11A 598 596 60/60 CS 12A Gray (10YR4/1), sandy SILT, trace gravel [Continued from previous page] 60/60 590 CS 100% 8 13A 588 586 60/60 100% 584 14A 13

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 5/11/2006 **Finish:** 5/11/2006

WEATHER: Partly sunny, cool (mid-50's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS & CME samplers

Eng/Geo: R. Hasenyager

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy BOREHOLE ID: SB-04

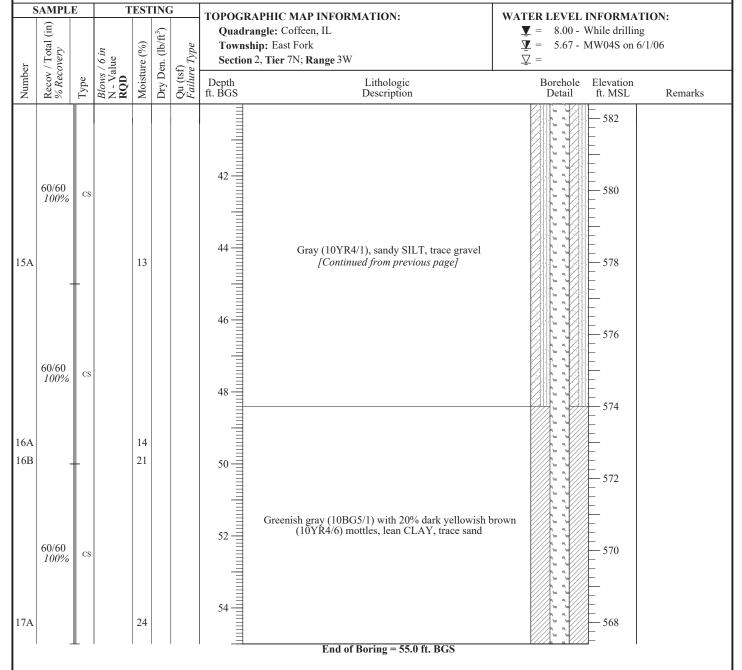
Well ID: n/a

 Surface Elev:
 622 ft. MSL

 Completion:
 55 ft. BGS

 Station:
 878,000.0N

 2,514,445.0E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Sh

NOTE(S): MW05D installed in SB-05.

Location: Coffeen, Illinois Project: 05S3004A **DATES: Start: 5/12/2006**

Finish: 5/17/2006 WEATHER: Sunny, mild (mid-60's) **CONTRACTOR:** Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS sampler & 41/4" HSA overdrill

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-05 Well ID: MW5D

> **Surface Elev:** 623 ft. MSL **Completion:** 54 ft. BGS **Station:** 878,174.8N

2,513,290.3E SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Ξ $\mathbf{V} = 10.00$ - While drilling Quadrangle: Coffeen, IL Dry Den. (lb/ft3 Recov / Total (% Recovery Township: East Fork $\nabla = 6.74 - MW05S \text{ on } 6/1/06$ Qu (tsf) Failure Type Moisture (%) Blows / 6 in N - Value RQD Section 3, Tier 7N; Range 3W ∇ = 50.44 - MW05D on 6/1/06 Number Depth ft. BGS Lithologic Borehole Elevation Description Detail ft. MSL Remarks Dark grayish brown (10YR4/2), clayey SILT, trace sand 29 1A 0 - 219/24 SS 3-4 79% N=51B 28 Gray (10YR5/1 with 50% yellowish brown (10YR5/6) mottles, clayey SILT 620 2-5 5-7 2A 27 SS 92% BSh N=10Very dark brown (10YR2/2) with 20% dark gray (10YR4/1) mottles, clayey SILT, trace sand 2B22 2.13 618 24/24 Dark gray (10YR4/1) with 30% light gray (10YR7/1) 3-6 100% mottles, lean CLAY N=53A 21 2.33 BSP \mathbf{I} 616 7-6 24/24 Gray (10YR6/1), lean CLAY, trace sand 6-8 100% N=1221 4A 1.90 BSh1-3 18/24 Gray (10YR6/1) with 50% yellowish brown (10YR5/6) 4-5 75% mottles, lean CLAY, trace sand 22 1.78 5A В **▼** 10 Yellowish brown (10YR5/6), clayey SAND, trace gravel, 16 6A 612 0 - 120/24 Yellowish brown (10YR5/6) with 50% gray 10YR5/1) 3-4 83% 22 mottles, sandy CLAY 6B 0.70 BSh Gray 10YR6/1), clayey, fine to medium SAND, trace gravel, wet 19 7A 610 24/24 17-20 100% Brownish yellow (10YR6/6), silty, fine SAND, trace N = 23medium sand 7В 16 608 Yellowish brown (10YR5/6), silty, fine SAND, wet 4-16 20/24 25-25 N=41 83% 20 8A 8B11 Brown (10YR5/3), silty SAND and GRAVEL, wet 16 9A 12 606 14-18 24/24 38-62 100% 9B 8 Dark gray (10YR4/1), sandy SILT, trace clay and gravel 604 14-39 18/24 77 SS 75% N=1169 10A 3.27

CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/12/2006

Finish: 5/17/2006 WEATHER: Sunny, mild (mid-60's) CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS sampler & 41/4" HSA overdrill

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager



BOREHOLE ID: SB-05 Well ID: MW5D

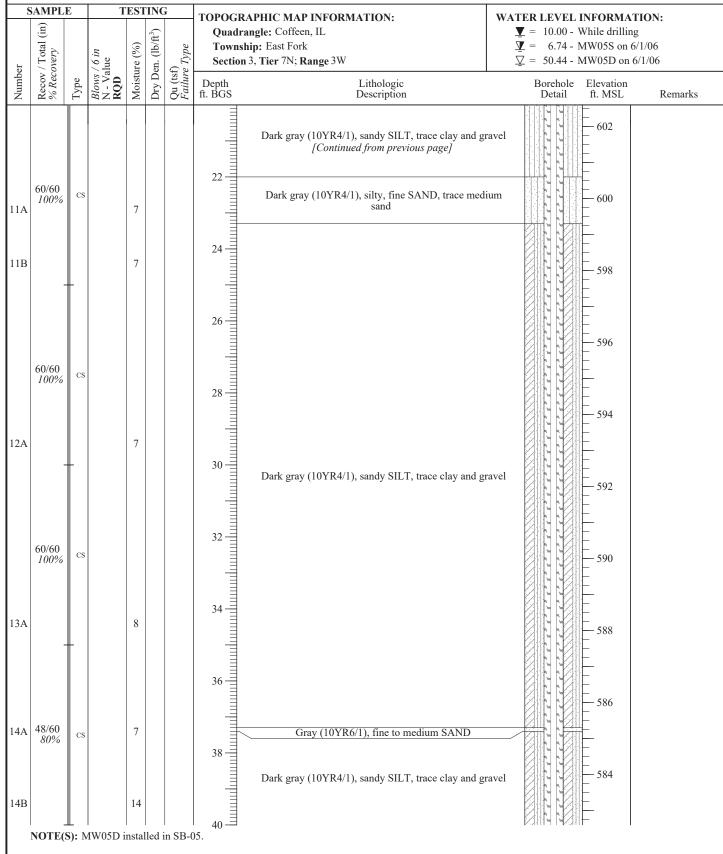
 Surface Elev:
 623 ft. MSL

 Completion:
 54 ft. BGS

 Station:
 878,174.8N

 2,513,290.3E

Page 2 of 3



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/12/2006

DATES: Start: 5/12/2006 Finish: 5/17/2006 WEATHER: Sunny, mild (mid-60's) CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS sampler & 41/4" HSA overdrill

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

2,513,290.3E

BOREHOLE ID: SB-05

Surface Elev:

Completion:

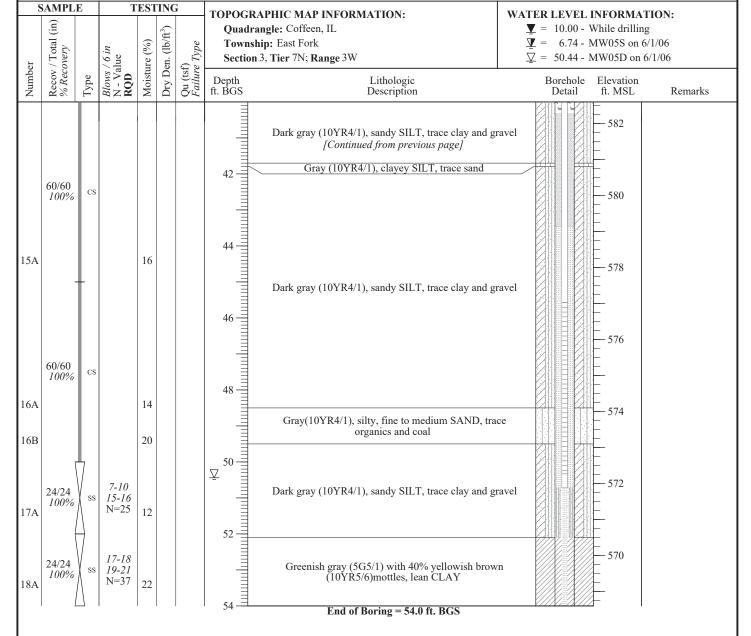
Station:

Well ID: MW5D

623 ft. MSL

54 ft. BGS

878,174.8N



CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/17/2006

DATES: Start: 5/17/2006 **Finish:** 5/17/2006

WEATHER: Sunny, mild (mid-60's)

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 41/4" HSA (blind drill)

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

HANSON

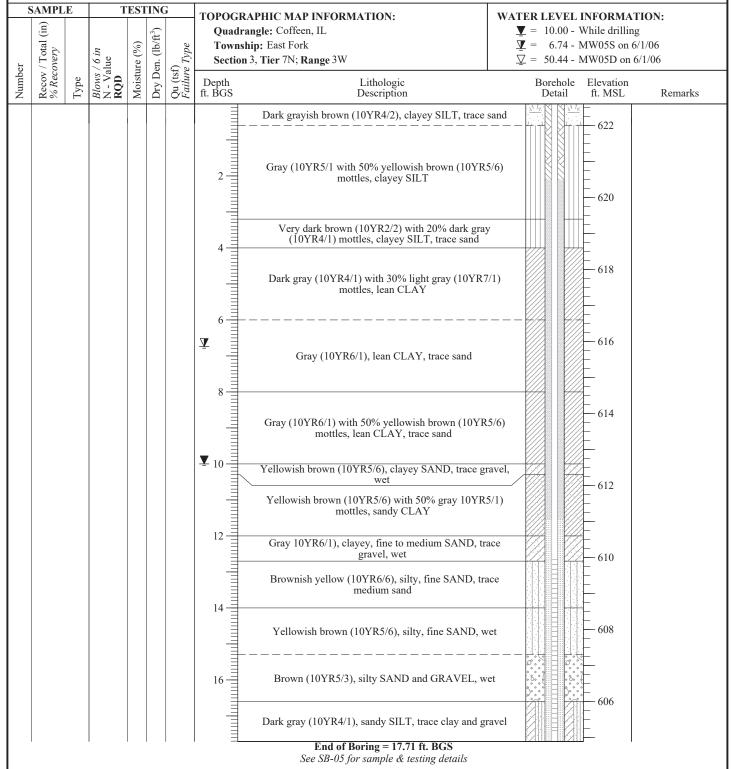
BOREHOLE ID: SB-05a Well ID: MW5S

 Surface Elev:
 623 ft. MSL

 Completion:
 18 ft. BGS

 Station:
 878,175.6N

 2,513,285.5E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Sh

NOTE(S): MW05D installed in SB-05.

Location: Coffeen, Illinois Project: 05S3004A **DATES: Start: 5/12/2006**

Finish: 5/17/2006 WEATHER: Sunny, mild (mid-60's) **CONTRACTOR:** Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS sampler & 41/4" HSA overdrill

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-05 Well ID: MW5D

> **Surface Elev:** 623 ft. MSL **Completion:** 54 ft. BGS **Station:** 878,174.8N

2,513,290.3E SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Ξ $\mathbf{V} = 10.00$ - While drilling Quadrangle: Coffeen, IL Dry Den. (lb/ft3 Recov / Total (% Recovery Township: East Fork $\nabla = 6.74 - MW05S \text{ on } 6/1/06$ Qu (tsf) Failure Type Moisture (%) Blows / 6 in N - Value RQD Section 3, Tier 7N; Range 3W ∇ = 50.44 - MW05D on 6/1/06 Number Depth ft. BGS Lithologic Borehole Elevation Description Detail ft. MSL Remarks Dark grayish brown (10YR4/2), clayey SILT, trace sand 29 1A 0 - 219/24 SS 3-4 79% N=51B 28 Gray (10YR5/1 with 50% yellowish brown (10YR5/6) mottles, clayey SILT 620 2-5 5-7 2A 27 SS 92% BSh N=10Very dark brown (10YR2/2) with 20% dark gray (10YR4/1) mottles, clayey SILT, trace sand 2B22 2.13 618 24/24 Dark gray (10YR4/1) with 30% light gray (10YR7/1) 3-6 100% mottles, lean CLAY N=53A 21 2.33 BSP \mathbf{I} 616 7-6 24/24 Gray (10YR6/1), lean CLAY, trace sand 6-8 100% N=1221 4A 1.90 BSh1-3 18/24 Gray (10YR6/1) with 50% yellowish brown (10YR5/6) 4-5 75% mottles, lean CLAY, trace sand 22 1.78 5A В **▼** 10 Yellowish brown (10YR5/6), clayey SAND, trace gravel, 16 6A 612 0 - 120/24 Yellowish brown (10YR5/6) with 50% gray 10YR5/1) 3-4 83% 22 mottles, sandy CLAY 6B 0.70 BSh Gray 10YR6/1), clayey, fine to medium SAND, trace gravel, wet 19 7A 610 24/24 17-20 100% Brownish yellow (10YR6/6), silty, fine SAND, trace N = 23medium sand 7В 16 608 Yellowish brown (10YR5/6), silty, fine SAND, wet 4-16 20/24 25-25 N=41 83% 20 8A 8B11 Brown (10YR5/3), silty SAND and GRAVEL, wet 16 9A 12 606 14-18 24/24 38-62 100% 9B 8 Dark gray (10YR4/1), sandy SILT, trace clay and gravel 604 14-39 18/24 77 SS 75% N=1169 10A 3.27

CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/12/2006

Finish: 5/17/2006 WEATHER: Sunny, mild (mid-60's) CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS sampler & 41/4" HSA overdrill

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager



BOREHOLE ID: SB-05 Well ID: MW5D

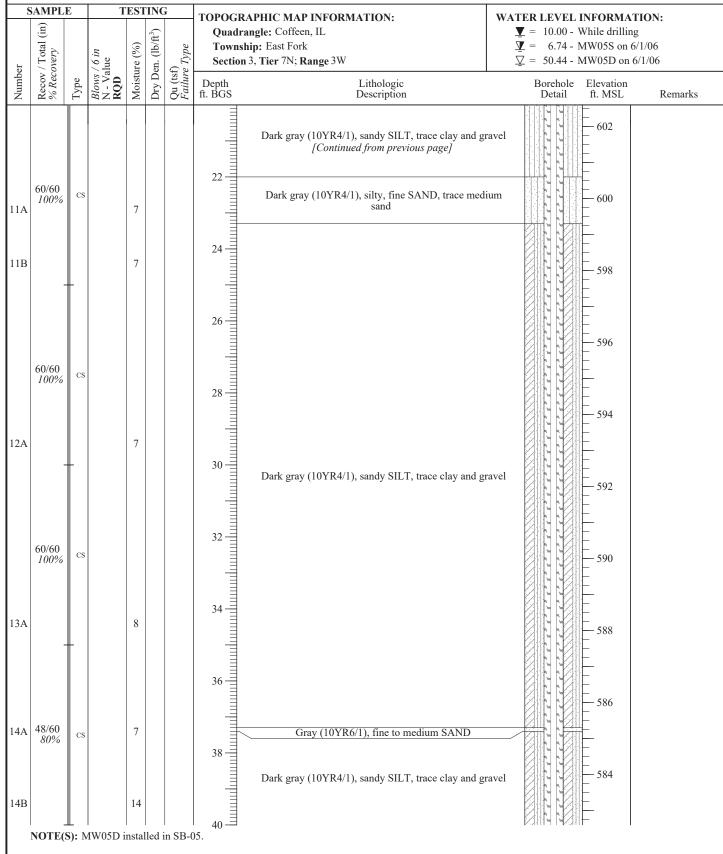
 Surface Elev:
 623 ft. MSL

 Completion:
 54 ft. BGS

 Station:
 878,174.8N

 2,513,290.3E

Page 2 of 3



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/12/2006

DATES: Start: 5/12/2006 Finish: 5/17/2006 WEATHER: Sunny, mild (mid-60's) CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS sampler & 41/4" HSA overdrill

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

2,513,290.3E

BOREHOLE ID: SB-05

Surface Elev:

Completion:

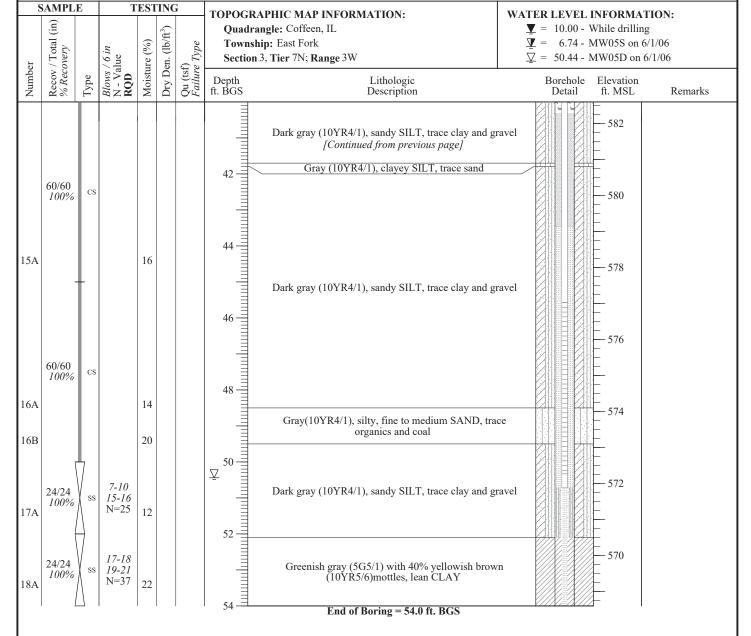
Station:

Well ID: MW5D

623 ft. MSL

54 ft. BGS

878,174.8N



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/4/2006

Finish: 5/4/2006
WEATHER: Partly sunny, cool (mid-50's)

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 31/4" HSA (blind drill)

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

HANSON

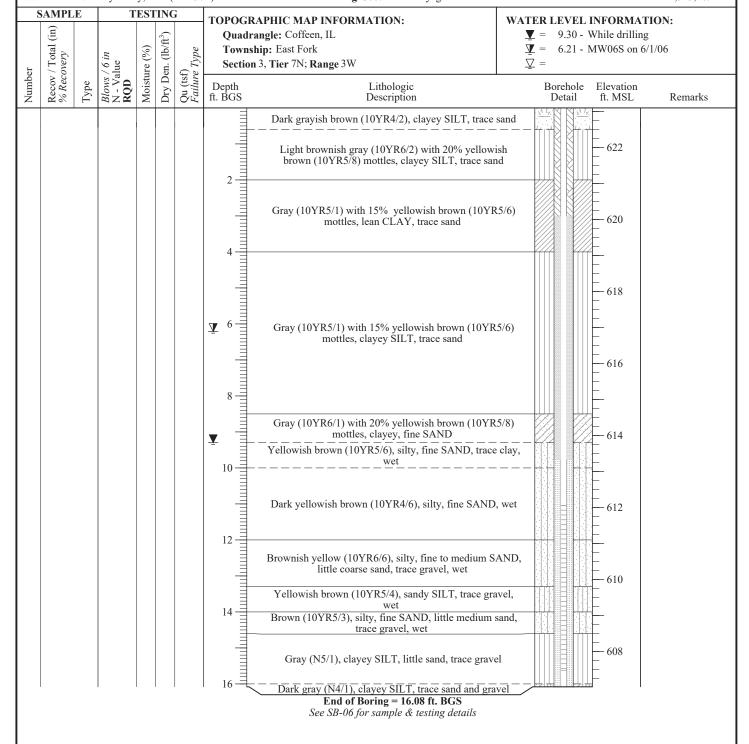
BOREHOLE ID: SB-06a **Well ID:** MW6S

 Surface Elev:
 623 ft. MSL

 Completion:
 16 ft. BGS

 Station:
 879,021.2N

 2,513,189.4E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility Location: Coffeen, Illinois

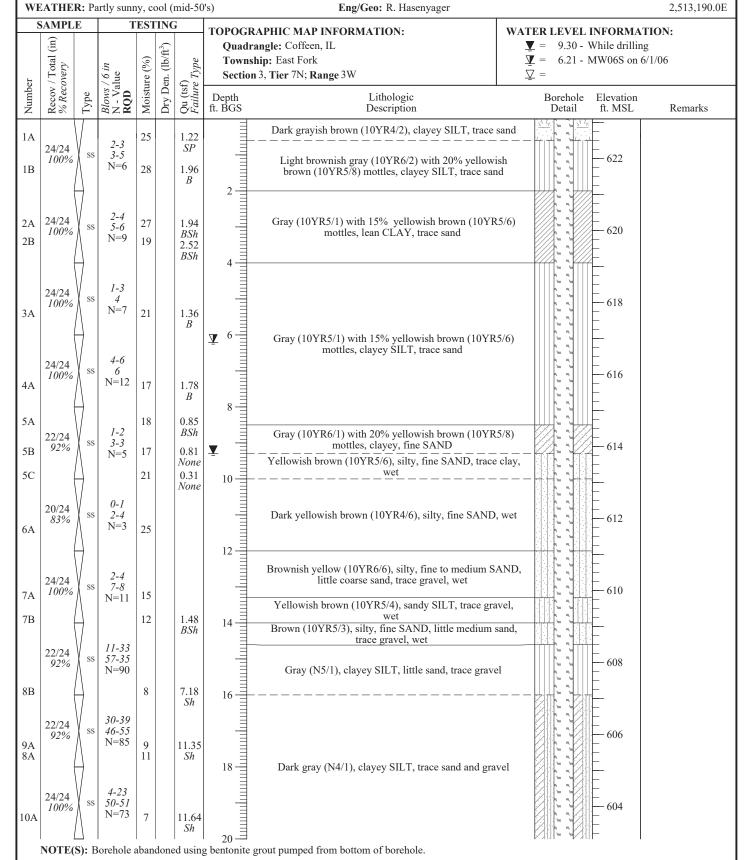
Project: 05S3004A **DATES: Start: 5/4/2006** Finish: 5/4/2006

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 31/4" HSA w/SS sampler

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

BOREHOLE ID: SB-06

Well ID: n/a **Surface Elev:** 623 ft. MSL **Completion:** 60 ft. BGS Station: 879,015.0N



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 5/4/2006

Finish: 5/4/2006 WEATHER: Partly sunny, cool (mid-50's) CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 31/4" HSA w/SS sampler

FIELD STAFF: Driller: B. Williamson **Helper:** R. Keedy

Eng/Geo: R. Hasenyager



BOREHOLE ID: SB-06 Well **ID:** n/a

 Surface Elev:
 623 ft. MSL

 Completion:
 60 ft. BGS

 Station:
 879,015.0N

 2,513,190.0E

5	SAMPL	E	T	EST	INC	3	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) Failure Type	Quadra	ngle: Coffeen, IL hip: East Fork 3, Tier 7N; Range 3W Lithologic	$\underline{\underline{V}} = 9.30 - \underline{\underline{V}} = 6.21 - \underline{\underline{\nabla}} = Borehole$	While drilling MW06S on 6/1/06 Elevation			
Ź	% R	T	BN X	Σ	ā	P.F.		Description	Detail	ft. MSL Remarks			
11A	20/24 83%	ss	6-16 33-58 N=49	11		3.49 BSh	22			602			
12A	20/24 83%	ss	45-56 54-50/3' N=110	9		11.64 Sh	24 —			600			
13A	12/24 50%	ss	26-78	12		2.84 Sh	26			598			
14A	8/24 33%	ss	52-48/2'	9		5.43 BSh	28 —			596			
15A	24/24 100%	ss	10-24 30-40 N=54	13		4.95 BSh	30 —	Dark gray (N4/1), clayey SILT, trace sand and grav [Continued from previous page]	vel	— 594 — 594			
16A	21/24 88%	ss	10-16 37-38 N=53	8		10.91 BSh	32 —						
17A	17/24 71%	ss	36-47 61/5"	9						590			
18A	22/24 92%	ss	11-36 45-60 N=81	9		10.04 Sh	34 = 36 = 36			— 588 — 588			
19A 19B	22/24 92%	ss	40-35 34-29 N=69	10 13		9.60 Sh 8.92	38-			586			
20A	24/24 100%	ss	3-8 12-15 N=20	14		8.53 BSh	40	Very dark greenish gray (10Y3/1), lean CLAY, trace	sand	584			
	NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.												

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility Location: Coffeen, Illinois

DATES: Start: 5/4/2006 Finish: 5/4/2006

Drilling Method: 31/4" HSA w/SS sampler Project: 05S3004A FIELD STAFF: Driller: B. Williamson

Helper: R. Keedy WEATHER: Partly sunny, cool (mid-50's) Eng/Geo: R. Hasenyager

TOPOGRAPHIC MAP INFORMATION.

CONTRACTOR: Testing Service Corporation

Rig mfg/model: CME-650 Track Rig

HANSON

BOREHOLE ID: SB-06 Well ID: n/a

> 623 ft. MSL **Surface Elev: Completion:** 60 ft. BGS 879,015.0N **Station:** 2,513,190.0E

S	SAMPLE			EST	INC	J	TOPOGRAPHIC MAP INFORMATION: W		WATI	WATER LEVEL INFORMATION:				
er	Recov / Total (in) % Recovery	covery (16 in alue (%) oen. (16/ft³)			Qu (tsf) Failure Type	Quadra Townsh	ingle: Coffeen, IL ing: East Fork 3, Tier 7N; Range 3W	WATER LEVEL INFORMATION: $\underline{\Psi} = 9.30$ - While drilling $\underline{\Psi} = 6.21$ - MW06S on 6/1/06 $\underline{\nabla} =$						
Number	Recov % Rea	Type	Blows N - V RQD	Moist	Dry D	Qu (ts Failu	Depth ft. BGS	Lithologic Description		Borehole Detail	Elevation ft. MSL	Remarks		
21A	23/24 96%	SS	3-5 8-12 N=13	15		4.27 B	42 —				582			
22A	22/24 92%	ss	6-6 7-8 N=13	14		3.49 B	44				580			
23A	24/24 100%	ss	5-7 9-12 N=16	16		2.72 B	46	42————————————————————————————————————	sand		578			
24A	24/24 100%	ss	4-8 10-11 N=18	15		4.07 B	48				576			
25A	24/24 100%	ss	5-6 9-13 N=15	15		3.10 B	50				574			
26A	24/24 100%	ss	5-6 8-12 N=14	22		1.94 B	52				572			
27A	24/24 100%	ss	5-6 8-8 N=14	23		2.13 B	54 —	Dark greenish gray (10BG4/1), lean CLAY			570			
28A	24/24 100%	ss	3-5 6-8 N=11	24		2.33 BSh	56				568			
29A	24/24 100%	ss	12-10 12-14 N=22	24		3.30 BSh	58	Greenish gray (5G4/1) with 10% dark yellowish bro (10YR3/4) mottles, lean CLAY, trace sand	own		566			
30A	24/24 100%	ss	5-8 12-13 N=20	27		2.13 BSh	60	Dark greenish gray (10G4/1), lean CLAY, trace sa End of Boring = 60.0 ft. BGS rout pumped from bottom of borehole.	and		564			

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A **DATES: Start:** 5/9/2006

Finish: 5/9/2006 WEATHER: Overcast, mild (mid-60's) **CONTRACTOR:** Reynolds Drilling Corp. Rig mfg/model: CME-1050 ATV Rig Drilling Method: 41/4" HSA (blind drill)

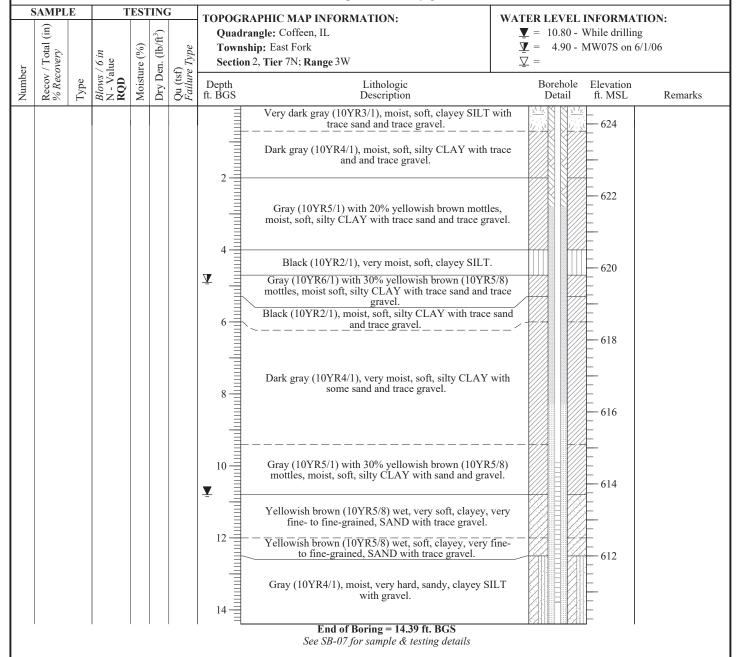
FIELD STAFF: Driller: P. McIntire Helper: S. McCartney Eng/Geo: R. Hasenyager

Well ID: MW7S **Surface Elev:** 625 ft. MSL **Completion:** 14 ft. BGS

Station:

BOREHOLE ID: SB-07a

879,181.1N 2,514,397.5E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A **DATES: Start: 5/5/2006** Finish: 5/8/2006

WEATHER: Partly cloudy, mild (mid-70s)

CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-1050 ATV Rig

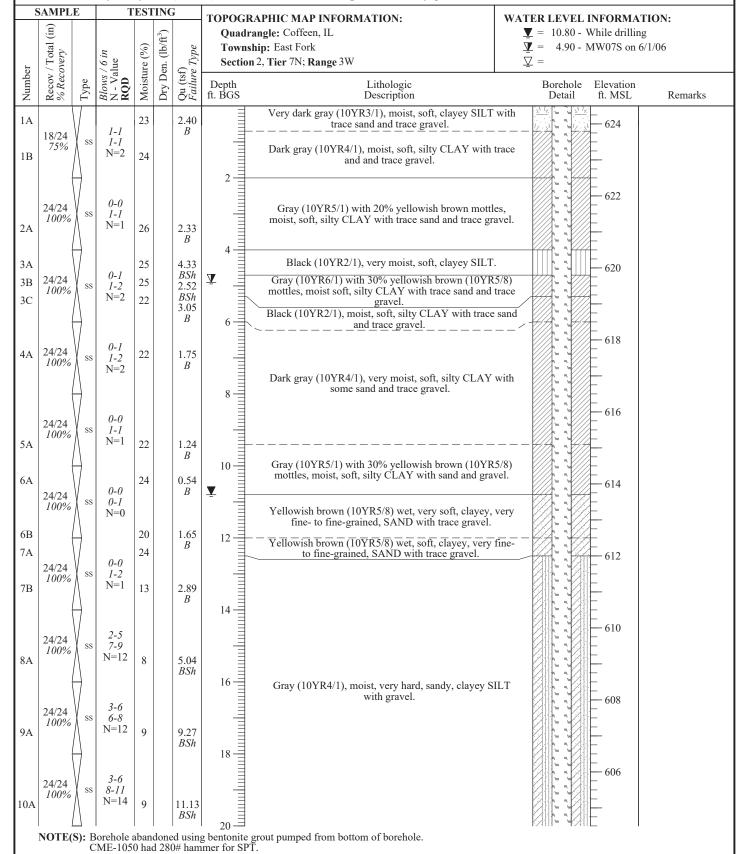
Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: P. McIntire Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-07 Well ID: n/a

Surface Elev: 625 ft. MSL **Completion:** 54 ft. BGS Station: 879,180.0N 2,514,390.0E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

TESTING

Location: Coffeen, Illinois Project: 05S3004A **DATES: Start:** 5/5/2006

SAMPLE

Finish: 5/8/2006 WEATHER: Partly cloudy, mild (mid-70s) **CONTRACTOR:** Reynolds Drilling Corp. Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

Eng/Geo: R. Hasenyager

FIELD STAFF: Driller: P. McIntire Helper: S. McCartney

TOPOGRAPHIC MAP INFORMATION:

WATER LEVEL INFORMATION:

 $\mathbf{V} = 10.80$ - While drilling

BOREHOLE ID: SB-07

Surface Elev:

Completion:

Station:

Well ID: n/a

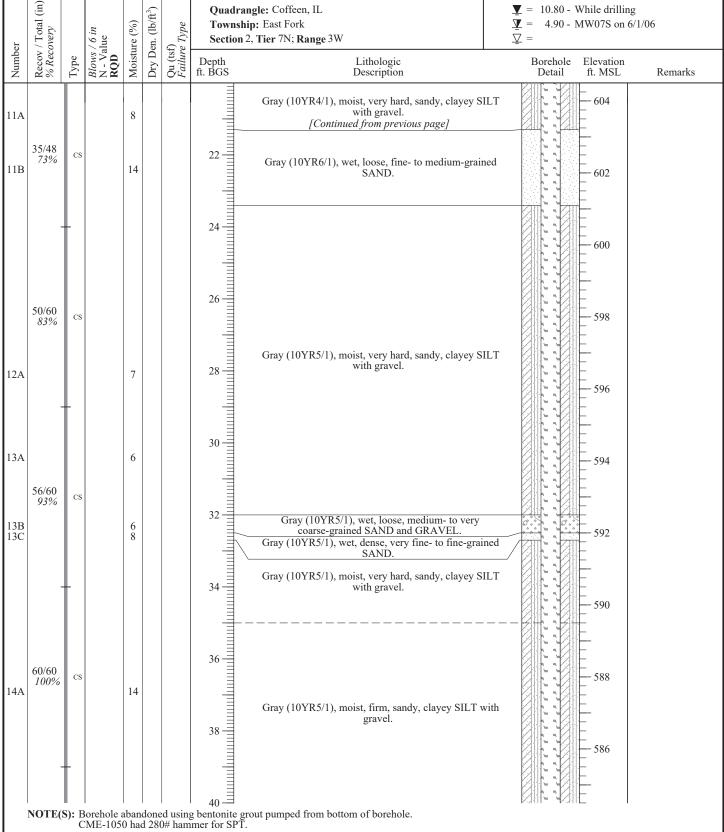
625 ft. MSL

54 ft. BGS

879,180.0N

2,514,390.0E

Page 2 of 3



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

TESTING

Location: Coffeen, Illinois Project: 05S3004A **DATES: Start:** 5/5/2006

Finish: 5/8/2006 WEATHER: Partly cloudy, mild (mid-70s)

SAMPLE

CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: P. McIntire Helper: S. McCartney

Eng/Geo: R. Hasenyager

WATER LEVEL INFORMATION:

HANSON

625 ft. MSL

54 ft. BGS

879,180.0N

2,514,390.0E

BOREHOLE ID: SB-07

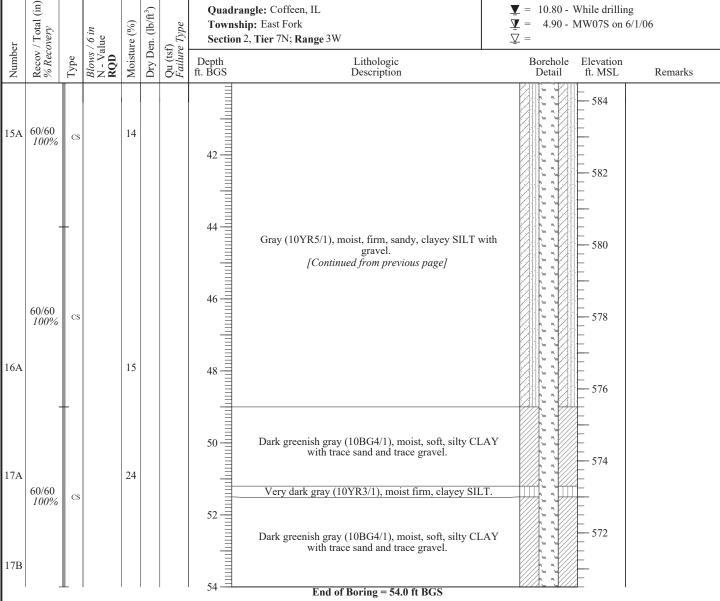
Surface Elev:

Completion:

Station:

Well ID: n/a

TOPOGRAPHIC MAP INFORMATION: Quadrangle: Coffeen, IL $\mathbf{Y} = 10.80$ - While drilling Township: East Fork Section 2, Tier 7N; Range 3W



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois

Project: 05S3004A **DATES: Start:** 5/10/2006

Finish: 5/10/2006

SAMPLE

WEATHER: Foggy to partly sunny, mild (hi-60's)

TESTING

CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-1050 ATV Rig Drilling Method: 4¹/₄" HSA (blind drill)

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

TOPOGRAPHIC MAP INFORMATION:

Eng/Geo: R. Hasenyager

2,514,478.8E

WATER LEVEL INFORMATION:

HANSON

625 ft. MSL

17 ft. BGS

879,776.6N

BOREHOLE ID: SB-08a

Surface Elev:

Completion:

Station:

Well ID: MW8S

 Ξ $\mathbf{Y} = 12.70$ - While drilling Quadrangle: Coffeen, IL Dry Den. (lb/ft3 Recov / Total % Recovery Township: East Fork $\Psi = 5.33 - MW08S \text{ on } 6/1/06$ Qu (tsf) Failure Type Moisture (%) Blows / 6 in N - Value RQD Section 2, Tier 7N; Range 3W Number Depth ft. BGS Lithologic Borehole Elevation Description Detail ft. MSL Remarks Very dark gray (10YR3/1), moist, soft, clayey SILT with 624 trace sand and trace gravel. Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with little sand. 622 Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with little sand. 620 Gray (10YR5/1) with 40% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with little sand. 1 Gray (10YR5/1), moist, soft, clayey SILTwith little sand and trace gravel. 618 Yellowish brown (10YR5/6) with 20% gray (10YR5/1) mottles, moist, soft, sandy CLAY with sl. trace gravel. 616 614 Yellowish brown (10YR5/6) with 10% gray (10YR5/1) mottles, moist, soft, sandy CLAY with sl. trace gravel. 12 Y 612 Gray (10YR5/1), wet, soft, very silty, very fine- to coarse-grained SAND. Light gray (10YR6/1) with 50% yellowish brown 610 (10YR5/6) mottles, wet, very dense, very fine- to fine-grained SAND. Gray (10YR5/1) with 30% yellowish brown (10YR5/6) mottles, wet, dense, fine- to very coarse-grained SAND. 16 Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel. 608

> End of Boring = 17.08 ft. BGS See SB-08 for sample & testing details

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois
Project: 05S3004A

DATES: Start: 5/9/2006

Finish: 5/10/2006
WEATHER: Foggy to partly sunny, mild (hi-60's)

CONTRACTOR: Reynolds Drilling Corp. **Rig mfg/model:** CME-1050 ATV Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

Eng/Geo: R. Hasenyage

Helper: S. McCartney **Eng/Geo:** R. Hasenyager

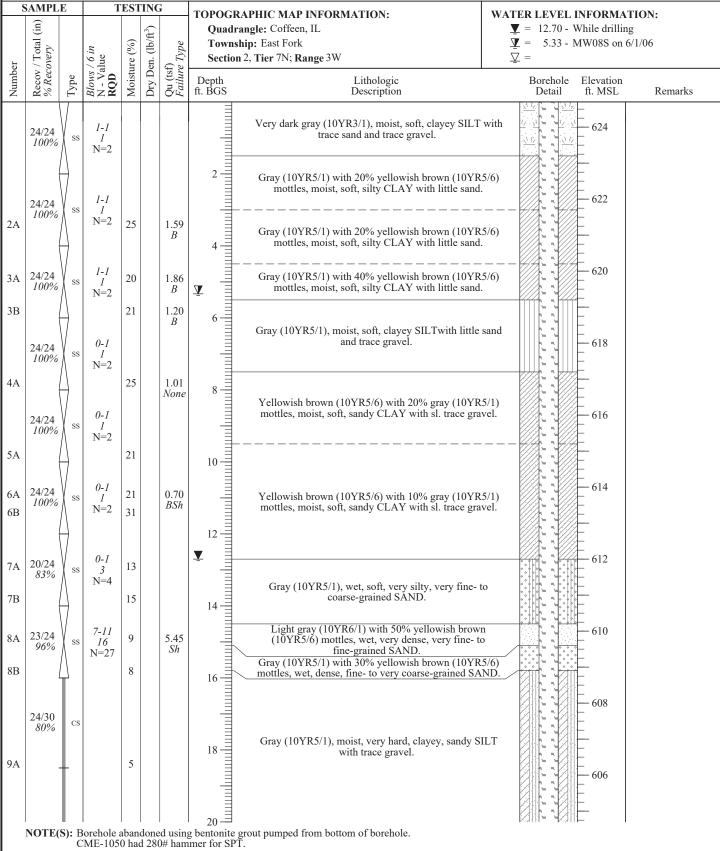
BOREHOLE ID: SB-08 Well ID: n/a

> Surface Elev: 625 ft. MSL Completion: 59 ft. BGS Station: 879,770.0N

HANSON

2,514,480.0E

Page 1 of 3



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A **DATES: Start:** 5/9/2006

SAMPLE

Finish: 5/10/2006 WEATHER: Foggy to partly sunny, mild (hi-60's)

TESTING

CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

Eng/Geo: R. Hasenyager

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

TOPOGRAPHIC MAP INFORMATION:

Quadrangle: Coffeen, IL Township: East Fork

WATER LEVEL INFORMATION:

HANSON

625 ft. MSL

59 ft. BGS

879,770.0N

2,514,480.0E

BOREHOLE ID: SB-08

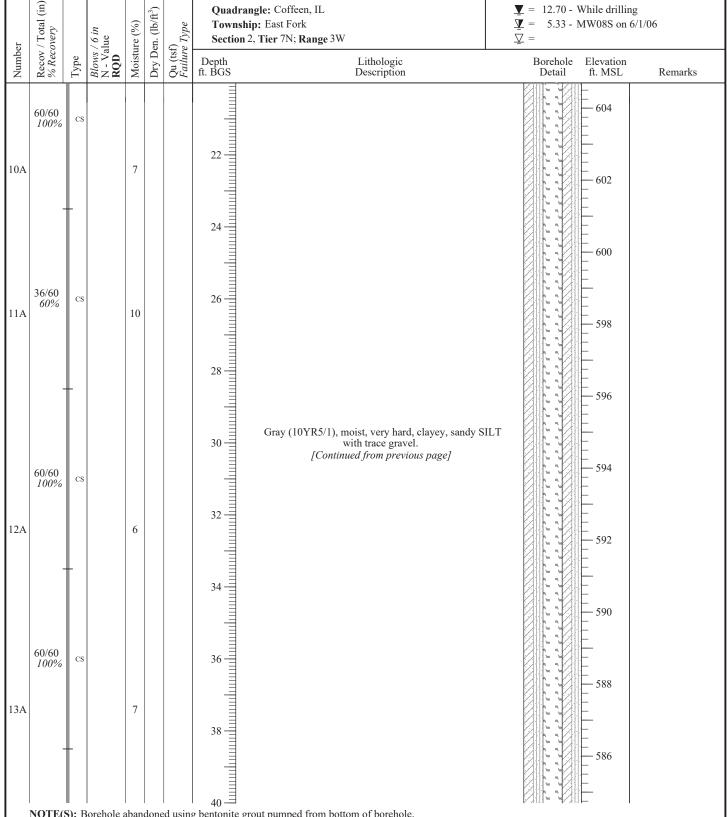
Surface Elev:

Completion:

Station:

Well ID: n/a

 $\mathbf{Y} = 12.70$ - While drilling



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A **DATES:** Start: 5/9/2006

Finish: 5/10/2006
WEATHER: Foggy to partly sunny, mild (hi-60's)

CONTRACTOR: Reynolds Drilling Corp. **Rig mfg/model:** CME-1050 ATV Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney

Eng/Geo: R. Hasenyager

HANSON
BOREHOLE ID: SB-08

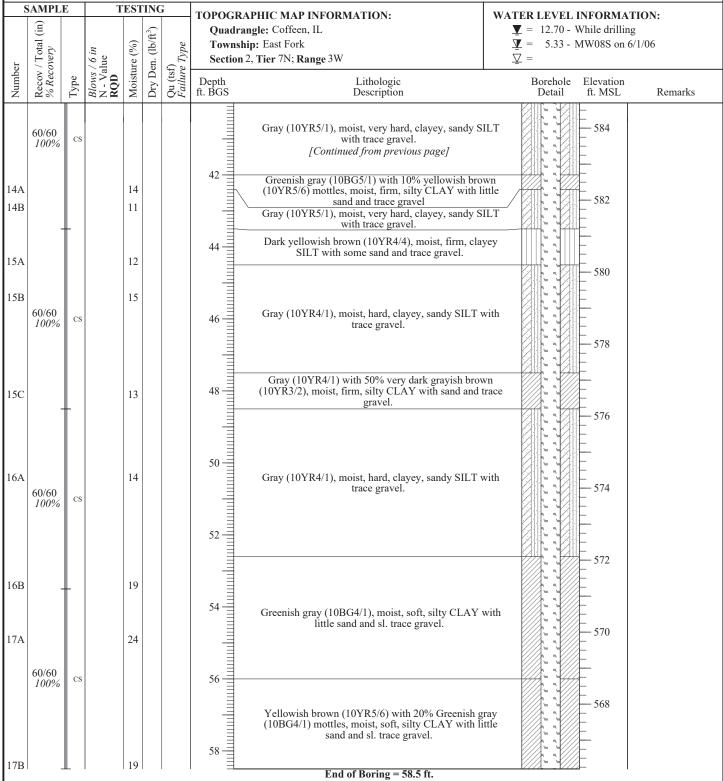
Well ID: n/a

 Surface Elev:
 625 ft. MSL

 Completion:
 59 ft. BGS

 Station:
 879,770.0N

 2,514,480.0E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A **DATES: Start:** 5/3/2006 Finish: 5/3/2006

WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager



HANSON

625 ft. MSL

54 ft. BGS

Page 1 of 3

879,679.7N

BOREHOLE ID: SB-09

Surface Elev:

Completion:

Station:

Well ID: MW9D

SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Ξ $\mathbf{V} = 14.00$ - While drilling Quadrangle: Coffeen, IL Dry Den. (lb/ft3 Recov / Total % Recovery Township: East Fork $\nabla = 5.23 - MW09S \text{ on } 6/1/06$ Qu (tsf) Failure Type Moisture (%) Blows / 6 in N - Value RQD Section 2, Tier 7N; Range 3W ∇ = 52.46 - MW09D on 6/1/06 Number Depth ft. BGS Lithologic Borehole Elevation Description Detail ft. MSL Remarks Very dark gray (10YR3/1), moist, soft, clayey SILT with trace sand and trace gravel. 0 - 122/24 SS 1-1 92% N=21 A 24 1.65 R622 1-2 20/24 Yellowish brown (10YR5/6), moist, soft, silty CLAY with 1-1 83% little sand. N=32A 27 2.06 620 24/24 1-1 <u>v</u> 100% N=23A 24 1.65 RGray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with little sand. 618 1-1 24/24 1-2 100% N=223 4A 1.57 В Gray (10YR5/1) with 50% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with sand. 616 0 - 124/24 100% N=227 5A 1.85 BYellowish brown (10YR5/6) with 10% Gray (10YR5/1) mottles, moist, soft, silty CLAY with sand. 614 0-0 24/24 0-0 100% N=022 6A 1.44 Yellowish brown (10YR5/8), very moist, soft, sandy CLAY with trace gravel. 21 7A 0.93 612 Yellowish brown (10YR5/6) with 30% gray (10YR5/1) mottles, moist to very moist, clayey SILT with sand and 0-0 BSh24/24 100% N=17В 18 0.97 trace gravel. BShGray (10YR5/1), wet, soft, clayey very fine- to **▼** 14 7C 15 fine-grained SAND. Gray (10YR5/1), wet, loose, fine- to very coarse-grained 84 18 610 SAND with trace gravel. 1-3 24/24 Gray (10YR5/1), wet, dense, silty very fine- to fine-grained SAND. 6-6 100% N=98B18 608 7-13 24/24 19-25 100% N = 329A 8 7.86 Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel. 12/12 CS 606 100% NOTE(S): MW09D installed in SB-09. CME-1050 had 280# hammer for SPT.

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/3/2006

Finish: 5/3/2006 WEATHER: Overcast, mild (mid-60's) **CONTRACTOR:** Reynolds Drilling Corp. **Rig mfg/model:** CME-1050 ATV Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

Helper: S. McCartney

FIELD STAFF: Driller: K. Doetzel

Eng/Geo: R. Hasenyager

HANSON

BOREHOLE ID: SB-09 Well ID: MW9D

 Surface Elev:
 625 ft. MSL

 Completion:
 54 ft. BGS

 Station:
 879,679.7N

 2,515,666.3E

SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Œ, Quadrangle: Coffeen, IL $\mathbf{Y} = 14.00$ - While drilling Dry Den. (lb/ft³) Recov / Total (% Recovery Qu (tsf) Failure Type Township: East Fork $\Psi = 5.23 - MW09S \text{ on } 6/1/06$ Moisture (%) Blows / 6 in N - Value RQD Section 2, Tier 7N; Range 3W ∇ = 52.46 - MW09D on 6/1/06 Number Lithologic Borehole Elevation Description Detail ft. MSL Remarks 604 60/60 CS 100% 2 11A 602 600 49/60 CS 598 82% 12A 3 2" Sand stringer Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel. [Continued from previous page] 596 594 60/60 CS 100% 592 13A 3 590 Yellowish brown (10YR5/6) with 40% gray (10YR5/1) mottles, moist, very hard, clayey SILT with little sand and 60/60 10 14A CS occasional dry, silt stringers (<1"). 100% 588 Gray (10YR5/1), moist, very hard, clayey SILT with little sand and trace gravel. 14B 11 586 DRILLER NOTE: Appears more plastic Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel. NOTE(S): MW09D installed in SB-09. CME-1050 had 280# hammer for SPT. Page 2 of 3

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois
Project: 05S3004A

DATES: Start: 5/3/2006

Finish: 5/3/2006 WEATHER: Overcast, mild (mid-60's) **CONTRACTOR:** Reynolds Drilling Corp. **Rig mfg/model:** CME-1050 ATV Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

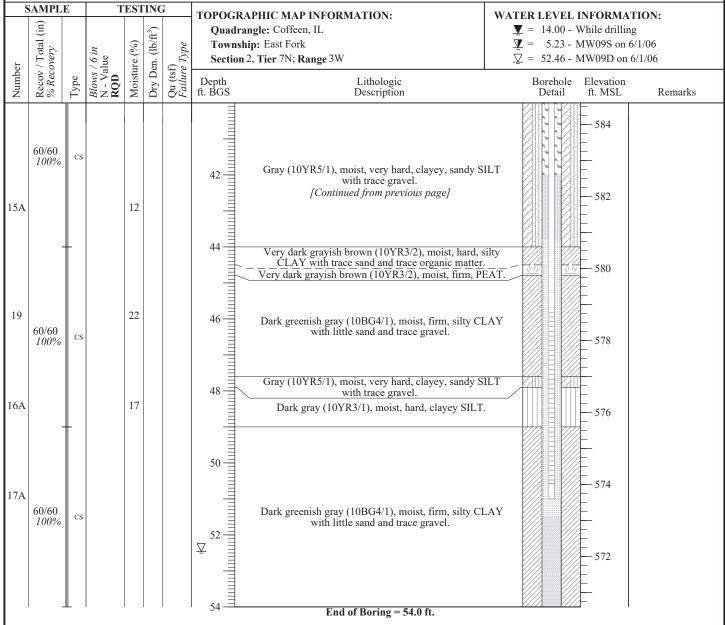
FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-09

Well ID: MW9D Surface Elev: 625 ft. MSL

Completion: 54 ft. BGS Station: 879,679.7N 2,515,666.3E



NOTE(S): MW09D installed in SB-09.

CME-1050 had 280# hammer for SPT.

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois

Project: 05S3004A **DATES: Start:** 5/3/2006

Finish: 5/3/2006 WEATHER: Overcast, mild (mid-60's) CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-1050 ATV Rig Drilling Method: 4¹/₄" HSA (blind drill)

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager

HANSON

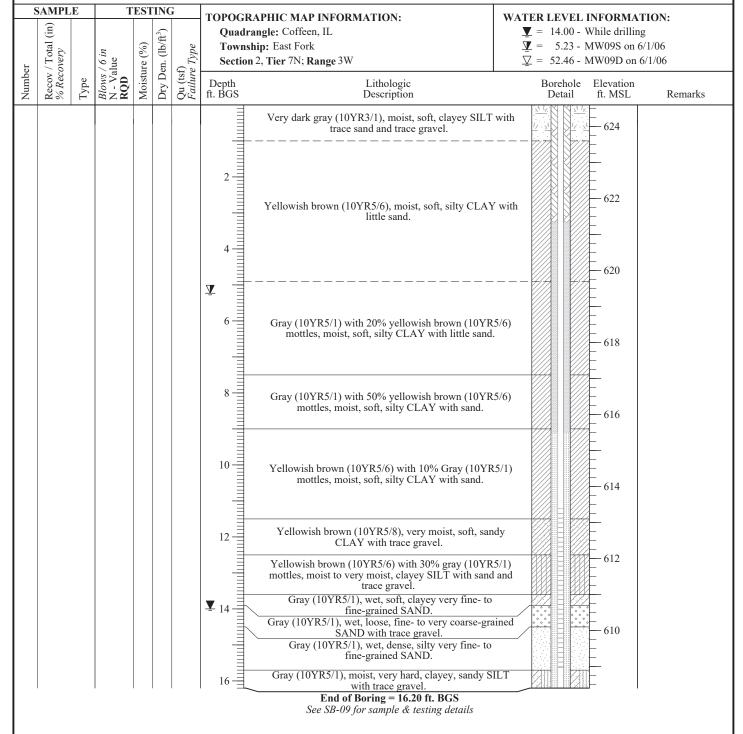
BOREHOLE ID: SB-09a **Well ID:** MW9S

 Surface Elev:
 625 ft. MSL

 Completion:
 16 ft. BGS

 Station:
 879,684.9N

 2,515,666.2E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A **DATES: Start:** 5/3/2006 Finish: 5/3/2006

WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager



HANSON

625 ft. MSL

54 ft. BGS

Page 1 of 3

879,679.7N

BOREHOLE ID: SB-09

Surface Elev:

Completion:

Station:

Well ID: MW9D

SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Ξ $\mathbf{V} = 14.00$ - While drilling Quadrangle: Coffeen, IL Dry Den. (lb/ft³ Recov / Total % Recovery Township: East Fork $\nabla = 5.23 - MW09S \text{ on } 6/1/06$ Qu (tsf) Failure Type Moisture (%) Blows / 6 in N - Value RQD Section 2, Tier 7N; Range 3W ∇ = 52.46 - MW09D on 6/1/06 Number Depth ft. BGS Lithologic Borehole Elevation Description Detail ft. MSL Remarks Very dark gray (10YR3/1), moist, soft, clayey SILT with trace sand and trace gravel. 0 - 122/24 SS 1-1 92% N=21 A 24 1.65 R622 1-2 20/24 Yellowish brown (10YR5/6), moist, soft, silty CLAY with 1-1 83% little sand. N=32A 27 2.06 620 24/24 1-1 <u>v</u> 100% N=23A 24 1.65 RGray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with little sand. 618 1-1 24/24 1-2 100% N=223 4A 1.57 В Gray (10YR5/1) with 50% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with sand. 616 0 - 124/24 100% N=227 5A 1.85 BYellowish brown (10YR5/6) with 10% Gray (10YR5/1) mottles, moist, soft, silty CLAY with sand. 614 0-0 24/24 0 - 0100% N=022 6A 1.44 Yellowish brown (10YR5/8), very moist, soft, sandy CLAY with trace gravel. 21 7A 0.93 612 Yellowish brown (10YR5/6) with 30% gray (10YR5/1) mottles, moist to very moist, clayey SILT with sand and 0-0 BSh24/24 100% N=17В 18 0.97 trace gravel. BShGray (10YR5/1), wet, soft, clayey very fine- to **▼** 14 7C 15 fine-grained SAND. Gray (10YR5/1), wet, loose, fine- to very coarse-grained 84 18 610 SAND with trace gravel. 1-3 24/24 Gray (10YR5/1), wet, dense, silty very fine- to fine-grained SAND. 6-6 100% N=98B18 608 7-13 24/24 19-25 100% N = 329A 8 7.86 Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel. 12/12 CS 606 100% NOTE(S): MW09D installed in SB-09. CME-1050 had 280# hammer for SPT.

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/3/2006

Finish: 5/3/2006 WEATHER: Overcast, mild (mid-60's) **CONTRACTOR:** Reynolds Drilling Corp. **Rig mfg/model:** CME-1050 ATV Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

Helper: S. McCartney

FIELD STAFF: Driller: K. Doetzel

Eng/Geo: R. Hasenyager

HANSON

BOREHOLE ID: SB-09 Well ID: MW9D

 Surface Elev:
 625 ft. MSL

 Completion:
 54 ft. BGS

 Station:
 879,679.7N

 2,515,666.3E

SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Œ, Quadrangle: Coffeen, IL $\mathbf{Y} = 14.00$ - While drilling Dry Den. (lb/ft³) Recov / Total (% Recovery Qu (tsf) Failure Type Township: East Fork $\Psi = 5.23 - MW09S \text{ on } 6/1/06$ Moisture (%) Blows / 6 in N - Value RQD Section 2, Tier 7N; Range 3W ∇ = 52.46 - MW09D on 6/1/06 Number Lithologic Borehole Elevation Description Detail ft. MSL Remarks 604 60/60 CS 100% 2 11A 602 600 49/60 CS 598 82% 12A 3 2" Sand stringer Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel. [Continued from previous page] 596 594 60/60 CS 100% 592 13A 3 590 Yellowish brown (10YR5/6) with 40% gray (10YR5/1) mottles, moist, very hard, clayey SILT with little sand and 60/60 10 14A CS occasional dry, silt stringers (<1"). 100% 588 Gray (10YR5/1), moist, very hard, clayey SILT with little sand and trace gravel. 14B 11 586 DRILLER NOTE: Appears more plastic Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel. NOTE(S): MW09D installed in SB-09. CME-1050 had 280# hammer for SPT. Page 2 of 3

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/3/2006

Finish: 5/3/2006 WEATHER: Overcast, mild (mid-60's) **CONTRACTOR:** Reynolds Drilling Corp. **Rig mfg/model:** CME-1050 ATV Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

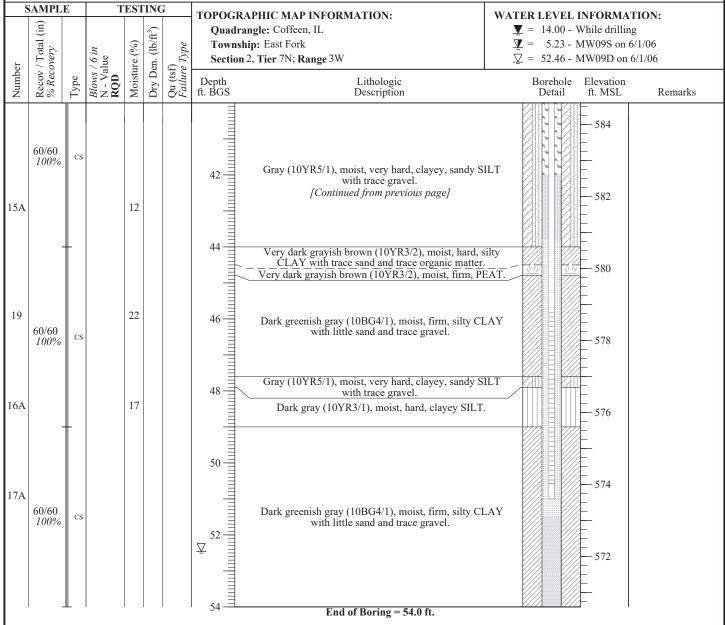
FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-09

Well ID: MW9D Surface Elev: 625 ft. MSL

Completion: 54 ft. BGS Station: 879,679.7N 2,515,666.3E



NOTE(S): MW09D installed in SB-09.

CME-1050 had 280# hammer for SPT.

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois
Project: 05S3004A

DATES: Start: 5/1/2006

DATES: Start: 5/1/2006 Finish: 5/1/2006 WEATHER: Overcast, mild (mid-60's) CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager

HANSON

BOREHOLE ID: SB-10 Well ID: MW10D

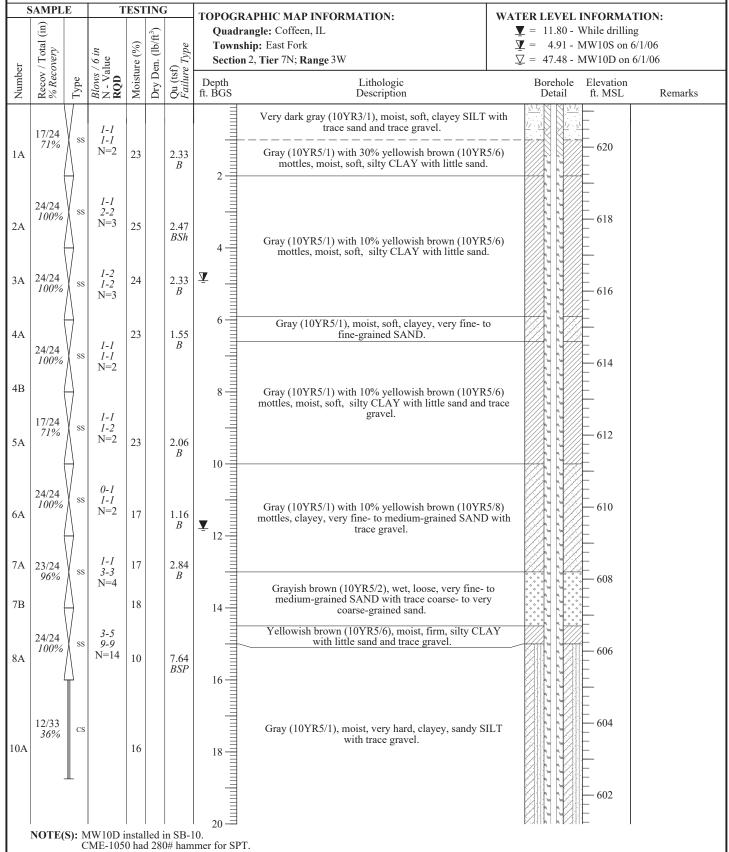
 Surface Elev:
 621 ft. MSL

 Completion:
 49 ft. BGS

 Station:
 878,245.1N

 2,515,914.0E

Page 1 of 3



CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/1/2006

DATES: Start: 5/1/2006 Finish: 5/1/2006 WEATHER: Overcast, mild (mid-60's) CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-1050 ATV Rig

Drilling Method: $4\frac{1}{4}$ " HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager



BOREHOLE ID: SB-10 Well ID: MW10D

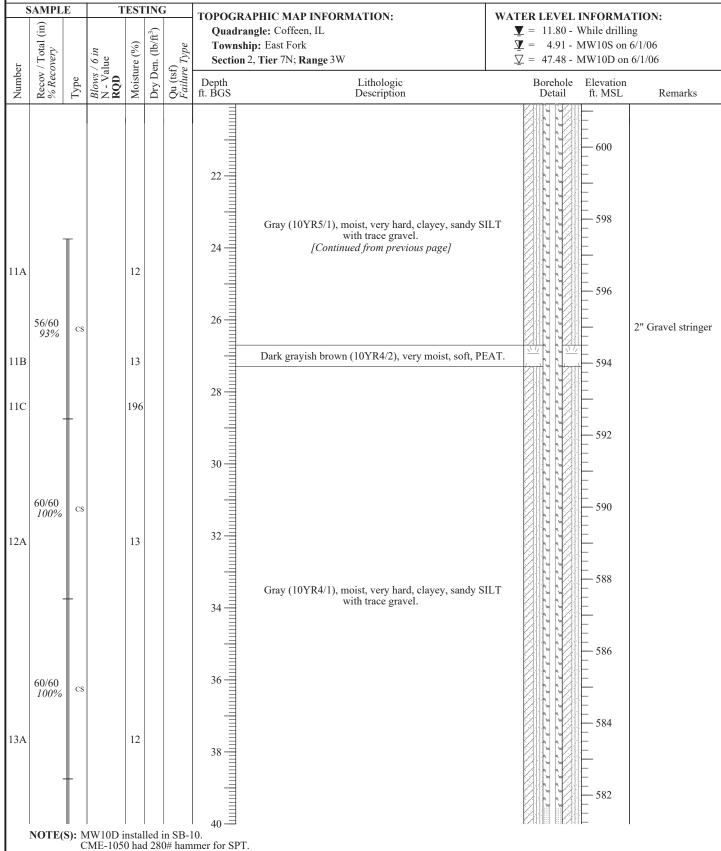
 Surface Elev:
 621 ft. MSL

 Completion:
 49 ft. BGS

 Station:
 878,245.1N

 2,515,914.0E

Page 2 of 3



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A **DATES: Start:** 5/1/2006

Finish: 5/1/2006

WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

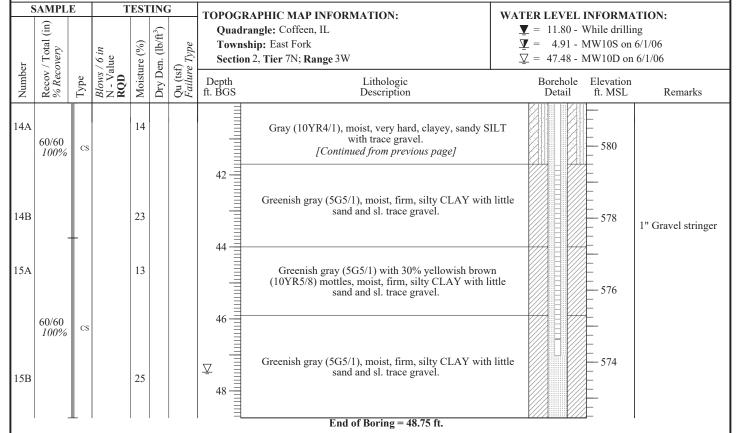
FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-10

Well ID: MW10D **Surface Elev:** 621 ft. MSL 49 ft. BGS **Completion:** Station: 878,245.1N 2,515,914.0E

HANSON



NOTE(S): MW10D installed in SB-10.

CME-1050 had 280# hammer for SPT.

CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/2/2006

Finish: 5/2/2006
WEATHER: Sunny, mild (mid-60's)

CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-1050 ATV Rig Drilling Method: 4¹/₄" HSA (blind drill)

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney Eng/Geo: R. Hasenyager **HANSON**

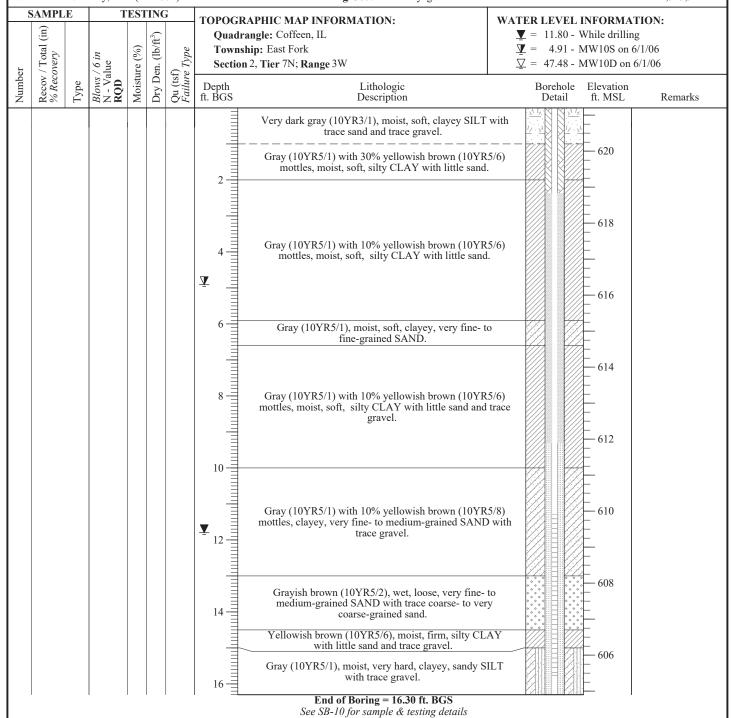
BOREHOLE ID: SB-10a **Well ID:** MW10S

 Surface Elev:
 621 ft. MSL

 Completion:
 16 ft. BGS

 Station:
 878,250.5N

 2,515,914.4E



NOTE(S): MW10S installed in blind-drilled borehole within 10 ft of SB-10.

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois
Project: 05S3004A

DATES: Start: 5/1/2006

DATES: Start: 5/1/2006 Finish: 5/1/2006 WEATHER: Overcast, mild (mid-60's) CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager

HANSON

BOREHOLE ID: SB-10 Well ID: MW10D

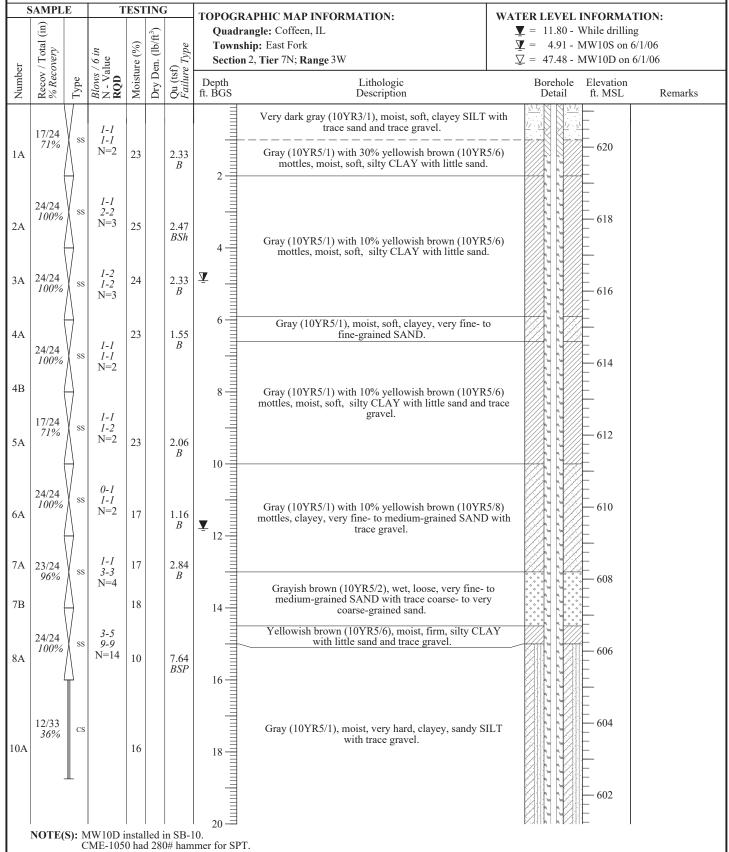
 Surface Elev:
 621 ft. MSL

 Completion:
 49 ft. BGS

 Station:
 878,245.1N

 2,515,914.0E

Page 1 of 3



CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/1/2006

DATES: Start: 5/1/2006 Finish: 5/1/2006 WEATHER: Overcast, mild (mid-60's) CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-1050 ATV Rig

Drilling Method: $4\frac{1}{4}$ " HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager



BOREHOLE ID: SB-10 Well ID: MW10D

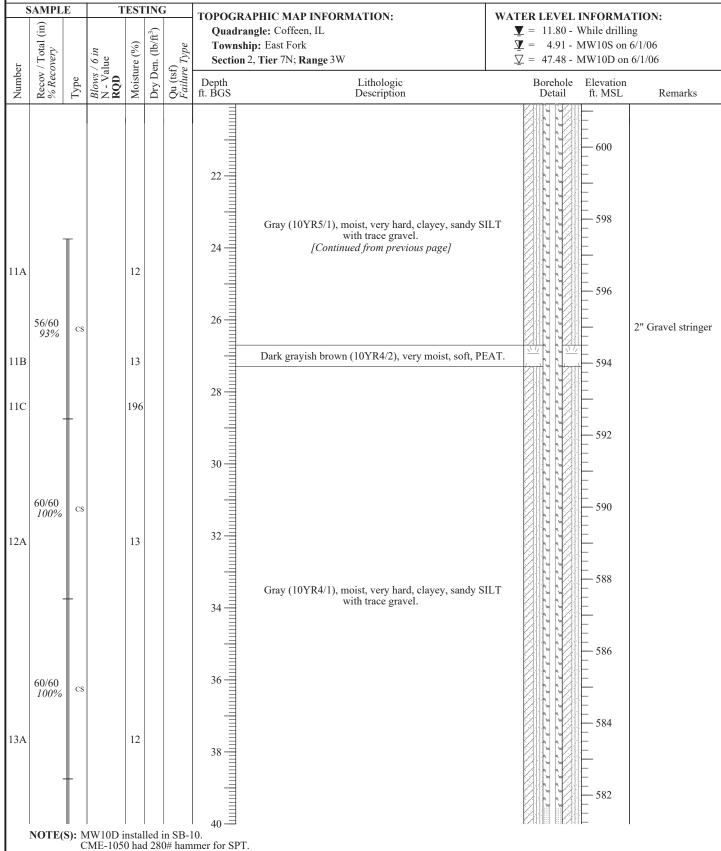
 Surface Elev:
 621 ft. MSL

 Completion:
 49 ft. BGS

 Station:
 878,245.1N

 2,515,914.0E

Page 2 of 3



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A **DATES: Start:** 5/1/2006

Finish: 5/1/2006

WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

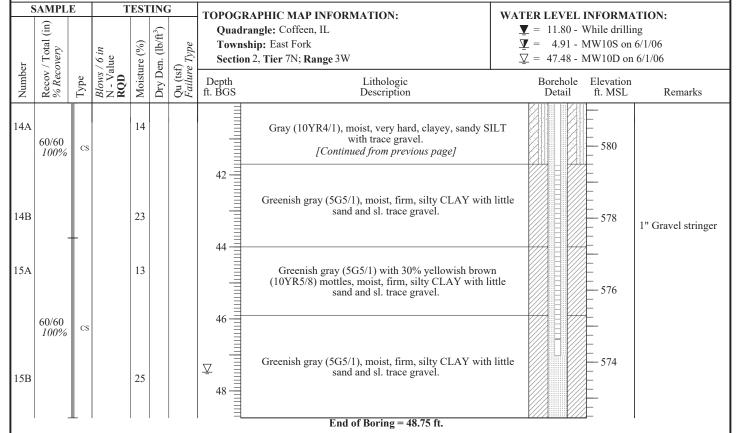
FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-10

Well ID: MW10D **Surface Elev:** 621 ft. MSL 49 ft. BGS **Completion:** Station: 878,245.1N 2,515,914.0E

HANSON



NOTE(S): MW10D installed in SB-10.

CME-1050 had 280# hammer for SPT.

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A **DATES: Start: 4/27/2006** Finish: 4/28/2006

SAMPLE

WEATHER: Partly cloudy, mild (mid-60's)

TESTING

CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-850 Track Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager

2,515,976.7E

HANSON

622 ft. MSL

36 ft. BGS

Page 1 of 2

876,749.6N

Well ID: MW11D

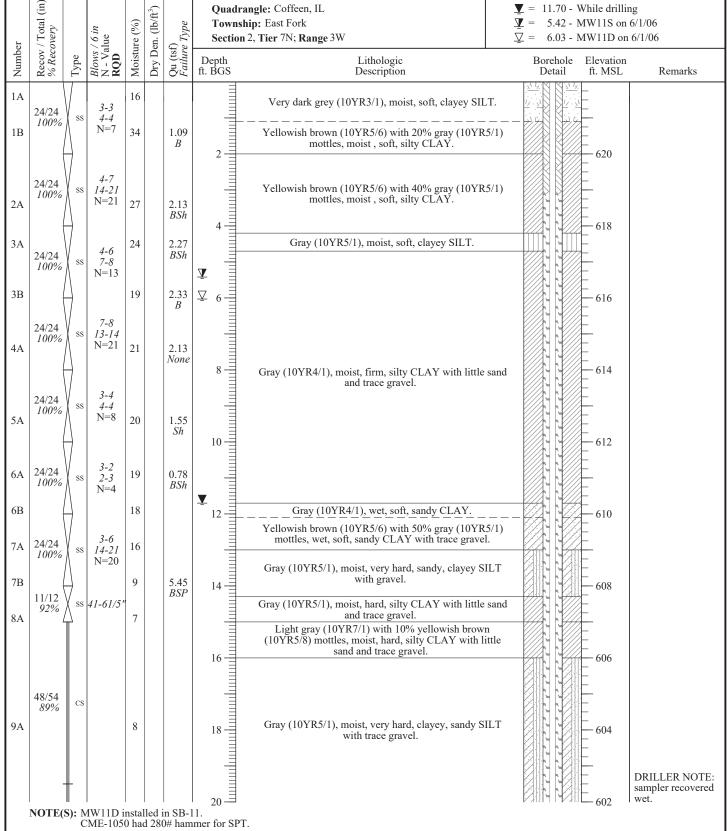
BOREHOLE ID: SB-11

Surface Elev:

Completion:

Station:

TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: $\mathbf{V} = 11.70$ - While drilling Quadrangle: Coffeen, IL



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A **DATES: Start: 4/27/2006** Finish: 4/28/2006

SAMPLE

WEATHER: Partly cloudy, mild (mid-60's)

TESTING

CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-850 Track Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager

Quadrangle: Coffeen, IL Township: East Fork

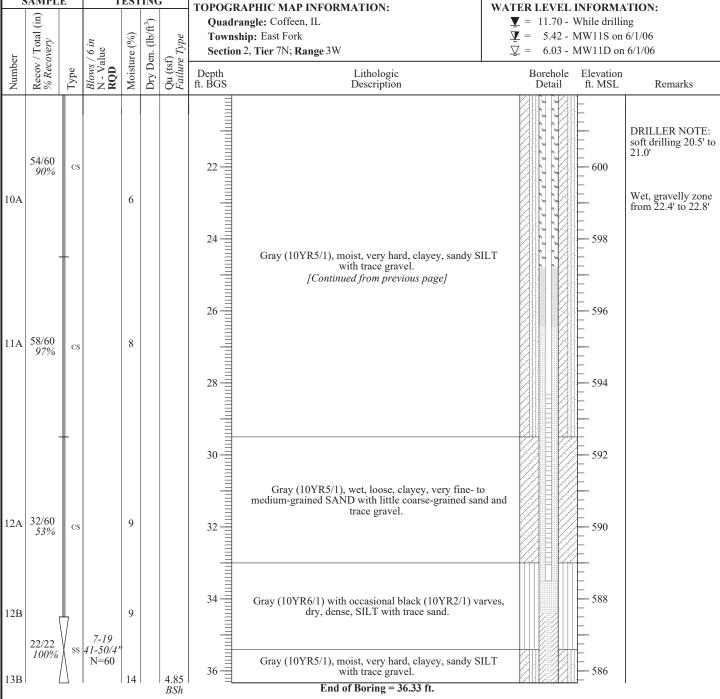
BOREHOLE ID: SB-11

Well ID: MW11D

Surface Elev: 622 ft. MSL 36 ft. BGS **Completion:** Station: 876,749.6N

HANSON

2,515,976.7E



NOTE(S): MW11D installed in SB-11.

CME-1050 had 280# hammer for SPT.

CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 4/28/2006

Finish: 4/28/2006 WEATHER: Partly cloudy, mild (mid-60's) CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-850 Track Rig

Drilling Method: 4¼" HSA (blind drill)

FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

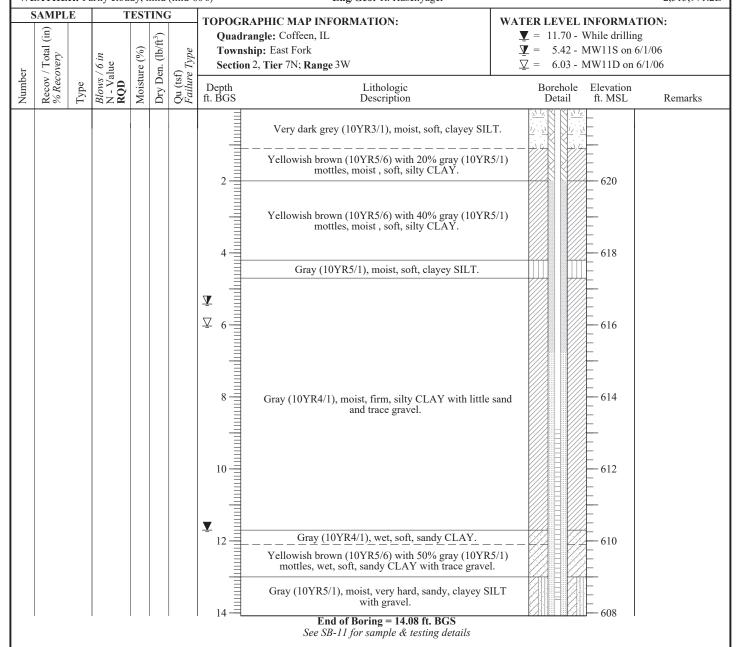
HANSON
BOREHOLE ID: SB-11a

Well ID: MW11S

Surface Elev: 622 ft. MSL

Completion: 14 ft. BGS

Station: 876,749.4N
2,515,971.2E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A **DATES: Start: 4/27/2006** Finish: 4/28/2006

SAMPLE

WEATHER: Partly cloudy, mild (mid-60's)

TESTING

CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-850 Track Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager

2,515,976.7E

HANSON

622 ft. MSL

36 ft. BGS

Page 1 of 2

876,749.6N

Well ID: MW11D

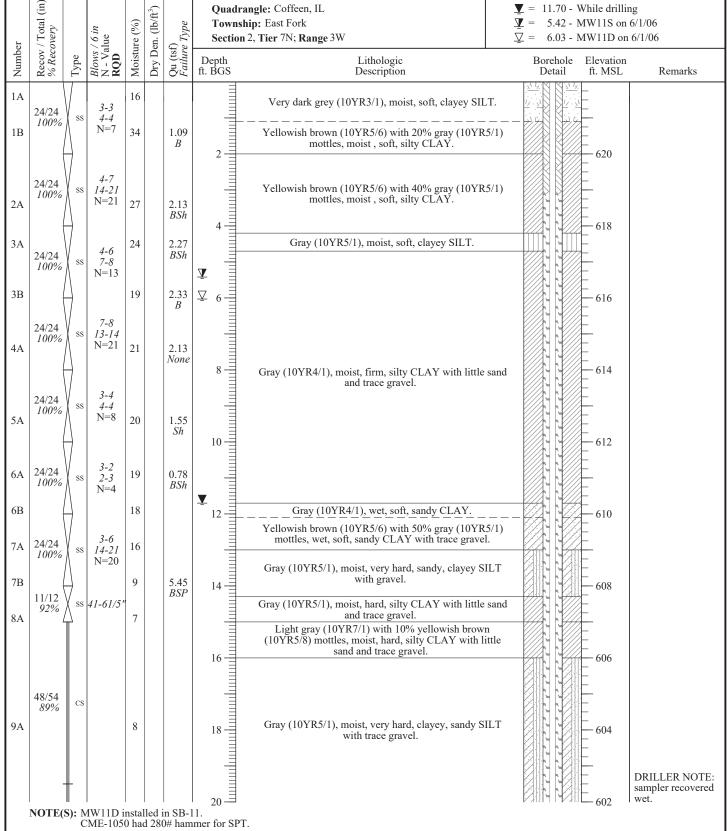
BOREHOLE ID: SB-11

Surface Elev:

Completion:

Station:

TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: $\mathbf{V} = 11.70$ - While drilling Quadrangle: Coffeen, IL



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A **DATES: Start: 4/27/2006** Finish: 4/28/2006 **CONTRACTOR:** Reynolds Drilling Corp. Rig mfg/model: CME-850 Track Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

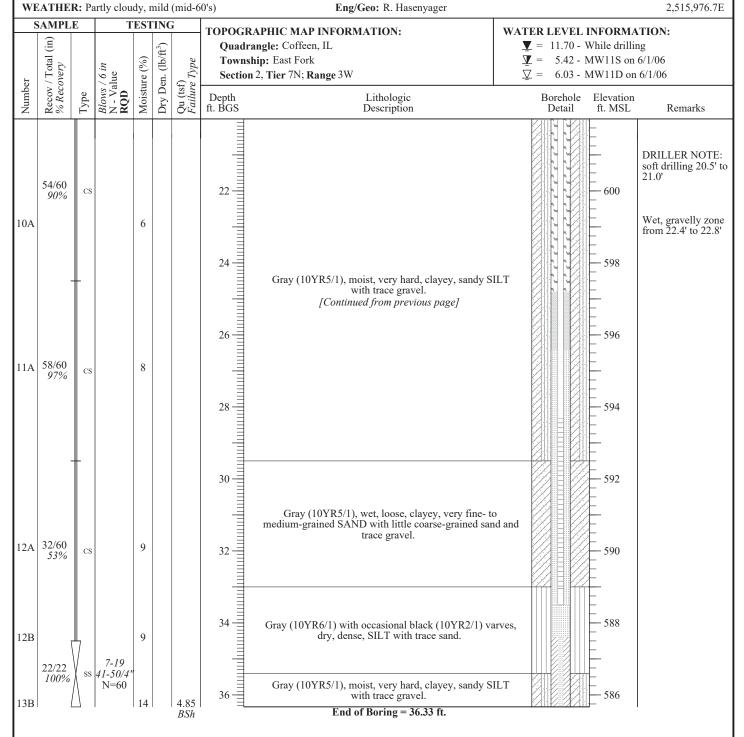
FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-11 Well ID: MW11D

Surface Elev: 622 ft. MSL 36 ft. BGS **Completion:** Station: 876,749.6N 2,515,976.7E

HANSON



NOTE(S): MW11D installed in SB-11.

CME-1050 had 280# hammer for SPT.

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

TESTING

Location: Coffeen, Illinois Project: 05S3004A **DATES: Start:** 5/10/2006

Finish: 5/10/2006

SAMPLE

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

WEATHER: Foggy to partly sunny, mild (hi-60's) Eng/Geo: R. Hasenyager

> Quadrangle: Coffeen, IL Township: East Fork

TOPOGRAPHIC MAP INFORMATION:

WATER LEVEL INFORMATION: $\mathbf{V} = 12.00$ - While drilling

 Ξ Dry Den. (lb/ft3 Recov / Total (% Recovery $\nabla = 6.76 - MW12S \text{ on } 6/1/06$ Qu (tsf) Failure Type Moisture (%) Blows / 6 in N - Value RQD Section 11, Tier 7N; Range 3W ∇ = 46.90 - MW12D on 6/1/06 Number Depth ft. BGS Lithologic Borehole Elevation Description Detail ft. MSL Remarks Very dark gray (10YR3/1), clayey SILT, trace sand 2-3 24"/24 22 1.27 1A SS 4-5 Sh Dark gray (10YR4/1) with 15% yellowish brown (10YR5/6) mottles, lean CLAY 1B 2.7 2.33 620 19"/24 Yellowish brown (10YR5/8) with 40% grayish brown 5-7 (10YR5/2) mottles, lean CLAY N=92A 24 2.91 В 618 2-2 20"/24 3-4 Gray (10YR5/1), lean CLAY, trace sand and gravel N=53A 21 2.13 B616 1 4-5 24/24 Gray (10YR5/1) with 10% yellowish brown (10YR5/6) 5-6 100% mottles, lean CLAY, trace sand N=1021 1.36 BSh1-2 2-5 24"/24 Yellowish brown (10YR5/8) with 20% gray (10YR6/1) mottles, lean CLAY, trace sand and gravel 20 5A 1.47 BSh612 0 - 120"/24 Yellowish brown (10YR 5/8) with 25% gray (10YR6/1) 3-3 mottles, clayey SAND, trace gravel N=421 0.62 6A B**▼** 12 610 Gray (10YR6/1), clayey SAND, trace gravel, wet 22 0.19 2-2 3-5 N=5 21"/24 Dark yellowish brown (10YR4/6), clayey SAND, trace gravel, wet 22 7В Light yellowish brown (10YR6/4) with 30% brownish 7C 15 3.71 yellow (10YR6/6) mottles, clayey SILT, trace sand and 608 gravel 4-13 24"/24 18-29 N = 319 8A 5.15 BSh606 26-32 24"/24 Dark greenish gray (N4/1), clayey SILT, trace sand and 46-50 gravel N = 786.59 9 9A 604 21-31 24"/24 63-71 SS N = 946.39 10A 11 ShNOTE(S): MW12D installed in SB-12.

HANSON

622 ft. MSL

50 ft. BGS

2,515,900.6E

875,515.1N

Well ID: MW12D

BOREHOLE ID: SB-12

Surface Elev:

Completion:

Station:

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A **DATES:** Start: 5/10/2006

DATES: Start: 5/10/2006 Finish: 5/10/2006

WEATHER: Foggy to partly sunny, mild (hi-60's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager



BOREHOLE ID: SB-12 Well ID: MW12D

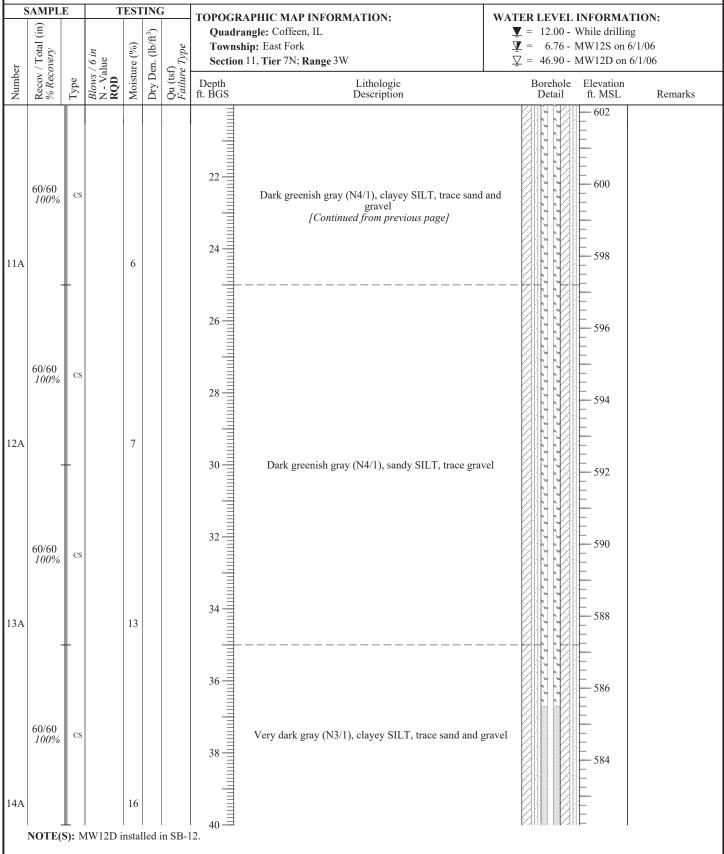
 Surface Elev:
 622 ft. MSL

 Completion:
 50 ft. BGS

 Station:
 875,515.1N

 2,515,900.6E

Page 2 of 3



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A

SAMPLE

DATES: Start: 5/10/2006 Finish: 5/10/2006

WEATHER: Foggy to partly sunny, mild (hi-60's)

TESTING

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

2,515,900.6E

BOREHOLE ID: SB-12

Surface Elev:

Completion:

Station:

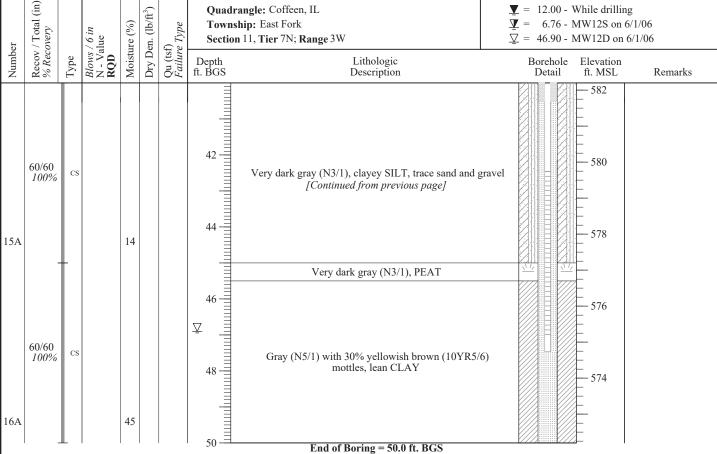
Well ID: MW12D

622 ft. MSL

50 ft. BGS

875,515.1N

TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Quadrangle: Coffeen, IL $\mathbf{\underline{\underline{V}}} = 12.00$ - While drilling Township: East Fork Section 11, Tier 7N; Range 3W



CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A

DATES: Start: 5/10/2006 **Finish:** 5/10/2006

WEATHER: Foggy to partly sunny, mild (hi-60's)

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 31/4" HSA (blind drill)

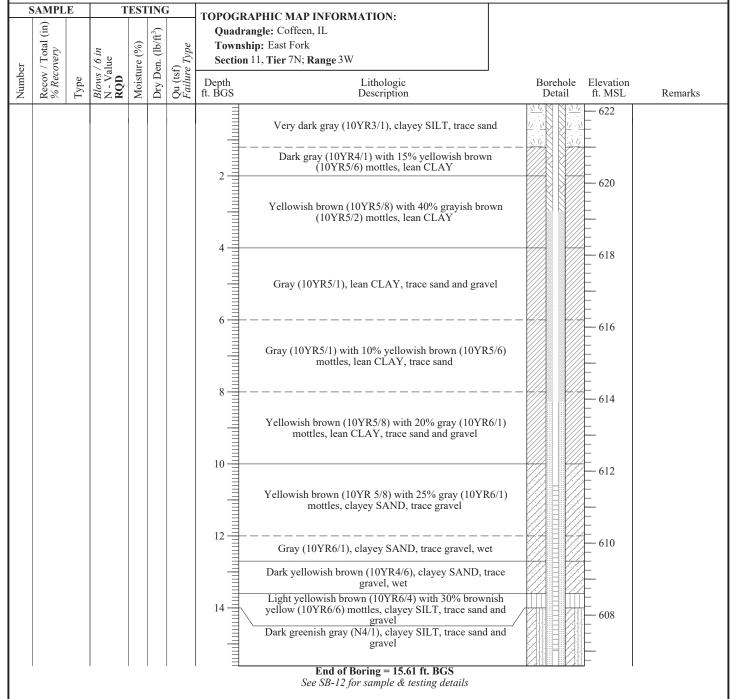
Drilling Method: 3¼" HSA (blind drill)

FIELD STAFF: Driller: B. Williamson

Helper: R. Keedy Eng/Geo: R. Hasenyager BOREHOLE ID: SB-12a

Well ID: MW12S
Surface Elev: 622 ft. MSL
Completion: 16 ft. BGS
Station: 875,520.1N

2,515,900.5E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

TESTING

Location: Coffeen, Illinois Project: 05S3004A **DATES: Start:** 5/10/2006

Finish: 5/10/2006

SAMPLE

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

WEATHER: Foggy to partly sunny, mild (hi-60's) Eng/Geo: R. Hasenyager

> Quadrangle: Coffeen, IL Township: East Fork

TOPOGRAPHIC MAP INFORMATION:

WATER LEVEL INFORMATION: $\mathbf{V} = 12.00$ - While drilling

 Ξ Dry Den. (lb/ft3 Recov / Total (% Recovery $\nabla = 6.76 - MW12S \text{ on } 6/1/06$ Qu (tsf) Failure Type Moisture (%) Blows / 6 in N - Value RQD Section 11, Tier 7N; Range 3W ∇ = 46.90 - MW12D on 6/1/06 Number Depth ft. BGS Lithologic Borehole Elevation Description Detail ft. MSL Remarks Very dark gray (10YR3/1), clayey SILT, trace sand 2-3 24"/24 22 1.27 1A SS 4-5 Sh Dark gray (10YR4/1) with 15% yellowish brown (10YR5/6) mottles, lean CLAY 1B 2.7 2.33 620 19"/24 Yellowish brown (10YR5/8) with 40% grayish brown 5-7 (10YR5/2) mottles, lean CLAY N=92A 24 2.91 В 618 2-2 20"/24 3-4 Gray (10YR5/1), lean CLAY, trace sand and gravel N=53A 21 2.13 B616 1 4-5 24/24 Gray (10YR5/1) with 10% yellowish brown (10YR5/6) 5-6 100% mottles, lean CLAY, trace sand N=1021 1.36 BSh1-2 2-5 24"/24 Yellowish brown (10YR5/8) with 20% gray (10YR6/1) mottles, lean CLAY, trace sand and gravel 20 5A 1.47 BSh612 0 - 120"/24 Yellowish brown (10YR 5/8) with 25% gray (10YR6/1) 3-3 mottles, clayey SAND, trace gravel N=421 0.62 6A B**▼** 12 610 Gray (10YR6/1), clayey SAND, trace gravel, wet 22 0.19 2-2 3-5 N=5 21"/24 Dark yellowish brown (10YR4/6), clayey SAND, trace gravel, wet 22 7В Light yellowish brown (10YR6/4) with 30% brownish 7C 15 3.71 yellow (10YR6/6) mottles, clayey SILT, trace sand and 608 gravel 4-13 24"/24 18-29 N = 319 8A 5.15 BSh606 26-32 24"/24 Dark greenish gray (N4/1), clayey SILT, trace sand and 46-50 gravel N = 786.59 9 9A 604 21-31 24"/24 63-71 SS N = 946.39 10A 11 ShNOTE(S): MW12D installed in SB-12.

HANSON

622 ft. MSL

50 ft. BGS

2,515,900.6E

875,515.1N

Well ID: MW12D

BOREHOLE ID: SB-12

Surface Elev:

Completion:

Station:

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A **DATES:** Start: 5/10/2006

DATES: Start: 5/10/2006 Finish: 5/10/2006

WEATHER: Foggy to partly sunny, mild (hi-60's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager



BOREHOLE ID: SB-12 Well ID: MW12D

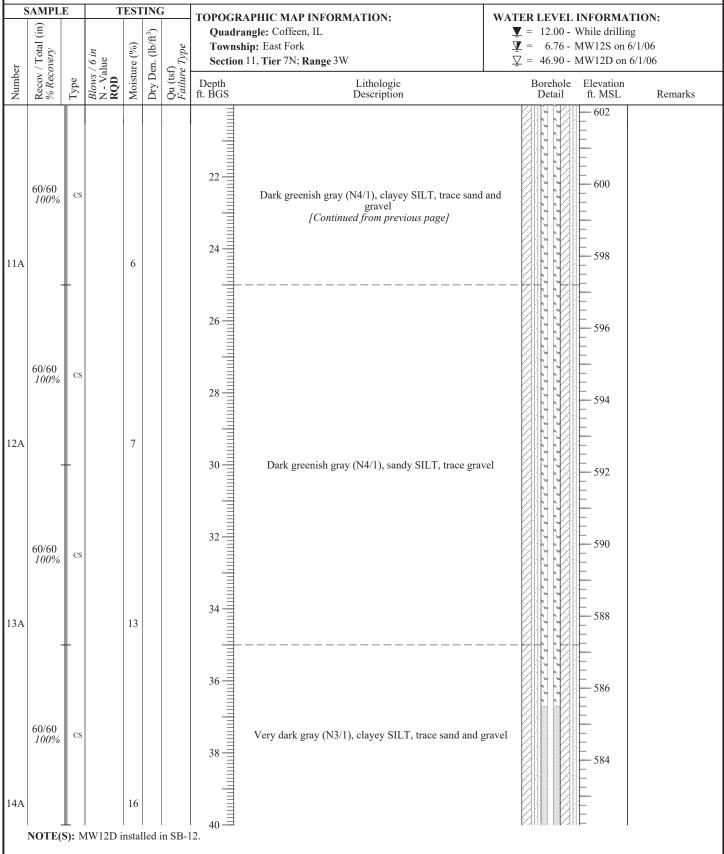
 Surface Elev:
 622 ft. MSL

 Completion:
 50 ft. BGS

 Station:
 875,515.1N

 2,515,900.6E

Page 2 of 3



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A

SAMPLE

DATES: Start: 5/10/2006 Finish: 5/10/2006

WEATHER: Foggy to partly sunny, mild (hi-60's)

TESTING

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

2,515,900.6E

BOREHOLE ID: SB-12

Surface Elev:

Completion:

Station:

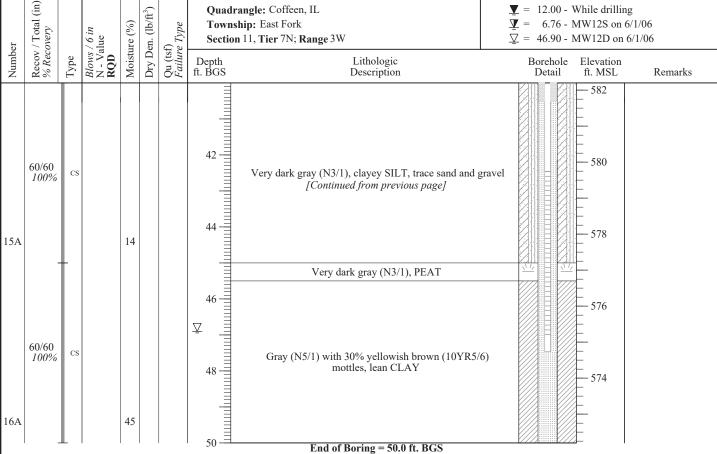
Well ID: MW12D

622 ft. MSL

50 ft. BGS

875,515.1N

TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Quadrangle: Coffeen, IL $\mathbf{\underline{\underline{V}}} = 12.00$ - While drilling Township: East Fork Section 11, Tier 7N; Range 3W



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois

Project: 05S3004A DATES: Start: 5/9/2006 Finish: 5/9/2006

WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS sampler & 41/4" HSA overdrill

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

HANSON

BOREHOLE ID: SB-13 Well ID: MW13D

Surface Elev: 623 ft. MSL Completion: 55 ft. BGS Station: 874,694.3N

2,513,929.9E

	SAMPLE TESTING			APHIC MAP INFORMATION:	WATER LEVEL INFORMATION:			
Recov / Total (in)		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) Failure Type	Townsl	angle: Coffeen, IL nip: East Fork 10, Tier 7N; Range 3W	Ψ = 12.40 - While drilling Ψ = 8.24 - MW12S on 6/1/06 Ψ = 56.03 - MW13D on 6/1/06
Recov // % Recov	Type	Blows N - V RQD	Moist	Dry D	Qu (ts Failun	Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL Reman
A 22/24 B 92%	ss	4-5 4-4 N=9	21 15 28		2.13	2	Grayish brown (10YR5/2), clayey SILT, trace sar Light gray (10YR7/2), clayey SILT, trace sand Light brownish gray (10YR6/2) with 15%yellowish b (10YR5/6) mottles, lean CLAY, trace sand	622
A 21/24 88%	ss	3-4 5-8 N=9	25		2.13 BSh	4-	Gray (10YR6/1) with 40% yellowish brown (10YR mottles, lean CLAY, trace sand	5/6)
3A 24/24 100%	SS	4-5 6-8 N=11	22		2.84 Sh 2.91		Dark gray (10YR4/1), lean CLAY, trace sand and gr	ravel 618
24/24 100%	ss	9-12 10-10 N=22	23		2.33 B			616
24/24 100%	ss	3-4 6-7 N=10	21		2.72 Sh	▼ 8 = 10 = 10 = 10 = 10 = 10 = 10 = 10 =	Gray (10YR5/1) with 25% yellowish brown (10YR mottles, lean CLAY, trace sand and gravel	5/6)
5A 19/24 79%	ss	1-3 6-8 N=9	23		1.94 Sh	12		612
ZA 21/24 88%	SS	6-8 10-12 N=18	18		1.94 Sh	¥ =	Yellowish brown (10YR5/6), silty SAND, trace gra	vel.
A 22/24 92%	SS	7-21 29-30	13 11		1.55 BSh	14	wet wet	
3B 3270		N=50	9			16	Yellowish brown (10YR5/8) with 30% light brown gray (10YR6/2) mottles, sandy SILT, trace grave	
23/24 96%	ss	25-28 28-45 N=56	9		9.16 Sh	18	Dark gray (10YR4/1), sandy SILT, trace gravel	606
0A 24/24 100%	SS	18-27 31-36 N=58	8		12.00 Sh			604

CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 5/9/2006 Finish: 5/9/2006 WEATHER: Overcast, mild (mid-60's) **CONTRACTOR:** Testing Service Corporation **Rig mfg/model:** CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS sampler & 41/4" HSA overdrill

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-13

Well ID: MW13D
Surface Elev: 623 ft. MSL
Completion: 55 ft. BGS

Station: 874,694.3N 2,513,929.9E

Page 2 of 3

HANSON

SAMPLE TESTING			j	TOPOGRA	PHIC MAP INFORMATION:	WATER LEVEL INFORMATION:		
Number Recov / Total (in) % Recovery Type	/ 6 in ılue	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) Failure Type	Quadrar	gle: Coffeen, IL p: East Fork 0, Tier 7N; Range 3W	Ψ = 12.40 - While drilling Ψ = 8.24 - MW12S on 6/1/06 Ψ = 56.03 - MW13D on 6/1/06	
Recov / 1 % Recov Type	Blows / 6 in N - Value RQD	Moist	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL Remarks	
60/60 100% cs		13			22		— 602 — 600 — 598	
60/60 100% cs					26 —	Dark gray (10YR4/1), sandy SILT, trace gravel [Continued from previous page]	596	
60/60 100% cs					30 - 32 - 32 - 32 - 33		592	
A		15			34		588	
60/60 100% cs					38-	Dark gray (10YR4/1), lean CLAY, trace sand and gr	ravel 586	
A		15					584	

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 5/9/2006 Finish: 5/9/2006 WEATHER: Overcast, mild (mid-60's) **CONTRACTOR:** Testing Service Corporation **Rig mfg/model:** CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS sampler & 41/4" HSA overdrill

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

HANSO

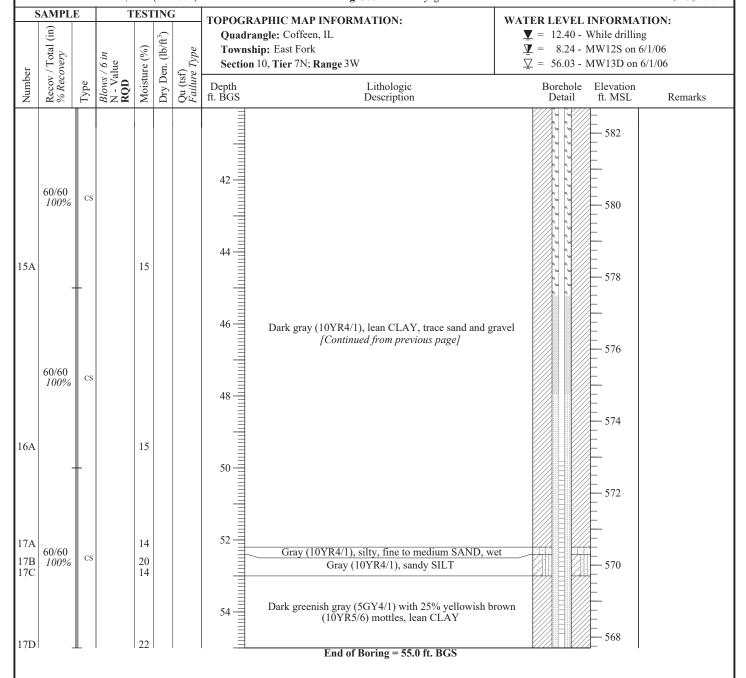
BOREHOLE ID: SB-13 Well ID: MW13D

 Surface Elev:
 623 ft. MSL

 Completion:
 55 ft. BGS

 Station:
 874,694.3N

 2,513,929.9E



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A **DATES: Start:** 5/9/2006

Finish: 5/9/2006 WEATHER: Overcast, mild (mid-60's) CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 31/4" HSA (blind drill)

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

HANSON

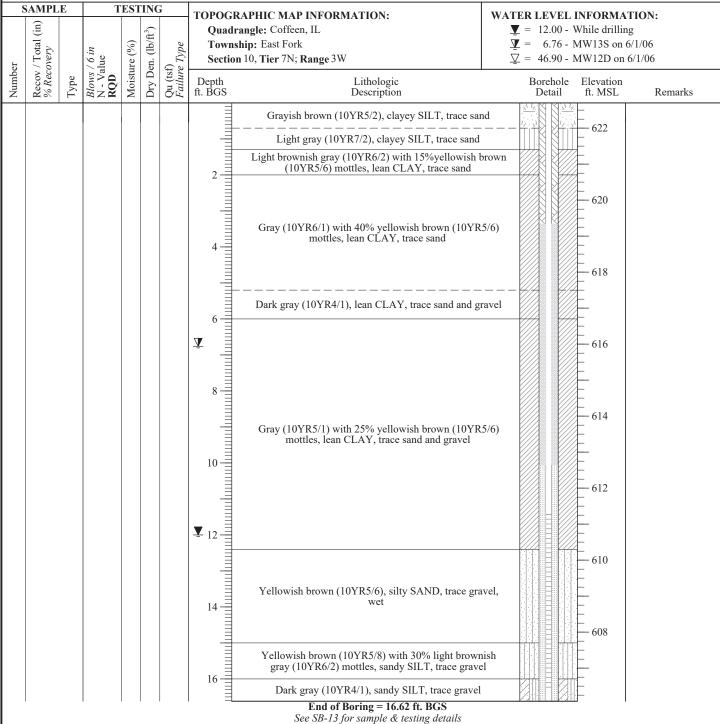
BOREHOLE ID: SB-13a Well ID: MW13S

 Surface Elev:
 623 ft. MSL

 Completion:
 17 ft. BGS

 Station:
 874,695.7N

 2,513,925.3E



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois

Project: 05S3004A DATES: Start: 5/9/2006 Finish: 5/9/2006

WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS sampler & 41/4" HSA overdrill

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

HANSON

BOREHOLE ID: SB-13 Well ID: MW13D

Surface Elev: 623 ft. MSL Completion: 55 ft. BGS Station: 874,694.3N

2,513,929.9E

	SAMPLE TESTING			APHIC MAP INFORMATION:	WATER LEVEL INFORMATION:			
Recov / Total (in)		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) Failure Type	Townsl	angle: Coffeen, IL nip: East Fork 10, Tier 7N; Range 3W	Ψ = 12.40 - While drilling Ψ = 8.24 - MW12S on 6/1/06 Ψ = 56.03 - MW13D on 6/1/06
Recov // % Recov	Type	Blows N - V RQD	Moist	Dry D	Qu (ts Failun	Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL Reman
A 22/24 B 92%	ss	4-5 4-4 N=9	21 15 28		2.13	2	Grayish brown (10YR5/2), clayey SILT, trace sar Light gray (10YR7/2), clayey SILT, trace sand Light brownish gray (10YR6/2) with 15%yellowish b (10YR5/6) mottles, lean CLAY, trace sand	622
A 21/24 88%	ss	3-4 5-8 N=9	25		2.13 BSh	4-	Gray (10YR6/1) with 40% yellowish brown (10YR mottles, lean CLAY, trace sand	5/6)
3A 24/24 100%	SS	4-5 6-8 N=11	22		2.84 Sh 2.91		Dark gray (10YR4/1), lean CLAY, trace sand and gr	ravel 618
24/24 100%	ss	9-12 10-10 N=22	23		2.33 B			616
24/24 100%	ss	3-4 6-7 N=10	21		2.72 Sh	▼ 8 = 10 = 10 = 10 = 10 = 10 = 10 = 10 =	Gray (10YR5/1) with 25% yellowish brown (10YR mottles, lean CLAY, trace sand and gravel	5/6)
5A 19/24 79%	ss	1-3 6-8 N=9	23		1.94 Sh	12		612
ZA 21/24 88%	SS	6-8 10-12 N=18	18		1.94 Sh	¥ =	Yellowish brown (10YR5/6), silty SAND, trace gra	vel.
A 22/24 92%	SS	7-21 29-30	13 11		1.55 BSh	14	wet wet	
3B 3270		N=50	9			16	Yellowish brown (10YR5/8) with 30% light brown gray (10YR6/2) mottles, sandy SILT, trace grave	
23/24 96%	ss	25-28 28-45 N=56	9		9.16 Sh	18	Dark gray (10YR4/1), sandy SILT, trace gravel	606
0A 24/24 100%	SS	18-27 31-36 N=58	8		12.00 Sh			604

CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 5/9/2006 Finish: 5/9/2006 WEATHER: Overcast, mild (mid-60's) **CONTRACTOR:** Testing Service Corporation **Rig mfg/model:** CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS sampler & 41/4" HSA overdrill

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-13

Well ID: MW13D
Surface Elev: 623 ft. MSL
Completion: 55 ft. BGS

Station: 874,694.3N 2,513,929.9E

Page 2 of 3

HANSON

SAMPLE TESTING			j	TOPOGRA	PHIC MAP INFORMATION:	WATER LEVEL INFORMATION:		
Number Recov / Total (in) % Recovery Type	/ 6 in ılue	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) Failure Type	Quadrar	gle: Coffeen, IL p: East Fork 0, Tier 7N; Range 3W	Ψ = 12.40 - While drilling Ψ = 8.24 - MW12S on 6/1/06 Ψ = 56.03 - MW13D on 6/1/06	
Recov / 1 % Recov Type	Blows / 6 in N - Value RQD	Moist	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL Remarks	
60/60 100% cs		13			22		— 602 — 600 — 598	
60/60 100% cs					26 —	Dark gray (10YR4/1), sandy SILT, trace gravel [Continued from previous page]	596	
60/60 100% cs					30 - 32 - 32 - 32 - 33		592	
A		15			34		588	
60/60 100% cs					38-	Dark gray (10YR4/1), lean CLAY, trace sand and gr	ravel 586	
A		15					584	

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 5/9/2006 Finish: 5/9/2006 WEATHER: Overcast, mild (mid-60's) **CONTRACTOR:** Testing Service Corporation **Rig mfg/model:** CME-650 Track Rig

Drilling Method: 31/4" HSA w/SS sampler & 41/4" HSA overdrill

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

HANSO

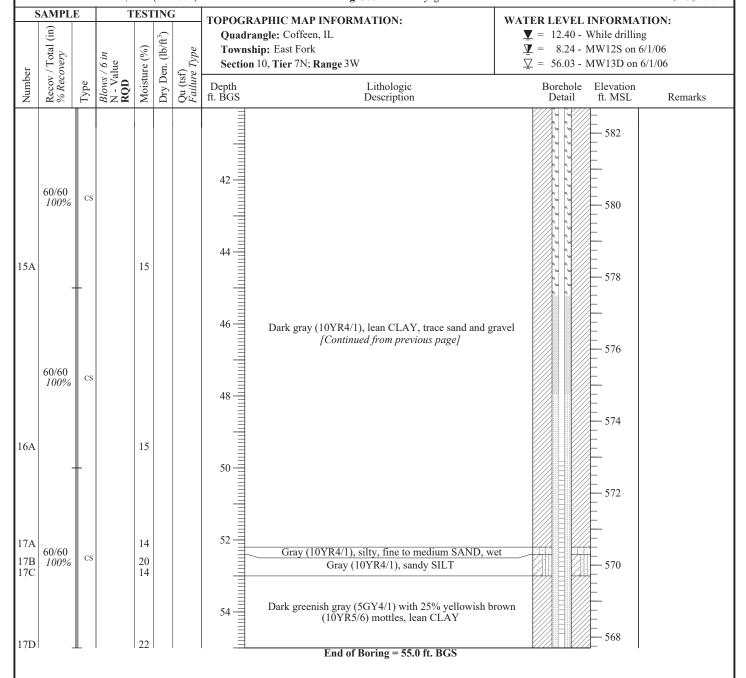
BOREHOLE ID: SB-13 Well ID: MW13D

 Surface Elev:
 623 ft. MSL

 Completion:
 55 ft. BGS

 Station:
 874,694.3N

 2,513,929.9E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A **DATES: Start:** 5/2/2006

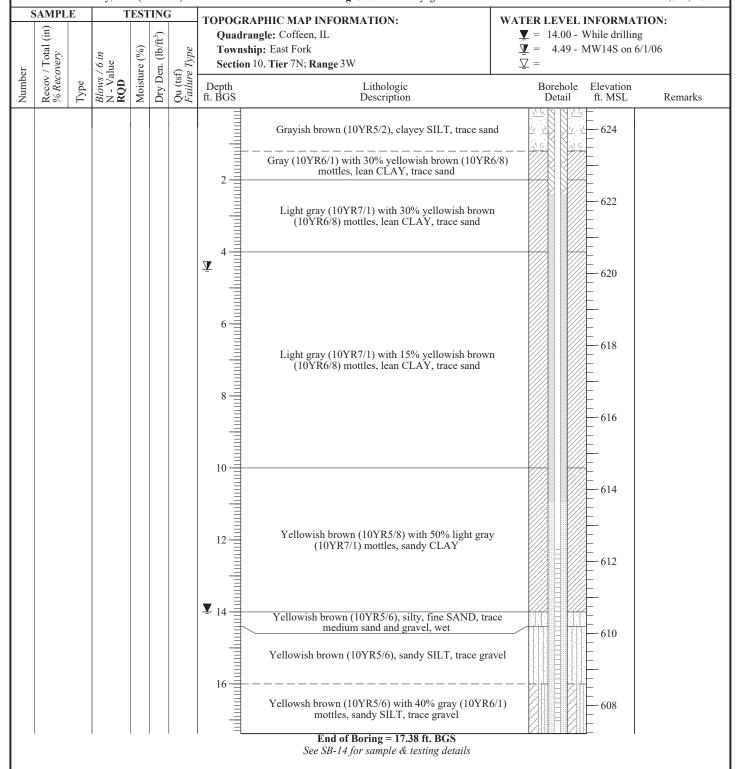
Finish: 5/2/2006 WEATHER: Sunny, mild (mid-60's) **CONTRACTOR:** Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 41/4" HSA (blind drill)

FIELD STAFF: Driller: B. Williamson

Helper: R. Keedy Eng/Geo: R. Hasenyager HANSON

BOREHOLE ID: Sb-14a Well ID: MW14S

Surface Elev: 625 ft. MSL 17 ft. BGS **Completion: Station:** 875,737.8N 2,514,125.9E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility Location: Coffeen, Illinois

Project: 05S3004A **DATES: Start:** 5/1/2006 Finish: 5/2/2006

WEATHER: Sunny, mild (mid-60's)

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig **Drilling Method:** 31/4" HSA w/SS sampler

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager



BOREHOLE ID: SB-14 Well ID: n/a

> 625 ft. MSL **Surface Elev: Completion:** 60 ft. BGS **Station:** 875,740.0N

2,514,130.0E

5	Recov / Total (in) % Recovery Type Blows / 6 in N - Value RQD Moisture (%) Dry Den. (lb/ft³) Qu (tsf) Failure Type		TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:						
er			/ 6 in alue	ıre (%)	en. (lb/ft³)	e Type	Quadra Townsh	ningle: Coffeen, IL nip: East Fork 10, Tier 7N; Range 3W	<u>_</u> = 14.00 -	$\underline{\Psi}$ = 14.00 - While drilling $\underline{\Psi}$ = 4.49 - MW14S on 6/1/06	
Number	Recov % Rec	Type	Blows N - V	Moist	Dry D	Qu (tsf) Failure I	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	23/24 96%	SS	2-3 2-3	16				Grayish brown (10YR5/2), clayey SILT, trace sar	nd (2.34 (2.45)	624	
1B	7070	$\stackrel{\wedge}{\rightarrow}$	N=5	26		2.33 B	2	Gray (10YR6/1) with 30% yellowish brown (10YR mottles, lean CLAY, trace sand	6/8)		
2A	24/24 100%	ss	3-4 5-7 N=9	23		3.10 B	4	Light gray (10YR7/1) with 30% yellowish brown (10YR6/8) mottles, lean CLAY, trace sand	n	622	
ВА	23/24 96%	ss	3-3 5-5 N=8	19		2.33 B	6 — Ā			620	
4A	24/24 100%	ss	5-6 5-7 N=11	23		2.68 BSh	2	Light gray (10YR7/1) with 15% yellowish brown (10YR6/8) mottles, lean CLAY, trace sand	n	618	
5A	24/24 100%	ss	2-2 3-4 N=5	26		1.83 B	10			616	
5A	19/24 79%	ss	2-2 3-5 N=5	17		2.18 B	12 —	Yellowish brown (10YR5/8) with 50% light gray (10YR7/1) mottles, sandy CLAY	·	614	
7A	20/24 83%	ss	2-3 3-3 N=6	22		1.16 B	■	(**************************************		612	
8A	24/24 100%	ss	5-14 14-20 N=28	16		1.36 B	14	Yellowish brown (10YR5/6), silty, fine SAND, tra medium sand and gravel, wet Yellowish brown (10YR5/6), sandy SILT, trace gra		610	
8B	+			11		5.77 BSh	16				
9A	12/24 50%	ss	57-65	10			18	Yellowsh brown (10YR5/6) with 40% gray (10YR6 mottles, sandy SILT, trace gravel	5/1)	608	
0A	24/24 100%	ss	6-8 16-18 N=24	12		5.04 BSh	16	Dark gray (10YR4/1), clayey SILT, trace sand and g	ravel	606	

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Project: 05S3004A **DATES: Start:** 5/1/2006 Finish: 5/2/2006

Rig mfg/model: CME-650 Track Rig Location: Coffeen, Illinois Drilling Method: 31/4" HSA w/SS sampler FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

HANSON BOREHOLE ID: SB-14

Well ID: n/a

Surface Elev: 625 ft. MSL 60 ft. BGS **Completion: Station:** 875,740.0N 2,514,130.0E

WEATHER: Sunny, mild (mid-60's) Eng/Geo: R. Hasenyager SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Œ, Quadrangle: Coffeen, IL $\mathbf{Y} = 14.00$ - While drilling Township: East Fork Ψ = 4.49 - MW14S on 6/1/06

CONTRACTOR: Testing Service Corporation

Dry Den. (lb/ft³) Recov / Total (% Recovery Qu (tsf) Failure Type Moisture (%) Section 10, Tier 7N; Range 3W Number Elevation Lithologic Borehole Description Remarks 604 24/24 13-30 100% N = 2011A 13 9.70 602 26-40 24/24 36-40 100% N = 769 12A 13.09 600 8-18 24/24 28-34 100% N=46 13A 9 8.73 BSP 598 20-18 24-30 Dark gray (10YR4/1), clayey SILT, trace sand and gravel 92% N = 42[Continued from previous page] 9 7.42 *BSP* 596 19/24 33-67 79% N = 6015A 9 594 8-25 27-33 24/24 100% N=52 10 16A 9.60 BSh 592 20/24 20-24 83% 17A 14 6.80 590 24/24 7-9 100% N=11 18A 16 3.88 588 8-12 24/24 13-15 Dark gray (N4/1), lean CLAY, trace sand and gravel 100% 19A 16 6.18 586 3-7 24/24 10-13 SS 100% N=1720A 14 3.10 В

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

DATES: Start: 5/1/2006 Finish: 5/2/2006

Location: Coffeen, Illinois **Drilling Method:** 31/4" HSA w/SS sampler Project: 05S3004A FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

HANSON

BOREHOLE ID: SB-14 Well ID: n/a

> **Surface Elev:** 625 ft. MSL **Completion:** 60 ft. BGS **Station:** 875,740.0N 2,514,130.0E

WEATHER: Sunny, mild (mid-60's) Eng/Geo: R. Hasenyager SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Ψ = 14.00 - While drilling Ψ = 4.49 - MW14S on 6/1/06 Quadrangle: Coffeen, IL

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig

<u>.</u>	Total very		6 in	re (%)	n. (lb/f	Туре	Township: East Fork Section 10, Tier 7N; Range 3W	$\underline{\underline{\mathbf{Y}}} = 4.49 - \text{MW} 14 \text{S on } 6/1/06$ $\underline{\underline{\nabla}} =$
Number	Recov / Total % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/f	Qu (tsf) Failure Type	Depth Lithologic ft. BGS Description	Borehole Elevation Detail ft. MSL Remarks
21A	23/24 96%	SS	3-6 8-13 N=14	15		4.80 B	42 — Dark gray (N4/1), lean CLAY, trace sand and grav [Continued from previous page]	584
2A	24/24 100%	ss	13-15 16-18 N=31	14		5.62 B	Dark gray (N4/1), lean CLAY, trace sand and grav	7582 Vel
3A	24/24 100%	ss	4-8 11-13 N=19	15		4.65 B	[Continued from previous page]	580
4A	24/24 100%	ss	18-18 20-20 N=38	15		4.65 B	48	578
5A	24/24 100%	SS	4-7 9-11 N=16	19		2.13 BSh	Dark gray (N4/1), clayey SILT, trace sand and grade Gray (N4/1), wet, loose, fine- to medium-grained SAD Dark gray (N4/1), clayey SILT, trace sand and grade	AND
6A	22/24 92%	SS	3-5 6-8 N=11	22		3.30 BSh	Greenish gray (5BG5/1), lean CLAY	574
7A	24/24 100%	SS	3-5 5-7 N=10	25		2.89 BSh	Greenish gray (5BG5/1) with 15% yellowish brov (10YR5/6) mottles, lean CLAY	vn 572
8A	21/24 88% 0/24 0%	SS	4-6 7-8 N=13	22		3.71 BSh		vn 570
9A	14/24 58%	ss	0-0 0-0 N=0	22		3.09 BSh	Greenish gray (5BG5/1) with 25% yellowish brow (10YR5/6) mottles, lean CLAY Greenish gray (5BG5/1) with 50% yellowish brow (10YR5/6) mottles, lean CLAY Yellowish brown (10YR4/6) with 10% greenish gray (5BG5/1) mottles, lean CLAY End of Boring = 60.0 ft. BGS	vn 568
60A	22/24 92%	ss	5-6 8-12 N=14	19		4.46 BSh	Yellowish brown (10YR4/6) with 10% greenish gr (5BG5/1) mottles, lean CLAY End of Boring = 60.0 ft. BGS	ray 566

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A **DATES:** Start: 4/24/2006

Finish: 4/25/2006 WEATHER: Overcast, cool (lo-50's) CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 4¹/₄" HSA (blind drill)

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

HANSON

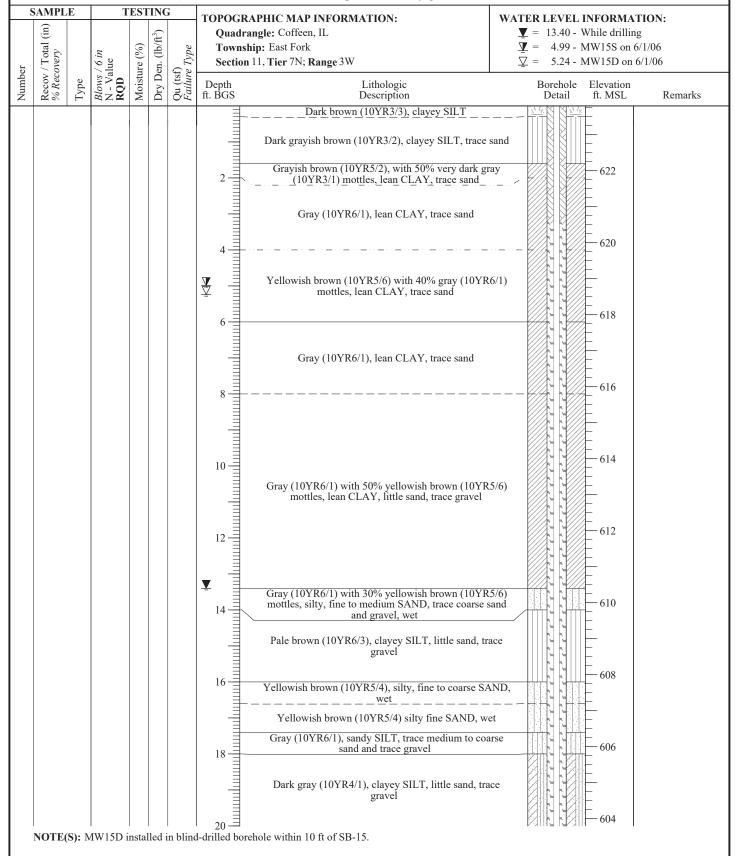
BOREHOLE ID: SB-15b Well ID: MW15D

 Surface Elev:
 624 ft. MSL

 Completion:
 39 ft. BGS

 Station:
 875,970.5N

 2,515,080.7E



CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A **DATES:** Start: 4/24/2006

DATES: Start: 4/24/2006 Finish: 4/25/2006 CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 41/4" HSA (blind drill)

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Helper: R. Keedy Eng/Geo: R. Hasenyager **HANSON**

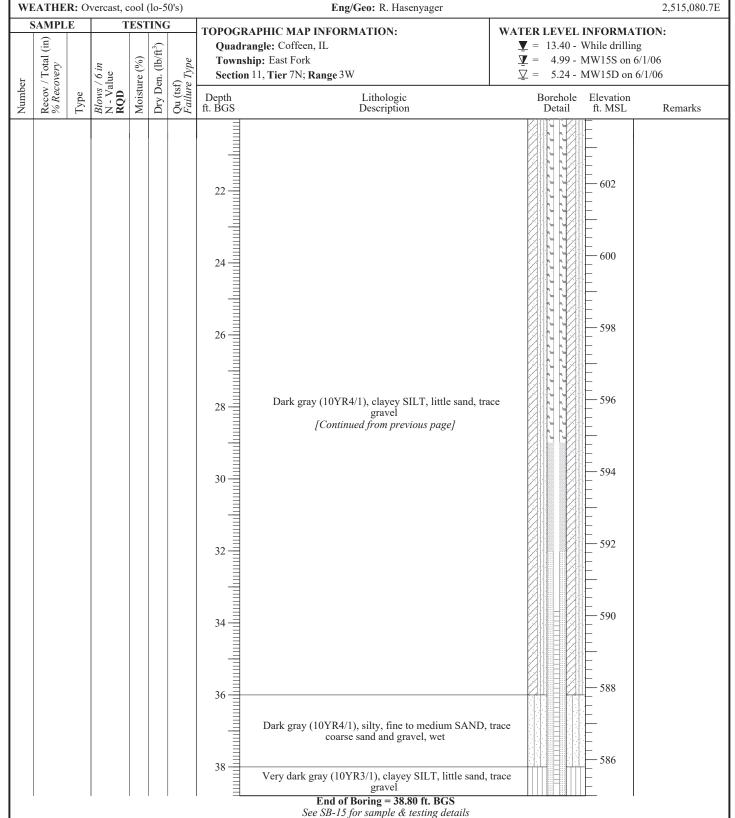
BOREHOLE ID: SB-15b
Well ID: MW15D

 Surface Elev:
 624 ft. MSL

 Completion:
 39 ft. BGS

 Station:
 875,970.5N

 2,515,080.7E



CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 4/24/2006

Finish: 4/25/2006 WEATHER: Overcast, cool (lo-50's) CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 3¹/₄" HSA w/SS sampler

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager



BOREHOLE ID: SB-15 **Well ID:** n/a

 Surface Elev:
 624 ft. MSL

 Completion:
 84 ft. BGS

 Station:
 875,970.0N

 2,515,080.0E

Page 1 of 5

SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Ξ $\mathbf{Y} = 13.40$ - While drilling Quadrangle: Coffeen, IL Dry Den. (lb/ft3 Recov / Total % Recovery Township: East Fork $\nabla = 4.99 - MW15S \text{ on } 6/1/06$ Qu (tsf) Failure Type Moisture (%) Blows / 6 in N - Value RQD Section 11, Tier 7N; Range 3W ∇ = 5.24 - MW15D on 6/1/06 Number Depth Lithologic Borehole Elevation ft. BGS Description Detail ft. MSL Remarks Dark brown (10YR3/3), clayey SILT 24/24 19 1A SS 3-4 Dark grayish brown (10YR3/2), clayey SILT, trace sand 100% N=5Grayish brown (10YR5/2), with 50% very dark gray 622 1B 2.7 1.94 (10YR3/1) mottles, lean CLAY, trace sand B 2-2 24/24 Gray (10YR6/1), lean CLAY, trace sand 4-6 100% N=62A 25 3.10 B620 2-3 20/24 $\bar{\Lambda}$ Yellowish brown (10YR5/6) with 40% gray (10YR6/1) 3-5 83% mottles, lean CLAY, trace sand N=629 3A 2.10 B618 4-6 24/24 5-5 Gray (10YR6/1), lean CLAY, trace sand 100% N=1124 4A 1.75 В 616 22/24 3-4 92% 5A 26 1.55 В 614 Gray (10YR6/1) with 50% yellowish brown (10YR5/6) 2-3 22/24 mottles, lean CLAY, little sand, trace gravel 3-4 92% N=622 6A 1.85 B612 19/24 Shelby tube taken 79% from shallow well borehole at 24/24 indicated depth. 5-5 100% Y 7A 23 1.22 Gray (10YR6/1) with 30% yellowish brown (10YR5/6) 610 mottles, silty, fine to medium SAND, trace coarse sand 7В 17 and gravel, wet 2-6 21/24 Pale brown (10YR6/3), clayey SILT, little sand, trace 15-19 88% N=218A 11 BSP 608 16 Yellowish brown (10YR5/4), silty, fine to coarse SAND, 20 9A _ _ _ _ <u>wet</u> _ _ _ _ 18-29 24/24 40-50 Yellowish brown (10YR5/4) silty fine SAND, wet 100% N=69 9B 21 Gray (10YR6/1), sandy SILT, trace medium to coarse 606 9C 9 sand and trace gravel 17/24 11-43 Dark gray (10YR4/1), clayey SILT, little sand, trace SS 71% 59/5" gravel 7 10A 7.42 В 20 NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois

Project: 05S3004A DATES: Start: 4/24/2006

Finish: 4/25/2006 WEATHER: Overcast, cool (lo-50's) CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 31/4" HSA w/SS sampler

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager



BOREHOLE ID: SB-15 Well ID: n/a

 Surface Elev:
 624 ft. MSL

 Completion:
 84 ft. BGS

 Station:
 875,970.0N

 2,515,080.0E

S	SAMPL	E	Т	EST	INC	j	TOPOGE	APHIC MAP INFORMATION:	WATER	I EVEI	INFORMAT	ION:
ber	Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) Failure Type	Quadi Towns Section	angle: Coffeen, IL hip: East Fork 11, Tier 7N; Range 3W	$\bar{\Lambda} = \bar{\Lambda} = $	13.40 - 4.99 - 5.24 -	While drilling MW15S on 6/ MW15D on 6/	1/06
Number	Reco % Re	Type	Blow. N - V RQD	Mois	Dry I	Qu (t Failu	Depth ft. BGS	Lithologic Description	I	Borehole Detail	Elevation ft. MSL	Remarks
11A	14/24 58%	ss	14-55 45/2"	8			22				602	
12A	8/24 33%	ss	100/8"	8		6.76 SP	24				600	
13A	23/24 96%	ss	12-28 43-57/5" N=71	5			26				598	
14A	8/24 33%	ss	59-41/2"	6		7.95 <i>BSh</i>	28	Dark gray (10YR4/1), clayey SILT, little sand, tra	ace			
15A	16/24 67%	ss	11-26 74/4"	12		4.74 <i>BSh</i>	30	[Continued from previous page]			594	
16A	12/24 50%	ss	39-61	7			30				592	
17A	10/24 42%	ss	49-51/4"	9		5.43 B					590	
18A	11/24 46%	ss	100-95	11			34 = 36				588	
19A	8/24 33%	ss	61-39/2"	10			38	Dark gray (10YR4/1), silty, fine to medium SAND, coarse sand and gravel, wet	trace		586	
20A	24/24 100%	ss	21-41 21-24 N=62	12		16.00 None	40	Very dark gray (10YR3/1), clayey SILT, little sand, gravel				
20B	NOTE	S): B	orehole :	13 aban	done	None 9.38	40 =	Very dark gray (10YR3/1) with 20% dark grayish b (10YR4/2) mottles, clayey SILT, trace sand and grayiout pumped from bottom of borehole.	ravel		584	

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois

Project: 05S3004A **DATES: Start:** 4/24/2006

Finish: 4/25/2006 WEATHER: Overcast, cool (lo-50's) CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 3¹/₄" HSA w/SS sampler

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

HANSON

BOREHOLE ID: SB-15 Well ID: n/a

 Surface Elev:
 624 ft. MSL

 Completion:
 84 ft. BGS

 Station:
 875,970.0N

 2,515,080.0E

5	SAMPL	E	T	EST	INC	j	TOPOGR	APHIC MAP INFORMATION:	WATI	RIEVEL	INFORMAT	ION:
er	Recov / Total (in) % Recovery		/ 6 in Ilue	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) Failure Type	Quadr Towns	angle: Coffeen, IL hip: East Fork 111, Tier 7N; Range 3W	Ā Ā	= 13.40 - = 4.99 -	While drilling MW15S on 66 MW15D on 6	5 /1/06
Number	Recov % Rec	Type	Blows / 6 in N - Value RQD	Moist	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description		Borehole Detail	Elevation ft. MSL	Remarks
21A	22/24 92%	ss	3-7 11-18 N=18	19		BSh 6.11 BSh	42	Dark gray (10YR4/1), clayey SILT, trace sand and g	ravel		582	
2A	24/24 100%	ss	4-7 8-10 N=15	23		4.46 B	44	Dark greenish gray (5GY4/1) with 30% dark gray (N mottles, lean CLAY, trace sand	N4/1)		580	
3A	24/24 100%	SS	3-6 6-10 N=12	21								
23B				16		3.69 B	46				578	
24A	24/24 100%	ss	10-12 10-15 N=22	16		4.58 B	48	Dark gray (N4/1), lean CLAY, trace sand and grav	zel .		576	
25A	24/24 100%	ss	2-4 7-9 N=11	16		4.65 B	48	Daik gray (1947), tean CLA1, trace saild and grav	VCI		574	
26A	19/24 79%	ss	3-5 7-13 N=12	21		3.88 B	52				572	
27A	24/24 100%	ss	8-10 8-13 N=18	25		3.49 <i>BSh</i>	54	Dark yellowish brown (10YR3/4) with 50% dark grabrown (10YR4/2) mottles, lean CLAY, trace sand a gravel	ayish and		570	
28A	24/24 100%	ss	4-5 8-12 N=13	22		3.49 <i>BSh</i>	56	Greenish gray (10YR5/1) with 20% dark yellowish b (10YR4/4) mottles, lean CLAY, trace sand and gra	orown wel		568	
29A	24/24 100%	ss	5-9 15-18 N=24	20			58	-,			566	
30A	24/24 100%	SS	8-9 14-18 N=23	18		5.82 BSh	60	Olive (5Y4/3) with 15% greenish gray (10GY5/1) molean CLAY, trace sand and gravel	ottles,		564	

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois

Project: 05S3004A **DATES: Start:** 4/24/2006

Finish: 4/25/2006 WEATHER: Overcast, cool (lo-50's) CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 3¹/₄" HSA w/SS sampler

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager



BOREHOLE ID: SB-15 Well ID: n/a

 Surface Elev:
 624 ft. MSL

 Completion:
 84 ft. BGS

 Station:
 875,970.0N

 2,515,080.0E

S	AMPL	E	Т	EST	INC	j	TOPOGR	APHIC MAP INFORMATION:	WAT	ER LEVEL	INFORMAT	ΓΙΟΝ:
er	Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) Failure Type	Quadra Townsl	nngle: Coffeen, IL nip: East Fork 11, Tier 7N; Range 3W	Ž	Z = 13.40 - 4.99 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.	While drilling MW15S on 6 MW15D on 6	g /1/06
Number	Recov % Rec	Type	Blows N - Vs RQD	Moist	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description		Borehole Detail	Elevation ft. MSL	Remarks
1A	24/24 100%	ss	4-7 13-15 N=20	18		5.42 BSh	62	Olive (5Y4/3) with 15% greenish gray (10GY5/1) mo	ottles,		562	
2A	24/24 100%	ss	10-15 11-16 N=26	20		4.74 BSh	64	lean CLAY, trace sand and gravel [Continued from previous page]			560	
3A	24/24 100%	ss	6-10 11-13 N=21	16		6.98 BSh	66	Greenish gray (10Y5), lean CLAY, trace sand and gr	ravel		558	
64A	24/24 100%	ss	11-14 18-31 N=32	18		6.98 BSh	68-				556	
5A	23/24 96%	ss	9-18 27-40 N=45	15		11.95 BSh	70				554	
66A	24/24 100%	ss	4-12 18-24 N=30	16		7.15 BSh	72				552	
57A	24/24 100%	ss	17-29 36-47 N=65	17		8.24 <i>BSh</i>	74	Dark yellowish brown (10YR4/4), lean CLAY, trace and gravel	sand		550	
38A	20/24 83%	ss	12-18 23-28 N=41	17		6.59 BSh	76				548	
59A	9/24 38%	ss	29-39 48-66 N=87	16			78 —				546	
-0A	24/24 100%	SS	5-9 13-18 N=22	18		6.21 B	80				544	

CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A **DATES:** Start: 4/24/2006

Finish: 4/25/2006
WEATHER: Overcast, cool (lo-50's)

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 3¹/₄" HSA w/SS sampler

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Helper: R. Keedy Eng/Geo: R. Hasenyager **HANSON**

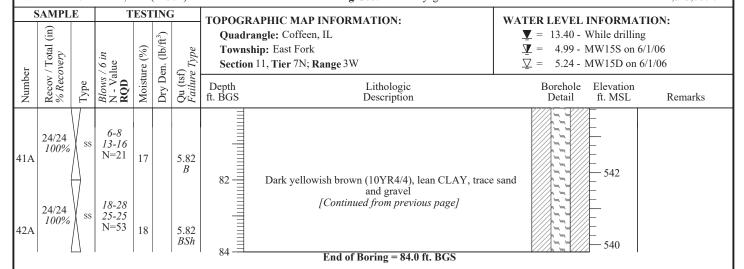
BOREHOLE ID: SB-15 **Well ID:** n/a

 Surface Elev:
 624 ft. MSL

 Completion:
 84 ft. BGS

 Station:
 875,970.0N

 2,515,080.0E



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A **DATES: Start:** 4/25/2006

DATES: Start: 4/25/2006 Finish: 4/25/2006 WEATHER: Overcast, cool (lo-50's) CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 41/4" HSA (blind drill)

FIELD STAFF: Driller: B. Williamson

Helper: R. Keedy **Eng/Geo:** R. Hasenyager



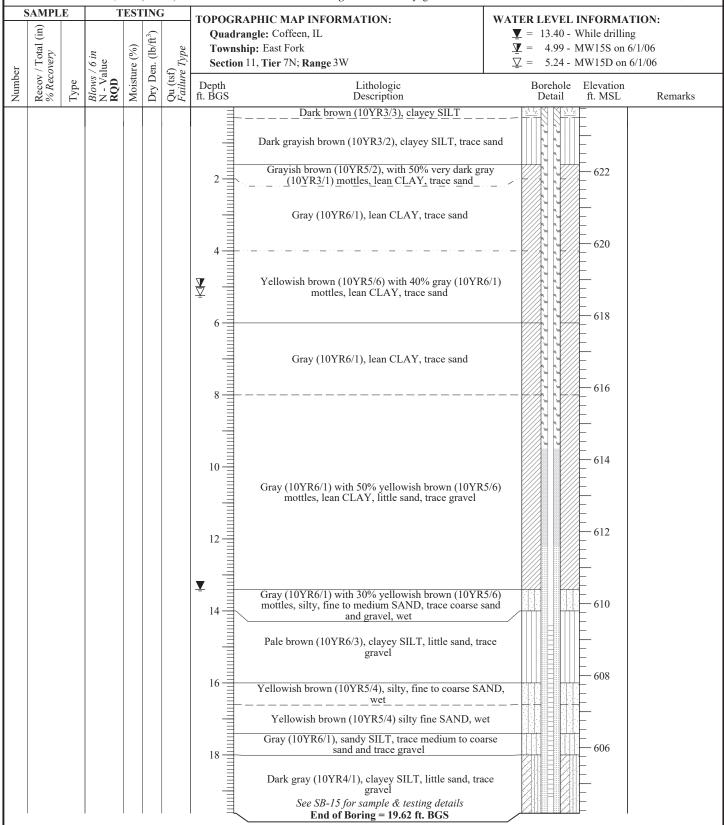
BOREHOLE ID: SB-15a **Well ID:** MW15S

 Surface Elev:
 624 ft. MSL

 Completion:
 20 ft. BGS

 Station:
 875,971.1N

 2,515,076.3E



NOTE(S): MW15S installed in blind-drilled borehole within 10 ft of SB-15.

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 4/24/2006

Finish: 4/25/2006

Finish: 4/25/2006
WEATHER: Overcast, cool (lo-50's)

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 3¹/₄" HSA w/SS sampler

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager



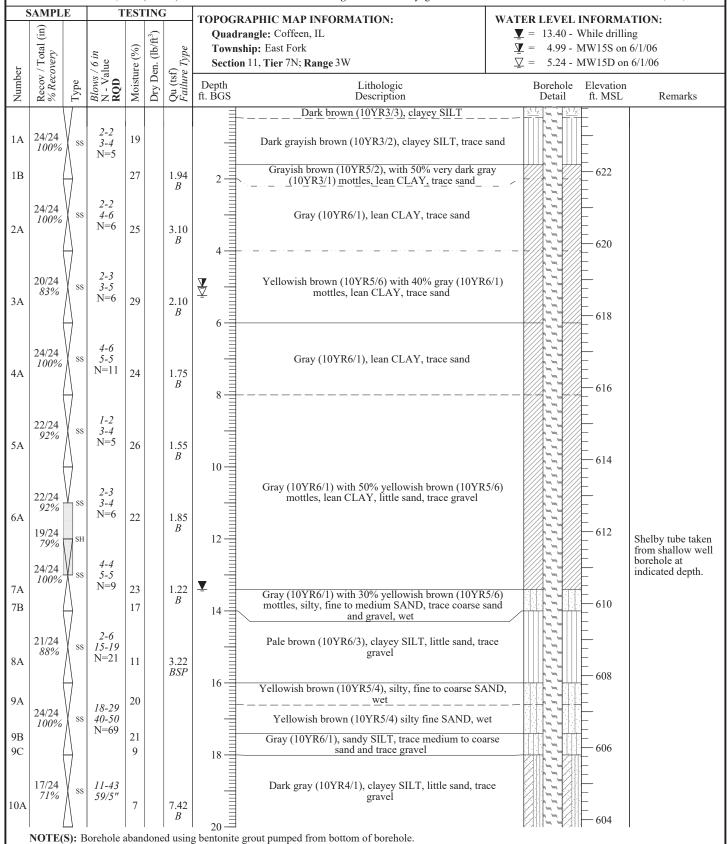
BOREHOLE ID: SB-15 Well **ID:** n/a

 Surface Elev:
 624 ft. MSL

 Completion:
 84 ft. BGS

 Station:
 875,970.0N

 2,515,080.0E



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois

Project: 05S3004A DATES: Start: 4/24/2006

Finish: 4/25/2006 WEATHER: Overcast, cool (lo-50's) CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 31/4" HSA w/SS sampler

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager



BOREHOLE ID: SB-15 Well ID: n/a

 Surface Elev:
 624 ft. MSL

 Completion:
 84 ft. BGS

 Station:
 875,970.0N

 2,515,080.0E

S	SAMPL	E	Т	EST	INC	j	TOPOGE	APHIC MAP INFORMATION:	WATER	I EVEI	INFORMAT	ION:
ber	Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) Failure Type	Quadi Towns Section	angle: Coffeen, IL hip: East Fork 11, Tier 7N; Range 3W	$\bar{\Lambda} = \bar{\Lambda} = $	13.40 - 4.99 - 5.24 -	While drilling MW15S on 6/ MW15D on 6/	1/06
Number	Reco % Re	Type	Blow. N - V RQD	Mois	Dry I	Qu (t Failu	Depth ft. BGS	Lithologic Description	I	Borehole Detail	Elevation ft. MSL	Remarks
11A	14/24 58%	ss	14-55 45/2"	8			22				602	
12A	8/24 33%	ss	100/8"	8		6.76 SP	24				600	
13A	23/24 96%	ss	12-28 43-57/5" N=71	5			26				598	
14A	8/24 33%	ss	59-41/2"	6		7.95 <i>BSh</i>	28	Dark gray (10YR4/1), clayey SILT, little sand, tra	ace			
15A	16/24 67%	ss	11-26 74/4"	12		4.74 <i>BSh</i>	30	[Continued from previous page]			594	
16A	12/24 50%	ss	39-61	7			30				592	
17A	10/24 42%	ss	49-51/4"	9		5.43 B					590	
18A	11/24 46%	ss	100-95	11			34 = 36				588	
19A	8/24 33%	ss	61-39/2"	10			38	Dark gray (10YR4/1), silty, fine to medium SAND, coarse sand and gravel, wet	trace		586	
20A	24/24 100%	ss	21-41 21-24 N=62	12		16.00 None	40	Very dark gray (10YR3/1), clayey SILT, little sand, gravel				
20B	NOTE	S): B	orehole :	13 aban	done	None 9.38	40 =	Very dark gray (10YR3/1) with 20% dark grayish b (10YR4/2) mottles, clayey SILT, trace sand and grayiout pumped from bottom of borehole.	ravel		584	

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois

Project: 05S3004A **DATES: Start:** 4/24/2006

Finish: 4/25/2006 WEATHER: Overcast, cool (lo-50's) CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 3¹/₄" HSA w/SS sampler

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager

HANSON

BOREHOLE ID: SB-15 Well ID: n/a

 Surface Elev:
 624 ft. MSL

 Completion:
 84 ft. BGS

 Station:
 875,970.0N

 2,515,080.0E

5	SAMPL	E	T	EST	INC	j	TOPOGR	APHIC MAP INFORMATION:	WATI	RIEVEL	INFORMAT	ION:
er	Recov / Total (in) % Recovery		/ 6 in Ilue	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) Failure Type	Quadr Towns	angle: Coffeen, IL hip: East Fork 111, Tier 7N; Range 3W	Ā Ā	= 13.40 - = 4.99 -	While drilling MW15S on 66 MW15D on 6	5 /1/06
Number	Recov % Rec	Type	Blows / 6 in N - Value RQD	Moist	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description		Borehole Detail	Elevation ft. MSL	Remarks
21A	22/24 92%	ss	3-7 11-18 N=18	19		BSh 6.11 BSh	42	Dark gray (10YR4/1), clayey SILT, trace sand and g	ravel		582	
2A	24/24 100%	ss	4-7 8-10 N=15	23		4.46 B	44	Dark greenish gray (5GY4/1) with 30% dark gray (N mottles, lean CLAY, trace sand	N4/1)		580	
3A	24/24 100%	SS	3-6 6-10 N=12	21								
23B				16		3.69 B	46				578	
24A	24/24 100%	ss	10-12 10-15 N=22	16		4.58 B	48	Dark gray (N4/1), lean CLAY, trace sand and grav	zel .		576	
25A	24/24 100%	ss	2-4 7-9 N=11	16		4.65 B	48	Daik gray (1947), tean CLA1, trace saild and grav	VCI		574	
26A	19/24 79%	ss	3-5 7-13 N=12	21		3.88 B	52				572	
27A	24/24 100%	ss	8-10 8-13 N=18	25		3.49 <i>BSh</i>	54	Dark yellowish brown (10YR3/4) with 50% dark grabrown (10YR4/2) mottles, lean CLAY, trace sand a gravel	ayish and		570	
28A	24/24 100%	ss	4-5 8-12 N=13	22		3.49 <i>BSh</i>	56	Greenish gray (10YR5/1) with 20% dark yellowish b (10YR4/4) mottles, lean CLAY, trace sand and gra	orown wel		568	
29A	24/24 100%	ss	5-9 15-18 N=24	20			58	-,			566	
30A	24/24 100%	SS	8-9 14-18 N=23	18		5.82 BSh	60	Olive (5Y4/3) with 15% greenish gray (10GY5/1) molean CLAY, trace sand and gravel	ottles,		564	

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois

Project: 05S3004A **DATES: Start:** 4/24/2006

Finish: 4/25/2006 WEATHER: Overcast, cool (lo-50's) CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 3¹/₄" HSA w/SS sampler

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager



BOREHOLE ID: SB-15 Well ID: n/a

 Surface Elev:
 624 ft. MSL

 Completion:
 84 ft. BGS

 Station:
 875,970.0N

 2,515,080.0E

S	AMPL	E	Т	EST	INC	j	TOPOGR	APHIC MAP INFORMATION:	WAT	ER LEVEL	INFORMAT	ΓΙΟΝ:
er	Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) Failure Type	Quadra Townsl	nngle: Coffeen, IL nip: East Fork 11, Tier 7N; Range 3W	Ž	Z = 13.40 - 4.99 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.90 - 4.	While drilling MW15S on 6 MW15D on 6	g /1/06
Number	Recov % Rec	Type	Blows N - Vs RQD	Moist	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description		Borehole Detail	Elevation ft. MSL	Remarks
1A	24/24 100%	ss	4-7 13-15 N=20	18		5.42 BSh	62	Olive (5Y4/3) with 15% greenish gray (10GY5/1) mo	ottles,		562	
2A	24/24 100%	ss	10-15 11-16 N=26	20		4.74 BSh	64	lean CLAY, trace sand and gravel [Continued from previous page]			560	
3A	24/24 100%	ss	6-10 11-13 N=21	16		6.98 BSh	66	Greenish gray (10Y5), lean CLAY, trace sand and gr	ravel		558	
64A	24/24 100%	ss	11-14 18-31 N=32	18		6.98 BSh	68-				556	
5A	23/24 96%	ss	9-18 27-40 N=45	15		11.95 BSh	70				554	
66A	24/24 100%	ss	4-12 18-24 N=30	16		7.15 BSh	72				552	
57A	24/24 100%	ss	17-29 36-47 N=65	17		8.24 <i>BSh</i>	74	Dark yellowish brown (10YR4/4), lean CLAY, trace and gravel	sand		550	
38A	20/24 83%	ss	12-18 23-28 N=41	17		6.59 BSh	76				548	
59A	9/24 38%	ss	29-39 48-66 N=87	16			78 —				546	
-0A	24/24 100%	SS	5-9 13-18 N=22	18		6.21 B	80				544	

CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A **DATES:** Start: 4/24/2006

Finish: 4/25/2006
WEATHER: Overcast, cool (lo-50's)

CONTRACTOR: Testing Service Corporation Rig mfg/model: CME-650 Track Rig Drilling Method: 3¹/₄" HSA w/SS sampler

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Helper: R. Keedy Eng/Geo: R. Hasenyager **HANSON**

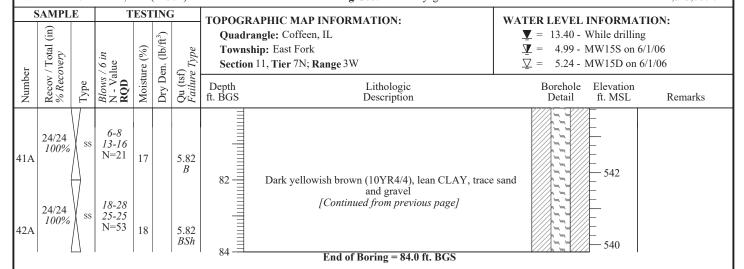
BOREHOLE ID: SB-15 **Well ID:** n/a

 Surface Elev:
 624 ft. MSL

 Completion:
 84 ft. BGS

 Station:
 875,970.0N

 2,515,080.0E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility Location: Coffeen, Illinois

Project: 05S3004A **DATES: Start: 4/21/2006**

SAMPLE

Finish: 4/25/2006 WEATHER: Overcast, cool (mid-40's)

TESTING

CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-850 Track Rig Drilling Method: 41/4" HSA (blind drill)

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager

WATER LEVEL INFORMATION:

BOREHOLE ID: SB-16b

Surface Elev:

Completion:

Station:

Well ID: MW16D

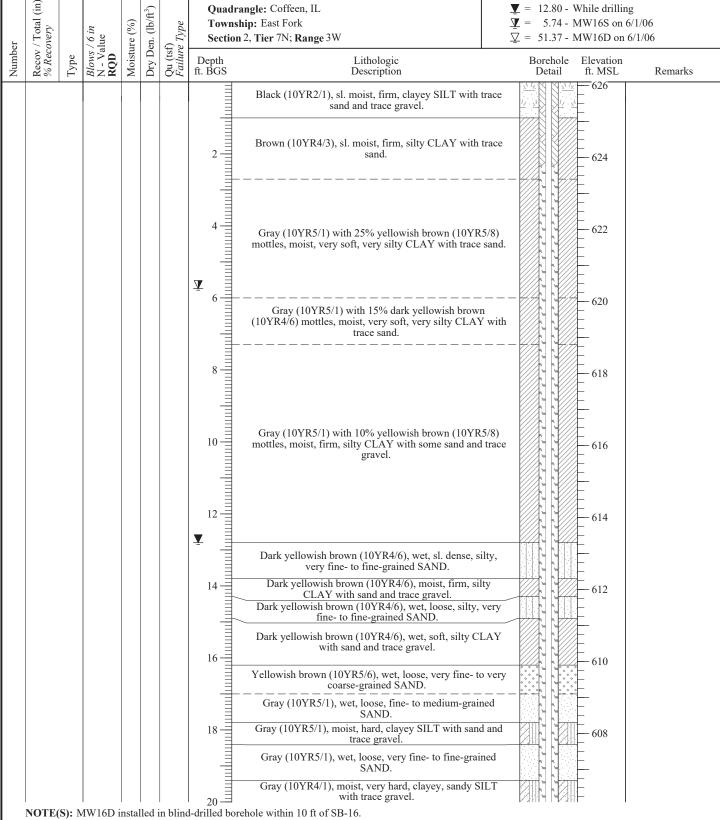
626 ft. MSL

51 ft. BGS

2,515,079.4E

877,354.9N

TOPOGRAPHIC MAP INFORMATION: $\mathbf{\underline{\Psi}} = 12.80$ - While drilling $\nabla = 5.74 - MW16S \text{ on } 6/1/06$



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois

Project: 05S3004A **DATES: Start:** 4/21/2006

Finish: 4/25/2006 WEATHER: Overcast, cool (mid-40's) CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-850 Track Rig

Drilling Method: 4¼" HSA (blind drill)

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager

HANSON

BOREHOLE ID: SB-16b
Well ID: MW16D
Surface Elev: 626 ft. MSL
Completion: 51 ft. BGS
Station: 877,354.9N

2,515,079.4E

	SAMPL		T		INC		TOROGRA	Eng/Geo: R. Hasenyager	XXA TEED I EXTEL	INFORMATI	2,515,079.4E
	Recov / Total (in) % Recovery			Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) Failure Type	Quadran	PHIC MAP INFORMATION: ngle: Coffeen, IL p: East Fork r, Tier 7N; Range 3W	$\underline{\underline{\mathbf{y}}} = 5.74 -$	While drilling MW16S on 6/1 MW16D on 6/	/06
Number	Recov % Rec	Type	Blows / 6 in N - Value RQD	Moistu	Dry Do	Qu (tsf Failur	Depth ft. BGS	Lithologic Description	Borehole Detail	ft. MSL	Remarks
	NOTE(S): M	IW16D i	nstal	lled i	n bline	22 ———————————————————————————————————	Gray (10YR4/1), moist, very hard, clayey, sandy SI with trace gravel. [Continued from previous page]			

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 4/21/2006

Finish: 4/25/2006 WEATHER: Overcast, cool (mid-40's) CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-850 Track Rig
Drilling Method: 4¹/₄" HSA (blind drill)

FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

HANSON

BOREHOLE ID: SB-16b Well ID: MW16D

- 578

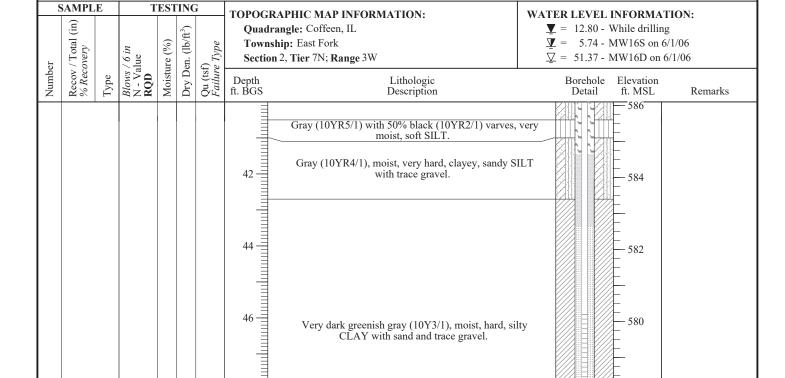
576

 Surface Elev:
 626 ft. MSL

 Completion:
 51 ft. BGS

 Station:
 877,354.9N

 2,515,079.4E



with trace sand and trace gravel.

End of Boring = 51.00 ft. BGS

See SB-16 for sample & testing details

Very dark bluish gray (5BG3/1), moist, firm, silty CLAY

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 4/21/2006

Finish: 4/25/2006

WEATHER: Overcast, cool (mid-40's)

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-850 Track Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

Helper: S. McCartney Eng/Geo: R. Hasenyager

HANSON
BOREHOLE ID: SB-16

Well ID: n/a

 Surface Elev:
 626 ft. MSL

 Completion:
 92 ft. BGS

 Station:
 877,355.0N

 2,515,080.0E

SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Ξ $\mathbf{V} = 12.80$ - While drilling Quadrangle: Coffeen, IL Dry Den. (lb/ft3 Recov / Total % Recovery Township: East Fork $\nabla = 5.74 - MW16S \text{ on } 6/1/06$ Moisture (%) Blows / 6 in N - Value RQD Section 2, Tier 7N; Range 3W ∇ = 51.37 - MW16D on 6/1/06 Qu (tsf) Failure 1 Number Lithologic Borehole Elevation Description Detail ft. MSL Remarks Black (10YR2/1), sl. moist, firm, clayey SILT with trace 22 1A sand and trace gravel. 21/24 SS 6-7 88% N = 101B 29 2.13 Brown (10YR4/3), sl. moist, firm, silty CLAY with trace B sand. 624 4-6 24/24 7-9 SS 100% N=132A 25 2.13 Gray (10YR5/1) with 25% yellowish brown (10YR5/8) mottles, moist, very soft, very silty CLAY with trace sand. 20/24 5-7 83% 3A 21 2.33 $\bar{\Lambda}$ B 620 Gray (10YR5/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, very soft, very silty CLAY with 24/24 trace sand. 4-6 100% 25 4A 2.13 В 618 24/24 5-5 100% N=95A 24 2.33 В Gray (10YR5/1) with 10% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with some sand and trace 616 gravel. 24/24 6A 24 1.75 4-5 Shelby tube taken SS SH 100% В from shallow well 24/24 borehole at 100% indicated depth. 12 7A 22 1.94 BSh \blacksquare 24/24 100% Dark yellowish brown (10YR4/6), wet, sl. dense, silty, N=147В 18 very fine- to fine-grained SAND. Dark yellowish brown (10YR4/6), moist, firm, silty 612 CLAY with sand and trace gravel. Dark yellowish brown (10YR4/6), wet, loose, silty, very 1-2 21/24 fine- to fine-grained SAND. 2-4 88% N=4 Dark yellowish brown (10YR4/6), wet, soft, silty CLAY 8A 20 with sand and trace gravel. 16 610 Yellowish brown (10YR5/6), wet, loose, very fine- to very coarse-grained SAND. 18/24 9A 14 4-10 75% Gray (10YR5/1), wet, loose, fine- to medium-grained SAND. 9B 15 Gray (10YR5/1), moist, hard, clayey SILT with sand and 608 trace gravel. 10A 10 27-54 Gray (10YR5/1), wet, loose, very fine- to fine-grained 20/24 10B 17 59-59 SS SAND. 83% N = 113Gray (10YR4/1), moist, very hard, clayey, sandy SILT with trace gravel. 20 NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A **DATES: Start:** 4/21/2006

Finish: 4/25/2006
WEATHER: Overcast, cool (mid-40's)

CONTRACTOR: Reynolds Drilling Corp. **Rig mfg/model:** CME-850 Track Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager

HANSON

BOREHOLE ID: SB-16 Well **ID:** n/a

 Surface Elev:
 626 ft. MSL

 Completion:
 92 ft. BGS

 Station:
 877,355.0N

 2,515,080.0E

S	AMPL	E	Т	EST	INC	3	TOPOGR	APHIC MAP INFORMATION:	WATI	R LEVEL	INFORMA	TION:
ber	Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) Failure Type	Quadr Towns Section	angle: Coffeen, IL hip: East Fork 12, Tier 7N; Range 3W	Ā Ā	= 12.80 - = 5.74 - = 51.37 -	While drillin MW16S on MW16D on	ng 6/1/06
Number	Reco % Re	Type	Blow N - V RQD	Mois	Dry 1	Qu (t Failu	Depth ft. BGS	Lithologic Description		Borehole Detail	ft. MSL	Remarks
A	10/24 42%	SS	10-96	8			22				606	
A	14/24 58%	SS	84-132	10		3.10 BSh	24				602	
A	20/24 83%	ss	41-68 82 N=150	10		7.56 B	26				600	
A	12/24 50%	ss	58-119	10		9.89 B	28				598	Dusky red (7.5YR3/4) staining.
A	24/24 100%	SS	30-48 70-71 N=118	9		5.62 B	22	Gray (10YR4/1), moist, very hard, clayey, sandy SI with trace gravel.	LT		596	
A	24/24 100%	SS	50-54 68-93 N=122	9			32	[Continued from previous page]			594	
7A	35/36 97%	CS		17			34 —				592	
33A	60/60 100%	CS		10			36				590	
							40	grout pumped from bottom of borehole.				Wood fragment

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A **DATES:** Start: 4/21/2006

Finish: 4/25/2006 WEATHER: Overcast, cool (mid-40's) **CONTRACTOR:** Reynolds Drilling Corp. **Rig mfg/model:** CME-850 Track Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager

Well ID: n/a
Surface Fley: 62

BOREHOLE ID: SB-16

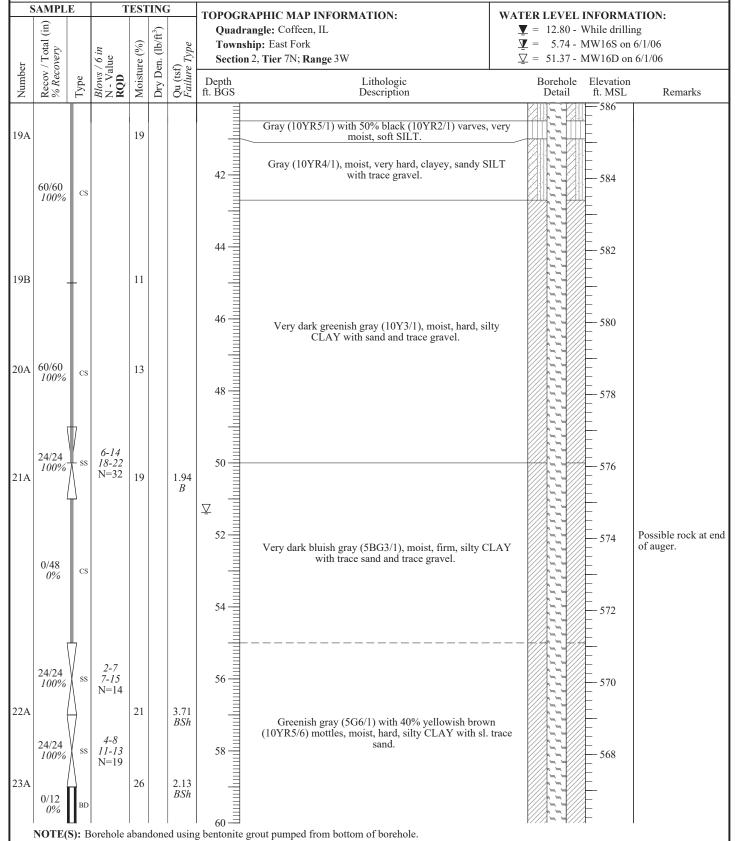
 Surface Elev:
 626 ft. MSL

 Completion:
 92 ft. BGS

 Station:
 877,355.0N

 2,515,080.0E

HANSON



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A

DATES: Start: 4/21/2006 Finish: 4/25/2006 WEATHER: Overcast, cool (mid-40's)

Drilling Method: 41/4" HSA w/SS & CME samplers FIELD STAFF: Driller: K. Doetzel

Eng/Geo: R. Hasenyager

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-850 Track Rig

Helper: S. McCartney

HANSON

BOREHOLE ID: SB-16 Well ID: n/a

> 626 ft. MSL **Surface Elev: Completion:** 92 ft. BGS 877,355.0N **Station:** 2,515,080.0E

S	SAMPLI	E	Т	EST	INC	j	TOPOGRAP	PHIC MAP INFORMATION:	WATER LEVEL INFORMATION:
er	Recov / Total (in) % Recovery		/ 6 in Ilue	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) Failure Type	Quadrang Township	gle: Coffeen, IL o: East Fork Tier 7N; Range 3W	$ \underline{\Psi} $ = 12.80 - While drilling $ \underline{\Psi} $ = 5.74 - MW16S on 6/1/06 $ \underline{\nabla} $ = 51.37 - MW16D on 6/1/06
Number	Recov % Rec	Type	Blows / 6 in N - Value RQD	Moist	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL Remarks
	0/48 0%	RC					62	Greenish gray (5G6/1) with 40% yellowish brow (10YR5/6) mottles, moist, hard, silty CLAY with sl. sand. [Continued from previous page]	n trace
26A	24/24 100%	SS	32-34 42-51 N=76	25		2.72 BSh	64-		562
	0/24 0%	RC					66 -	Yellowish brown (10YR5/6) with 20% greenish gr (5G6/1) mottles, moist, hard, silty CLAY with trace and trace coal fragments.	ray sand 560
28A	24/24 100%	SS	15-21 21-21 N=42	18		2.72 BSh	68 —		558
29A	24/24 100%	ss	14-17 21-25 N=38	20		2.91 BSh	70		70' to 79.5' - possible oxidation rinds.
30A	24/24 100%	ss	12-21 34-35 N=55	18		5.04 <i>BSh</i>	72		
31A	24/24 100%	ss	16-21 27-35 N=48	16		8.15 <i>BSh</i>	74	Yellowish brown (10YR5/6) with zones of gray (10YR4/1) mottles, moist, hard, clayey SILT with so sand and trace gravel.	ome = 552
32A	60/60 100%	cs		19			76	cana and duce graven	550 548

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility Location: Coffeen, Illinois

Project: 05S3004A **DATES: Start: 4/21/2006** Finish: 4/25/2006

WEATHER: Overcast, cool (mid-40's)

CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-850 Track Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

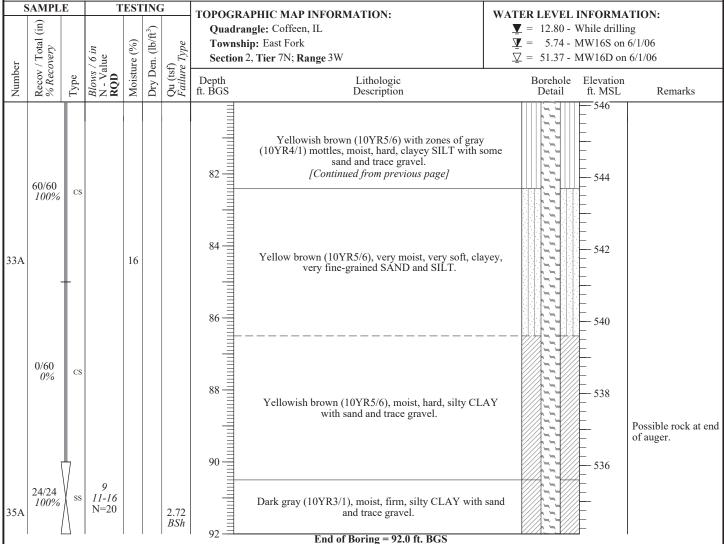
Eng/Geo: R. Hasenyager TOPOGRAPHIC MAP INFORMATION:

Helper: S. McCartney

HANSON

BOREHOLE ID: SB-16 Well ID: n/a

> **Surface Elev:** 626 ft. MSL 92 ft. BGS **Completion:** Station: 877,355.0N 2,515,080.0E



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 4/25/2006

Finish: 4/25/2006 WEATHER: Overcast, cool (mid-40's) CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-850 Track Rig Drilling Method: 4¹/₄" HSA (blind drill)

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager

HANSON
BOREHOLE ID: SB-16a

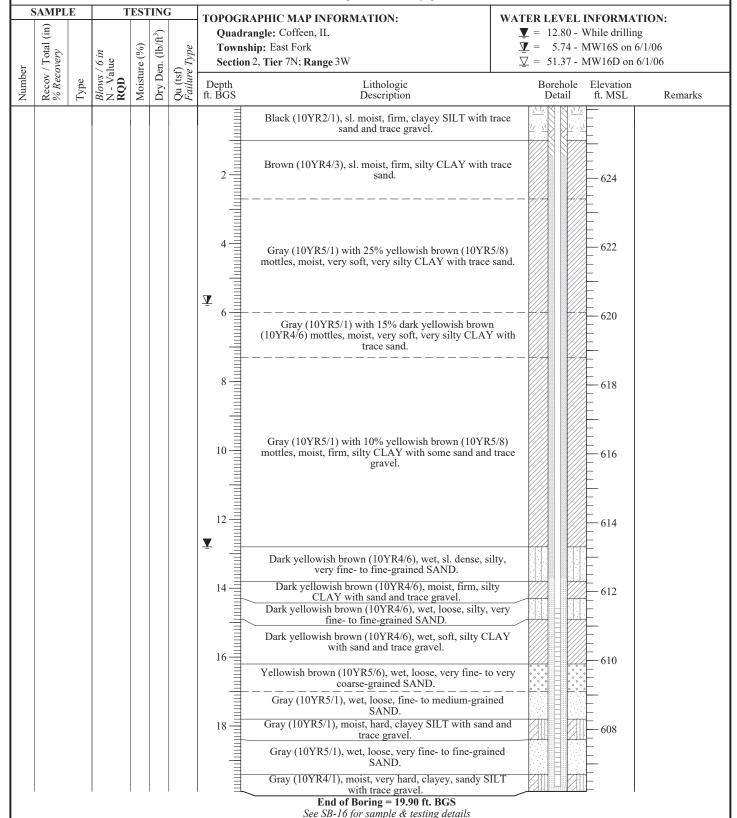
Well ID: MW16S

 Surface Elev:
 626 ft. MSL

 Completion:
 20 ft. BGS

 Station:
 877,355.1N

 2,515,088.0E



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 4/21/2006

Finish: 4/25/2006

WEATHER: Overcast, cool (mid-40's)

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-850 Track Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

Helper: S. McCartney Eng/Geo: R. Hasenyager

HANSON
BOREHOLE ID: SB-16

Well ID: n/a

 Surface Elev:
 626 ft. MSL

 Completion:
 92 ft. BGS

 Station:
 877,355.0N

 2,515,080.0E

SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Ξ $\mathbf{V} = 12.80$ - While drilling Quadrangle: Coffeen, IL Dry Den. (lb/ft3 Recov / Total % Recovery Township: East Fork $\nabla = 5.74 - MW16S \text{ on } 6/1/06$ Moisture (%) Blows / 6 in N - Value RQD Section 2, Tier 7N; Range 3W ∇ = 51.37 - MW16D on 6/1/06 Qu (tsf) Failure 1 Number Lithologic Borehole Elevation Description Detail ft. MSL Remarks Black (10YR2/1), sl. moist, firm, clayey SILT with trace 22 1A sand and trace gravel. 21/24 SS 6-7 88% N = 101B 29 2.13 Brown (10YR4/3), sl. moist, firm, silty CLAY with trace B sand. 624 4-6 24/24 7-9 SS 100% N=132A 25 2.13 Gray (10YR5/1) with 25% yellowish brown (10YR5/8) mottles, moist, very soft, very silty CLAY with trace sand. 20/24 5-7 83% 3A 21 2.33 $\bar{\Lambda}$ B 620 Gray (10YR5/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, very soft, very silty CLAY with 24/24 trace sand. 4-6 100% 25 4A 2.13 В 618 24/24 5-5 100% N=95A 24 2.33 В Gray (10YR5/1) with 10% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with some sand and trace 616 gravel. 24/24 6A 24 1.75 4-5 Shelby tube taken SS SH 100% В from shallow well 24/24 borehole at 100% indicated depth. 12 7A 22 1.94 BSh \blacksquare 24/24 100% Dark yellowish brown (10YR4/6), wet, sl. dense, silty, N=147В 18 very fine- to fine-grained SAND. Dark yellowish brown (10YR4/6), moist, firm, silty 612 CLAY with sand and trace gravel. Dark yellowish brown (10YR4/6), wet, loose, silty, very 1-2 21/24 fine- to fine-grained SAND. 2-4 88% N=4 Dark yellowish brown (10YR4/6), wet, soft, silty CLAY 8A 20 with sand and trace gravel. 16 610 Yellowish brown (10YR5/6), wet, loose, very fine- to very coarse-grained SAND. 18/24 9A 14 4-10 75% Gray (10YR5/1), wet, loose, fine- to medium-grained SAND. 9B 15 Gray (10YR5/1), moist, hard, clayey SILT with sand and 608 trace gravel. 10A 10 27-54 Gray (10YR5/1), wet, loose, very fine- to fine-grained 20/24 10B 17 59-59 SS SAND. 83% N = 113Gray (10YR4/1), moist, very hard, clayey, sandy SILT with trace gravel. 20 NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A **DATES: Start:** 4/21/2006

Finish: 4/25/2006
WEATHER: Overcast, cool (mid-40's)

CONTRACTOR: Reynolds Drilling Corp. **Rig mfg/model:** CME-850 Track Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager

HANSON

BOREHOLE ID: SB-16 Well **ID:** n/a

 Surface Elev:
 626 ft. MSL

 Completion:
 92 ft. BGS

 Station:
 877,355.0N

 2,515,080.0E

S	AMPL	E	Т	EST	INC	3	TOPOGR	APHIC MAP INFORMATION:	WATI	R LEVEL	INFORMA	TION:
ber	Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) Failure Type	Quadr Towns Section	angle: Coffeen, IL hip: East Fork 12, Tier 7N; Range 3W	Ā Ā	= 12.80 - = 5.74 - = 51.37 -	While drillin MW16S on MW16D on	ng 6/1/06
Number	Reco % Re	Type	Blow N - V RQD	Mois	Dry 1	Qu (t Failu	Depth ft. BGS	Lithologic Description		Borehole Detail	ft. MSL	Remarks
A	10/24 42%	SS	10-96	8			22				606	
A	14/24 58%	SS	84-132	10		3.10 BSh	24				602	
A	20/24 83%	ss	41-68 82 N=150	10		7.56 B	26				600	
A	12/24 50%	ss	58-119	10		9.89 B	28				598	Dusky red (7.5YR3/4) staining.
A	24/24 100%	SS	30-48 70-71 N=118	9		5.62 B	22	Gray (10YR4/1), moist, very hard, clayey, sandy SI with trace gravel.	LT		596	
A	24/24 100%	SS	50-54 68-93 N=122	9			32	[Continued from previous page]			594	
7A	35/36 97%	CS		17			34 —				592	
33A	60/60 100%	CS		10			36				590	
							40	grout pumped from bottom of borehole.				Wood fragment

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A **DATES:** Start: 4/21/2006

Finish: 4/25/2006 WEATHER: Overcast, cool (mid-40's) **CONTRACTOR:** Reynolds Drilling Corp. **Rig mfg/model:** CME-850 Track Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager

Well ID: n/a
Surface Fley: 62

BOREHOLE ID: SB-16

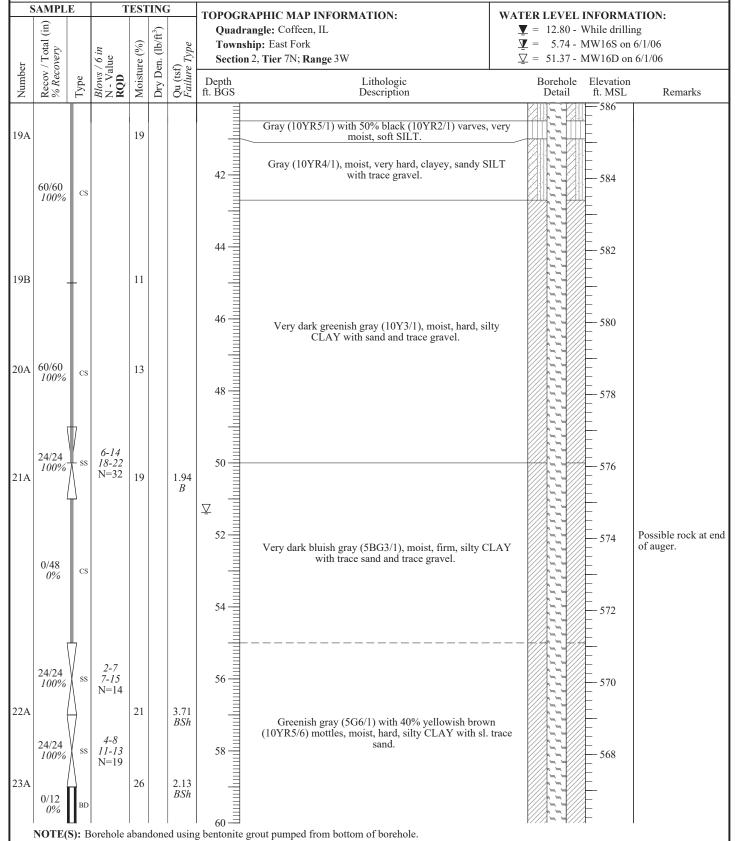
 Surface Elev:
 626 ft. MSL

 Completion:
 92 ft. BGS

 Station:
 877,355.0N

 2,515,080.0E

HANSON



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A

DATES: Start: 4/21/2006 Finish: 4/25/2006 WEATHER: Overcast, cool (mid-40's)

Drilling Method: 41/4" HSA w/SS & CME samplers FIELD STAFF: Driller: K. Doetzel

Eng/Geo: R. Hasenyager

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-850 Track Rig

Helper: S. McCartney

HANSON

BOREHOLE ID: SB-16 Well ID: n/a

> 626 ft. MSL **Surface Elev: Completion:** 92 ft. BGS 877,355.0N **Station:** 2,515,080.0E

S	SAMPLI	E	Т	EST	INC	j	TOPOGRAP	PHIC MAP INFORMATION:	WATER LEVEL INFORMATION:
er	Recov / Total (in) % Recovery		/ 6 in Ilue	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) Failure Type	Quadrang Township	gle: Coffeen, IL o: East Fork Tier 7N; Range 3W	$ \underline{\Psi} $ = 12.80 - While drilling $ \underline{\Psi} $ = 5.74 - MW16S on 6/1/06 $ \underline{\nabla} $ = 51.37 - MW16D on 6/1/06
Number	Recov % Rec	Type	Blows / 6 in N - Value RQD	Moist	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL Remarks
	0/48 0%	RC					62	Greenish gray (5G6/1) with 40% yellowish brow (10YR5/6) mottles, moist, hard, silty CLAY with sl. sand. [Continued from previous page]	n trace
26A	24/24 100%	SS	32-34 42-51 N=76	25		2.72 BSh	64-		562
	0/24 0%	RC					66 -	Yellowish brown (10YR5/6) with 20% greenish gr (5G6/1) mottles, moist, hard, silty CLAY with trace and trace coal fragments.	ray sand 560
28A	24/24 100%	SS	15-21 21-21 N=42	18		2.72 BSh	68 —		558
29A	24/24 100%	ss	14-17 21-25 N=38	20		2.91 BSh	70		70' to 79.5' - possible oxidation rinds.
30A	24/24 100%	ss	12-21 34-35 N=55	18		5.04 <i>BSh</i>	72		
31A	24/24 100%	ss	16-21 27-35 N=48	16		8.15 <i>BSh</i>	74	Yellowish brown (10YR5/6) with zones of gray (10YR4/1) mottles, moist, hard, clayey SILT with so sand and trace gravel.	ome = 552
32A	60/60 100%	cs		19			76	cana and duce graven	550 548

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility Location: Coffeen, Illinois

Project: 05S3004A **DATES: Start: 4/21/2006** Finish: 4/25/2006

WEATHER: Overcast, cool (mid-40's)

CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-850 Track Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

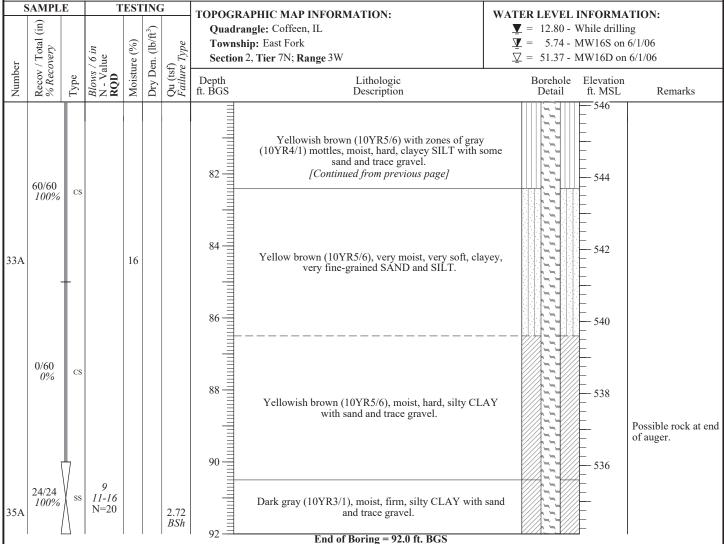
Eng/Geo: R. Hasenyager TOPOGRAPHIC MAP INFORMATION:

Helper: S. McCartney

HANSON

BOREHOLE ID: SB-16 Well ID: n/a

> **Surface Elev:** 626 ft. MSL 92 ft. BGS **Completion:** Station: 877,355.0N 2,515,080.0E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/4/2006 Finish: 5/4/2006

WEATHER: Partly sunny, cool (mid-50's)

NOTE(S): CME-1050 had 280# hammer for SPT.

CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager

HANSON
BOREHOLE ID: SB-17

Well ID: MW17D
Surface Elev: 627 ft. MSL
Completion: 54 ft. BGS
Station: 878.659.0N

2,515,090.4E

TESTING SAMPLE TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Ξ $\mathbf{V} = 11.70$ - While drilling Quadrangle: Coffeen, IL Dry Den. (lb/ft3 Recov / Total % Recovery Township: East Fork $\nabla = 6.89 - MW17S \text{ on } 6/1/06$ Qu (tsf) Failure Type Moisture (%) Blows / 6 in N - Value RQD Section 2, Tier 7N; Range 3W ∇ = 54.45 - MW17D on 6/1/06 Number Depth ft. BGS Lithologic Borehole Elevation Description Detail ft. MSL Remarks Dark grayish brown (10YR4/2), moist, soft, clayey SILT with trace sand and trace gravel. 20/24 1-1 SS 83% 626 N=21 A 29 Yellowish brown (10YR5/8), moist, soft, silty CLAY. Gray (10YR5/1) with 40% yellowish brown (10YR5/6) 24/24 2-2 100% mottles, moist, firm, silty CLAY with little sand and trace 624 N=32A 26 1.71 gravel. 24/24 1-3 622 100% Yellowish brown (10YR5/6) with 10% gray (10YR5/1) N=23A 16 2.62 mottles, moist, firm sandy, clayey SILT. BSh1-2 1 24/24 2-3 620 100% N=418 2.33 Yellowish brown (10YR5/6) with 30% gray (10YR5/1) mottles, moist, firm sandy, clayey SILT. 24/24 2-3 100% N=318 5A 10 Yellowish brown (10YR5/6), moist, soft, very sandy, clayey SÍLT. 24/24 1-1 616 100% Gray (10YR5/1) with 30% yellowish brown (10YR5/6) N=221 0.58 6A mottles, very moist, soft, very sandy, clayey SILT. 18 6B Moderate yellowish brown (10YR5/4), wet, loose, very 12 fine- to fine-grained SAND. Yellowish brown (10YR5/6), wet, dense, silty, very fine-24/24 21 to fine-grained SAND. 3-7 100% Yellowish brown (10YR5/6), very moist, dense, silty, very fine-grained SAND. 7B 19 Yellowish brown (10YR5/6) wet, dense, fine- to 84 15 medium-grained SAND. 24/24 Moderate yellowish brown (10YR5/4), wet, dense, SILT 5-7 612 100% and very fine-grained SAND N=10 8B13 Yellowish brown (10YR5/6), moist, clayey SILT and very fine-grained SAND with trace gravel. 8C 13 0-6 Yellowish brown (10YR5/6), wet, loose, very fine- to 24/24 6-6 medium-grained SAND. 610 100% N=129A 17 Yellowish brown (10YR5/6), wet, loose, very fine- to 0-2 fine-grained SAND. 24/24 2-3 SS 608 100% Yellowish brown (10YR5/6), wet, sl. dense, SILT with N=410A 19 some very fine-grained SAND. Gray (10YR5/1), wet, sl. dense, SILT with some very 10B 23

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/4/2006 Finish: 5/4/2006 CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

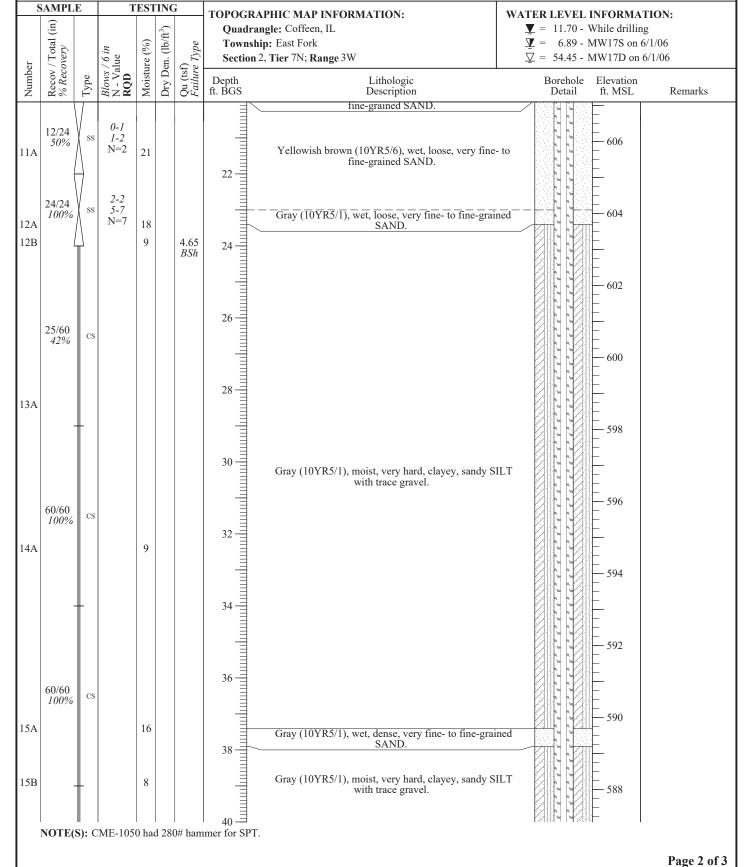
FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

WEATHER: Partly sunny, cool (mid-50's) Eng/Geo: R. Hasenyager



BOREHOLE ID: SB-17

Well ID: MW17D
Surface Elev: 627 ft. MSL
Completion: 54 ft. BGS
Station: 878,659.0N
2,515,090.4E



CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/4/2006 Finish: 5/4/2006

Finish: 5/4/2006 WEATHER: Partly sunny, cool (mid-50's) **CONTRACTOR:** Reynolds Drilling Corp. **Rig mfg/model:** CME-1050 ATV Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

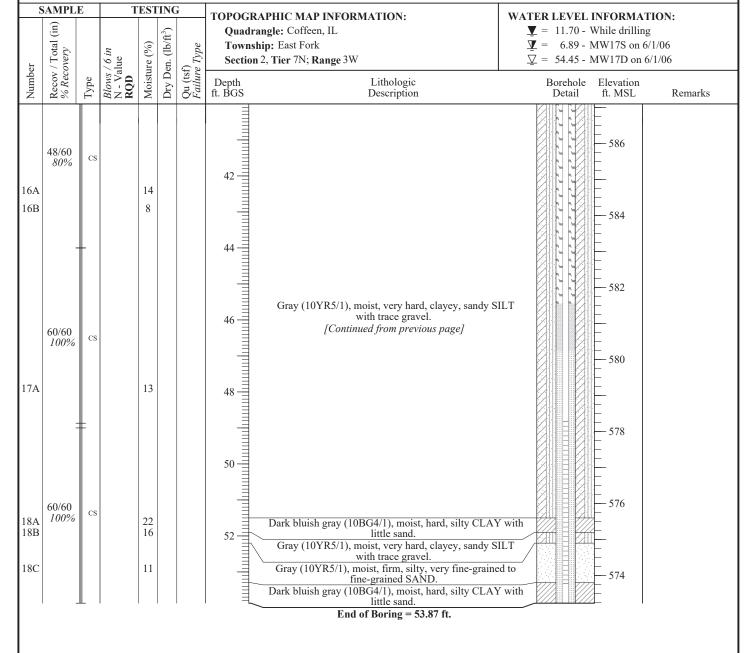
Eng/Geo: R. Hasenyager

HANSON
BOREHOLE ID: SB-17

Station:

Well ID: MW17D
Surface Elev: 627 ft. MSL
Completion: 54 ft. BGS

878,659.0N 2,515,090.4E



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois
Project: 05S3004A

DATES: Start: 5/4/2006

Finish: 5/4/2006

CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-1050 ATV Rig Drilling Method: 4¼" HSA (blind drill)

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney Eng/Geo: R. Hasenyager **HANSON**

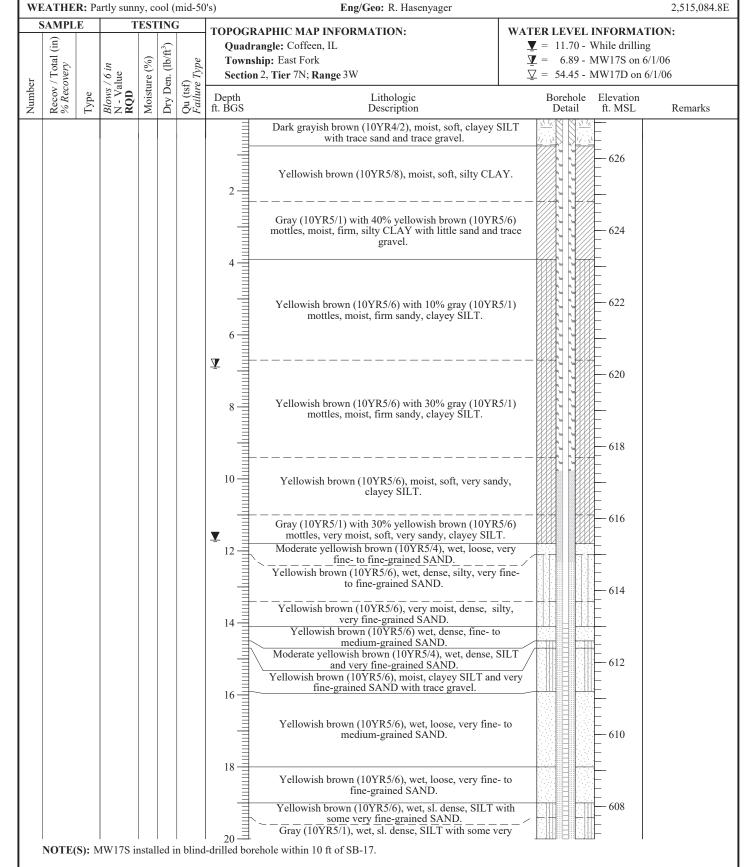
BOREHOLE ID: SB-17a **Well ID:** MW17S

 Surface Elev:
 627 ft. MSL

 Completion:
 24 ft. BGS

 Station:
 878,658.5N

 2,515,084.8E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility Location: Coffeen, Illinois

Project: 05S3004A **DATES: Start:** 5/4/2006 Finish: 5/4/2006

WEATHER: Partly sunny, cool (mid-50's)

CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-1050 ATV Rig **Drilling Method:** 41/4" HSA (blind drill)

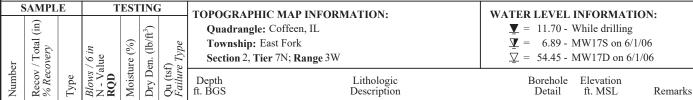
FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager

HANSON

BOREHOLE ID: SB-17a Well ID: MW17S

Surface Elev: 627 ft. MSL 24 ft. BGS **Completion: Station:** 878,658.5N 2,515,084.8E





End of Boring = 24.11 ft. BGS See SB-17 for sample & testing details

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/4/2006 Finish: 5/4/2006

WEATHER: Partly sunny, cool (mid-50's)

NOTE(S): CME-1050 had 280# hammer for SPT.

CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager

HANSON
BOREHOLE ID: SB-17

Well ID: MW17D
Surface Elev: 627 ft. MSL
Completion: 54 ft. BGS
Station: 878.659.0N

2,515,090.4E

TESTING SAMPLE TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Ξ $\mathbf{V} = 11.70$ - While drilling Quadrangle: Coffeen, IL Dry Den. (lb/ft3 Recov / Total % Recovery Township: East Fork $\nabla = 6.89 - MW17S \text{ on } 6/1/06$ Qu (tsf) Failure Type Moisture (%) Blows / 6 in N - Value RQD Section 2, Tier 7N; Range 3W ∇ = 54.45 - MW17D on 6/1/06 Number Depth ft. BGS Lithologic Borehole Elevation Description Detail ft. MSL Remarks Dark grayish brown (10YR4/2), moist, soft, clayey SILT with trace sand and trace gravel. 20/24 1-1 SS 83% 626 N=21 A 29 Yellowish brown (10YR5/8), moist, soft, silty CLAY. Gray (10YR5/1) with 40% yellowish brown (10YR5/6) 24/24 2-2 100% mottles, moist, firm, silty CLAY with little sand and trace 624 N=32A 26 1.71 gravel. 24/24 1-3 622 100% Yellowish brown (10YR5/6) with 10% gray (10YR5/1) N=23A 16 2.62 mottles, moist, firm sandy, clayey SILT. BSh1-2 1 24/24 2-3 620 100% N=418 2.33 Yellowish brown (10YR5/6) with 30% gray (10YR5/1) mottles, moist, firm sandy, clayey SILT. 24/24 2-3 100% N=318 5A 10 Yellowish brown (10YR5/6), moist, soft, very sandy, clayey SÍLT. 24/24 1-1 616 100% Gray (10YR5/1) with 30% yellowish brown (10YR5/6) N=221 0.58 6A mottles, very moist, soft, very sandy, clayey SILT. 18 6B Moderate yellowish brown (10YR5/4), wet, loose, very 12 fine- to fine-grained SAND. Yellowish brown (10YR5/6), wet, dense, silty, very fine-24/24 21 to fine-grained SAND. 3-7 100% Yellowish brown (10YR5/6), very moist, dense, silty, very fine-grained SAND. 7B 19 Yellowish brown (10YR5/6) wet, dense, fine- to 84 15 medium-grained SAND. 24/24 Moderate yellowish brown (10YR5/4), wet, dense, SILT 5-7 612 100% and very fine-grained SAND N=10 8B13 Yellowish brown (10YR5/6), moist, clayey SILT and very fine-grained SAND with trace gravel. 8C 13 0-6 Yellowish brown (10YR5/6), wet, loose, very fine- to 24/24 6-6 medium-grained SAND. 610 100% N=129A 17 Yellowish brown (10YR5/6), wet, loose, very fine- to 0-2 fine-grained SAND. 24/24 2-3 SS 608 100% Yellowish brown (10YR5/6), wet, sl. dense, SILT with N=410A 19 some very fine-grained SAND. Gray (10YR5/1), wet, sl. dense, SILT with some very 10B 23

CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/4/2006 Finish: 5/4/2006 CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

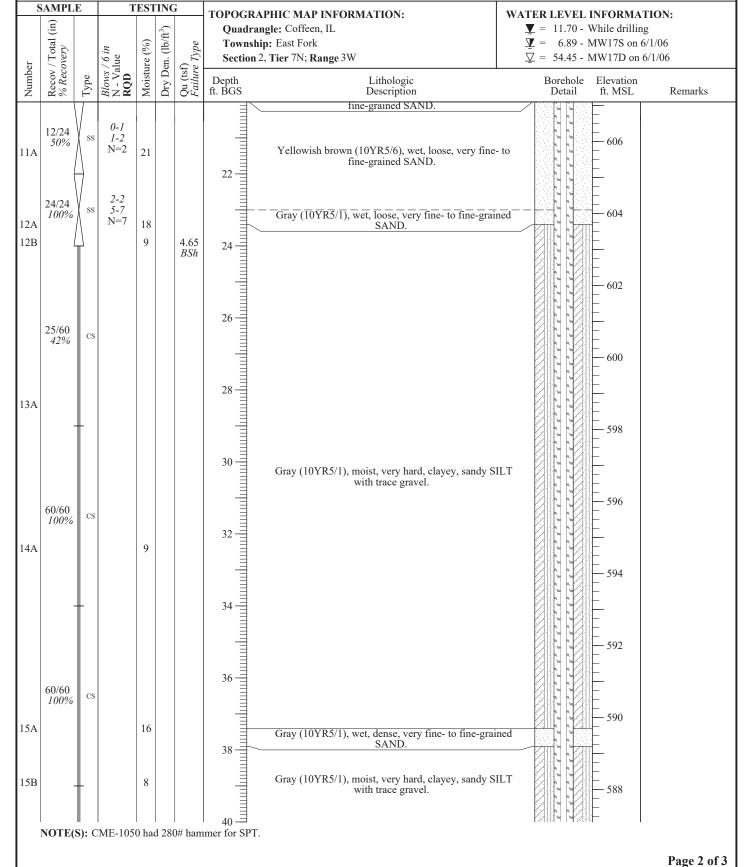
FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

WEATHER: Partly sunny, cool (mid-50's) Eng/Geo: R. Hasenyager



BOREHOLE ID: SB-17

Well ID: MW17D
Surface Elev: 627 ft. MSL
Completion: 54 ft. BGS
Station: 878,659.0N
2,515,090.4E



CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/4/2006 Finish: 5/4/2006

Finish: 5/4/2006 WEATHER: Partly sunny, cool (mid-50's) **CONTRACTOR:** Reynolds Drilling Corp. **Rig mfg/model:** CME-1050 ATV Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

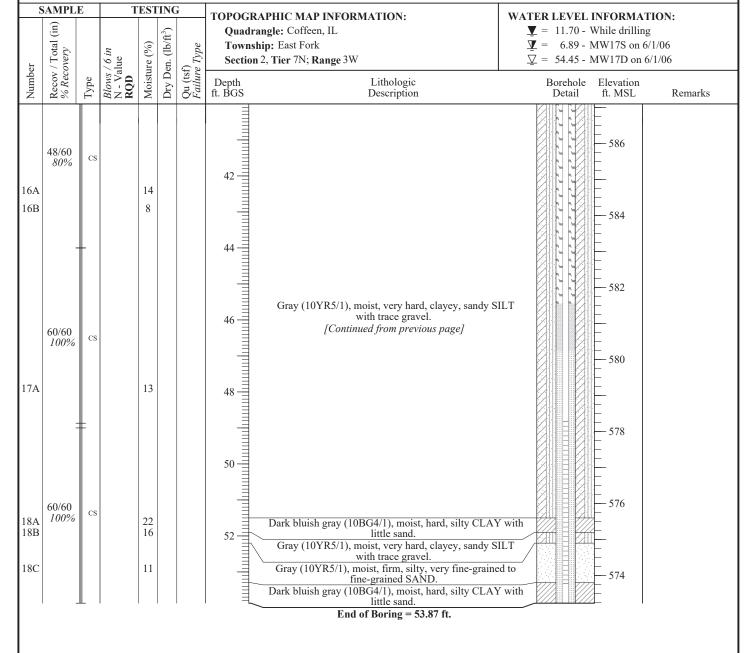
Eng/Geo: R. Hasenyager

HANSON
BOREHOLE ID: SB-17

Station:

Well ID: MW17D
Surface Elev: 627 ft. MSL
Completion: 54 ft. BGS

878,659.0N 2,515,090.4E



CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A DATES: Start: 5/11/2006

Finish: 5/11/2006
WEATHER: Partly sunny, cool (mid-50's)

CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-1050 ATV Rig Drilling Method: 41/4" HSA (blind drill)

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: R. Hasenyager



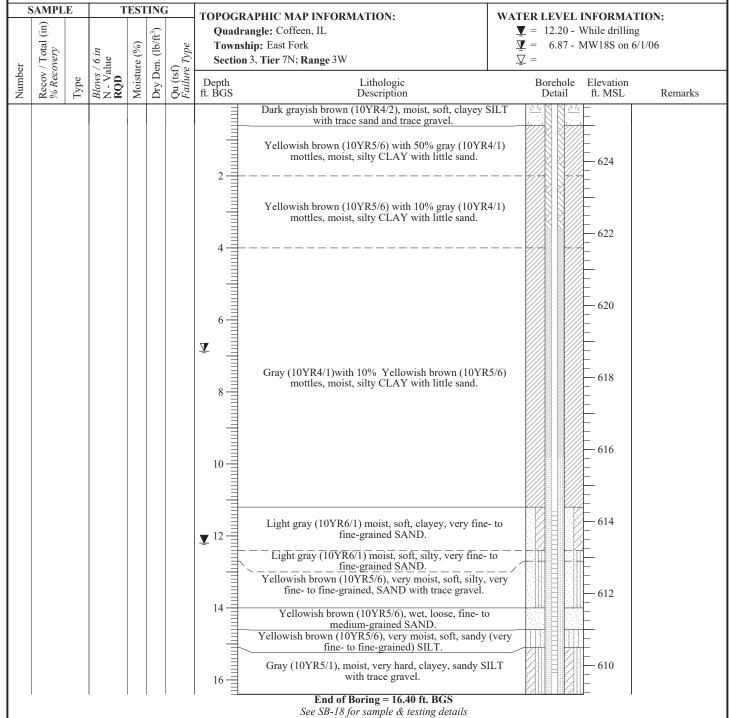
BOREHOLE ID: SB-18a **Well ID:** MW18S

 Surface Elev:
 626 ft. MSL

 Completion:
 16 ft. BGS

 Station:
 878,604.7N

 2,513,745.2E



NOTE(S): MW18S installed in blind-drilled borehole within 10 ft of SB-18.

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois
Project: 05S3004A

DATES: Start: 5/11/2006

DATES: Start: 5/11/2006 Finish: 5/11/2006 WEATHER: Partly sunny, cool (mid-50's) CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-1050 ATV Rig
Drilling Method: 4½" HSA w/SS & CME

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-18

Well ID: n/a
Surface Elev: 626 ft. MSL
Completion: 54 ft. BGS
Station: 878,605.0N

2,513,750.0E

SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Ξ $\mathbf{V} = 12.20$ - While drilling Quadrangle: Coffeen, IL Dry Den. (lb/ft3 Recov / Total (% Recovery Township: East Fork $\Psi = 6.87 - MW18S \text{ on } 6/1/06$ Qu (tsf) Failure Type Moisture (%) Blows / 6 in N - Value RQD Section 3, Tier 7N; Range 3W Number Lithologic Borehole Elevation Description Detail Remarks Dark grayish brown (10YR4/2), moist, soft, clayey SILT with trace sand and trace gravel. 0 - 118/24 1-1 SS 75% Yellowish brown (10YR5/6) with 50% gray (10YR4/1) N=21 A 24 1.31 mottles, moist, silty CLAY with little sand. 624 BSh 1-2 2-2 24/24 Yellowish brown (10YR5/6) with 10% gray (10YR4/1) mottles, moist, silty CLAY with little sand. 100% N=42A 28 1.78 622 24/24 2-2 100% 3A 23 1.32 620 BSh $\bar{\pmb{\Lambda}}$ 24/24 24 1.09 *1-2* N=2 100% Gray (10YR4/1)with 10% Yellowish brown (10YR5/6) 618 mottles, moist, silty CLAY with little sand. 5A 28 24/24 100% 616 0-0 24/24 1-2 100% N=121 0.39 6A Light gray (10YR6/1) moist, soft, clayey, very fine- to 614 **▼** 12 fine-grained SAND. 7A 17 Light gray (10YR6/1) moist, soft, silty, very fine- to 24/24 <u>fine-grained SAND.</u> 9-15 100% Yellowish brown (10YR5/6), very moist, soft, silty, very N = 14fine- to fine-grained, SAND with trace gravel. 612 7B 15 Yellowish brown (10YR5/6), wet, loose, fine- to 8A 14 medium-grained SAND. Yellowish brown (10YR5/6), very moist, soft, sandy (very 24/24 8B 11 9-10 fine- to fine-grained) SILT. 100% N=18 610 9 8C 24/36 Gray (10YR5/1), moist, very hard, clayey, sandy SILT 67% 608 with trace gravel. 9A NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole. CME-1050 had 280# hammer for SPT.

CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A

DATES: Start: 5/11/2006 **Finish:** 5/11/2006

WEATHER: Partly sunny, cool (mid-50's)

CONTRACTOR: Reynolds Drilling Corp. **Rig mfg/model:** CME-1050 ATV Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

Eng/Geo: R. Hasenyager

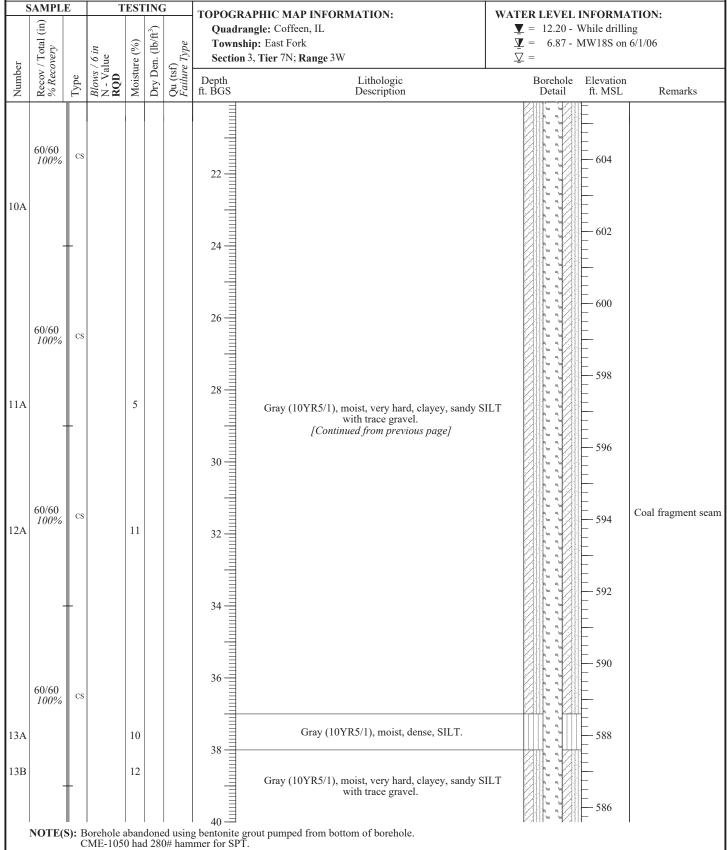
HANSON
BOREHOLE ID: SB-18

Well ID: n/a

Surface Elev: 626 ft. MSL Completion: 54 ft. BGS Station: 878,605.0N

2,513,750.0E

Page 2 of 3



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A

SAMPLE

DATES: Start: 5/11/2006 Finish: 5/11/2006

WEATHER: Partly sunny, cool (mid-50's)

TESTING

CONTRACTOR: Reynolds Drilling Corp. Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 41/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel Helper: S. McCartney

TOPOGRAPHIC MAP INFORMATION:

Eng/Geo: R. Hasenyager

WATER LEVEL INFORMATION:

HANSON

626 ft. MSL

54 ft. BGS

878,605.0N

2,513,750.0E

BOREHOLE ID: SB-18

Surface Elev:

Completion:

Station:

Well ID: n/a

 $\mathbf{Y} = 12.20$ - While drilling

Œ, Quadrangle: Coffeen, IL Dry Den. (lb/ft³) Recov / Total (% Recovery Qu (tsf) Failure Type Township: East Fork $\Psi = 6.87 - MW18S \text{ on } 6/1/06$ Moisture (%) Section 3, Tier 7N; Range 3W Number Elevation Lithologic Borehole Description Remarks 60/60 CS 584 100% Appears more clayey 14A 13 582 580 Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel. 60/60 [Continued from previous page] CS 100% 578 15A 13 13 16A 60/60 CS 100% Greenish gray (10BG5/1), moist, firm, silty CLAY with little sand and trace gravel. 16B End of Boring = 54.0 ft.

CLIENT: AEG Coffeen Power Station **Site:** Ash Pond Investigation

Location: Coffeen, IL Project: 05S3004B

DATES: Start: 5/1/2007 Finish: 5/1/2007

WEATHER: Partly sunny, warm

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-550 ATV Drill

Drilling Method: 41/4" Hollow stem auger with split spoon

sampler

FIELD STAFF: Driller: A. Rachford Helper: M. Brown

Eng/Geo: R. Hasenyager

BOREHOLE ID:SB20

Well ID: MW20S

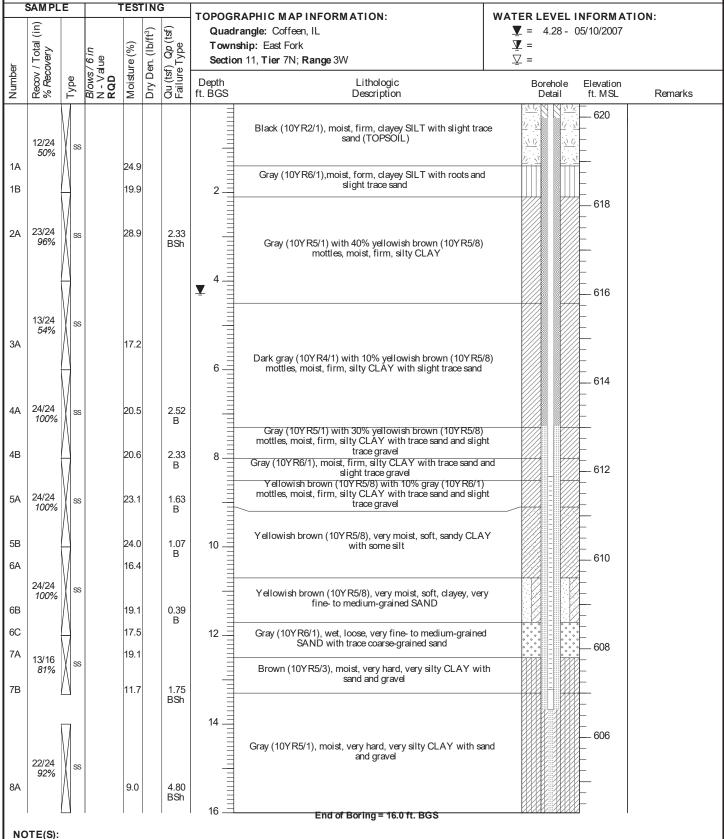
 Surface Elev:
 620.30 ft. MSL

 Completion:
 16.00 ft. BGS

 Station:
 874,226.44N

2,515,867.87E

Page 1 of 1



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois Project: 05S3004A

DATES: Start: 10/8/2010 Finish: 10/8/2010

WEATHER: Sunny, mild

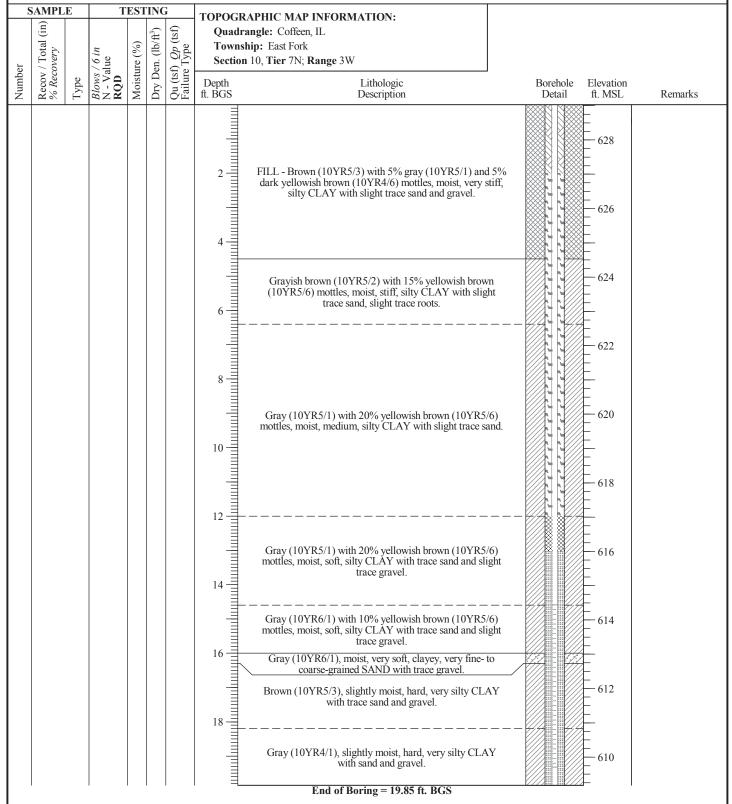
CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill **Drilling Method:** 41/4" HSA (blind drill)

FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: R. Hasenyager

BOREHOLE ID: R104 Well ID: R104

> Surface Elev: 629.03 ft. MSL Completion: 19.85 ft. BGS **Station:** 875,857.80N 2,514,503.41E



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A **DATES:** Start: 2/15/2010

Finish: 2/15/2010

WEATHER: Overcast, cold, windy (lo-20's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 4½" HSA w/SS samplers

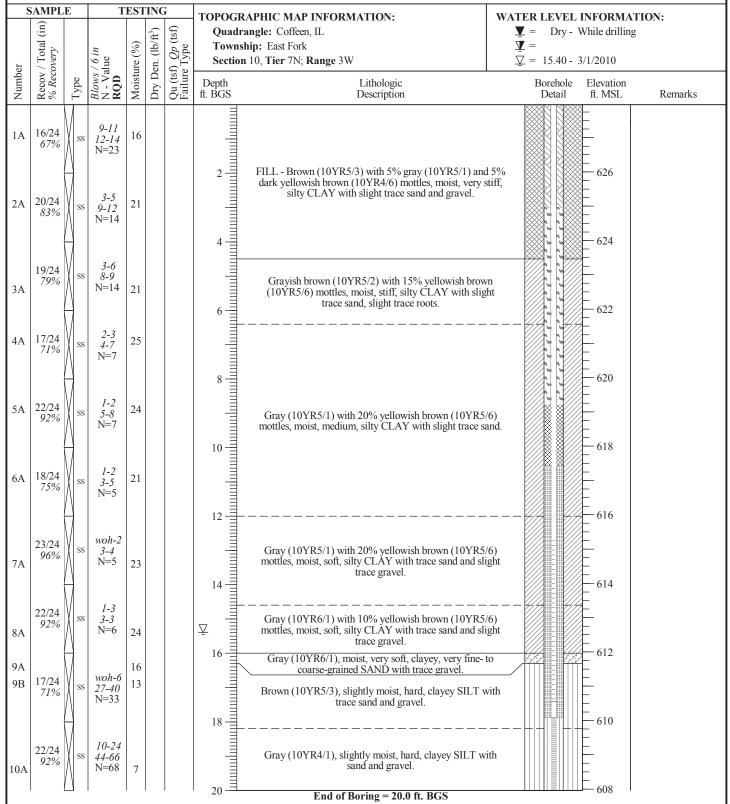
FIELD STAFF: Driller: T. List

Helper: M. Herbst/S. Hamby **Eng/Geo:** S. Suzanna Simpson

HANSON

BOREHOLE ID: G104 **Well ID:** G104

Surface Elev: 627.96 ft. MSL Completion: 20.00 ft. BGS Station: 875,849.26N 2.514.504.98E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 10/15/2010

Finish: 10/15/2010 WEATHER: Sunny (mid-50's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 41/4" HSA (blind drill)

FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: R. Hasenyager

HANSON

BOREHOLE ID: R201 **Well ID:** R201

 Surface Elev:
 624.02 ft. MSL

 Completion:
 17.22 ft. BGS

 Station:
 877,925.26N

 2,514,841.96E

TESTING SAMPLE TOPOGRAPHIC MAP INFORMATION: Qu (tsf) *Qp* (tsf) Failure Type Dry Den. (lb/ft³) Quadrangle: Coffeen, IL Recov / Total (% Recovery Moisture (%) Township: East Fork Blows / 6 in N - Value RQD Section 2, Tier 7N; Range 3W Number Lithologic Borehole Elevation ft. BGS Description Detail ft. MSL Remarks Very dark grayish brown (10YR3/2), moist, soft, friable, clayey SILT, slight trace sand and gravel Dark brown (10YR3/3), moist, soft, silty CLAY 622 Yellowish brown (10YR5/8) with 20% gray (10YR5/1) mottles, moist, firm, silty CLAY, trace sand, slight trace gravel 620 Gray (10YR5/1) with 5% yellowish brown (10YR5/6) mottles, moist, firm, silty CLAY, sand and trace gravel 618 Gray (10YR5/1), moist, firm, sandy CLAY, trace silt and slight trace gravel Yellowish brown (10YR5/8) with 10% gray (10YR5/1) mottles, moist, firm, sandy CLAY, trace gravel 614 Yellowish brown (10YR5/8), wet, soft, silty SAND, trace gravel Yellowish brown (10YR5/8), moist, firm, clayey SILT Greenish gray (5GY6/1), moist, firm, interbedded clayey SILT and SILT 610 Yellowish brown (10YR5/8), wet, soft, fine-to coarse-grained SAND, slight trace gravel 608 Yellowish brown (10YR5/8), wet, firm, very fine-to __fine-grained silty SAND Gray (10YR5/1), wet, soft, SILT Gray (10YR5/1), wet, soft, fine- to coarse-grained SAND, slight trace gravel Gray (10YR5/1), moist, hard, very silty CLAY, trace sand and gravel End of Boring = 17.22 ft. BGS

CLIENT: Illinois Power Generating Co.

TESTING

Site: Coffeen Power Station - Gypsum Mgmt Facility

Quadrangle: Coffeen, IL

Location: Coffeen, IL

SAMPLE

Œ)

Project: 15E0161A **DATES: Start:** 3/20/2017

Finish: 3/20/2017 WEATHER: Sunny, mild, (mid-60s) **CONTRACTOR:** Bulldog Drilling Rig mfg/model: CME-550 ATV Drill

Drilling Method: 41/4" HSA (blind drill)

FIELD STAFF: Driller: J. Gates Helper: C. Clines

Eng/Geo: R. Hasenyager TOPOGRAPHIC MAP INFORMATION:

WATER LEVEL INFORMATION:

BOREHOLE ID: R205

Well ID: R205

Surface Elev: 621.91 ft. MSL

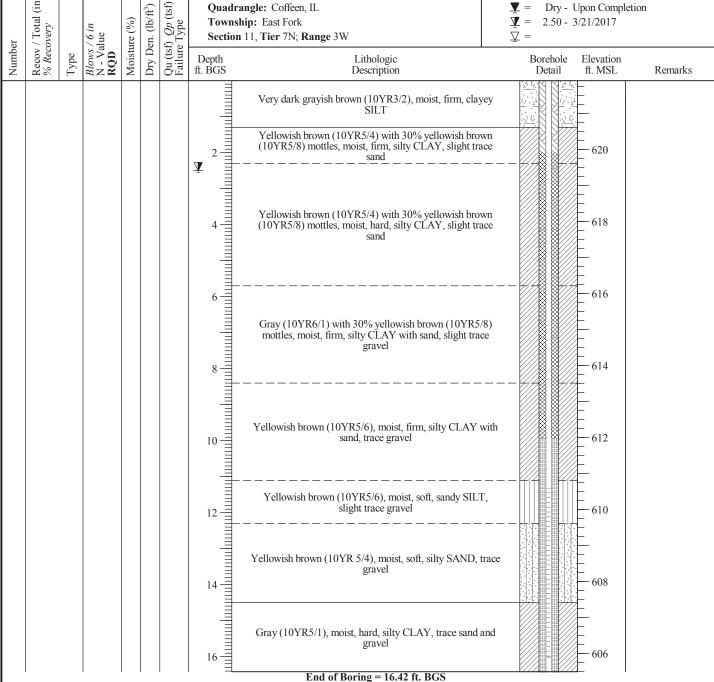
Completion: 16.42 ft. BGS

Station: 875,548.77N

2,515,910.12E

▼ = Dry - Upon Completion

 Ψ = 2.50 - 3/21/2017



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 2/21/2008 **Finish:** 2/21/2008

WEATHER: Overcast, cold

CONTRACTOR: Testing Service Corp.
Rig mfg/model: CME-650 Track Drill

Drilling Method: 31/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson Helper: R. Keedy

Eng/Geo: .

BOREHOLE ID: G205

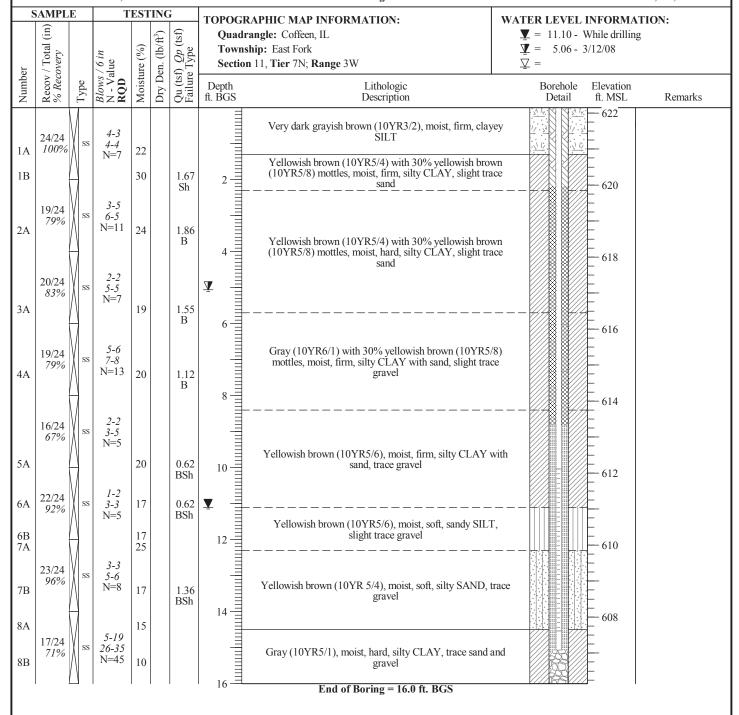
Well ID: G205

 Surface Elev:
 622.15 ft. MSL

 Completion:
 16.00 ft. BGS

 Station:
 875,550.19N

 2,515,914.87E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 2/10/2010 **Finish:** 2/10/2010

WEATHER: Overcast, wind 15mph, ~10-20F

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 41/4" HSA w/SS samplers

FIELD STAFF: Driller: T. List

Helper: M. Herbst/S. Hamby

Eng/Geo: .

HANSON

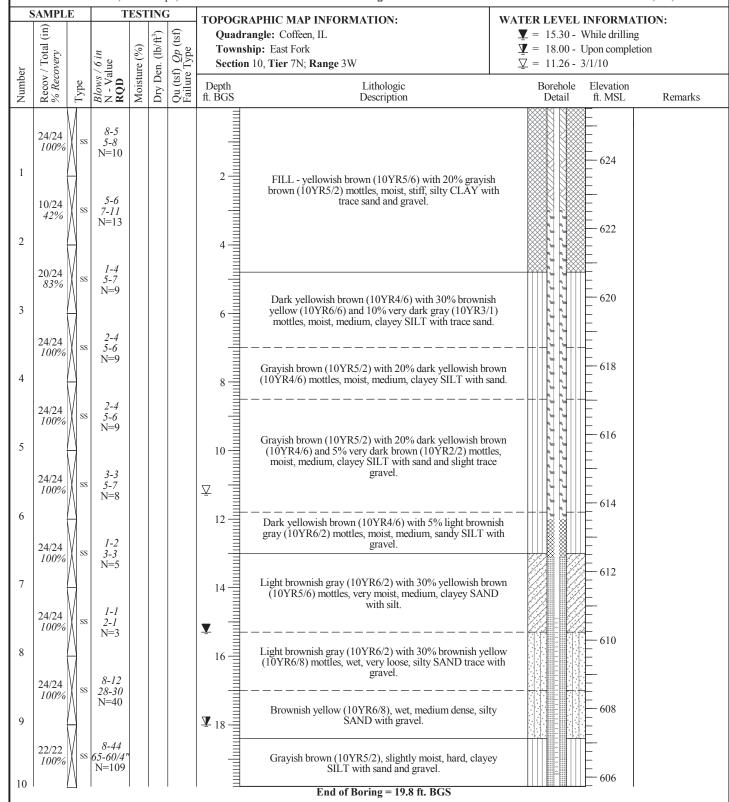
BOREHOLE ID: T127 Well ID: T127

 Surface Elev:
 625.53 ft. MSL

 Completion:
 19.80 ft. BGS

 Station:
 875,359.21N

 2.513,911.02E



CLIENT: AEG Coffeen Power Station Site: CCB Management Facility

Location: Coffeen, Illinois

Project:

DATES: Start: 2/9/2010

Finish: 2/9/2010 WEATHER: Cold, windy, snow (lo-20's) FIELD STAFF: Driller: T. List

CONTRACTOR: Layne-Western Co

Rig mfg/model: CME-750 ATV Drill

Drilling Method: 41/4" HSA w/SS samplers

Helper: M. Herbst/S. Hamby

Eng/Geo: S. Simpson

HANSON

BOREHOLE ID: T128

Well ID: T128

Surface Elev: 626.27 ft. MSL Completion: 22.00 ft. BGS

Station: 875,509.70N 2,513,909.45E

_ \$	SAMPL	E	T	EST	ING	i	TOPOGRA	PHIC MAP INFORMATION:	WATER LEVEL INFORMATION:				
ıe	Recov / Total (in) % Recovery		/ 6 in Iue	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) Failure Type	Quadra Townsh	ngle: Coffeen, IL ip: East Fork 10, Tier 7N; Range 3W	$ \begin{array}{c} \mathbf{V} = 16.84 - \mathbf{V} \\ \mathbf{V} = \\ \mathbf{V} = 12.35 - 3 \end{array} $	While drilling	O11.		
Number	Recov % Rec	Type	Blows / 6 in N - Value RQD	Moist	Dry D	Qu (tsː Failur	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks		
1A	12/24 50%	SS	37-17 15-18 N=32	14			2	FILL - Brown (10YR5/3) with 5% gray (10YR5/1) and yellowish brown (10YR5/6) mottles, moist, hard (froze silty CLAY with trace sand and slight trace gravel. FILL - Gray (10YR5/1) with 15% yellowish brown (10YR5/6) mottles, moist, hard (frozen), silty CLAY we slight trace sand and gravel.	en), 	626			
2A	7/24 29%	SS	4-7 7-13 N=14				4-	FILL - Brown (10YR5/3) with 10% gray (10YR5/1) a 5% yellowish brown (10YR5/6) mottles, moist, stiff, s CLAY with slight trace sand and gravel.	nd ilty	624			
3A	12/24 50%	SS	3-4 6-8 N=10	26			6	Gray (10YR5/1) with 20% yellowish brown (10YR5/mottles, moist, stiff, silty CLAY with slight trace san slight trace roots.	6) d,	622			
4A	19/24 79%	SS	2-4 5-7 N=9	24						620			
5A	22/24 92%	SS	1-3 4-5 N=7	20			8 = 10 = 10 = 1	Gray (10YR5/1) with 20% yellowish brown (10YR5/mottles, moist, stiff, silty CLAY with trace sand and slitrace gravel.	6) ght	618			
6A	22/24 92%	SS	1-3 5-5 N=8	20			12		7.7.	616			
7A	22/24 92%	SS	2-3 4-5 N=7	19						614			
8A	20/24 83%	SS	2-2 3-3 N=5	18			14	Gray (10YR6/1) with 30% yellowish brown (10YR5/mottles, moist, medium, silty CLAY with trace sand a slight trace gravel.		612			
9A 9B	22/24 92%	SS	woh-1 8-7 N=9	19 18			16 	Brown (10YR5/3), moist, soft, clayey SAND with slig trace gravel. Gray (10YR6/1), wet, loose, silty, very fine- to coarse-grained SAND.	cht	610			
10A 10B	18/24	SS	3-10 13-11	17			18	Brown (10YR5/3), wet, loose, silty, very fine- to fine-grained SAND.		608			
	75%	85	N=23	- /			20	Brown (10YR5/3), slightly moist, very stiff, clayey SI with trace sand and gravel.					

CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility

Location: Coffeen, Illinois

Project:

DATES: Start: 2/9/2010

Finish: 2/9/2010

FIELD STAFF: Driller: T. List

CONTRACTOR: Layne-Western Co **Rig mfg/model:** CME-750 ATV Drill

Drilling Method: 4¹/₄" HSA w/SS samplers

Helper: M. Herbst/S. Hamby

HANSON

BOREHOLE ID: T128

Well ID: T128 Surface Elev: 626.27 ft. MSL

Completion: 22.00 ft. BGS Station: 875,509.70N 2,513,909.45E

	WE	EATHEI	R: C	old, wind	y, sn	ow (lo-20's)	Eng/Geo: S. Simpson	2,513,909.45E
	S	SAMPL	E	Т	EST	ING	;	TOPOGRA	PHIC MAP INFORMATION:	WATER LEVEL INFORMATION:
1		(in)				ft³)		_	agle: Coffeen, IL	$\mathbf{\underline{V}} = 16.84$ - While drilling
	ıe	/ Total overy		/ 6 in alue	ure (%)	en. (lb/ft³)	f) e Type		p: East Fork 0, Tier 7N; Range 3W	$\underline{\Psi} = $ $\underline{\nabla} = 12.35 - 3/1/10$
	Number	Recov % Rec	Type	Blows N - Va RQD	Moisture	Dry Den.	Qu (tsf) Failure	Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL Remarks
	11A	24/24 100%	ss	8-16 18-25 N=34	16			22	Dark gray (10YR4/1), slightly moist, hard, clayey SI with trace sand and gravel.	LT
-1								44	End of Boring = 22.0 ft. BGS	

CLIENT: AEG Coffeen Power Station **Site:** CCB Management Facility

Location: Coffeen, Illinois **Project:** 05S3004A

DATES: Start: 10/15/2010

Finish: 10/15/2010 **WEATHER:** Sunny (mid-50's)

CONTRACTOR: Layne-Western Co Rig mfg/model: CME-750 ATV Drill Drilling Method: 4¼" HSA w/SS samplers

FIELD STAFF: Driller: D. Mahurin Helper: J. Litsch/D. Smail

Eng/Geo: R. Hasenyager



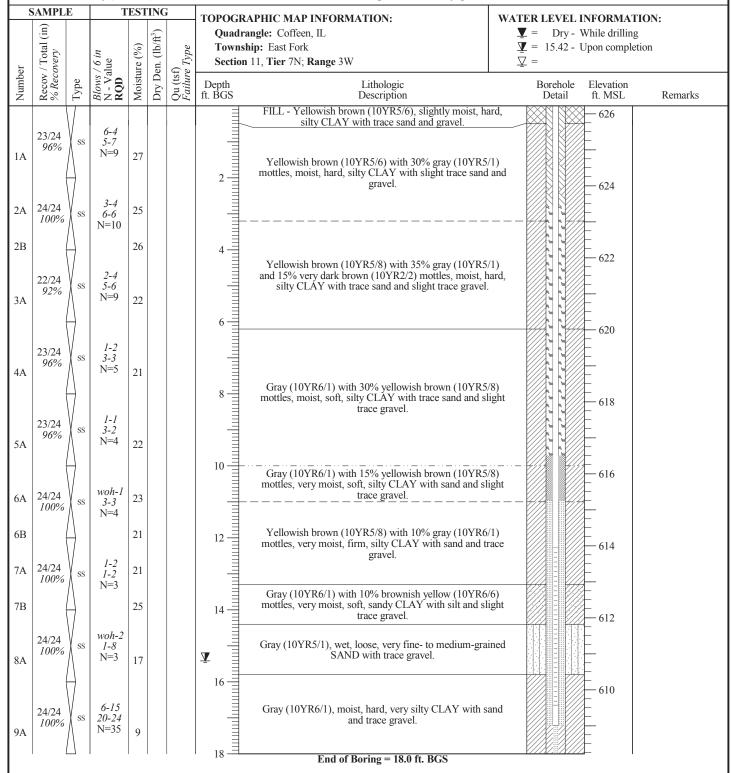
BOREHOLE ID: T202 **Well ID:** T202

 Surface Elev:
 626.22 ft. MSL

 Completion:
 18.00 ft. BGS

 Station:
 876,699.42N

 2,514,895.01E



CLIENT: Natural Resources Technology, Inc. Site: Coffeen Power Station - Ash Pond 2 Location: 134 CIPS Lane, Coffeen, IL 62017

Project: 16E0080

DATES: Start: 8/17/2016 Finish: 8/17/2016

WEATHER: Hazy, (low-80s)

CONTRACTOR: Bulldog Drilling, Inc. Rig mfg/model: CME-750 ATV Drill Drilling Method: 4 1/4" Hollow Stem Auger

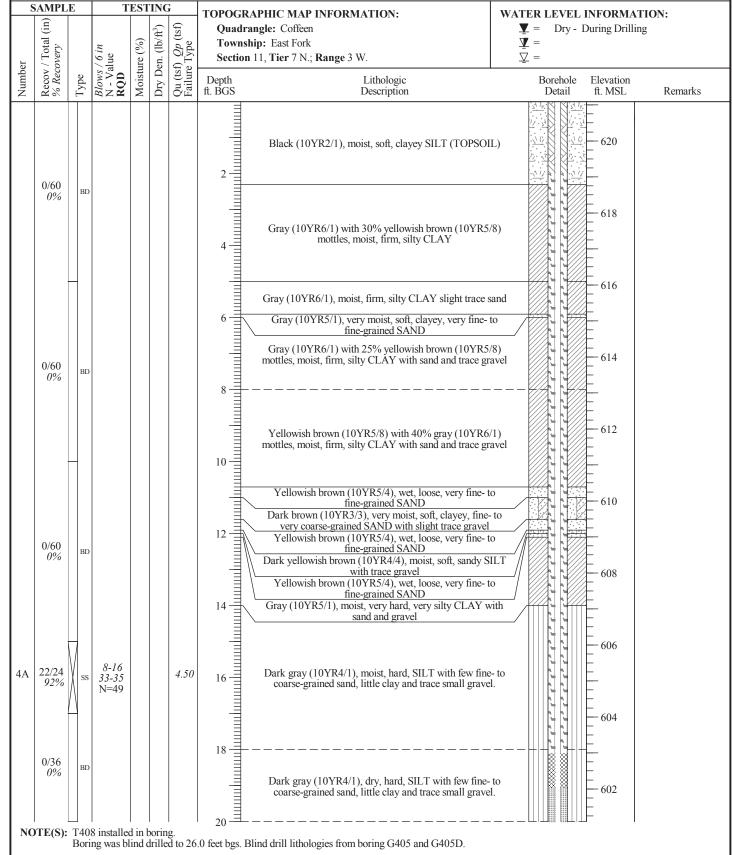
FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill

Eng/Geo: K. Theesfeld



BOREHOLE ID: T408 Well ID: T408

> Surface Elev: 621.09 ft. MSL Completion: 25.92 ft. BGS **Station:** 2,515,314.91N 2,515,314.91E



CLIENT: Natural Resources Technology, Inc. Site: Coffeen Power Station - Ash Pond 2 Location: 134 CIPS Lane, Coffeen, IL 62017

Project: 16E0080

DATES: Start: 8/17/2016 Finish: 8/17/2016

WEATHER: Hazy, (low-80s)

CONTRACTOR: Bulldog Drilling, Inc. Rig mfg/model: CME-750 ATV Drill Drilling Method: 4 1/4" Hollow Stem Auger

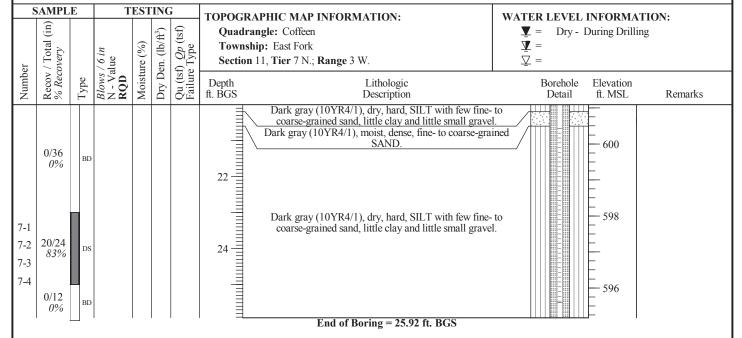
FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill

Eng/Geo: K. Theesfeld



BOREHOLE ID: T408 Well ID: T408

> Surface Elev: 621.09 ft. MSL Completion: 25.92 ft. BGS **Station:** 2,515,314.91N 2,515,314.91E



CLIENT: Natural Resources Technology, Inc.
Site: Coffeen Power Station - Ash Pond 2
Location: 134 CIPS Lane, Coffeen, IL 62017

Project: 16E0080

DATES: Start: 8/19/2016 **Finish:** 8/19/2016

WEATHER: Cloudy, (70s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" Hollow Stem Auger

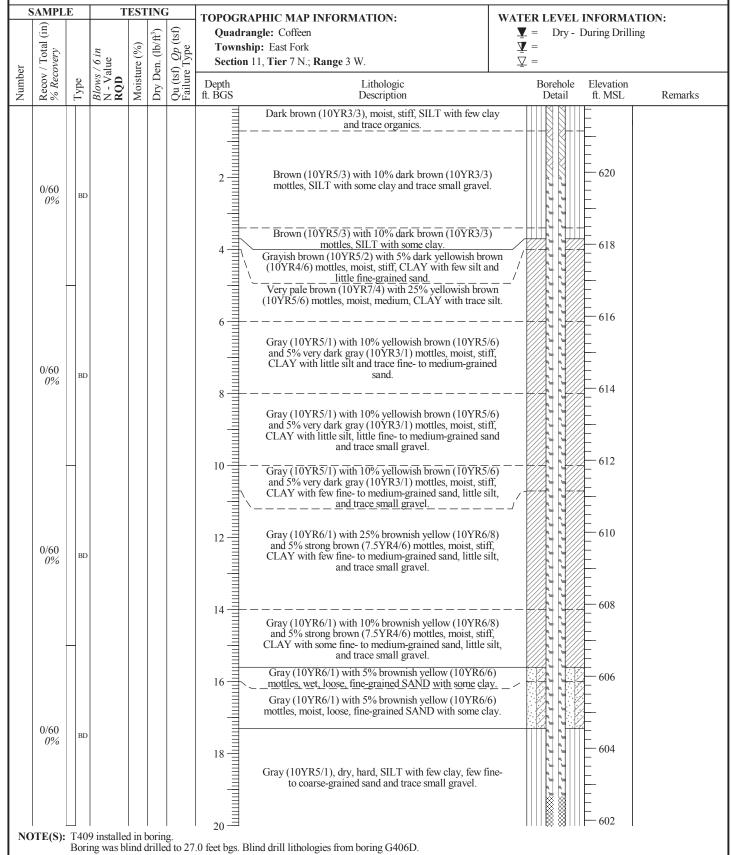
FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill

Eng/Geo: K. Theesfeld



BOREHOLE ID: T409 **Well ID:** T409

Surface Elev: 621.85 ft. MSL Completion: 26.99 ft. BGS Station: 2,514,693.89N 2,514,693.89E



CLIENT: Natural Resources Technology, Inc. Site: Coffeen Power Station - Ash Pond 2 Location: 134 CIPS Lane, Coffeen, IL 62017

Project: 16E0080

DATES: Start: 8/19/2016 Finish: 8/19/2016

WEATHER: Cloudy, (70s)

CONTRACTOR: Bulldog Drilling, Inc. Rig mfg/model: CME-750 ATV Drill Drilling Method: 4 1/4" Hollow Stem Auger

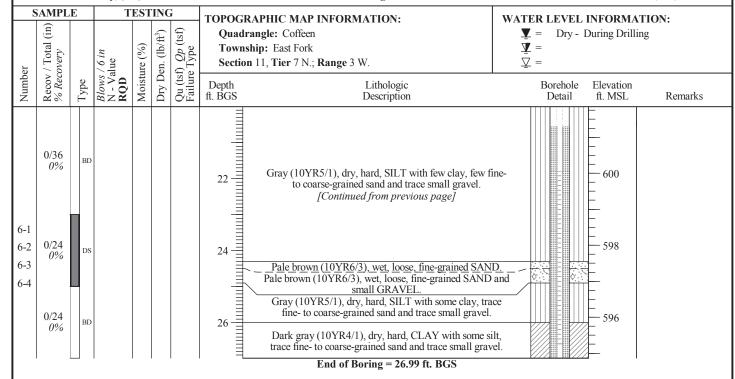
FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill

Eng/Geo: K. Theesfeld



BOREHOLE ID: T409 Well ID: T409

> Surface Elev: 621.85 ft. MSL Completion: 26.99 ft. BGS **Station:** 2,514,693.89N 2,514,693.89E



CLIENT: Illinois Power Holdings **Site:** Coffeen Power Station

Location: Coffeen, Montgomery County, Illinois

NOTE(S): Monitoring well TA31 installed in borehole.

Project: 14E0078

DATES: Start: 10/28/2014 **Finish:** 10/28/2014

WEATHER: Overcast, mild - mid 50's

CONTRACTOR: Ramsey

Rig mfg/model: D-50 Turbo Tracked MST 800ATV

Drilling Method: 41/4" Hollow Stem Auger with Split spoon

FIELD STAFF: Driller: B. Williamson Helper: D. Crump

Eng/Geo: R. Hasenyager

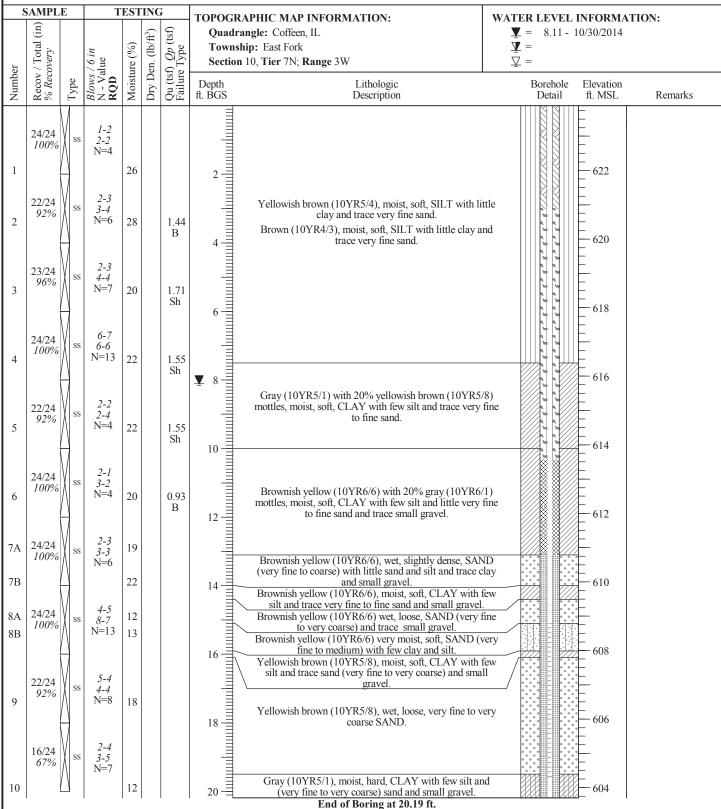
BOREHOLE ID: TA31

Well ID: TA31

Surface Elev: 623.89 ft. MSL **Completion:** 20.19 ft. BGS

Station: 876,542.25N 2,513,856.77E

Page 1 of 1



CLIENT: Illinois Power Holdings Site: Coffeen Power Station

Location: Coffeen, Montgomery County, Illinois

TESTING

Project: 14E0078

SAMPLE

DATES: Start: 10/27/2014 Finish: 10/27/2014

WEATHER: Partly sunny, mild - mid 70's

CONTRACTOR: Ramsey

TOPOGRAPHIC MAP INFORMATION:

Rig mfg/model: D-50 Turbo Tracked MST 800ATV

Drilling Method: 41/4" Hollow Stem Auger with Split spoon

FIELD STAFF: Driller: B. Williamson Helper: D. Crump

Eng/Geo: R. Hasenyager

WATER LEVEL INFORMATION:

BOREHOLE ID: TA32

Well ID: TA32

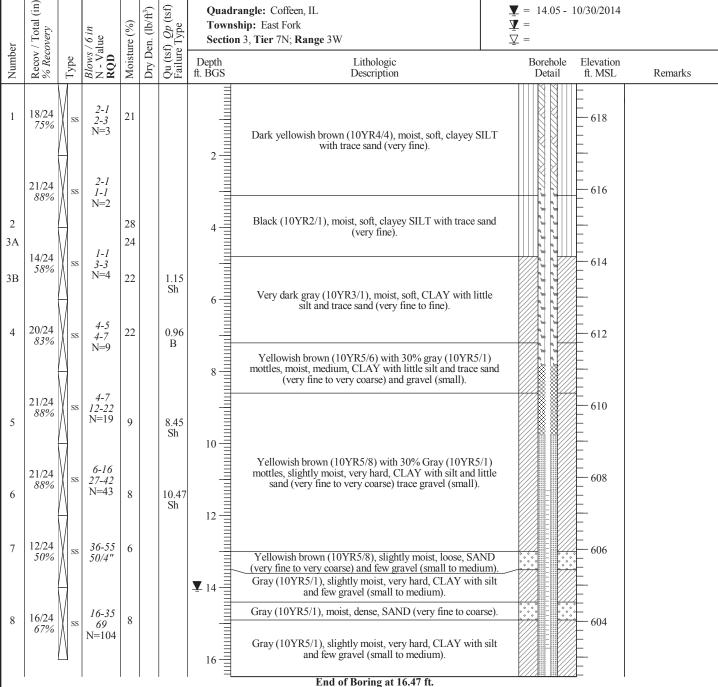
Surface Elev: 618.93 ft. MSL

Completion: 16.47 ft. BGS

Station: 877,532.57N

2,513,605.19E

 $\mathbf{V} = 14.05 - 10/30/2014$



CLIENT: Illinois Power Generating Company

Site: Coffeen Well Sealing & Assmt Well Install

Location: Coffeen, Illinois Project: 14E0078A

DATES: Start: 6/2/2015 Finish: 6/2/2015 WEATHER: Sunny, calm, hi-60s CONTRACTOR: Ramsey
Rig mfg/model: Diedrich D-50
Drilling Method: 3 ¼" HSA, blind drill

FIELD STAFF: Driller: B. Williamson

Helper: D. Crump Eng/Geo: S. Keim



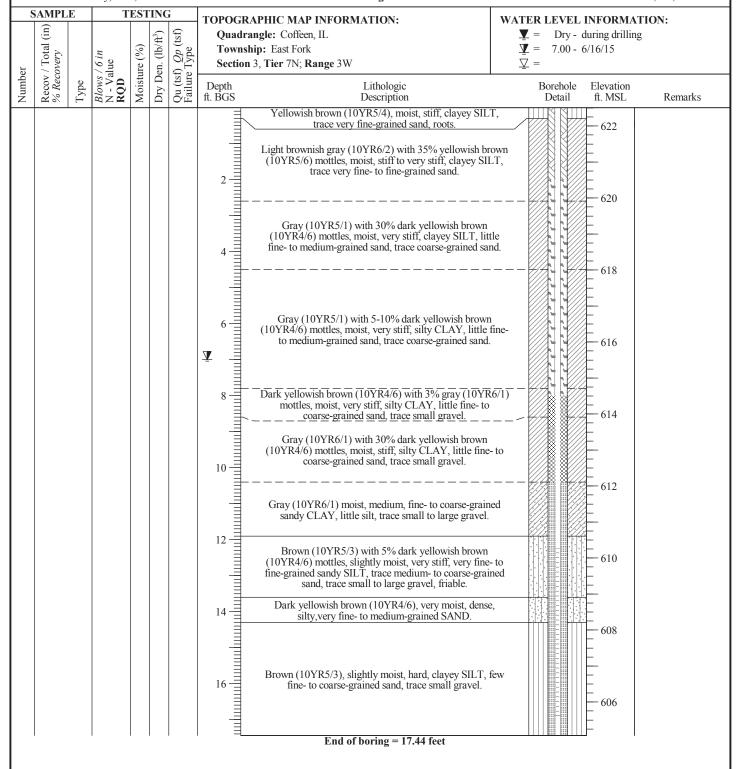
BOREHOLE ID: TA33b Well ID: TA33

 Surface Elev:
 622.51 ft. MSL

 Completion:
 17.44 ft. BGS

 Station:
 876,605.45N

 2.513,248.73E



CLIENT: Illinois Power Generating Company

Site: Coffeen Well Sealing & Assmt Well Install

Location: Coffeen, Illinois **Project:** 14E0078A

DATES: Start: 6/2/2015 **Finish:** 6/2/2015

WEATHER: Sunny, calm, hi-60s

CONTRACTOR: Ramsey **Rig mfg/model:** Diedrich D-50

Drilling Method: 3 1/4" HSA, split spoon sampler

FIELD STAFF: Driller: B. Williamson Helper: D. Crump

Eng/Geo: S. Keim

BOREHOLE ID: TA33a

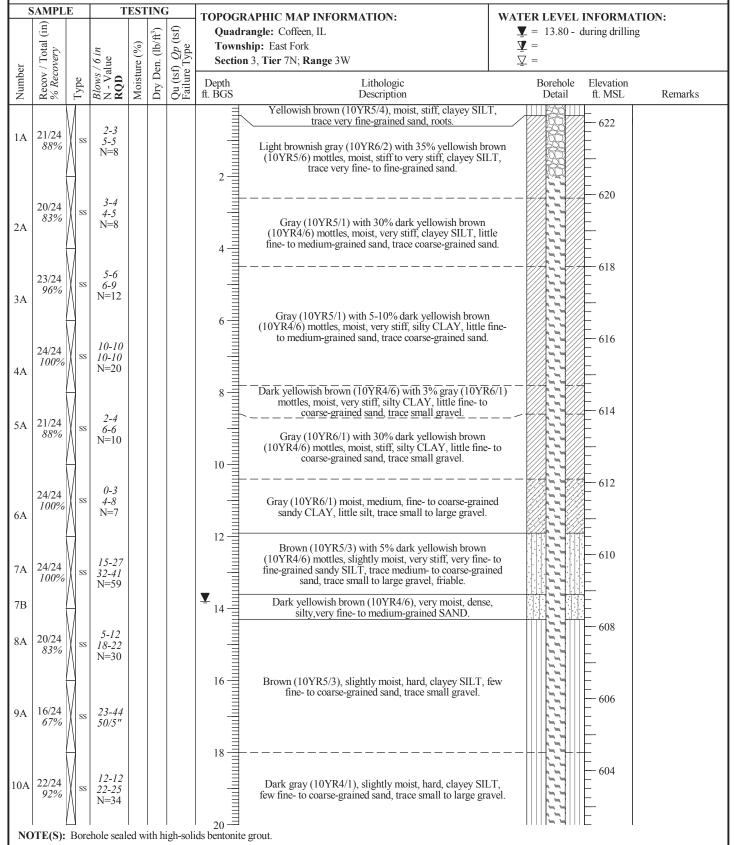
Well ID: n/a

 Surface Elev:
 622.5 ft. MSL

 Completion:
 30.0 ft. BGS

 Station:
 876,610.00N

 2.513,248.00E



CLIENT: Illinois Power Generating Company

Site: Coffeen Well Sealing & Assmt Well Install

Location: Coffeen, Illinois Project: 14E0078A

DATES: Start: 6/2/2015 **Finish:** 6/2/2015

WEATHER: Sunny, calm, hi-60s

CONTRACTOR: Ramsey **Rig mfg/model:** Diedrich D-50

Drilling Method: 3 ¹/₄" HSA, split spoon sampler

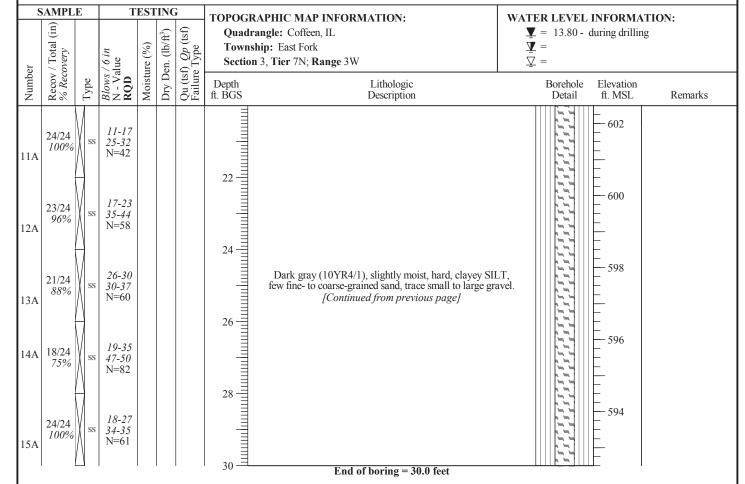
FIELD STAFF: Driller: B. Williamson Helper: D. Crump

Eng/Geo: S. Keim

BOREHOLE ID: TA33a
Well ID: n/a

Surface Elev: 622.5 ft. MSL Completion: 30.0 ft. BGS Station: 876,610.00N 2,513,248.00E

HANSON



CLIENT: Illinois Power Generating Company

Site: Coffeen Well Sealing & Assmt Well Install

Location: Coffeen, Illinois Project: 14E0078A DATES: Start: 6/3/2015

Finish: 6/3/2015 **WEATHER:** Mostly cloudy, windy, mid-60s

CONTRACTOR: Ramsey **Rig mfg/model:** Diedrich D-50

Drilling Method: 3 1/4" HSA, split spoon sampler

FIELD STAFF: Driller: B. Williamson Helper: D. Crump

Eng/Geo: S. Keim

HANSON

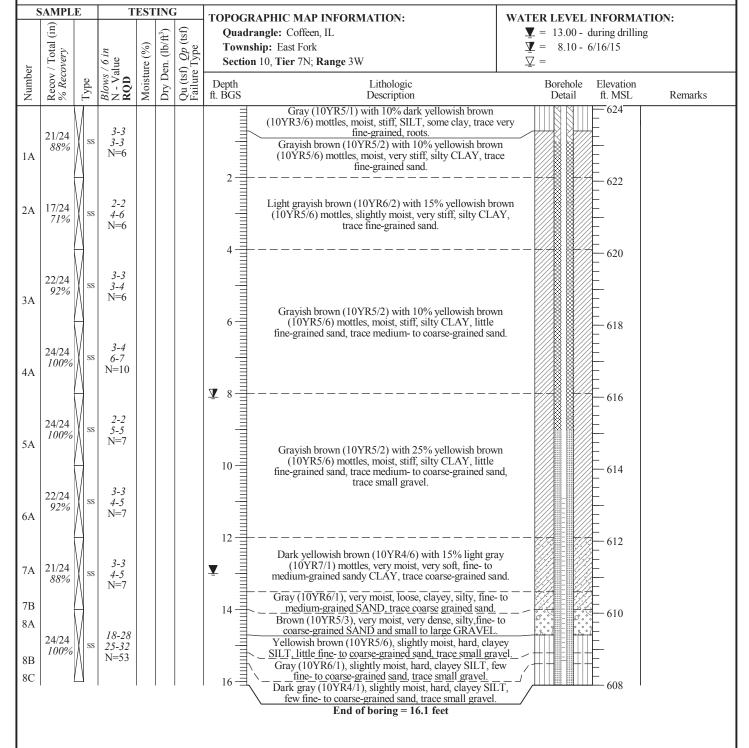
BOREHOLE ID: TA34 **Well ID:** TA34

 Surface Elev:
 624.10 ft. MSL

 Completion:
 16.10 ft. BGS

 Station:
 875,906.10N

 2.513,466.73E



Illinois Enviro	onmental Protection	n Agency		Well (Completion	Report
Site #:	Count	y:			Well #:T	R32
Site Name: Coffeen Power S	Station			E	Borehole #:	TR32
State Plan&oordinate: X_2,513,605	5.0 Y <u>877,523.7</u> (or)	Latitude: 39	9° 4' 26	663.000"Longitu	de: 89° 24	<u>4'</u> <u>8.070"</u>
Surveyed By: Kyle J. Nolan		IL R	egistration #: _	035-003919		
Drilling Contractor: Ramsey	Geotechnical Engineering, 1	LLC Drill	ler: <u>B. Willi</u> a	amson		
Consulting Firm: Hanson Pr	ofessional Services Inc.	Geo	logist: <u>Rhon</u>	ald W. Hasenya	nger, LPG #196-0	000246
Drilling Method: Hollow Ste	m Auger	Drill	ling Fluid (Type	e): <u>none</u>		
Logged By: Rhonald W. Has	enyager	Date	e Started:7	<u>/2/2019</u> Da	te Finished:7/	2/2019
Report Form Completed By:	Rhonald W. Hasenyager	Date	e: <u>7/3/2</u>	019		
ANNULAR SPA	CE DETAILS		Elevat (MSI	ions Depths	(0.01 ft.)
			621.9	, ,	Top of Protectiv	re Casing
			621.0	<u>-2.40</u>	Top of Riser Pip	е
Type of Surface Seal: <u>Concrete</u>	·		= 	28 0.00	Ground Surface	
Type of Annular Sealant: Bent	tonite		616.2	28 3.00	Top of Annular	Sealant
Installation Method: Grav			/			
Setting Time: 30 min.					Static Water Le	⁄el
betting Time:					(After Completion	
Type of Bentonite Seal Gran	ular Pellet Slurry					
Installation Method:			n/a	n/a	Top of Seal	
Setting Time:			609.	77 9.51	Top of Sand Pac	k
m 60 lb l					- 1	
Type of Sand Pack: Quartz Sa			608.2	28 11.00	Top of Screen	
Grain Size: 10/20 (sid	ŕ				- 1	
Installation Method: <u>Grav</u>	ity		603.6	50 15.68	Bottom of Scree	n
Type of Backfill Material:n/a	(if applicable)		603.		-	
Installation Method:			603.3	11 16.17	Bottom of Borel	nole
				nced to a National Geo	-	1010
				CASING MEA	SUREMENTS	
			Diameter o	f Borehole	(inches)	8.0
	TRUCTION MATERIALS type of material for each area)		ID of Riser	Pipe	(inches)	2.0
			Protective (Casing Length	(feet)	5.0
Duoto chives Coming	CC204 CC247 DWDD DVG	OTHER C. 1	Riser Pipe I		(feet)	13.40
Protective Casing Riser Pipe Above W.T.	SS304 SS316 PTFE PVC SS304 SS316 PTFE PVC	OTHER: (Steel		creen to End Cap		0.49
Riser Pipe Below W.T.		OTHER:		gth (1st slot to last h of Casing		4.68 18.57
F			_ rotal Leligt	ii oi casiiig	(feet)	10.0/

PTFE PVC OTHER:

SS304

Well Completion Form (revised 02/06/02)

SS316

Total Length of Casing

**Hand-Slotted Well Screens Are Unacceptable

Screen Slot Size **

0.010

(inches)

 $\begin{tabular}{ll} \textbf{CLIENT:} & \textbf{Illinois Power Generating Co.} \\ \end{tabular}$

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 1/28/2021 **Finish:** 2/3/2021

WEATHER: Clear, cold (20s)

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter



BOREHOLE ID: G275D

Well ID: G275

 Surface Elev:
 617.52 ft. MSL

 Completion:
 99.70 ft. BGS

 Station:
 874,285.30N

2,516,366.50E

Ť	SAMPLE				ING		TOPOGRA	APHIC MAP INFORMATION:	WATER LEVEL INFORMATION:			
	Recov / Total (in) % Recovery			Water Content (%)	Dry Density (lb/ft ³	, Qp (tsf) Type	Quadra	angle: Coffeen, IL	$\mathbf{\underline{\nabla}}$ = 10.90 - During Drilling			
	otal 7		2	ent	 	p (t	Towns	hip: East Fork Township	<u>Ā</u> =			
_	/Tc		, 6 <i>i</i>	ont	nsit	ωĘ	Section	n 11, Tier 7N; Range 3W	<u>_</u> =			
ge) o o	Φ	Blows / 6 in N - Value RQD	or O	Del	Qu (tsf) Failure T	Danth	-	5			
Number	Sec % F	Type	3628	Nat	7	Su (Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL	Remarks		
_		i	~~-		_				11/14/21/21/14			
	1	1					2	Dayle valley righ brown (40VD4/4) maint stiff loop CLAV				
1A	21/24	/	3-4 5-10				<u> </u>	Dark yellowish brown (10YR4/4), moist, stiff, lean CLAY, v some silt, few very fine- to fine-grained sand and small gra				
IA	88%	ss	N=9				∃	trace roots. [FILL]				
	/	1							616			
	+	4					2=					
	١	1										
	22/24	/	3-4					Gray (10YR5/1) with 15% yellowish brown (10YR5/6) moti moist, very stiff, lean CLAY, with some silt, few very fine-				
2A	92%	ss	6-9 N=10					fine-grained sand, trace small gravel.				
	/	\	14-10						614			
	1	_										
	١	1					. ∃					
	19/24	/	2-4				8 10					
3A	79%	ss	6-8				📑	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) moti				
			N=10					moist, very stiff, lean CLAY, with some silt, few very fine- fine-grained sand, trace small gravel.	612			
	1						6-	5, g g.				
	1	1					"∃					
		/	2-5				=					
4A	23/24 96%	ss	5-6				-					
	3076	'\	N=10					O (40)/DE(4) 'II 000/ II 'I I (40)/DE(0) I	5% 610			
	1	1						Gray (10YR5/1) with 20% yellowish brown (10YR5/6) and yellowish red (5YR4/6) mottles, moist, very stiff, lean CL/4	570 P///XII I: P///X			
	Ţ	7					8 =	with some silt, few very fine- to fine-grained sand, trace si				
	1	/	2.2					gravel.				
5A	24/24	ss	2-3 5-6]					
	100%	/	N=8									
5B	1	1					=	Dark yellowish brown (10YR3/6), moist, stiff, SILT, with so				
6A	t	1					10 =	very fine- to medium-grained sand, few clay and small gra Gray (10YR6/1) with 10% dark yellowish brown (10YR3/				
	1	/						mottles, moist, stiff, lean CLAY, with some silt, little very fi				
	24/24	ss	0-1 2-2				₹ ‡	to fine-grained sand, few small gravel				
CD	100%	\mathbb{N}	N=3				12 -					
6B	/	1					∃	Dark yellowish brown (10YR3/6), wet, loose, SILT, with so	ome			
7A							12 =	very fine- to fine-grained sand, few clay and small grave				
	18/24	SH										
	75%							Gray (10YR6/1) with 20% yellowish brown (10YR5/6) moti	tles,			
								moist, hard, SILT, with some very fine- to fine-grained sa few clay and small gravel.	ind,			
	7	7					14 🖶					
ва	14/14	/ ss	26-43									
	100%	\	50/2"					Grayish brown (10YR5/2) with 10% yellowish brown (10YR	R5/6)			
	L							mottles, moist, hard, SILT, with some very fine- to fine-gra	nined 🌂 🌂 _			
								sand, few clay and small gravel.				
	Γ	7					16					
	١											
_	24/24		5-12									
9A	100%	ss	18-22 N=30]					
		\	00									
	ŀ	-					18 🗏	Brown (10YR5/3) with 10% yellowish brown (10YR5/6) more				
	١							moist, hard, SILT, with some clay and very fine- to fine-gra sand, few small gravel.				
	24/24	$\parallel \parallel$	4-11					Sana, 1500 Sinali gravel.				
I0A	100%	ss	13-20 N=24				📑					
			IN-24						598			
	1	V					20 ∄					

 $\begin{tabular}{ll} \textbf{CLIENT:} & \textbf{Illinois Power Generating Co.} \\ \end{tabular}$

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 1/28/2021 **Finish:** 2/3/2021

WEATHER: Clear, cold (20s)

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig **Drilling Method:** 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt
Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

BOREHOLE ID: G275D

Well ID: G275

 Surface Elev:
 617.52 ft. MSL

 Completion:
 99.70 ft. BGS

 Station:
 874,285.30N

2,516,366.50E

,	SAMPLE TESTING					i	TOPOGRA	PHIC MAP INFORMATION:	WATE	RIEVELIN	FORMATIO	N•			
er	Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Township Section 11, Tier 7N; Range 3W			$\underline{\underline{Y}}$ = 10.90 - During Drilling $\underline{\underline{Y}}$ = $\underline{\underline{Y}}$ =					
Number	Reco	Туре	Blows N - V	Water	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description		Borehole Detail	Elevation ft. MSL	Remarks			
11A 12A	24/24 100%	ss	3-9 13-22 N=22				22	Brown (10YR5/3) with 10% yellowish brown (10YR5/6) moist, hard, SILT, with some clay and very fine- to fine-grand, few small gravel. [Continued from previous page]	ottles, ained	(, (, (, (, (, (, (, (, (, (, (, (, (, (596	Vertical fractures with			
	24/24 100%	ss	7-14 20-24 N=34				24			,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	594	oxidation from 22 to 24 ft, no oxidation below 24 ft.			
13A	24/24 100%	ss	6-11 15-21 N=26				26				592	Occasional thin SILT and SAND lenses from 25.3 to 25.8 ft.			
14A	18/24 75%	ss	4-8 12-10 N=20				28			(, (, (, (, (, (, (, (, (, (, (, (, (, (590	Trace wood			
15A	24/24 100%	ss	5-7 13-17 N=20				30			(, (, (, (, (, (, (, (, (, (, (, (, (, (588	fragments below 28 ft.			
16A 17A	23/24 96%	ss	4-7 12-16 N=19				32 —	Dark gray (10YR4/1) with frequent dark yellowish brow (10YR3/6) oxidation along fractures, moist, hard, lean Cl with some silt, few very fine- to fine-grained sand and sr gravel.	_AY,		586				
18A	2/24 8%	ss	4-10 13-17 N=23							3,	584				
	21/24 88%	SH					34 ====================================			,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	582				
19A 20A	24/24 100%	ss	3-6 10-14 N=16				38				580				
	4/24 17%	ss	3-8 11-17 N=19								578				
NO	TE(S):] G275	installed	l in a	 idjac	ent blir	40 ∄ nd drill boreh	ole.			#				

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

TESTING

Location: Coffeen, Illinois Project: 20E0111A

SAMPLE

DATES: Start: 1/28/2021 Finish: 2/3/2021

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig

WEATHER: Clear, cold (20s) Eng/Geo: C. Colin Winter

> TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION:

Quadrangle: Coffeen, IL

BOREHOLE ID: G275D

Station:

Completion:

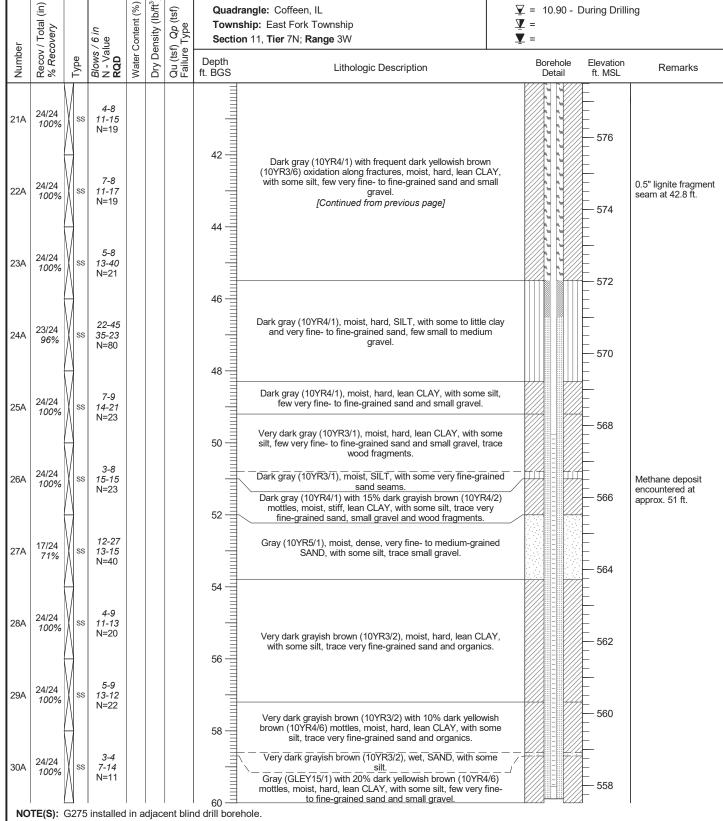
Well ID: G275

Surface Elev: 617.52 ft. MSL

99.70 ft. BGS

874,285.30N

2.516.366.50E



CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 1/28/2021

Finish: 2/3/2021 WEATHER: Clear, cold (20s)

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt
Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

BOREHOLE ID: G275D

Well ID: G275

 Surface Elev:
 617.52 ft. MSL

 Completion:
 99.70 ft. BGS

 Station:
 874,285.30N

2,516,366.50E

5	SAMPLE						TOPOGRA	WATER LEVEL INFORMATION:						
er	Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD Water Content (%) Dry Density (lb/ft³ Qu (tsf) Qp (tsf) Failure Type		if) Qp (tsf) e Type	TOPOGRAPHIC MAP INFORMATION: Quadrangle: Coffeen, IL Township: East Fork Township Section 11, Tier 7N; Range 3W			▼ = 10.90 - During Drilling ▼ =					
Number	Recov % Re	Type	Blows N - Va RQD	Water	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description		Boreho Detai		Elevation ft. MSL	Remarks	
31A	24/24 100%	ss	<i>0-4</i> <i>5-7</i> N=9				62	Gray (GLEY15/) with 30% dark yellowish brown (10YR4 mottles, moist, hard, lean CLAY, with some silt, few very to fine-grained sand and small gravel.	/6) fine-					
32A 33A	24/24 100%	SS	4-6 8-11 N=14				62 62 64 66 68 68 70 72 72 72 72 73 74 75 75 75 75 75 75 75							
	24/24 100%	SH					66	Greenish gray (GLEY15/1) with 15% very dark gray (10Yf mottles, moist, hard, lean CLAY, with some silt, few very to fine-grained sand and small gravel.	R3/1) fine-					
34A	24/24 100%	ss	5-10 22-41 N=32				68	. — — — — — — — — — — — — — — — — — — —	- — —					
35A	24/24 100%	ss	12-24 33-45 N=57				70 =	Yellowish brown (10YR5/4) with occasional thin greenish	gray				Trace medium grav	
36A	23/24 96%	ss	6-14 25-30 N=39				72	(GLEY15/1) seams, moist, hard, lean CLAY, with some few small gravel, trace very fine-grained sand.	silt,				at 70 ft.	
37A	24/24 100%	ss	8-18 24-32 N=42											
38A	24/24 100%	ss	7-16 25-29 N=41				74	Yellowish brown (10YR5/4) with 15% gray (10YR6/1) mot	tles					
39A	24/24 100%	ss	7-15 20-21 N=35				76 -	moist, hard, lean CLAY, with some silt, few small gravel, t very fine-grained sand.				 540		
40A	19/24 79%	ss	3-5 7-10 N=12				78	Greenish gray (GLEY15/1) with 5% yellowish brown (10Yf mottles, moist, stiff, lean CLAY, with some silt, trace ve				 538		

 $\begin{tabular}{ll} \textbf{CLIENT:} & \textbf{Illinois Power Generating Co.} \\ \end{tabular}$

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 1/28/2021 **Finish:** 2/3/2021

WEATHER: Clear, cold (20s)

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter



BOREHOLE ID: G275D

Well ID: G275 Surface Elev: 617.52 ft. MSL

 Surface Elev:
 617.52 π. MSL

 Completion:
 99.70 ft. BGS

 Station:
 874,285.30N

2,516,366.50E

;	SAMPLE		1	EST	ING		TOPOGRAPHIC MAP INFORMATION:	WATER LEVEL INFORMATION:	
	<u>E</u>			(%))/ft³)	(jg	Quadrangle: Coffeen, IL	$\nabla = 10.90$ - During Drilling	
	tal (jnt (e) Qp (tsf) Type	Township: East Fork Township	<u>V</u> =	
.	Tol Ver		6 ir. Ie	onte	sity	ğδ	Section 11, Tier 7N; Range 3W	▼ =	
per	000	a	/s/ /alu /	ŭ)en	tsf)		<u>-</u>	
Numper	Recov / Total (in) % Recovery	Туре	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft³	Qu (tsf) Failure T	Depth ft. BGS Lithologic Description	Detail It. MSL	marks
	1						fine-grained sa Greenish gray (GLEY15/1) with 5% y	Y///1 Y/// - I	
	22/24	VI	1-5				mottles, moist, stiff, lean CLAY, w	th some silt, trace very	
-1A	92%	ss	7-11				fine-grained sa		
		/\l	N=12				[Continued from prev		
	1	1					Very dark gray (10YR3/1), moist, sti	II, lean OLAT, with some	
	Ţ						Greenish gray (GLEY15/1) with 5% y		
	1	M.	444				82 silt, trace very fine gra Greenish gray (GLEY15/1) with 5% y mottles, moist, stiff, lean CLAY, w		
2A	24/24	ss	4-14 19-20				fine-grained sa	and.	
	100%	۸I	N=33						
	/	' \					₫	534	
13A	+	-					84 =		
	١	./					∄		
	8/24	VI.	6-20				Greenish gray (GLEY15/1) with		
	33%	ss	22-23 N=42				(10YR5/6) mottles, moist, stiff, lea		
	J	'\	14-72				= trace very inte-granted salid	— 532	
	<u> </u>	_					86 —		
	V						=		
	24/24	VI	7-8				Greenish gray (GLEY15/1) with 5% y	rellowish brown (10YR5/6)	
14A	24/24 100%	ss	16-17				mottles, moist, stiff, lean CLAY, w	th some silt, trace very	
	. 5570	/\	N=24				fine-grained sand and		
	1	1					Brown (10YR4/3) with 5% dark yel mottles, moist, hard, lean CLAY, w	OWISH DIOWH (10114-70)	
	Ţ						88 — fine-grained sand and		
	1	I	E 10				=		
15A	24/24	ss	5-13 16-21				<u> </u>		
.0.	100%	1	N=29				≣		
	/	' \					Brown (10YR4/3) with 5% dark yel mottles, moist, hard, lean CLAY, v gravel, trace very fine-grained sand, (10YR2/1) orga	lowish brown (10YR4/6)	
	+	-					90 mottles, moist, hard, lean CLAY, v	vith some silt, few small	
	١	./					gravel, trace very fine-grained sand,		
	24/24	VI	4-8				(10YR2/1) orga	IIIUS.	
16A	100%	ss	<i>15</i> -9 N=23				す		
	1	/\	IN-23				-	526	
17A	L						92 — — — Proum (10VP/1/2) with 59/ dark val		
	V	1					Brown (10YR4/3) with 5% dark yel mottles, moist, hard, lean CLAY, v		
	04/04	VI	5-6				gravel and wood fragments, trace	very fine-grained sand,	
47B	24/24 100%	ss	8-10					DYR2/1) organics/	
	. 5570	/\	N=14				Very dark grayish brown (10YR3/2),	moist, stiff, fat CLAY, with524	
	1	1					some silt.		
	Ţ						94 =		
	N	M	2.4				Dark grayish brown (10YR4/2), mo	oist, stiff, fat CLAY, with	
18A	24/24	ss	2-4 7-8				some silt.		
•	100%	Λl	N=11				∄		
	1	1					∃	522	
	+	-					96 =		
	١	1					₫		
10.4	24/24	VI	2-6				=		
19A	100%	ss	<i>7-11</i> N=13						
	J.	'\	11.10				∄	520	
	1	4					98 Greenish gray (GLEY15/1) with 5% y mottles, moist, stiff, lean CLAY, wit	rellowish brown (10YR5/6)	
	١	./					mottles, moist, stiff, lean CLAY, wit very fine-grained sand, tra		
	18/20	VI	3-15						
50A	90%	SS	28-50/2" N=43				킄		
	/	\						518	
	L		l	I	1	I	Greenish gray (GLEY15/1) with 5% y mottles, moist, stiff, lean CLAY, w	ellowish brown (10YR5/6) / Y/// Y/// 310 1	

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois Project: 20E0111A

WEATHER: Clear, cold (20s)

DATES: Start: 1/28/2021

Finish: 2/3/2021

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

BOREHOLE ID: G275D

Well ID: G275

Surface Elev: 617.52 ft. MSL Completion: 99.70 ft. BGS **Station:** 874,285.30N

2,516,366.50E

	SAMPL	.E	1	TES1	ΓING	i	TOPOGRAPH	IC MAP INFORMATION:	WATER LEVEL INFORMATION:			
	er //Total (in) covery		/ 6 <i>in</i> lue	Content (%)	ensity (lb/ft³)	sity (lb/f Qp (tsf) Type	Quadrangle: Coffeen, IL Township: East Fork Township Section 11, Tier 7N; Range 3W		<u>▼</u> = 10.90 - <u>▼</u> = <u>▼</u> =			
1.	Recov % Rec	Туре	Blows N - Va RQD	Water (Dry De	Qu (tsf Failure	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	

fine-grained sand, trace small gravel. End of boring = 99.7 feet

NOTE(S): G275 installed in adjacent blind drill borehole.

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois Project: 20E0111A **DATES: Start:** 1/25/2021

Finish: 1/25/2021

WEATHER: Rain, cold (30s)

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter



BOREHOLE ID: G282D Well ID: 282D

Surface Elev: ft. MSL Completion: 60.00 ft. BGS Ν

Station: Ε

	SAMPLE	AMPLE TESTING					TOPOGR/	APHIC MAP INFORMATION:	WATER LEVEL INFORMATION:				
ber	Recov / Total (in) % Recovery		<i>Blows / 6 in</i> N - Value RQD	Water Content (%)	Dry Density (lb/ft³)	Qu (tsf) Qp (tsf) Failure Type	Quadra Towns Section	ingle: Coffeen, IL hip: East Fork Township n 11, Tier 7N; Range 3W	$ \mathbf{Y} = 18.80 - \text{During Drilling} $ $ \mathbf{Y} = 55.90 - \text{During Drilling} $ $ \mathbf{Y} = 55.90 - \text{During Drilling} $				
Number	Reco % Re	Туре	<i>Blow</i> N - V RQD	Wate	Dry	Qu (t Failu	Depth ft. BGS	Lithologic Description	Borehole Elevation Remarks Detail ft. MSL				
	15/24	M	3-3					Gray (10YR6/1), wet, loose, GRAVEL, with some sand. [F	FILL]				
1A	63%	ss	3-4 N=6				2	Brown (10YR5/3), moist, stiff, lean CLAY, with some silt, t very fine- to fine-grained sand. [FILL]	trace				
2A	17/24 71%	ss	3-4 4-5 N=8				4-	Yellowish brown (10YR5/4) with 10% gray (10YR6/1) mot moist, stiff, lean CLAY, with some silt, trace very fine- t fine-grained sand and small gravel. [FILL]	ttles, to				
3A	22/24 92%	ss	2-3 5-6 N=8				6	Grayish brown (10YR5/2), moist, stiff, lean CLAY, with so silt, trace small gravel.	ome				
4A	20/24 83%	ss	3-4 4-5 N=8				8	Grayish brown (10YR5/2) with 10% yellowish brown (10YF mottles, moist, stiff, lean CLAY, with some silt, trace sm gravel.					
5A	22/24 92%	ss	2-3 5-7 N=8				10 =	Grayish brown (10YR5/2) with 10% yellowish brown (10YF and 5% gray (10YR5/1) mottles, moist, stiff, lean CLAY, some silt, trace small gravel.	R5/4) with				
6A	20/24 83%	ss	3-4 4-7 N=8				12 -	Grayish brown (10YR5/2) with 20% yellowish brown (10YR and 5% gray (10YR5/1) mottles, moist, stiff, lean CLAY, some silt, trace small gravel.	R5/4) with				
7A	20/24 83%	ss	2-3 4-5 N=7				14 -	Gray (10YR6/1) with 20% yellowish brown (10YR5/6) mot moist, stiff, lean CLAY, with some silt, little very fine- to fine-grained sand, trace small gravel.					
8A	20/24 83%	ss	1-2 3-4 N=5				16	Dark gray (10YR4/1), moist, stiff, lean CLAY, with some Gray (10YR6/1) with 20% yellowish brown (10YR5/6) mot moist, stiff, lean CLAY, with some silt, little very fine-to fine-grained sand, trace small gravel.	itles,				
9A 0A	21/24 88%	ss	1-2 2-3 N=4				18 -	Gray (10YR6/1) with 30% yellowish brown (10YR5/6) mot moist, stiff, lean CLAY, with some silt, little very fine- to fine-grained sand, trace small gravel.					
0B	24/24 100%	ss	0-1 1-0 N=2				20	Yellowish brown (10YR5/6) with 10% gray (10YR6/1) mot wet, very loose, SILT, with some very fine- to fine-grain sand, few small gravel, trace clay.	ttles, ed				

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois Project: 20E0111A

DATES: Start: 1/25/2021

Finish: 1/25/2021 WEATHER: Rain, cold (30s)

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter

BOREHOLE ID: G282D

Well ID: 282D Surface Elev: ft. MSL

60.00 ft. BGS Completion: Ν Station:

Ε SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Ξ Dry Density (Ib/ft³ ▼ = 18.80 - During Drilling Water Content (%)) Qp (tsf) Type Quadrangle: Coffeen, IL Recov / Total (% Recovery Ψ = 55.90 - During Drilling Township: East Fork Township Blows / 6 in N - Value **RQD** ▼ = Section 11, Tier 7N; Range 3W Qu (tsf) Failure T Number Borehole Elevation Lithologic Description Remarks Detail ft. MSL Yellowish brown (10YR5/4) with 20% yellowish brown (10YR5/6) and 5% yellowish red (5YR4/6) mottles, moist, hard, 3-7 24/24 SS 11-16 11A SILT, with some very fine- to fine-grained sand, little clay, few 100% N=18 small gravel. Yellowish brown (10YR5/4), wet, fine- to medium-grained 22 SAND Yellowish brown (10YR5/4) with 20% yellowish brown (10YR5/6) and 5% yellowish red (5YR4/6) mottles, moist, hard, 7-12 24/24 SILT, with some very fine- to fine-grained sand, little clay, few 12A 17-24 100% small gravel, trace medium gravel. N=29 9-15 24/24 13A 22-22 100% N=37 26 8-17 Dark gray (10YR4/1), moist, hard, SILT, with some very fine-22/24 to fine-grained sand, little clay, few small gravel, trace medium 14A SS 16-22 92% N = 33gravel. 5-11 21/24 15A 15-19 88% 30 Dark gray (10YR4/1), moist, SAND, little silt and clay. 5-25 22/22 SS 33-50/4 16A 100% Dark gray (10YR4/1), moist, hard, SILT, with some very fine-N=58 to fine-grained sand, little clay, few small gravel, trace medium gravel. 7-10 22/24 15-20 17A 92% N=25 4-8 24/24 18A 10-16 100% N=18 Dark gray (10YR4/1), moist, hard, lean CLAY, with some silt, few very fine- to fine-grained sand, little small gravel. 5-8 24/24 19A 13-15 100% N=21 2-4 21/24 20A SS 8-11 88% N=12 NOTE(S): G282D installed in borehole.

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 1/25/2021

Finish: 1/25/2021 WEATHER: Rain, cold (30s)

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

BOREHOLE ID: G282D

Well ID: 282D

Surface Elev: ft. MSL
Completion: 60.00 ft. BGS

Station: N E

Page 3 of 4

	SAMPLE		1		ING		TOPOGRA	PHIC MAP INFORMATION:	WATER LEVEL IN	FORMATION	N:
er	Recov / Total (in) % Recovery		/ 6 <i>in</i> Ilue	Water Content (%)	Dry Density (lb/ft³)	Qu (tsf) Qp (tsf) Failure Type	Townsh	ngle: Coffeen, IL ip: East Fork Township 11, Tier 7N; Range 3W	$\underline{\underline{Y}} = 18.80 - \underline{\underline{Y}} = 55.90 - \underline{\underline{Y}} = 55.90 - \underline{\underline{Y}} = 0.00$		
Number	Recov % Red	Туре	Blows / 6 in N - Value RQD	Water	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
21A	24/24 100%	ss	4-8 11-14 N=19				42				
2A	22/24 92%	ss	3-7 8-12 N=15				44 —				
23A	24/24 100%	ss	3-6 9-13 N=15				46		7,7,2,2		Trace wood fragments below 4
4A	24/24 100%	ss	4-6 9-12 N=15				48 —	Dark gray (10YR4/1), moist, hard, lean CLAY, with some s few very fine- to fine-grained sand, little small gravel. [Continued from previous page]	ilt,		ft.
25A	24/24 100%	ss	4-6 12-13 N=18				50 —				0.5" gravel seam a 48.5 ft.
6A	24/24 100%	ss	2-7 9-13 N=16				46				
7A	24/24 100%	ss	4-7 11-14 N=18				_,				
8A 8B	24/24 100%	ss	6-12 9-18 N=21				<u>₹</u> 56	Light yellowish brown (10YR6/5), moist, very fine- to medium-grained SAND, with some silt, little small to mediu gravel. Dark gray (10YR4/1), moist, hard, lean CLAY, with some s few very fine- to fine-grained sand, little small gravel, trace	ilt,		
29A	24/24 100%	ss	6-10 11-11 N=21					wood fragments. Light yellowish brown (10YR6/5), wet, medium dense, ver fine- to coarse-grained SAND, little small gravel, few silt.	v		
0A	24/24 100%	ss	4-5 8-9 N=13				58 —	Dark gray (10YR4/1) with 5% dark yellowish brown (10YR3 mottles, moist, stiff, lean CLAY, with some silt.	/6)		

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 1/25/2021 **Finish:** 1/25/2021

WEATHER: Rain, cold (30s)

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

BOREHOLE ID: G282D

Well ID: 282D

Surface Elev: ft. MSL

Completion: 60.00 ft. BGS

Station: N E

	SAMPL	E	1	EST	ING		TOPOGRAPI	IIC MAP INFORMATION:	WATER LEVEL INFORMATION:			
	otal (in) ery			Content (%)	(lb/ft³)	(tsf) e	Quadrang	le: Coffeen, IL : East Fork Township	▼ = 18.80 - ▼ = 55.90 -	During Drilling		
l -	/ Tot		/ 6 in lue	Conte	ensity	_ ·		1, Tier 7N; Range 3W	<u>T</u> =	g		
Numbe	Recov % Rec	Type	Blows N - Va RQD	Water (Dry De	Qu (tsf Failure	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	

End of boring = 60.0 feet

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 1/18/2021 **Finish:** 1/18/2021

WEATHER: Overcast, cold (30s)

NOTE(S): G283 installed in borehole.

CONTRACTOR: Roberts

Rig mfg/model: GeoProbe 8040DT **Drilling Method:** 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

BOREHOLE ID: G283

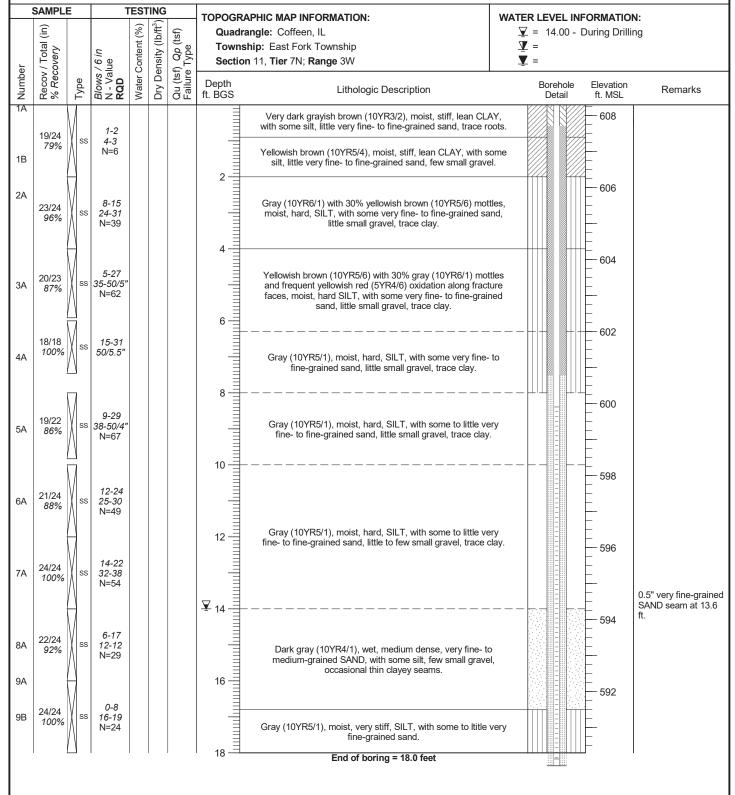
Well ID: G283

 Surface Elev:
 608.30 ft. MSL

 Completion:
 18.00 ft. BGS

 Station:
 874.113.00N

2.516.503.00E



CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 1/20/2021 **Finish:** 1/20/2021

WEATHER: Clear, cool (40s)

CONTRACTOR: Roberts

Rig mfg/model: GeoProbe 8040DT

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

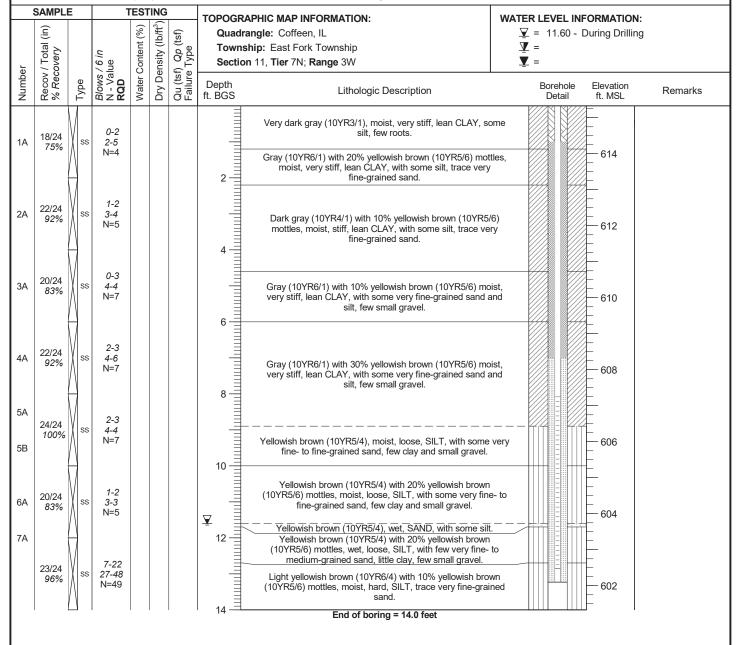
Eng/Geo: C. Colin Winter

HANSON

Well ID: G284

Surface Elev: 615.33 ft. MSL Completion: 14.00 ft. BGS

Station: 874,423.60N 2,516,922.90E



CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 1/19/2021 **Finish:** 1/19/2021

WEATHER: Clear, cold (20s)

CONTRACTOR: Roberts

Rig mfg/model: GeoProbe 8040DT

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

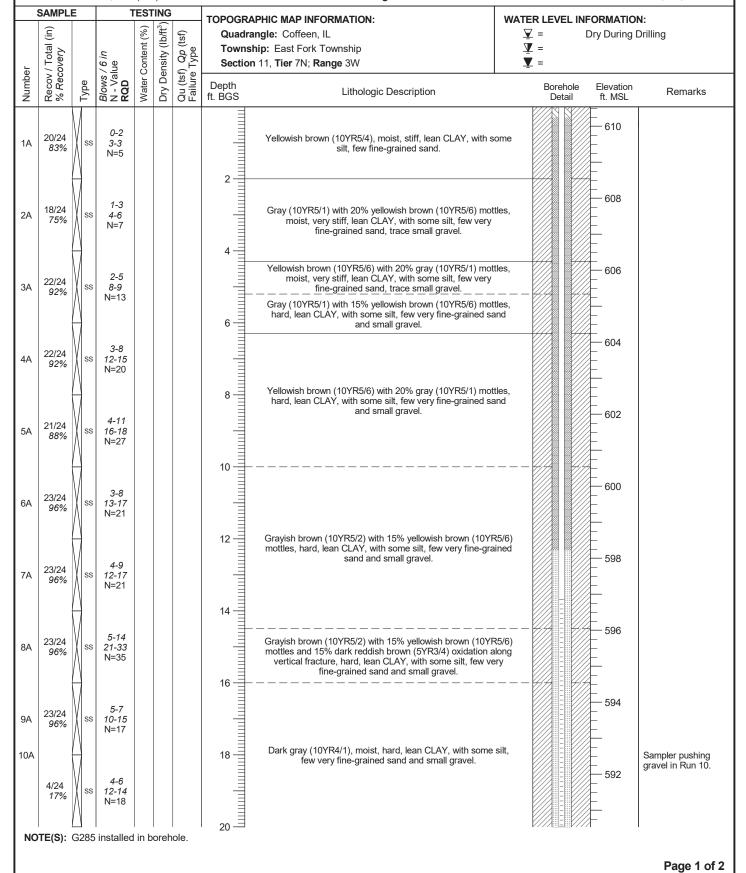
BOREHOLE ID: G285 Well ID: G285

 Surface Elev:
 610.54 ft. MSL

 Completion:
 26.00 ft. BGS

 Station:
 874.795.00N

2.516.680.40E



CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois Project: 20E0111A

DATES: Start: 1/19/2021

Finish: 1/19/2021

WEATHER: Clear, cold (20s)

CONTRACTOR: Roberts

Rig mfg/model: GeoProbe 8040DT

Drilling Method: 4.25" HSA w/SS sampler

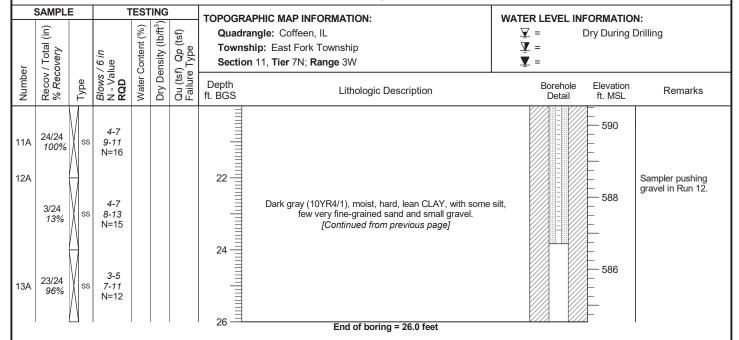
FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter

BOREHOLE ID: G285 Well ID: G285

Surface Elev: 610.54 ft. MSL Completion: 26.00 ft. BGS Station: 874,795.00N

2,516,680.40E



CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 1/19/2021

Finish: 1/19/2021 WEATHER: Clear, cold (30s)

CONTRACTOR: Roberts

Rig mfg/model: GeoProbe 8040DT

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

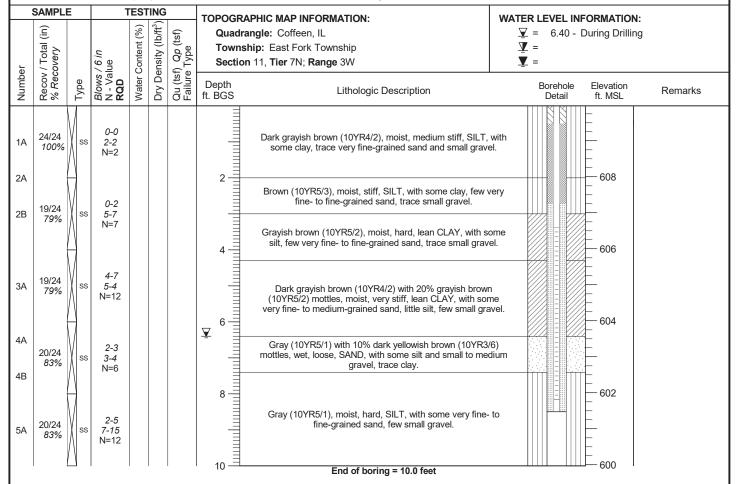
Eng/Geo: C. Colin Winter



BOREHOLE ID: G286 Well ID: G286

Surface Elev: 609.97 ft. MSL Completion: 10.00 ft. BGS

Station: 875,072.20N 2.516.561.80E



CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 1/18/2021 **Finish:** 1/18/2021

WEATHER: Overcast, cold (30s)

CONTRACTOR: Roberts

Rig mfg/model: GeoProbe 8040DT **Drilling Method:** 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter



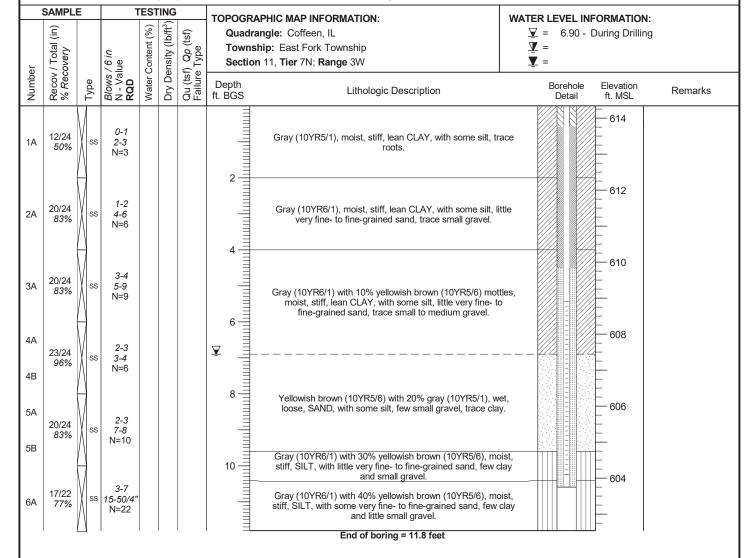
BOREHOLE ID: G287 Well ID: G287

 Surface Elev:
 614.34 ft. MSL

 Completion:
 11.80 ft. BGS

 Station:
 875.442.80N

2.516.415.50E



CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 1/21/2021

Finish: 1/21/2021 WEATHER: Clear, cold (30s)

CONTRACTOR: Roberts

Rig mfg/model: GeoProbe 8040DT

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

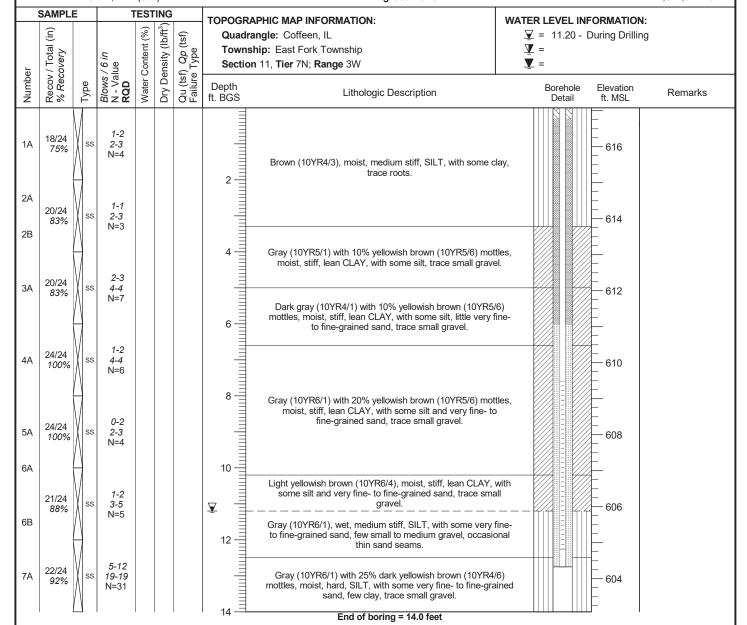
BOREHOLE ID: G288 Well ID: G288

 Surface Elev:
 617.08 ft. MSL

 Completion:
 14.00 ft. BGS

 Station:
 875.279.60N

2,517,071.40E



 $\begin{tabular}{ll} \textbf{CLIENT:} & \textbf{Illinois Power Generating Co.} \\ \end{tabular}$

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 1/26/2021 **Finish:** 1/27/2021

WEATHER: Overcast, cold (30s)

CONTRACTOR: Roberts

Rig mfg/model: GeoProbe 8040DT

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

Well ID: n/a

Surface Elev: ft. MSL

Completion: 60.0 ft. BGS

Station: N E

	SAMPLE		1	EST	ING		TOPOGRA	PHIC MAP INFORMATION:	WATER L	EVEL INF	ORMATION:	
er	Recov / Total (in) % Recovery		/ 6 in Ilue	Water Content (%)	Dry Density (lb/ft³)	f) Qp (tsf) e Type	Quadra Townsl	ingle: Coffeen, IL hip: East Fork Township 11, Tier 7N; Range 3W	=	18.60 - [During Drilling	1
Number	Recov % Rec	Type	Blows / 6 in N - Value RQD	Water	Dry De	Qu (tsf) Failure T	Depth ft. BGS	Lithologic Description	E	orehole Detail	Elevation ft. MSL	Remarks
IA	17/24 71%	ss	2-2 4-4 N=6				2	Brown (10YR4/3) with 10% gray (10YR6/1) and 10% yellow brown (10YR5/6) mottles, moist, stiff, SILT, with some classification trace very fine- to fine-grained sand.				
Α	19/24 79%	ss	3-5 6-8 N=11				4-	Dark gray (10YR4/1) with 20% yellowish brown (10YR5/ mottles, moist, stiff, lean CLAY, with some silt, few very fi to fine-grained sand, trace small gravel.				
ЗА	19/24 79%	ss	3-4 4-10 N=8					to me granes carta, trace circan granes.				
	21/24 88%	ss	2-2 4-6 N=6				6	Dark gray (10YR4/1) with 20% yellowish brown (10YR5/mottles, moist, stiff, lean CLAY, with some silt, little very fit to fine-grained sand, trace small gravel. Dark gray (10YR4/1), SILT, with some clay.				
1A 5A	18/24 75%	SH	N-0				Ā 8	Dark gray (10YR4/1) with 20% yellowish brown (10YR5/6) 5% very pale brown (10YR7/3) mottles, moist, stiff, lean Cl with some silt, little very fine- to fine-grained sand, trace signavel.	_AY, ///			
6A	22/24 92%	ss	3-4 4-6 N=8				10	Dark grayish brown (10YR4/3), moist, stiff, lean CLAY, w some silt.				
6B			N=0				12	Gray (10YR6/1) with 20% dark grayish brown (10YR4/2) at 10% yellowish brown (10YR5/6) mottles, moist, stiff, lea CLAY, with some silt, few very fine- to fine-grained sand, the small grayel.	n ///			
7A	19/24 79%	ss	3-4 6-6 N=10				14	Gray (10YR6/1) with 30% dark yellowish brown (10YR4/6) 20% dark grayish brown (10YR4/2) mottles, moist, stiff, le CLAY, with some silt, few very fine- to fine-grained sand, t small gravel.	ean ///			
ЗА	22/24 92%	ss	3-4 5-6 N=9					Very dark gray (10YR3/1), moist, stiff, lean CLAY, with so silt, few very fine- to fine-grained sand, trace small grave				
9A	24/24 100%	ss	2-3 4-4 N=7				18	Gray (10YR6/1) with 10% yellowish brown (10YR5/6) mott moist, medium stiff, lean CLAY, with some silt and very fin fine-grained sand, few small gravel.				
0A 0B	24/24 100%	ss	2-2 1-2 N=3				18	Gray (10YR6/1) with 10% yellowish brown (10YR5/6) mott moist to wet, SILT, with some very fine- to fine-grained sa and clay, few small gravel. Strong brown (7.5YR5/8), moist to wet, SILT, with some very fine- to fine-grained sa and clay, few small gravel.	and			

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 1/26/2021

Finish: 1/27/2021 WEATHER: Overcast, cold (30s)

CONTRACTOR: Roberts

Rig mfg/model: GeoProbe 8040DT

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter



BOREHOLE ID: G289 Well ID: n/a

> Surface Elev: ft. MSL Completion: 60.0 ft. BGS

Station: N

Ε

Page 2 of 4

SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Ξ Dry Density (lb/ft³ Water Content (%) Quadrangle: Coffeen, IL ∇ = 18.60 - During Drilling) Qp (tsf) Type Recov / Total (% Recovery Ψ = 8.00 - 1/27/2021 8am Township: East Fork Township *Blows / 6 ir* N - Value **RQD** Section 11, Tier 7N; Range 3W ▼ = Qu (tsf) Failure T Number Depth Borehole Elevation Lithologic Description Remarks ft. BGS Detail ft. MSL fine- to fine-grained sand and clay, few small gravel. Strong brown (7.5YR5/8) with 20% gray (10YR6/1) mottles, moist to wet, SILT, with some very fine- to fine-grained sand 2-2 22/24 SS 4-7 11A 92% N=6 and clay, few small gravel. 1" wet, fine- to coarse-grained SAND, with little 22 Gray (10YR6/1), moist, hard, SILT, with some very fine- to small gravel. fine-grained sand, little clay, few small gravel. 6-20 21/24 Gray (10YR6/1), wet, SILT, with some sand, little small to SS 17-26 88% medium gravel. N = 37Gray (10YR6/1), moist, hard, SILT, with some very fine- to fine-grained sand, little clay, few small gravel. 13A Gray (10YR6/1), wet, loose, very fine- to medium-grained SAND, with some silt, little small to medium gravel. 4-23 23/24 13B 32-41 96% N=55 26 14A 23-25 22/24 Trace large gravel from 27 to 28 ft. SS 24-27 92% N=49 14-22 23/23 32-50/5 15A 100% N=54 Gray (10YR6/1), moist, hard, SILT, with some very fine- to fine-grained sand, little clay, few small gravel. 4-6 82% 50/5" Trace large gravel 16A from 31 to 31.5 ft. 27-50/5 SS 20-17 19/24 18A 15-19 79% N = 3236 7-14 24/24 19A SS 16-16 Dark gray (10YR4/1), moist, hard, lean CLAY, with some silt, 100% N=30 few very fine- to fine- grained sand, few small gravel. 7-12 24/24 15-18 20A SS 100% N=27 NOTE(S):

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 1/26/2021

Finish: 1/27/2021 WEATHER: Overcast, cold (30s)

CONTRACTOR: Roberts

Rig mfg/model: GeoProbe 8040DT

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter



Well ID: n/a

Surface Elev: ft. MSL Completion: 60.0 ft. BGS

Station: N E

Page 3 of 4

SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Ξ Water Content (%) Dry Density (lb/ft³) Qp (tsf) Type Quadrangle: Coffeen, IL \mathbf{Y} = 18.60 - During Drilling Recov / Total (% Recovery $\underline{\Psi}$ = 8.00 - 1/27/2021 8am Township: East Fork Township Blows / 6 in N - Value **RQD** Section 11, Tier 7N; Range 3W ▼ = Qu (tsf) Failure T Number Borehole Elevation Lithologic Description Remarks ft. BGS ft. MSL 24/24 SH 100% Trace wood fragments below 42 5-13 24/24 22A SS 20-22 100% N = 333-7 22/24 23A 11-14 92% N=18 46 Dark gray (10YR4/1), moist, hard, lean CLAY, with some silt, 4-9 24/24 24A 10-14 few very fine- to fine- grained sand, few small gravel. 100% N=19 [Continued from previous page] 2-5 23/24 25A 7-10 96% N=12 4-10 24/24 12-18 N=22 26A 100% 52 5-9 24/24 27A 11-15 100% N=20 3-5 24/24 Gray (10YR5/1) with 15% dark brown (10YR3/6) mottles, 28A 7-8 100% moist, stiff, lean CLAY, with some silt. N=12 56 3-5 24/24 29A 9-9 100% N=14 Gray (10YR5/1) with 20% dark brown (10YR3/6) and 10% dark gray (10YR4/1) mottles, moist, stiff, lean CLAY, with some silt. 0-4 24/24 Few fine- to 30A SS 8-9 100% medium-grained N=12 sand below 59 ft. NOTE(S):

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 1/26/2021

Finish: 1/27/2021 WEATHER: Overcast, cold (30s)

CONTRACTOR: Roberts

Rig mfg/model: GeoProbe 8040DT

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

BOREHOLE ID: G289

Well ID: n/a
Surface Elev:

ft. MSL 60.0 ft. BGS

Remarks

Completion: 60.0

Station: N E

	SAMPL	E	1	EST	ING		TOPOGE	APHIC MAP INFORMATION:	WATER	I EVEL INF	ORMATION:
er	/ Total (in) overy		/ 6 in ue	Content (%)	ensity (lb/ft³)	f) <i>Qp</i> (tsf) s Type	Quad Town	rangle: Coffeen, IL ship: East Fork Township on 11, Tier 7N; Range 3W	. ₹	= 18.60 - [During Drilling 1/27/2021 8am
Numbe	Recov % Rec	Туре	Blows N - Val	Water (Dry De	Qu (tsf Failure	Depth ft. BGS	Lithologic Description		Borehole Detail	Elevation ft. MSL

End of boring = 60.0 feet

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 2/9/2021 **Finish:** 2/9/2021

WEATHER: Overcast, very cold (10s)

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt
Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

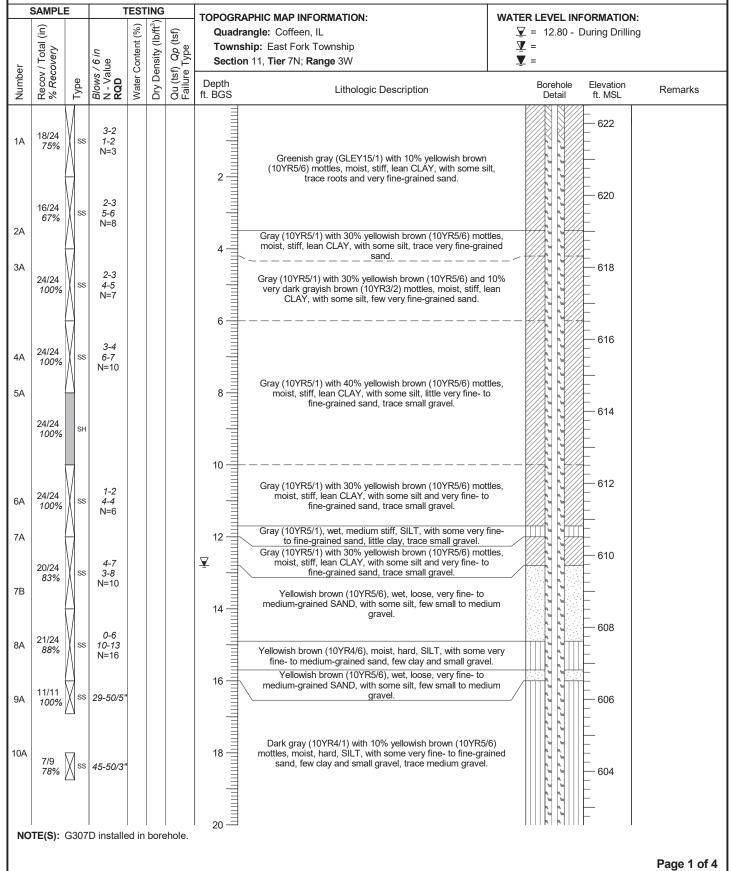
BOREHOLE ID: G307D Well ID: G307D

 Surface Elev:
 622.51 ft. MSL

 Completion:
 60.00 ft. BGS

 Station:
 871,397.20N

2,515,560.30E



CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 2/9/2021 **Finish:** 2/9/2021

WEATHER: Overcast, very cold (10s)

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

BOREHOLE ID: G307D Well ID: G307D

 Surface Elev:
 622.51 ft. MSL

 Completion:
 60.00 ft. BGS

 Station:
 871,397.20N

2,515,560.30E

	AMPLE		vercast, v	_	ING	(103)	TOPOGRAI	Eng/Geo: C. Colin Winter PHIC MAP INFORMATION:	WATER	I EVEL INI	FORMATIO	2,515,560.30E
e	Recov / Total (in) % Recovery		/ 6 in Ilue	Water Content (%)	Dry Density (lb/ft³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Quadraı Townsh	ngle: Coffeen, IL ip: East Fork Township 11, Tier 7N; Range 3W		= 12.80 - =	During Drilli	
	Recov % Red	Туре	Blows / 6 in N - Value RQD	Water	Dry De	Qu (ts Failure	Depth ft. BGS	Lithologic Description		Borehole Detail	Elevation ft. MSL	Remarks
11A 12A	8/8 100% 8/9 89%		35-50/2" 30-50/3"				22			6, 6, 6, 6, 6, 6, 6, 6, 6, 6		Little sand below 20 ft.
	17/17 100%	ss	23-39 50/5"				24			,,,,,,,,,	_ _ _ _ 598 _ _ _	
I4A	9/9 100%	ss	38-50/3"							,,,,,,,,	 596 	
5A	23/23 100%	ss	31-44 34-50/5" N=78				28	Dark gray (10YR4/1) with 10% yellowish brown (10YR5, mottles, moist, hard, SILT, with some very fine- to fine-gra sand, few clay and small gravel, trace medium gravel [Continued from previous page]	ined	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	 594 	1" dark gray (10/R4/1), fine- to medium-grained sand, with some sil little small gravel.
7A	15/15 100% 10/10	ss	3-44 50/3"				32			,,,,,,,,	— 592 - - - - - - -	
8A	10/10 100% 11/11 100%		34-50/4"				34 —			,,,,,,,,,	590 588	
	17/17 100%	ss	28-44 50/5"				36			,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		
20A	24/24 100%	ss	14-19 13-24 N=32				38	Dark gray (10YR4/1), moist, hard, lean CLAY, with some little to few very fine- to fine-grained sand, few small grav	silt, vel.		584 	
NOT	E(S): (G307	D installe	ed in	bore	hole.	40 —		<i>V</i>			•
												Page 2 of

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 2/9/2021 **Finish:** 2/9/2021

WEATHER: Overcast, very cold (10s)

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig **Drilling Method:** 4.25" HSA w/SS sampler

Drilling Metriod. 4.25 FISA W/SS Sain

FIELD STAFF: Driller: Matt
Helper: Corey

Eng/Geo: C. Colin Winter



BOREHOLE ID: G307D Well ID: G307D

 Surface Elev:
 622.51 ft. MSL

 Completion:
 60.00 ft. BGS

 Station:
 871,397.20N

2,515,560.30E

Page 3 of 4

•	SAMPLE		7	EST				HIC MAP INFORMATION:	WATER LEVEL IN	FORMATIO	N:
Number	Recov / Total (in) % Recovery	ø.	<i>Blows / 6 in</i> N - Value RQD	Water Content (%)	Dry Density (lb/ft³)	Qu (tsf) Qp (tsf) Failure Type		gle: Coffeen, IL p: East Fork Township 11, Tier 7N; Range 3W	<u>▼</u> = 12.80 - <u>▼</u> = <u>▼</u> =	During Drilli	
Nun	Rec % R	Туре	B/o ₁ N - ,	Wate	Dry	Qu (Failt	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
21A 22A	24/24 100%	SS	7-8 12-15 N=20							582	Trace medium grav below 40 ft.
3A	24/24 100%	SH	6-8 10-12 N=18				42 44 44 46 48 50 52 11 11 11 11 11 11 11			578	
4A	24/24 100%	ss	5-6 10-12 N=16				46 —	Dark gray (10YR4/1), moist, hard, lean CLAY, with some little to few very fine- to fine-grained sand, few small gra [Continued from previous page]	silt, vel.	576	
5A	24/24 100%	ss	3-6 11-11 N=17				50				Trace wood fragments below 48 ft.
6A	24/24 100%	ss	3-7 9-10 N=16				52			572 	Gravel plugged sho
7A	16/24 67%	ss	4-7 11-9 N=18							570 	in Run 27. Large lignite fragment at 52.4 ft.
8A	24/24 100%	ss	3-6 10-9 N=16					Greenish gray (GLEY15/1) with 15% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some s trace small gravel.	ilt,	568	
9A	24/24 100%	ss	4-6 8-12 N=14				58	Yellowish brown (10YR5/6) with 30% greenish gray (GLEY15/1) and 5% very dark grayish brown (10YR3//mottles, moist, stiff, lean CLAY, with some silt, trace sm gravel.	2) aall	566	
80A	24/24 100%	ss	2-5 8-10 N=13				60	Yellowish brown (10YR5/6) with 20% greenish gray (GLEY15/1) and 5% very dark grayish brown (10YR3/mottles, moist, stiff, lean CLAY, with some silt, trace sm gravel.	2) nall	564	Large wood fragme

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 2/9/2021 **Finish:** 2/9/2021

WEATHER: Overcast, very cold (10s)

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig **Drilling Method:** 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt

Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

BOREHOLE ID: G307D

Well ID: G307D

 Surface Elev:
 622.51 ft. MSL

 Completion:
 60.00 ft. BGS

 Station:
 871,397.20N

2,515,560.30E

							Quadran Townshi	HIC MAP INFORMATION: gle: Coffeen, IL p: East Fork Township 1, Tier 7N: Range 3W	WATER LEVEL IN	FORMATION: During Drilling	
Number	Recov / % Reco	Type	Blows / N - Valu RQD	Water Co	Dry Den	Qu (tsf) Failure ⁻	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks

End of boring = 60.0 feet

at 59.8 ft.

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

TESTING

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 1/13/2021 **Finish:** 1/13/2021

WEATHER: Clear, cold (30s)

SAMPLE

CONTRACTOR: Roberts

TOPOGRAPHIC MAP INFORMATION:

Rig mfg/model: GeoProbe 8040DT **Drilling Method:** 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

BOREHOLE ID: G308 Well ID: G308

WATER LEVEL INFORMATION:

 Surface Elev:
 621.59 ft. MSL

 Completion:
 15.80 ft. BGS

 Station:
 871.454.70N

2,515,101.40E

(lb/ft³ Ξ Water Content (%) ∇ = 13.00 - During Drilling) Qp (tsf) Type Quadrangle: Coffeen, IL Recov / Total (% Recovery **V** = Township: East Fork Township **Dry Density** *Blows / 6 ir* N - Value **RQD** Section 11, Tier 7N; Range 3W ▼ = Qu (tsf) Failure T Number Depth Borehole Elevation Lithologic Description Remarks ft. BGS ft. MSL 2-2 16/24 Grayish brown (10YR5/2), moist, stiff, lean CLAY, with some SS 2-3 1A 67% silt, trace roots. 620 0-2 19/24 2A SS 3-5 Brown (10YR5/3) with 20% yellowish brown (10YR5/6) mottles, 79% N=5moist, stiff, lean CLAY, with some silt, trace roots. 618 17/24 3A SS 5-6 71% 616 Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, trace very fine-grained sand and small gravel. 23/24 4A SS 5-6 96% N=9614 Gray (10YR5/1) with 30% yellowish brown (10YR5/6) mottles, 22/24 moist, very stiff, lean CLAY, with some silt, trace very 5A 6-7 92% fine-grained sand and small to medium gravel. N=10 612 10 2-3 21/24 4-5 6A 88% Gray (10YR5/1) with 40% yellowish brown (10YR5/6) mottles, N=7 moist, stiff to very stiff, lean CLAY, with some silt, little to trace 610 very fine-grained sand, trace small to medium gravel. 7A 12

Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles,

wet, stiff, lean CLAY, with some very fine- to fine-grained

sand, little silt, trace small gravel.

Light yellowish brown (10YR6/4) wet, hard, SILT, with some

very fine-grained sand, few clay and small to medium gravel.

End of boring = 15.8 feet

NOTE(S): G308 installed in borehole.

2-3

4-6

N=7

4-12

N=45

ss 33-50/4

 \mathbf{V}

14

24/24

100%

19/22

8A

86%

608

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 1/12/2021 **Finish:** 1/12/2021

WEATHER: Clear, cold (30s)

CONTRACTOR: Roberts

Rig mfg/model: GeoProbe 8040DT

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt
Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

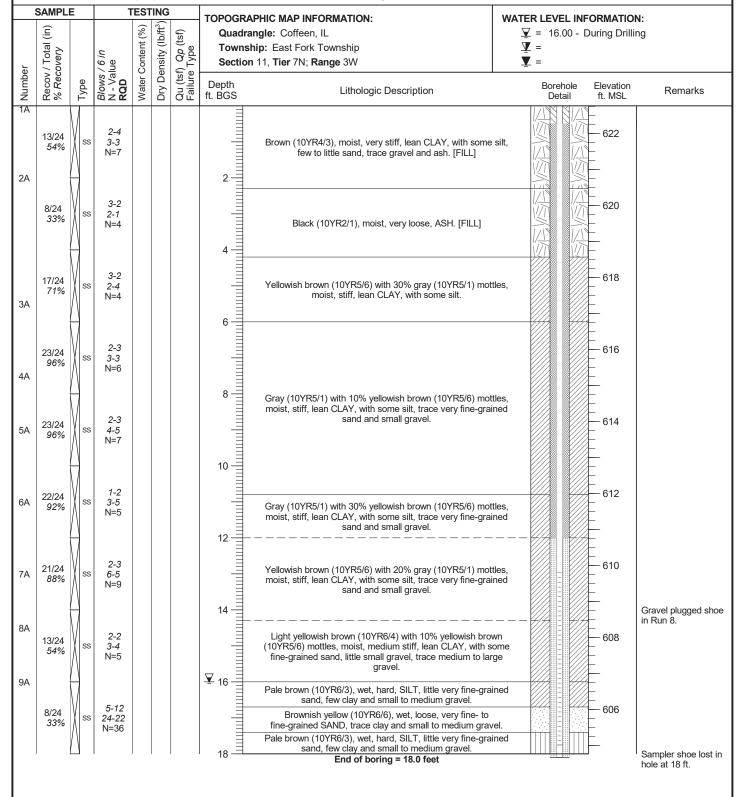
BOREHOLE ID: G309 Well ID: G309

 Surface Elev:
 622.77 ft. MSL

 Completion:
 18.00 ft. BGS

 Station:
 871.865.80N

2,515,067.10E



CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 1/15/2021 **Finish:** 1/15/2021

WEATHER: Clear, light snow, cold (30s)

CONTRACTOR: Roberts

Rig mfg/model: GeoProbe 8040DT **Drilling Method:** 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

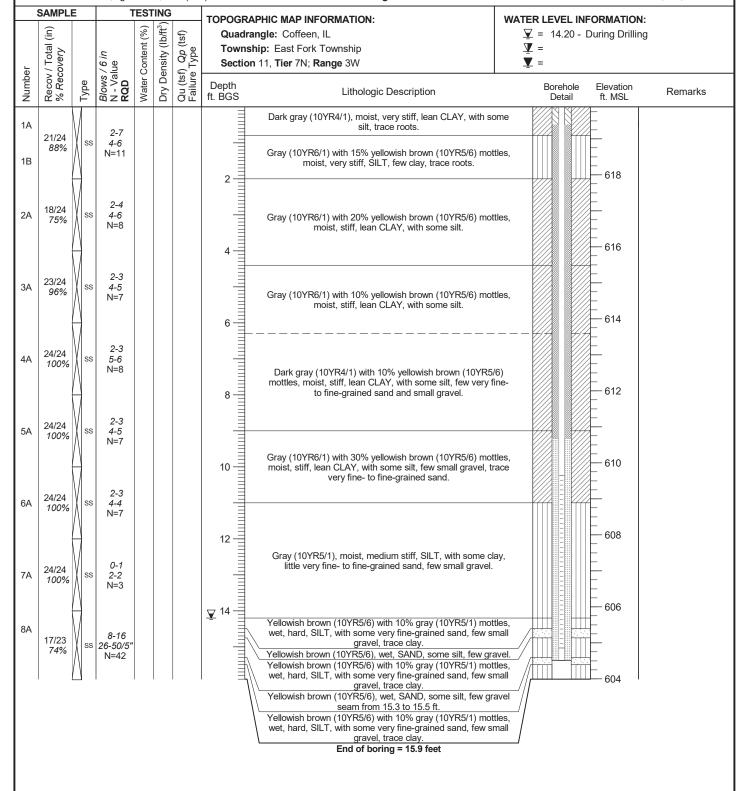
BOREHOLE ID: G310 **Well ID:** G310

 Surface Elev:
 619.89 ft. MSL

 Completion:
 15.90 ft. BGS

 Station:
 872,239.40N

2,515,159.40E



CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois

Project: 20E0111A

DATES: Start: 2/5/2021

FIELD STAFF: Driller: Matt Finish: 2/5/2021

WEATHER: Clear, cold (20s) SAMPLE TESTING

CONTRACTOR: Roberts BOREHOLE ID: G311 Rig mfg/model:

Well ID: G311

Surface Elev: 618.32 ft. MSL Completion: 14.40 ft. BGS

Station: 872,238.70N 2,515,881.80E

	SAMPLI	Ε	1	EST	ING		TOPOGRAPHI	C MAP INFORMATION:				
er	Recov / Total (in) % Recovery		/ 6 in Ilue	Water Content (%)	Dry Density (lb/ft³)	Qu (tsf) Q <i>p</i> (tsf) Failure Type	Quadrangle Township:	e: Coffeen, IL				
Number	Recov % Rec	Type	Blows / 6 in N - Value RQD	Water	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description	Bo [orehole Detail	Elevation ft. MSL	Remarks
							2 4 4 6 8 10 11 12 14 14 14 14 14 14 14 14 14 14 14 14 14				618 	

Drilling Method: 4.25" HSA w/SS sampler

Helper: Corey

Eng/Geo: C. Colin Winter

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 2/5/2021 **Finish:** 2/5/2021

WEATHER: Clear, cold (20s)

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig

Drilling Method: 3.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt
Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

BOREHOLE ID: G311D Well ID: G311D

 Surface Elev:
 618.39 ft. MSL

 Completion:
 60.00 ft. BGS

 Station:
 872,238.70N

2,515,881.80E

Page 1 of 4

•	SAMPLE		1	EST	ING		TOPOGRA	PHIC MAP INFORMATION:	WATER LEVEL IN	FORMATIO	N:
	Ë			(%	/ft³)	(j.		ngle: Coffeen, IL	▼ = 11.20 -		
	<u>a</u> (nt ((B)) Qp (tsf) Type	1	ip: East Fork Township	<u>*</u> = 11.20 =	_ ag D.IIII	a
	Tot.		ni S	nte	sity	8,8	Section	11, Tier 7N; Range 3W	<u> </u>		
per	006		s/(alu	ပို)en	sf) re J	0000000	TH, Hel M, Range OV			
Number	Recov / Total (in) % Recovery	Туре	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft³	Qu (tsf) Failure T	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
Α	13/24 54%	ss	<i>0-4</i> <i>4-4</i> N=8				2	Yellowish brown (10YR5/6), moist, stiff, lean CLAY, with s silt, few very fine- to fine-grained sand, trace small grav		618	
2A	24/24 100%	ss	3-4 5-6 N=9				l]	Gray (10YR5/1) with 20% yellowish brown (10YR5/6) and very dark grayish brown (10YR3/2) mottles, moist, stiff, le CLAY, with some silt, trace very fine- to fine-grained sand small gravel.	ean /// 4	616	
BA BA	24/24 100%	ss	2-4 5-7 N=9				6	Very dark grayish brown (10YR3/2) with 10% gray (10YR mottles, moist, stiff, lean CLAY, with some silt, trace very to fine-grained sand, and small gravel.		614	
, 7.	24/24 100%	SH								612	
δA	18/24 75%	ss	1-2 4-5 N=6				8	Gray (10YR5/1) with 15% yellowish brown (10YR5/6) mot moist, stiff, lean CLAY, with some silt, little very fine- to fine-grained sand, few small gravel.		610	
6A	24/24 100%	ss	1-3 3-4 N=6				10	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mot moist to wet, medium stiff, lean CLAY, with some very fin		608	
7A	24/24 100%	ss	2-6 12-13 N=18				12	fine-grained sand, little silt, few small gravel. Yellowish brown (10YR5/6), moist to wet, medium stiff, le CLAY, with some very fine- to fine-grained sand, little silt, small gravel. Gray (10YR5/1), moist to wet, medium stiff, SILT, with so very fine- to fine-grained sand, few clay, trace small gravel.	few ome	606	1" wet, SAND at 13
3A	24/24 100%	ss	2-6 12-13 N=18				14		00000	604	
)A	24/24 100%	ss	6-15 18-22 N=33				16	Yellowish brown (10YR5/6), moist, hard, SILT, with some some to little sand, few small gravel.	clay,		Vertical fracture wi very fine- to fine-grained sand from 16.9 to 18 ft.
0A	24/24 100%	ss	6-13 16-22 N=29				18 -	Dark grayish brown (10YR4/2) with frequent yellowish re (5YR4/6) oxidation along fractures, moist, hard, lean CL with some silt, few very fine- to fine-grained sand, trace s gravel.	AY, ///	600	

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois Project: 20E0111A

DATES: Start: 2/5/2021

Finish: 2/5/2021 WEATHER: Clear, cold (20s) **CONTRACTOR:** Roberts

Rig mfg/model: CME-75 Track Rig

Drilling Method: 3.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter

BOREHOLE ID: G311D

Well ID: G311D Surface Elev: 618.39 ft. MSL

Completion: 60.00 ft. BGS **Station:** 872,238.70N

2,515,881.80E

% Recovery			(o	t ₃)		101 00104	PHIC MAP INFORMATION:	WATER LEVEL IN	CIMATIO	41
cover		/ 6 in Ilue	Water Content (%)	Dry Density (Ib/ft ³)	f) Qp (tsf) e Type	Townsh	ngle: Coffeen, IL ip: East Fork Township 11, Tier 7N; Range 3W	<u>▼</u> = 11.20 - <u>▼</u> = <u>▼</u> =	During Drilli	ng
% Rec	Туре	Blows / 6 in N - Value RQD	Water	Dry De	Qu (tsf) Failure T	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1/24 00%	SS	5-15 20-26 N=35				22 —			598 	No oxidation below
2/24	ss	2-3 14-17 N=17				24			596 	22 ft. Trace medium grave
1/24 00%	ss	6-11 14-20 N=25				24		1,	594	below 24 ft.
1/24 00%	ss	4-8 11-16 N=19				26			592 	
1/24 00%	SH					30 —	Dark gray (10YR4/1) with frequent yellowish red (5YR4/6 oxidation along fractures, moist, hard, lean CLAY, with so	5) me	590 	
1/24 00%	ss	<i>0-3</i> <i>5-8</i> N=8				32 —	silt, few very fine- to fine-grained sand, trace small grave		588 	
1/24 00%	ss	2-4 6-8 N=10							586 	
1/24 00%	ss	2-5 7-7 N=12				34 = = = = = = = = = = = = = = = = = = =			584	Gravel plugged shoe in Run 18. Trace large gravel from 35 to 36 ft.
3/24 54%	ss	2-7 8-11 N=15				36		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	582	Trace lignite and wood fragments below 36 ft.
1/24	ss	2-6 10-8 N=16				38			580	
1/3/2000 11/3/20	24 0% 24 0% 24 0% 24 0% 24 1% 24 1%	24	24	24	24	24	24	24	24	24 SS 2-16 SS 14-17 SS SS 14-17 SS SS SS SS SS SS SS

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 2/5/2021 **Finish:** 2/5/2021

WEATHER: Clear, cold (20s)

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig

Drilling Method: 3.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter



BOREHOLE ID: G311D Well ID: G311D

 Surface Elev:
 618.39 ft. MSL

 Completion:
 60.00 ft. BGS

 Station:
 872,238.70N

2.515.881.80E

Page 3 of 4

SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Dry Density (Ib/ft³ Ξ Water Content (%) ▼ = 11.20 - During Drilling) Qp (tsf) Type Quadrangle: Coffeen, IL Recov / Total (% Recovery <u>v</u> = Township: East Fork Township *Blows / 6 ir* N - Value **RQD** ▼ = Section 11, Tier 7N; Range 3W Qu (tsf) Failure T Number Borehole Elevation Lithologic Description Remarks ft. MSL Dark gray (10YR4/1) with frequent yellowish red (5YR4/6) 3-3 16/24 oxidation along fractures, moist, hard, lean CLAY, with some SS 21A 6-8 67% silt, few very fine- to fine-grained sand, trace small gravel. N=9 [Continued from previous page] 22A 42 Greenish gray (GLEY15/1) with 20% dark reddish brown (10YR3/2) mottles, moist, medium stiff, lean CLAY, with some 576 silt. 24/24 22B SS 7-8 100% N=11 574 1-3 24/24 23A 5-7 100% 46 572 20/24 24A 10-8 83% N = 14Dark grayish brown (10YR4/1) with frequent yellowish red (5YR4/6) oxidation along fractures, moist, hard, lean CLAY, with some silt, few very fine- to fine-grained sand, trace small 48 to large gravel. 570 3-5 24/24 25A 8-13 100% N=13 568 24/24 10-12 26A 100% N=15 52 566 2-6 24/24 10-14 100% N=16 27A Dark gray (10YR4/1) with 20% greenish gray (GLEY15/1) and Trace small gravel 564 5% yellowish brown (10YR5/6) mottles, moist, very stiff, lean below 54.3 ft. 4-7 24/24 CLAY, with some silt, trace very fine-grained sand and small 28A 7-11 100% gravel. N=14 562 Greenish gray (GLEY16/1) with 40% yellowish brown 2-5 24/24 9-11 (10YR5/6) mottles, moist, very stiff, lean CLAY, with some silt, 29A 100% N=14 trace very fine-grained sand and small gravel. 0.5" small to medium GRAVEL. 560 Yellowish brown (10YR5/6) with 30% greenish gray (GLEY16/1) mottles, moist, very stiff, lean CLAY, with some silt, trace very fine-grained sand and small gravel. 3-7 24/24 30A SS 10-13 100% N=17 60 NOTE(S): G311D installed in borehole.

 $\begin{cal}CLIENT: Illinois Power Generating Co.\end{cal}$

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 2/5/2021

DATES: Start: 2/5/2021 Finish: 2/5/2021

WEATHER: Clear, cold (20s)

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig **Drilling Method:** 3.25" HSA w/SS sampler

_____**g** ...**__**__ ...

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

BOREHOLE ID: G311D Well ID: G311D

Elevation

ft. MSL

Borehole

Detail

Surface Elev: 618.39 ft. MSL Completion: 60.00 ft. BGS

Station: 872,238.70N 2,515,881.80E

Remarks

SAMPLE	TESTING	TOPOGRAPHIC MAP INFORMATION:	WATER LEVEL INFORMATION:
(ii)	(%) (%)	Quadrangle: Coffeen, IL	∇ = 11.20 - During Drilling
<u>₹</u>	tent (%)	Township: East Fork Township	<u> </u>
60.	늘 늘 뜻)	O (: 44 T: 7N D 0)4/	

Section 11, Tier 7N; Range 3W

Depth Lithelegie Description

Lithologic Description

End of boring = 60.0 feet

NOTE(S): G311D installed in borehole.

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 2/4/2021 **Finish:** 2/4/2021

WEATHER: Rain, cold (30s)

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig **Drilling Method:** 4.25" HSA w/SS sampler

FIELD STAFF, Drillow Mott

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter



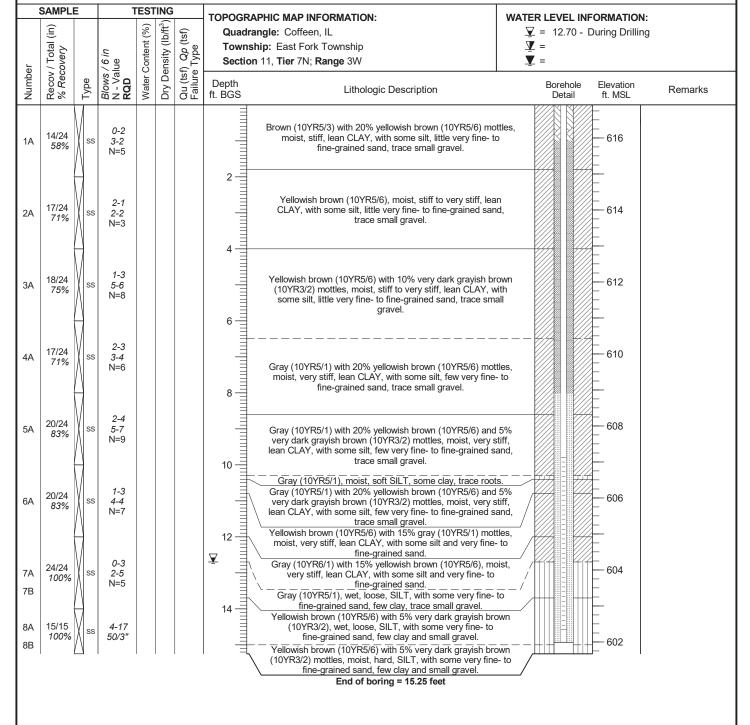
BOREHOLE ID: G312 Well ID: G312

 Surface Elev:
 616.92 ft. MSL

 Completion:
 15.25 ft. BGS

 Station:
 872,260.90N

2,516,557.40E



CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 1/14/2021 **Finish:** 1/14/2021

WEATHER: Overcast, cool (40s)

CONTRACTOR: Roberts

Rig mfg/model: GeoProbe 8040DT **Drilling Method:** 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

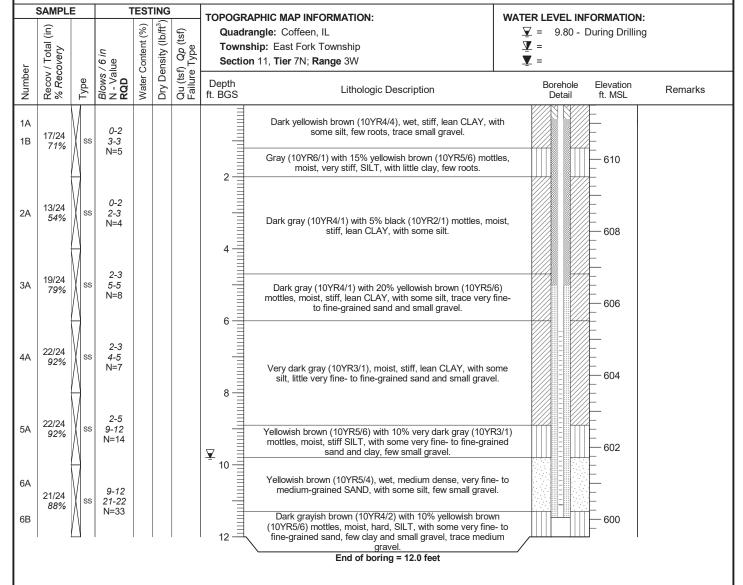
BOREHOLE ID: G313 Well ID: G313

 Surface Elev:
 611.51 ft. MSL

 Completion:
 12.00 ft. BGS

 Station:
 871.976.80N

2,516,803.70E



CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 2/26/2021

Finish: 2/26/2021
WEATHER: Clear, mild (40s)

CONTRACTOR: Holcomb Foundation Engineering Co.

Rig mfg/model:

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Steve Helper: Jeff

Eng/Geo: C. Colin Winter

HANSON

BOREHOLE ID: G314

Well ID: G314

 Surface Elev:
 611.11 ft. MSL

 Completion:
 20.05 ft. BGS

 Station:
 871,630.20N

2,516,852.10E

	SAMPLI		T		ING			21.9.0001 0. 00111 William			2,010,002.102
Number	Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft³)	Qu (tsf) Qp (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
NC	DTE(S):	G314	installed	in b	oreh	ole.	2 4 4 6 8 10 12 14 16 18 18 10 18 20 1				
1											Page 1 of 2

CLIENT: Illinois Power Generating Co. CONTR

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 2/26/2021

Finish: 2/26/2021 WEATHER: Clear, mild (40s)

CONTRACTOR: Holcomb Foundation Engineering Co.

Rig mfg/model:

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Steve Helper: Jeff

Eng/Geo: C. Colin Winter

HANSON

BOREHOLE ID: G314

Well ID: G314

 Surface Elev:
 611.11 ft. MSL

 Completion:
 20.05 ft. BGS

 Station:
 871,630.20N

2,516,852.10E

SAMPLE	TESTING				
Number Recov / Total (in) % Recovery Type	Type Blows / 6 in N - Value RQD Water Content (%) Dry Density (lb/ft²)	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks

NOTE(S): G314 installed in borehole.

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 2/10/2021 **Finish:** 2/12/2021

WEATHER: Overcast, very cold (10s)

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter



BOREHOLE ID: G314D Well ID: G314D

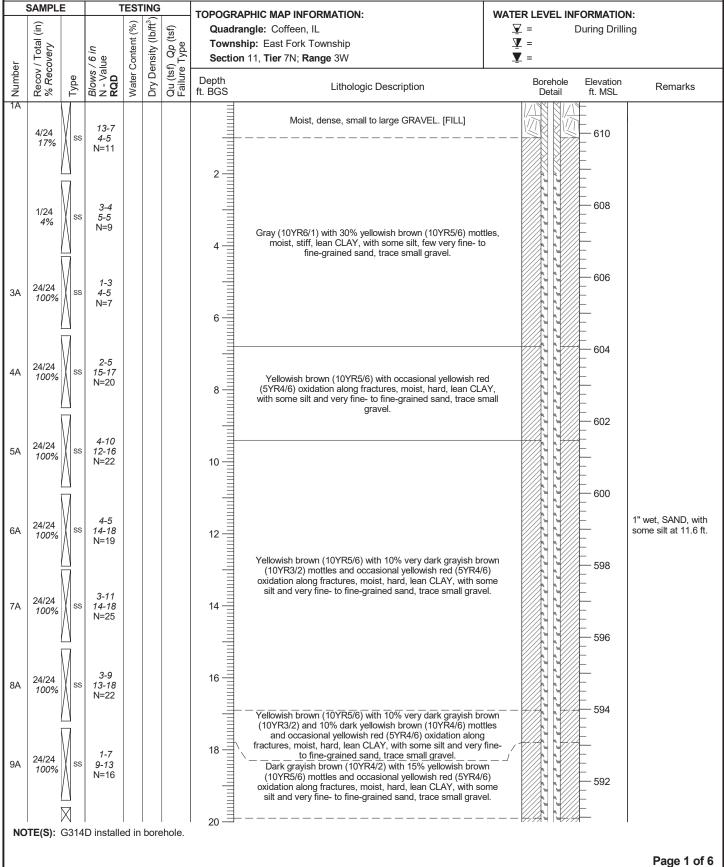
 Surface Elev:
 610.87 ft. MSL

 Completion:
 100.30 ft. BGS

 Station:
 871.642.00N

 Station:
 871,642.00N

 Vinter
 2,516,853.90E



CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 2/10/2021 **Finish:** 2/12/2021

WEATHER: Overcast, very cold (10s)

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig **Drilling Method:** 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt
Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

BOREHOLE ID: G314D Well ID: G314D

 Surface Elev:
 610.87 ft. MSL

 Completion:
 100.30 ft. BGS

 Station:
 871,642.00N

 2.516.853.90E

Page 2 of 6



CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 2/10/2021 **Finish:** 2/12/2021

WEATHER: Overcast, very cold (10s)

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig **Drilling Method:** 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt
Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

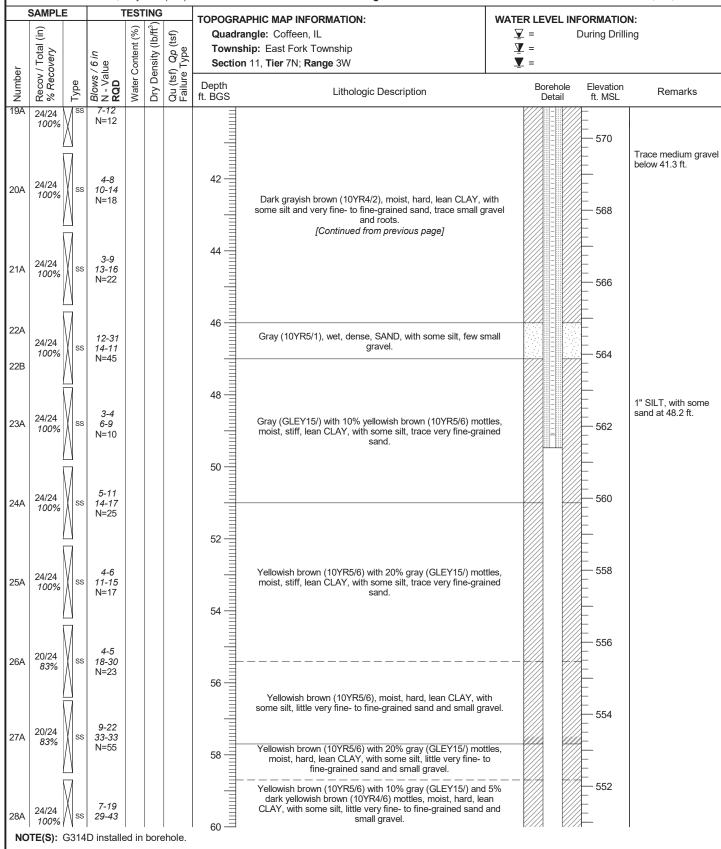
BOREHOLE ID: G314D Well ID: G314D

 Surface Elev:
 610.87 ft. MSL

 Completion:
 100.30 ft. BGS

 Station:
 871,642.00N

 2.516.853.90E



CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois Project: 20E0111A

DATES: Start: 2/10/2021 Finish: 2/12/2021

WEATHER: Overcast, very cold (10s)

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt

Helper: Corey

Eng/Geo: C. Colin Winter

BOREHOLE ID: G314D

Well ID: G314D

Surface Elev: 610.87 ft. MSL Completion: 100.30 ft. BGS 871,642.00N Station:

2.516.853.90E SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: (lb/ft³ **During Drilling** Ξ ▼ = Water Content (%) Qp (tsf) Type Quadrangle: Coffeen, IL Recov / Total (% Recovery <u>________</u> = Township: East Fork Township **Dry Density** *Blows / 6 ir* N - Value **RQD** ▼ = Section 11, Tier 7N; Range 3W Qu (tsf) Failure T Number Depth Borehole Elevation Lithologic Description Remarks ft. BGS ft. MSL 550 13-20 20/24 29A SS 30-38 62 83% N=50 548 Yellowish brown (10YR5/6) with 10% gray (GLEY15/) and 5% dark yellowish brown (10YR4/6) mottles, moist, hard, lean CLAY, with some silt, little very fine- to fine-grained sand and 8-15 24/24 30-34 30A small gravel. 100% N=45 [Continued from previous page] 546 5-17 24/24 66 31A SS 28-29 100% N=45 544 68 7-18 24/24 Yellowish brown (10YR5/6) with 10% gray (GLEY15/), 5% dark yellowish brown (10YR4/6) and 5% very dark grayish brown (10YR3/2) mottles, moist, hard, lean CLAY, with some silt, little SS 29-43 100% N=47 542 very fine- to fine-grained sand and small gravel. 70 Dark grayish brown (10YR4/2) with 5% gray (GLEY15/), 5% 5-14 24/24 32-26 yellowish brown (10YR5/6), and 5% light gray (10YR7/1) mottles, moist, hard, lean CLAY, with some silt, little sand and SS 100% 540 N=46 33A gravel. 24/24 Yellowish brown (10YR5/6) with 5% gray (GLEY15/), 5% dark grayish brown (10YR4/2), and 5% light gray (10YR7/1) 34A 26-38 538 100% N=43 mottles. 5-12 24/24 536 35A 24-24 100% N=36 Dark grayish brown (10YR4/2) with 5% gray (GLEY15/), 5% yellowish brown (10YR5/6), and 5% light gray (10YR7/1) 14-24 534 24/24 20-33 100% N=44 Dark grayish brown (10YR5/2) with 20% reddish brown 36A (5YR4/4) and 5% light gray (10YR7/1) mottles, moist, hard, lean CLAY, with some silt, trace very fine- to fine-grained sand and small gravel. Dark gray (10YR4/1) with 5% light gray (10YR7/1) mottles, 532 6-14 24/24 moist, hard, lean CLAY, with some silt, trace very fine- to 37A 17-31 100% fine-grained sand and small gravel. N = 3180 NOTE(S): G314D installed in borehole.

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 2/10/2021 **Finish:** 2/12/2021

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey **HANSON**

BOREHOLE ID: G314D Well ID: G314D

 Surface Elev:
 610.87 ft. MSL

 Completion:
 100.30 ft. BGS

 Station:
 871.642.00N

WEATHER: Overcast, very cold (10s) Eng/Geo: C. Colin Winter 2.516.853.90E SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Dry Density (lb/ft³ Ξ Water Content (%) ▼ = **During Drilling**) Qp (tsf) Type Quadrangle: Coffeen, IL Recov / Total (% Recovery **V** = Township: East Fork Township Blows / 6 in N - Value **RQD** Section 11, Tier 7N; Range 3W ▼ = Qu (tsf) Failure T Number Borehole Elevation Lithologic Description Remarks ft. BGS ft. MSL 530 7-17 Dark gray (10YR4/1) with 5% light gray (10YR7/1) mottles, 24/24 38A SS 21-29 moist, hard, lean CLAY, with some silt, trace very fine- to 100% N=38 fine-grained sand and small gravel. 82 Trace roots at 82 ft. [Continued from previous page] 528 8-24 24/24 39A 26-27 100% N=50 Very dark gray (10YR3/1), moist, hard, lean CLAY, with some silt. 526 5-9 24/24 Dark gray (10YR4/1) with 5% black (10YR2/1) mottles, moist, 40A 10-13 100% 86 hard, lean CLAY, with some silt. N=19 524 6-10 24/24 Dark greenish gray (GLEY14/1) with 5% yellowish brown (10YR5/6) mottles, moist, hard, lean CLAY, with some silt. 25-33 88 Few very fine- to 100% N=35 fine-grained sand below 88 ft. 522 42A 3-50/5' 100% 520 43A 5/5 100% Ss 50/5' 92 Light reddish brown (2.5YR6/3) with 10% gray (GLEY15/), dry, hard, SILT, with few clay and very fine-grained sand. 518 44A 5/5 100% Ss 50/5" 516 45A 8/8 100% SS 49-50/2 96 514 Light reddish brown (2.5YR6/3) with 10% gray (GLEY15/) and 5% dark yellowish brown (10YR4/6) mottles, dry, hard, SILT, 46A with few clay and very fine-grained sand. SS 25-50/5 512

100

NOTE(S): G314D installed in borehole.

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 2/10/2021

Finish: 2/12/2021

WEATHER: Overcast, very cold (10s)

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

BOREHOLE ID: G314D

Well ID: G314D Surface Elev: 610.87 ft. MSL

Completion: 100.30 ft. BGS
Station: 871,642.00N

2,516,853.90E

	SAMPL	E	1	EST			TOPOGRAPHIC MAP INFORI	MATION:	WATER LEVEL IN	IFORMATION	l:
1	(ii)			(%)	o/ft³)	sf)	Quadrangle: Coffeen, IL		<u>▼</u> =	During Drillin	ıg
	otal ery		u	ent	 	Qp (t ype	Township: East Fork Tow	vnship	<u>√</u> =		
Ē			/ 6 i, lue	Conte	nsity	O T	Section 11, Tier 7N; Rang	e 3W	▼ =		
qunN	Recov % Rec	Type	Blows N - Val RQD	Water (Dry De	Qu (tsf Failure	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
47A	3/3	⊠ ss	50/3"							∐ I	

100% End of boring = 100.3 feet

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

TESTING

Location: Coffeen, Illinois Project: 20E0111A

DATES: Start: 1/13/2021 Finish: 1/13/2021

WEATHER: Overcast, cool (40s)

SAMPLE

CONTRACTOR: Roberts

Rig mfg/model: GeoProbe 8040DT

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter

TOPOGRAPHIC MAP INFORMATION: Quadrangle: Coffeen, IL

WATER LEVEL INFORMATION:

 ∇ = 13.00 - During Drilling

BOREHOLE ID: G315

Surface Elev:

Completion:

Station:

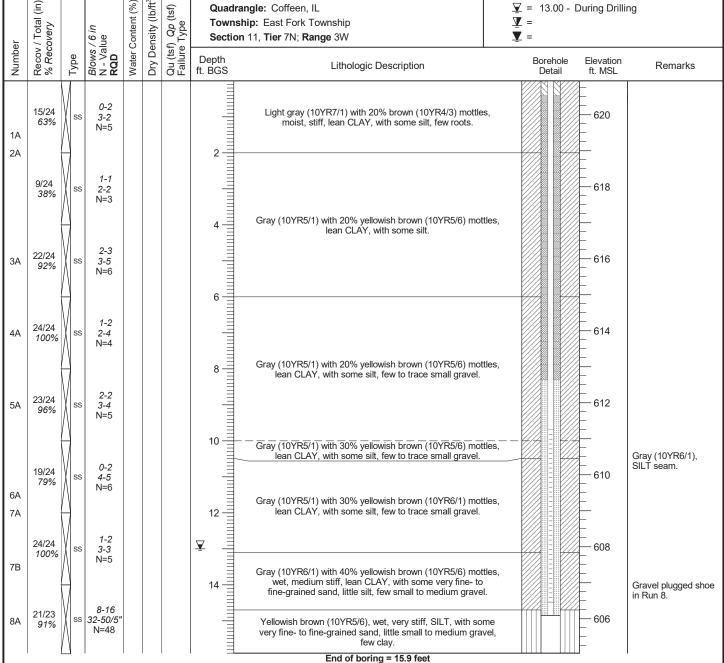
Well ID: G315

620.94 ft. MSL

15.90 ft. BGS

871,385.00N

2.516.086.60E



CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 1/14/2021

Finish: 1/14/2021 WEATHER: Overcast, cold (30s)

CONTRACTOR: Roberts

Rig mfg/model: GeoProbe 8040DT

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

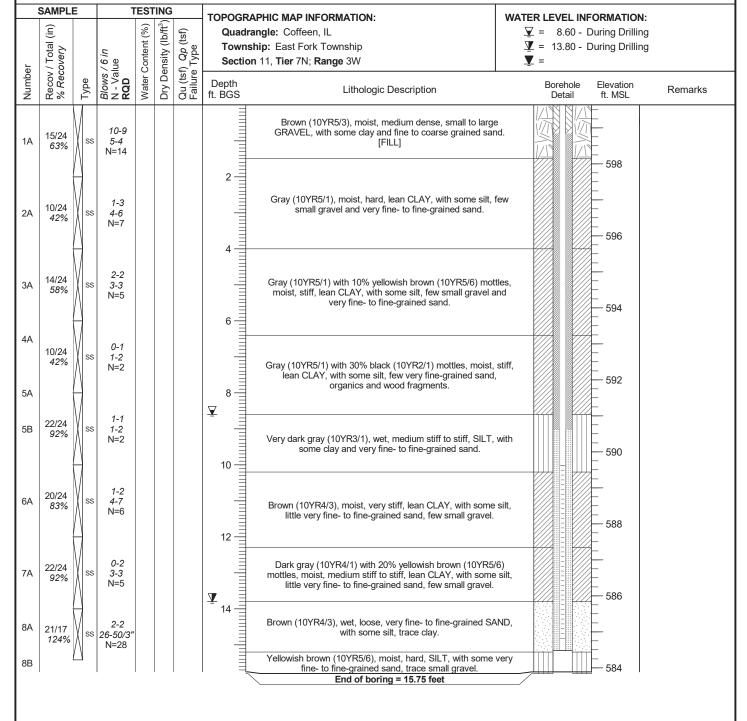
BOREHOLE ID: G316 Well ID: G316

 Surface Elev:
 599.64 ft. MSL

 Completion:
 15.75 ft. BGS

 Station:
 871.643.10N

2,517,211.60E



CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 1/13/2021 **Finish:** 1/14/2021

WEATHER: Overcast, cold (30s to 40s)

CONTRACTOR: Roberts

Rig mfg/model: GeoProbe 8040DT

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt
Helper: Corey

Eng/Geo: C. Colin Winter



BOREHOLE ID: G317 Well ID: G317

 Surface Elev:
 638.85 ft. MSL

 Completion:
 36.00 ft. BGS

 Station:
 871,234.20N

2,517,087.40E

Page 1 of 2

SAMPLE			Т	TING		TOPOGRAI	NEORMATIO	N:			
er	Recov / Total (in) % Recovery		<i>Blows / 6 in</i> N - Value RQD	Water Content (%)	Dry Density (lb/ft³)	Qu (tsf) Qp (tsf) Failure Type	Quadrai Townsh	ingle: Coffeen, IL ip: East Fork Township 11, Tier 7N; Range 3W	WATER LEVEL INFORMATION:		
Number	Reco	Туре	Blows N - Vs	Water	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A 17/24 71%		ss	2-2 5-3 N=7				2			638	
Α	12/24 50%	ss	3-5 4-4 N=9				2-11	Light yellowish brown (10YR6/4) with 20% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, fine-grained sand and small to medium gravel, trace to roots.	little ///	636	
A	15/24 63%	SS	1-2 3-4 N=5						2,7,7,7,7	634	
A	11/24 46%	ss	3-5 5-4 N=10				6 —	Gray (10YR6/1) with 20% yellowish brown (10YR5/6) mot moist, stiff, lean CLAY, with some silt, few fine-grained sa and small gravel.	tles, and	632	
A	12/24 50%	ss	3-4 4-4 N=8				8 = = = = = = = = = = = = = = = = = = =	Brownish yellow (10YR6/8) with 20% gray (10YR6/1) mot moist, stiff, SILT, with little fine-grained sand and small gra	tles, avel.	630	
A	4/24 17%	ss	2-2 3-4 N=5				10 =	Gray (10YR5/1) with 30% very dark gray (20YR3/1) mott moist, very stiff, lean CLAY, with some silt, trace lignite fragments and organics.		628	Wood fragment plugged shoe in R 6.
A B	14/24 58%	ss	1-3 4-3 N=7					Gray (10YR5/1) with 10% yellowish brown (10YR5/4) mot moist, stiff, SILT, with some fine-grained sand, little sm	tles,	626	Gravel plugged sh in Run 7.
A	20/24 83%	ss	2-2 3-4 N=5				14 =	Gray (10YR5/1) with 10% yellowish brown (10YR5/4) and very dark gray (10YR3/1) mottles, moist, very stiff, lean Cl with some silt, few fine-grained sand and small gravel.	10% _AY,	624	
A	17/24 71%	ss	1-2 3-4 N=5				16			622	
A	16/24 67%	ss	0-2 5-5 N=7				18	Gray (10YR5/1) with 10% very dark gray (10YR3/1) mott moist, medium stiff, lean CLAY, with some silt, little sma gravel, trace lignite fragments and organics.		620	Thin wood fragme seam at 19.4 ft.

CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 1/13/2021 **Finish:** 1/14/2021

WEATHER: Overcast, cold (30s to 40s)

CONTRACTOR: Roberts

Rig mfg/model: GeoProbe 8040DT

Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt
Helper: Corey

Eng/Geo: C. Colin Winter

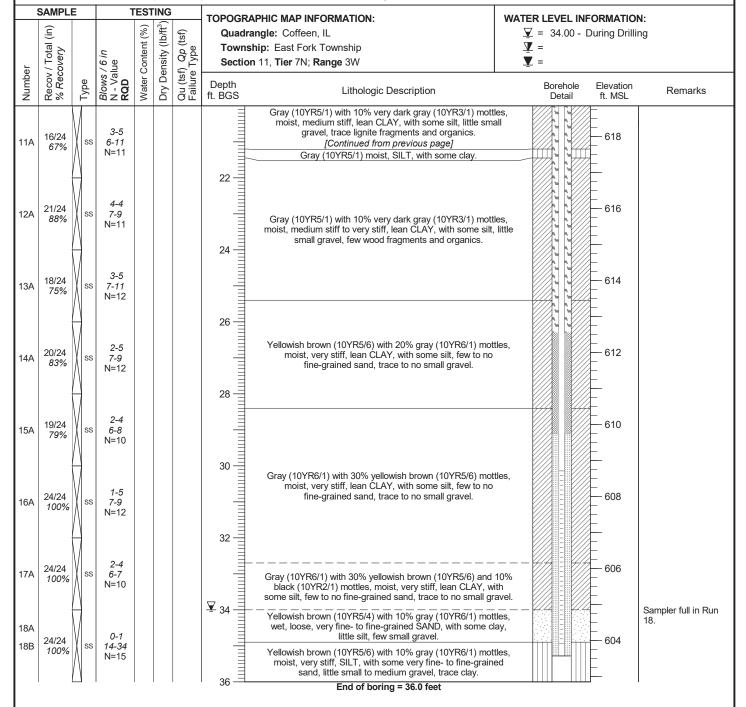
HANSON

BOREHOLE ID: G317

Well ID: G317 Surface Elev: 638.85 ft. MSL

Completion: 36.00 ft. BGS
Station: 871,234.20N

2,517,087.40E



CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois **Project:** 20E0111A

DATES: Start: 2/8/2021 **Finish:** 2/8/2021

WEATHER: Overcast, very cold (10s)

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

Eng/Geo: C. Colin Winter

HANSON

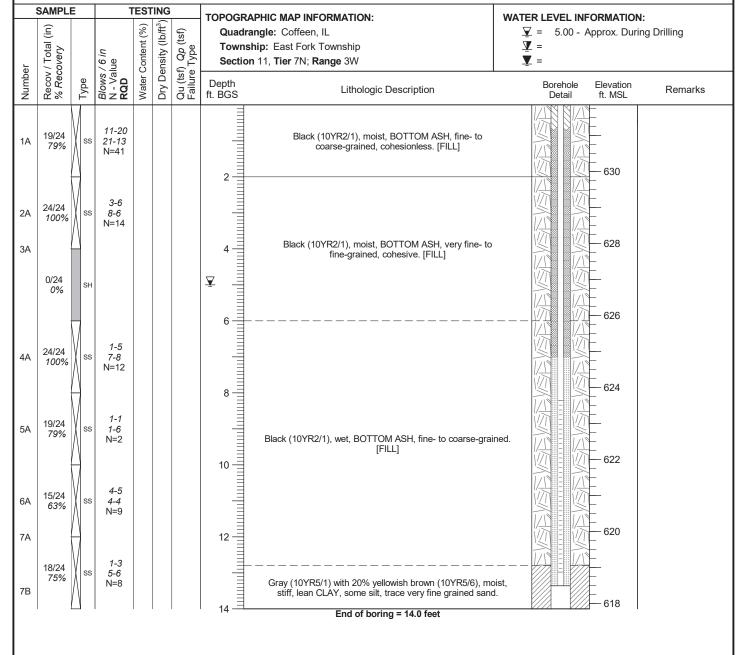
BOREHOLE ID: XPW01 Well ID: XPW01

 Surface Elev:
 631.85 ft. MSL

 Completion:
 14.00 ft. BGS

 Station:
 871,638.70N

2.515.366.30E



CLIENT: Illinois Power Generating Co.

Site: Coffeen Part 845 Groundwater

Location: Coffeen, Illinois Project: 20E0111A

DATES: Start: 2/8/2021 Finish: 2/8/2021

WEATHER: Overcast, very cold (10s)

CONTRACTOR: Roberts

Rig mfg/model: CME-75 Track Rig Drilling Method: 4.25" HSA w/SS sampler

FIELD STAFF: Driller: Matt Helper: Corey

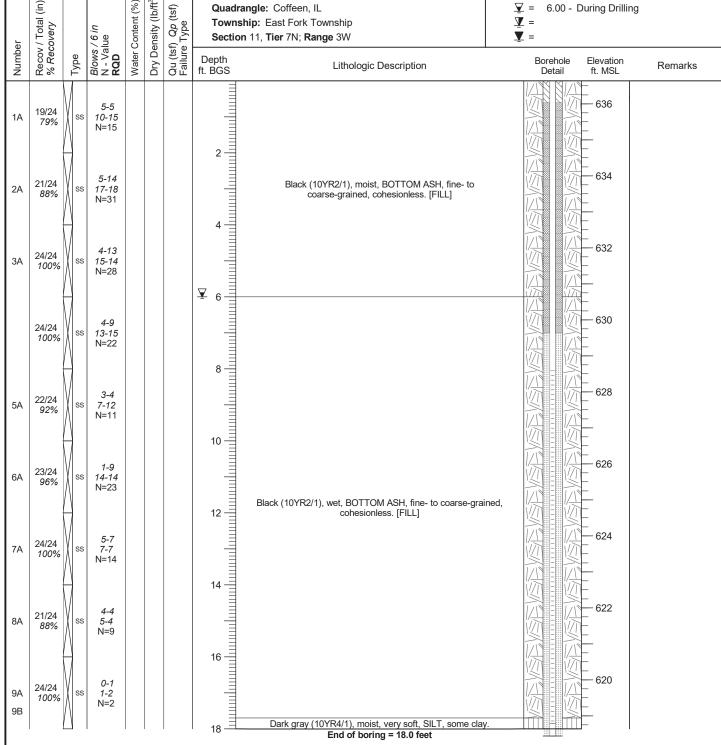
Eng/Geo: C. Colin Winter

BOREHOLE ID: XPW02 Well ID: XPW02

Surface Elev: 636.64 ft. MSL Completion: 18.00 ft. BGS Station: 871,987.10N

2,515,627.30E

SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION: Ξ Dry Density (lb/ft³) Qp (tsf) Type Quadrangle: Coffeen, IL \mathbf{Y} = 6.00 - During Drilling **V** = Township: East Fork Township **y** = Section 11, Tier 7N; Range 3W



NOTE(S): XPW02 installed in borehole.

WELL CONSTRUCTION LOGS

Illinois Enviror	nmental Protection Ag		Well Completion Report						
Site #:	Count	y: Mont	gomery		W	/ell #:	G45D		
Site Name: Coffeen Power Sta	ation - Ash Pond 2				В	orehole #:	G405D		
State Plane Coordinate: X 2,515,322	2.2 Y 873,998.0 (or)	Latitude: _	39°	3' 51.657"	Longitud	e: <u>-89°</u>	23' 46.612"		
Surveyed By: Gary C. Rogers			IL Regis	tration #: <u>035-0</u>	02957				
Drilling Contractor: Bulldog D	Orilling, Inc.		Driller:	J. Dittmaier					
Consulting Firm: Hanson Prof	essional Services Inc.	Geologist: Rhonald W. Hasenyager, LPG #196-000246							
Drilling Method: Hollow stem	auger		Drilling Fluid (Type):none						
Logged By: Kristen L. Theesf	îeld		Date Sta	rted: 8/16/20	016 Date	e Finished:	8/17/2016		
Report Form Completed By:Su	ızanna L. Keim		Date: _	8/24/2016					
ANNULAR SPA	CE DETAILS			Elevations (MSL)*	Depths (BGS)	(0.01 1	ft.)		
				624.16	3.22	Top of Protect	ive Casing		
				623.81	2.87	Top of Riser F	ipe		
Type of Surface Seal: Concrete				620.94	0.00	Ground Surfac	ce		
				618.94	2.00	Top of Annula			
Type of Annular Sealant: High-						Top of Funda	ii Souldiii		
Installation Method:Tremi	e								
Setting Time: >24 hours			-			Static Water L (After Completic			
Type of Bentonite Seal Gran		+							
Installation Method: Gravit	(choose one)	V V		_590.59_	30.35	Top of Seal			
Setting Time: <u>38 minutes</u>				589.62	31 32	Top of Sand P	'ack		
						Top of Sunu 1			
Type of Sand Pack: Quartz San				589.06	31.88	Top of Screen			
Grain Size: 10-20 (sie Installation Method: Gravit	,					•			
ilistaliation Method. Gravit	<u>y</u>			579.42	41.52	Bottom of Scr	een		
Type of Backfill Material: <u>n/a</u>	(if applicable)			579.02	_41.92_	Bottom of We	П		
Installation Method:				<u>578.94</u>	42.00	Bottom of Bor	ehole		
				* Referenced to a	i National Geodet	ic Datum			
		Г	CAS	SING MEAS	SUREMENTS				
WELL CONS	STRUCTION MATERIALS			Diameter of Boreho	ole	(inch			
	ne type of material for each area)		Г	ID of Riser Pipe	on atl-	(inch			
				Protective Casing I Riser Pipe Length			et) 5.0 et) 34.75		
Protective Casing	SS304 SS316 PTFE PVC	OTHER: Ste		Bottom of Screen to		•	et) 34.73		
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC	OTHER:		Screen Length (1s			et) 9.64		
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC	OTHER:		Total Length of Casing			et) 44.79		

SS304

Well Completion Form (revised 02/06/02)

SS316

PTFE PVC OTHER:

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environmental Protection Agency				Wel	l Completion	Report
Site #:	County: _	Montgomery	7	V	Vell #:G	16D
Site Name: Coffeen Power Sta	ation - Ash Pond 2			B	Sorehole #:C	3406D
State Plane Coordinate: X 2,514,697	7.8 Y 872,519.7 (or) Lati	itude: 39°	3' 37.09	98" Longitud	le: <u>-89°</u> 23	<u>3' 54.687"</u>
Surveyed By: Gary C. Rogers		IL Reg	istration #:03	5-002957		
Drilling Contractor: Bulldog D	rilling, Inc.	Driller:	J. Dittmaier			
Consulting Firm: Hanson Profe	essional Services Inc.	Geolog	ist: Rhonald	W. Hasenyage	r, LPG #196-000	246
Drilling Method: Hollow stem	auger	Drilling	g Fluid (Type): _	none		
Logged By: Kristen L. Theesfo	eld	Date St	arted:8/19	/2016 Dat	e Finished: 8/	19/2016
	zanna L. Keim		8/24/201	<u> </u>		
ANNULAR SPA			Elevation (MSL)*		(0.01 ft.)	
			625.53	, ,	Top of Protective	Casing
	c		625.24	-3.33	Top of Riser Pipe	
Type of Surface Seal: Concrete			621.91	0.00	Ground Surface	
			619.91	2.00	Top of Annular S	Sealant
Type of Annular Sealant: High-s						
Installation Method:Tremie	2				Static Water Lev	al
Setting Time: _ >24 hours					(After Completion)	21
Type of Bentonite Seal Grant	Pellet Slurry — (choose one)					
Installation Method: Gravity	y		582.34	39.57	Top of Seal	
Setting Time:>12 hours			581.21	40.70	Top of Sand Pack	ζ
Type of Sand Pack: Quartz Sand	d					
Grain Size: 10-20 (sie	_		_580.30	_41.61_	Top of Screen	
Installation Method: Gravit	y					
Type of Backfill Material:n/a			570.65 570.26		Bottom of Screen Bottom of Well	-
Type of Backfill Material	(if applicable)		370.20		Bottom of Wen	
Installation Method:			569.91 * Referenced	to a National Geode	Bottom of Borehotic Datum	ole
				A CDIC NEL		
			Diameter of Bo		SUREMENTS (inches)	8.0
	STRUCTION MATERIALS e type of material for each area)		ID of Riser Pipe		(inches)	2.0
(Choose on	JF- Or material for each area)		Protective Casir		(feet)	5.0
			Riser Pipe Leng	th	(feet)	44.94
Protective Casing		THER: Steel	Bottom of Scree		(feet)	0.39
Riser Pipe Above W.T.		HER:	Screen Length		ot) (feet)	9.65
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OT	HEK:	Total Length of	Casing	(feet)	54.98

SS304

Well Completion Form (revised 02/06/02)

SS316

Total Length of Casing

**Hand-Slotted Well Screens Are Unacceptable

Screen Slot Size **

Illinois Environmental Protection A	gency			Well	Completi	on Report
Site #: Coun	nty: Montgo	omery		W	/ell #:	G101
Site Name: CCB Management Facility				В	orehole #:	G101
State Plane Coordinate: X 2,514,214.3 Y 876,551.8 (or)	Latitude:	39° _	<u>4' 17.000"</u>	Longitude	e: <u>-89°</u> _	24' 0.400"
Surveyed By: Jeffrey D. Emrick	II	L Registra	ation #: <u>035-0</u>	03507		
Drilling Contractor: Layne-Western Co	D	Oriller:	T. List			
Consulting Firm: Hanson Professional Services Inc.	G	Geologist:	Rhonald W.	Hasenyager	<u>; LPG #196-</u> (000246
Drilling Method: Hollow stem auger	D	Drilling Flu	uid (Type):n/a	ì		
Logged By: Suzanna Simpson	D	ate Starte	ed:2/2/201	10 Date	e Finished:	2/2/2010
Report Form Completed By: Suzanna Simpson	D	Date:	2/4/2010			
ANNULAR SPACE DETAILS			Elevations (MSL)*	Depths (BGS)	(0.01	ft.)
			627.89	-2.62	Top of Protec	tive Casing
			627.60	-2.33	Top of Riser	
Type of Surface Seal: Concrete			625.27	0.00	Ground Surfa	ce
_			622.27	3.00	Top of Annul	
Type of Annular Sealant: High-solids bentonite			022.21		Top of Ailliui	ar Scaram
Installation Method: Tremie						
Setting Time:>24 hr.			617.96	7.31	Static Water 1 (After Completi	
Type of Bentonite Seal Granular Pellet Slurry						
(choose one) Installation Method: Gravity			614.27	11.00	Top of Seal	
Setting Time: 20 min						
			612.14	13.13	Top of Sand I	Pack
Type of Sand Pack: Quartz sand			(00.50	15 (0	T (0	
Grain Size:10/20 (sieve size)			609.59	15.68	Top of Screen	l
Installation Method: <u>Gravity</u>			604.95	20.32	Bottom of Sci	-een
Type of Backfill Material: Quartz sand			604.38	20.89	Bottom of We	
(if applicable) Installation Method: Gravity			603.35	21.92	Bottom of Bo	rehole
			* Referenced to a			
			CAS	SING MEAS	SUREMENTS	S
WELL CONCEDUCTION MATERIALS		Di	ameter of Boreho	ole	(incl	nes) 8.0
WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)			of Riser Pipe		(incl	
			otective Casing L	ength		(eet) 5.0
Protective Casing SS304 SS316 PTFE PVC	OTHER:	\neg	ser Pipe Length	F 16	•	eet) 18.01
Riser Pipe Above W.T. SS304 SS316 PIFE PVC			ereen Length (1s		•	(eet) 0.57 (eet) 4.64
Riser Pipe Below W.T. SS304 SS316 PTFE PVC			otal Length of Cas			(eet) 4.64 (eet) 23.22

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

Illinois Environ	mental Protection Ager	ıcy		Well C	Completion	Report
Site #:	County:	Montgomery		Well	#: <u>G102 (N</u>	/W3S)
Site Name: CCB Management	Facility			Bore	hole #:SI	3-03a
State Plane Coordinate: X 2,514,531	<u>.5</u> Y <u>876,554.8</u> (or) La	titude:		Longitude:		
Surveyed By: <u>Darren E. Forgy</u>		IL Regi	stration #:035-0	03637		
Drilling Contractor: Testing Ser	rvice Corp.	Driller:	B. Williamson			
Consulting Firm: Hanson Profe	essional Services Inc.	Geologi	st: Rhonald W.	Hasenyager, L	.PG #196-0002	46
Drilling Method: Hollow stem	auger	Drilling	Fluid (Type): Po	otable water		
Logged By: Testing Services C	Corp.	Date Sta	arted: 4/28/20	006 Date F	inished: 4/2	8/2006
Report Form Completed By: Rh	onald W. Hasenyager	Date: _	6/7/2006			
ANNULAR SPAC	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			629.45		op of Protective	Casing
			628.96	3.26 T	op of Riser Pipe	
Type of Surface Seal: Concrete		4 To	625.70	0.00 G	round Surface	
			623.70		op of Annular Se	alant
Type of Annular Sealant: Benton	ite chips				op of Familian Se	
Installation Method: <u>Gravity</u>						
Setting Time: +24 hr.			618.67		tatic Water Level After Completion) 6	
Type of Bentonite Seal Granu	llar Pellet Slurry – (choose one)					
Installation Method: Gravity	,		_623.70_	2.00 T	op of Seal	
Setting Time: +24 hr.			616.20	9.50 T	op of Sand Pack	
Type of Sand Pack: Quartz sand						
Grain Size: #5 (siev			613.68	_12.02 T	op of Screen	
Installation Method: Gravity						
			608.92		ottom of Screen	
Type of Backfill Material:	(if applicable)		608.55	17.15 B	ottom of Well	
Installation Method:n/a			608.55		ottom of Boreho	le
			* Referenced to a	National Geodetic D	atum	
		ı	CAS	SING MEASU	REMENTS	
WELL CONS	TRUCTION MATERIALS		Diameter of Boreho	ole	(inches)	8.0
	type of material for each area)		ID of Riser Pipe Protective Cosing I	anath	(inches)	5.0
			Protective Casing I Riser Pipe Length	zugui	(feet)	15.28
Protective Casing	SS304 SS316 PTFE PVC O	THER:	Bottom of Screen to	o End Can	(feet)	0.37
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC O	THER:	Screen Length (1s		(feet)	4.76
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC O	THER:	Total Length of Ca	sing	(feet)	20.41

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

Illinois Enviro	onmental Protection Agenc	e y		Well	Completion	Report
Site #:	County: N	Iontgomery	I	W	Vell #: G1	03
Site Name: AEG Coffeen Po	ower Station CCB Management Facili	ity		В	orehole #:	G103
State- Plant Plane Coordinate: X 2,514,50	01.2 Y 876,199.5 (or) Latitud	le:	·	Longitud	e:°	_'"
Surveyed By:	rick	_ IL Regi	istration #:035-0	03507		
Drilling Contractor: <u>Layne-W</u>	Vestern Co	_ Driller:	T. List			
Consulting Firm: Hanson Pro	ofessional Services Inc.	_ Geolog	ist: Rhonald W.	Hasenyage	r, LPG #196-000	246
Drilling Method: Hollow ster	m auger	_ Drilling	g Fluid (Type):n/a	ı		
Logged By: Suzanna L. Sim	pson	_ Date St	arted: 2/15/20	10 Dat	e Finished: 2/1	5/2010
Report Form Completed By:S	uzanna L. Simpson	_ Date: _	2/18/2010			
ANNULAR SPA	ACE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
	<u></u>		_634.07_	3.08	Top of Protective	Casing
			633.80	2.81	Top of Riser Pipe	
Type of Surface Seal: <u>Concrete</u>			630.99	0.00	Ground Surface	
Type of Annular Sealant: High	-solids bentonite		624.93	6.06	Top of Annular S	ealant
Installation Method:Trem	ie J					
Setting Time: >24 hr.		$ \underline{\nabla} $	614.00	16.99	Static Water Leve (After Completion)	
Type of Bentonite Seal Gra						
Installation Method: Gravi	(choose one)		618.51	12.48	Top of Seal	
Setting Time: 8 min			617.35	_13.64_	Top of Sand Pack	:
Type of Sand Pack: Quartz sar	nd					
	sieve size)		615.11	15.88	Top of Screen	
Installation Method: Gravi	ity					
Type of Backfill Material:n/a			610.32 609.90	<u>20.67</u> <u>21.09</u>	Bottom of Screen Bottom of Well	
Installation Mathed: n/o	(if applicable)		609.90	21.09	Dattam of Daroha	Jo.
Installation Method: <u>n/a</u>			* Referenced to a		Bottom of Boreho	ne
			CAS	ING MEA	SUREMENTS	
WELL CON			Diameter of Boreho	ole	(inches)	8.0
	STRUCTION MATERIALS one type of material for each area)		ID of Riser Pipe		(inches)	2.0
			Protective Casing L	ength	(feet)	5.0
Protective Casing	SS304 SS316 PTFE PVC OTHER	R: (Steel)	Riser Pipe Length Bottom of Screen to	End Con	(feet)	18.69 0.42
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER		Screen Length (1s		(feet)	4.79
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER	R:	Total Length of Cas		(feet)	23.90
Screen	SS304 SS316 PTFE PVC OTHE	R:	Screen Slot Size **		(inches)	0.010

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environmental Protection Agency			Well Completion Repo			
Site #:	County:	ontgomery		W	Vell #:G1	04
Site Name: AEG Coffeen Pov	wer Station CCB Management Facilit	У		В	orehole #:	104
State- Plant Plane Coordinate: X 2,514,505	5.0 Y 875,849.3 (or) Latitude	::°		Longitud	e:°	
Surveyed By:Jeffrey D. Emri	ck	IL Regis	tration #:035-0	03507		
Drilling Contractor: <u>Layne-We</u>	estern Co	Driller:	T. List			
Consulting Firm: Hanson Prof	essional Services Inc.	Geologis	et: Rhonald W.	Hasenyage	er, LPG #196-000	246
Drilling Method: Hollow stem	auger	Drilling l	Fluid (Type):n/a	a		
Logged By: Suzanna L. Simp	son	_ Date Sta	rted: 2/15/20	010 Dat	e Finished: 2/1	5/2010
Report Form Completed By: Su	zanna L. Simpson	Date:	2/18/2010			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			633.29	` /	Top of Protective	Casing
			632.94	-2.82	Top of Riser Pipe	
Type of Surface Seal: Concrete			630.12	0.00	Ground Surface	
Type of Annular Sealant: High-s	solids bentonite		624.92	5.20	Top of Annular So	ealant
Installation Method:Tremie	;					
Setting Time:>24 hr.		∇	614.72	_15.40_	Static Water Leve (After Completion)	
Type of Bentonite Seal Gran						
Installation Method: <u>Gravity</u>	(choose one)	X	619.17	_10.95_	Top of Seal	
Setting Time: 10 min			617.42	_12.70_	Top of Sand Pack	
Type of Sand Pack: Quartz sand	<u> </u>					
Grain Size:10/20 (sie	eve size)		615.21	14.91	Top of Screen	
Installation Method: Gravity	<u>/</u>		610.51	19.61	Bottom of Screen	
Type of Backfill Material: Quart	tz sand (if applicable)		610.04	20.08	Bottom of Well	
Installation Method: <u>Gravity</u>	<u> </u>		607.92 * Referenced to a	22.20 National Geodeti	Bottom of Boreho	le
		Г			SUREMENTS	
	TRUCTION MATERIALS		Diameter of Boreho ID of Riser Pipe	ole	(inches)	2.0
(Choose on	ne type of material for each area)		Protective Casing I	enoth	(inches)	5.0
			Riser Pipe Length	20115011	(feet)	17.73
Protective Casing	SS304 SS316 PTFE PVC OTHER		Bottom of Screen to	o End Cap	(feet)	0.47
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER	:	Screen Length (1:	st slot to last slo	ot) (feet)	4.70
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER	: []	Total Length of Ca	sino	(feet)	22 90

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

(inches)

SS304

Well Completion Form (revised 02/06/02)

SS316

ILLINOIS DEPARTMENT OF PUBLIC HEALTH DIVISION OF ENVIRONMENTAL HEALTH 525 W. JEFFERSON ST. SPRINGFIELD, IL 62761

WATER WELL SEALING FORM

TYPE OR PRESS FIRMLY

RETURN ALL COPIES TO IDPH OR LOCAL HEALTH DEPARTMENT

This form shall be submitted to this Department or the local health department not more than 30 days after a water well, boring or monitoring well is sealed. Such wells are to be sealed not more than 30 days after they are abandoned in accordance with the sealing requirements in the Water Well Construction Code. THE LOCAL HEALTH DEPARTMENT OR REGIONAL PUBLIC HEALTH DEPARTMENT MUST BE NOTIFIED AT LEAST 48 HOURS PRIOR TO SEALING.

Ownership (Name of Controlling Party) AEG C	offeen Power Stat	ion		(G104)	
Well Location 134 CIPS Trail		Coffeen		Montgomer	у
Address - Lot Number		City	y	County	
General Description Township 7 (N)	Range	3 (K)(W)	Section	10
SEQuarter of theNE	Quarter of the	NE	_Quarter		
Year Drilled 2010					
Drilling Permit Number (and date, if known) n/a					
Type of Well Bored Drilled_ ✓					
Total Depth 20.0 ft. Diamo	eter (inches)	2			
Formation clear of obstructionYes					
DETAILS OF PLUGGING					
Filled with Bentonite grout (cement or other materials)	from	0.5 to	20.0 ft.		
Kind of plug Random soil			0.5 ft.		
Filled with					
Kind of plug	from	to	ft.		
Filled with	from	to	ft.		
Kind of plug	from	to	ft.		
CASING RECORD Upper 2 feet of casing remove	ed <u>√</u> Ye	sN	0		
Date well was sealed Month October Da	ay8	Year 2010	.		
Licensed water well driller or other person approve	ed by the Departi	ment performin	g well sealing.		
Rhonald W. Hasenyager, L.P.G.	196-0002	246			
Name	Complete	License Numb	er		
Hanson Professional Services Inc., 1525 S. 6th St.	Springfie	eld	IL 62	2703	
Address	City		State	e/ZIP	

This state agency is requesting disclosure of information that is necessary to accomplish the statutory purpose as outlined under Public Act 85-0863. Disclosure of this information is mandatory. This form has been approved by the Forms Management Center.

IL 482-0631

Illinois Environmental Protection Agency	Well Completion Report
Site #: County: Mon	ontgomery Well #: G105
Site Name: AEG Coffeen Power Station CCB Management Facility	Borehole #: G105
State- Plant Plane Coordinate: X 2,514,509.2 Y 875,499.7 (or) Latitude:	
Surveyed By:	IL Registration #:035-003507
Drilling Contractor: <u>Layne-Western Co</u>	Driller: T. List
Consulting Firm: Hanson Professional Services Inc.	Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger	Drilling Fluid (Type): <u>n/a</u>
Logged By: Suzanna L. Simpson	Date Started: 2/16/2010 Date Finished: 2/16/2010
Report Form Completed By: Suzanna L. Simpson	Date:2/18/2010
ANNULAR SPACE DETAILS	Elevations Depths (0.01 ft.) (MSL)* (BGS)
	632.40 -3.14 Top of Protective Casing
Type of Surface Seal: Concrete	629.26 0.00 Ground Surface
Type of Annular Sealant: High-solids bentonite	
Installation Method: Tremie	
	(After Completion) 3/1/2010
Type of Bentonite Seal Granular Pellet Slurry (choose one)	
Installation Method: Gravity	616.55 12.71 Top of Seal
Setting Time: 10 min	
Type of Sand Pack: Quartz sand	
Grain Size: 10/20 (sieve size)	613.15 16.11 Top of Screen
Installation Method: <u>Gravity</u>	(00.26 20.00 D. 11
Type of Backfill Material: Quartz sand	608.36 20.90 Bottom of Screen 607.89 21.37 Bottom of Well
(if applicable) Installation Method: Gravity	606.80 22.46 Bottom of Borehole
	* Referenced to a National Geodetic Datum
	CASING MEASUREMENTS
WELL CONCEDUCATION MATTERIALS	Diameter of Borehole (inches) 8.0
WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)	ID of Riser Pipe (inches) 2.0
	Protective Casing Length (feet) 5.0
D + / C :	Riser Pipe Length (feet) 18.93
Protective Casing SS304 SS316 PTFE PVC OTHER: Riser Pipe Above W.T. SS304 SS316 PTFE PVC OTHER:	Bottom of Scient to End Cap (leet) 0.47
Riser Pipe Below W.T. SS304 SS316 PTFE PVC OTHER: Riser Pipe Below W.T. SS304 SS316 PTFE PVC OTHER:	Screen Length (1st slot to last slot) (feet) 4.79 Total Length of Casing (feet) 24.19

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

(inches)

SS304

Well Completion Form (revised 02/06/02)

SS316

Illinois Environ		Well Co	ompletion	Report		
Site #:	County:	Montgomery	r	Well #	#: <u>G1</u>	06
Site Name: CCB Management	Facility			Boreh	ole #:G	106
State Plane Coordinate: X 2,514,512	2.8 Y 875,149.8 (or) La	atitude: 39°	<u>4'</u> <u>3.100"</u>	Longitude: _	-89° 23	56.800"
Surveyed By: <u>Jeffrey D. Emric</u>	k	IL Regi	stration #: <u>035-00</u>	03507		
Drilling Contractor: <u>Layne-We</u>	stern Co	Driller:	T. List			
Consulting Firm: Hanson Profe	essional Services Inc.	Geolog	ist: Rhonald W.	Hasenyager, LI	PG #196-0002	246
Drilling Method: Hollow stem	auger	Drilling	g Fluid (Type):n/a	l .		
Logged By: Suzanna Simpson		Date St	arted: 2/16/20	10 Date Fir	nished: <u>2/1</u>	6/2010
Report Form Completed By: Su	zanna Simpson	Date: _	2/18/2010			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			631.45	, ,	p of Protective	Casing
			631.15		p of Riser Pipe	
Type of Surface Seal: Concrete	=======================================		628.39	0.00 Gr	ound Surface	
			622.94		p of Annular Se	ealant
Type of Annular Sealant: <u>High-s</u>			022.71	<u> </u>	p of Timulai Se	outuiit
Installation Method:Tremie						
Setting Time: >24 hr.			615.77		atic Water Leve after Completion) 3	
Type of Bentonite Seal Grant						
Installation Method: <u>Gravity</u>	(choose one)		_617.44_	10.95 To	p of Seal	
Setting Time:10 min						
			616.10		p of Sand Pack	
Type of Sand Pack: Quartz sand	<u> </u>		614.02	14.37 То	p of Screen	
(ve size)		014.02		p or screen	
Installation Method: Gravity	У		609.43	18.96 Bo	ttom of Screen	
Type of Backfill Material: Quart	zz sand (if applicable)		608.95		ttom of Well	
Installation Method: Gravity			607.94	20.45 Bo	ttom of Boreho	le
			* Referenced to a	National Geodetic Dat		
			CAS	ING MEASUF	REMENTS	
WELL CONS	TDI ICTIONI MATERIAL C		Diameter of Boreho	le	(inches)	8.0
	TRUCTION MATERIALS e type of material for each area)		ID of Riser Pipe		(inches)	2.0
			Protective Casing L	ength	(feet)	5.0
Protective Casing	SS304 SS316 PTFE PVC C	OTHER:	Riser Pipe Length Bottom of Screen to	End Com	(feet)	17.13 0.48
Riser Pipe Above W.T.		OTHER:	Screen Length (1s		(feet)	4.69
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC C	OTHER:	Total Length of Cas		(feet)	22.30

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

Illinois Enviro	onmental Protection Agenc	y		Well	Completion	Report
Site #:	County:M	ontgomery	V	W	Vell #: G1	.07
Site Name: AEG Coffeen Po	ower Station CCB Management Facili	ty		В	orehole #:	3107
State- Plant Plane Coordinate: X 2,514,35	58.3 Y 874,994.3 (or) Latitud	e:	·	Longitud	e:°	_'"
Surveyed By:	rick	_ IL Reg	istration #:035-0	03507		
Drilling Contractor: <u>Layne-W</u>	Vestern Co	_ Driller:	T. List			
Consulting Firm: Hanson Pro	ofessional Services Inc.	_ Geolog	ist: Rhonald W.	Hasenyage	r, LPG #196-000	246
Drilling Method: Hollow ster	m auger	_ Drilling	g Fluid (Type): <u>n/a</u>	ı		
Logged By: Suzanna L. Sim	pson	_ Date St	tarted: 2/17/20	10 Dat	e Finished: 2/1	7/2010
Report Form Completed By:S	uzanna L. Simpson	_ Date: _	2/18/2010			
ANNULAR SPA	ACE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			630.60	2.81	Top of Protective	Casing
			630.23	2.44	Top of Riser Pipe	
Type of Surface Seal: Concrete			627.79	0.00	Ground Surface	
Type of Annular Sealant: High	-solids bentonite		624.08	3.71	Top of Annular S	ealant
Installation Method:Trem	ie J					
Setting Time: >24 hr.		∇	617.39	_10.40_	Static Water Leve (After Completion)	
Type of Bentonite Seal Gra	nular Pellet Slurry (choose one)					
Installation Method: Grave	` '		617.41	_10.38_	Top of Seal	
Setting Time: 8 min			616.24	11.55	Top of Sand Pack	[
Type of Sand Pack: Quartz san	nd					
Grain Size:10/20 (s	sieve size)		613.92	13.87	Top of Screen	
Installation Method: Gravi	ity		(00.20	10.50	D. // C.C.	
Type of Backfill Material: Qua			609.29 608.82	18.50 18.97	Bottom of Screen Bottom of Well	
Installation Method: Gravi	(if applicable)		607.08	_20.71_	Bottom of Boreho	ole
			* Referenced to a	National Geodet	ic Datum	
			CAS	ING MEA	SUREMENTS	
WELL CON	ISTRUCTION MATERIALS		Diameter of Boreho	ole	(inches)	8.0
(Choose	one type of material for each area)		ID of Riser Pipe Protective Casing L	ength	(inches)	5.0
			Riser Pipe Length	Migui	(feet)	16.31
Protective Casing	SS304 SS316 PTFE PVC OTHER	R: Steel	Bottom of Screen to	End Cap	(feet)	0.47
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER	₹:	Screen Length (1s	st slot to last slo	ot) (feet)	4.63
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER		Total Length of Cas	sing	(feet)	21.41
Screen	SS304 SS316 PTFE PVC OTHER	K:	Screen Slot Size **		(inches)	0.010

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Enviro	onmental Protection Agend	ey .		Well	Completion	Report
Site #:	County: N	Iontgomery	V	W	Vell #: G1	.08
Site Name: AEG Coffeen Po	ower Station CCB Management Facil	ity		В	orehole #:	3108
State- Plant Plane Coordinate: X 2,514,24	18.3 Y 874,948.8 (or) Latitud	le:	·	Longitud	e:°	
Surveyed By: <u>Jeffrey D. Emr</u>	rick	IL Reg	istration #:035-0	03507		
Drilling Contractor: <u>Layne-W</u>	estern Co	Driller:	T. List			
Consulting Firm: Hanson Pro	ofessional Services Inc.	Geolog	ist: Rhonald W.	Hasenyage	r, LPG #196-000	246
Drilling Method: Hollow ster	n auger	_ Drilling	g Fluid (Type): <u>n/a</u>	ı		
Logged By: <u>Diane M. Lamb</u>		_ Date St	tarted: 2/12/20	10 Dat	e Finished: 2/1	2/2010
Report Form Completed By:	piane M. Lamb	_ Date: _	2/19/2010			
ANNULAR SPA	ACE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			630.52		Top of Protective	Casing
			630.22	2.72	Top of Riser Pipe	
Type of Surface Seal: Concrete			627.50	0.00	Ground Surface	
Type of Annular Sealant: High	-solids bentonite		622.50	5.00	Top of Annular S	ealant
Installation Method:Trem	ie J					
Setting Time: _ >24 hr.		\sqrt{\sq}}\sqrt{\sq}}}}}}}\sqrt{\sqrt{\sqrt{\sqrt{\sqrt{\sqrt{\sqrt{\sqrt{\sqrt{\sq}}}}}}}}\signtites\sqrt{\sqrt{\sqrt{\sqrt{\sqrt{\sqrt{\sqrt{\sq}}}}}}}\signtites\sqrt{\sqrt{\sqrt{\sqrt{\sq}}}}}}}\sqrt{\sqrt{\sq}}}}}}}\signtites\sqrt{\sinthintity}}}}\signtites\signtites\sintites}\signtifta\sintites\sintinitites\sintites}\sqrt{\sintita}}	618.57	8.93	Static Water Leve (After Completion)	
Type of Bentonite Seal Gra	nular Pellet Slurry (choose one)					
Installation Method: Granu	ılar		614.00	_13.50_	Top of Seal	
Setting Time: 10 min		1 1	612.80	_14.70_	Top of Sand Pack	
Type of Sand Pack: Quartz san	nd					
Grain Size:10/20 (s	sieve size)		610.68	16.82	Top of Screen	
Installation Method: <u>Gravi</u>	ty		606.00	21.50	2.0	
Type of Backfill Material:n/a_			606.00 605.50	<u>21.50</u> <u>22.00</u>	Bottom of Screen Bottom of Well	
Installation Method: n/a	(if applicable)		605.50	22.00	Bottom of Boreho	ale.
instantation rection. <u>II/a</u>			* Referenced to a			iic
			CAS	ING MEA	SUREMENTS	
WELL COM	CTDLICTION MATERIAL C		Diameter of Boreho	ole	(inches)	8.0
	STRUCTION MATERIALS one type of material for each area)		ID of Riser Pipe		(inches)	2.0
			Protective Casing L	ength	(feet)	5.0
Protective Casing	SS304 SS316 PTFE PVC OTHE	R: Steel	Riser Pipe Length Bottom of Screen to	End Can	(feet)	19.54 0.50
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHE		Screen Length (1s		`	4.68
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHE	R:	Total Length of Cas		(feet)	24.72
Screen	SS304 SS316 PTFE PVC OTHE	R:	Screen Slot Size **		(inches)	0.010

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environmental Protection Ager	ncy Well Completion Report
Site #: County:	Montgomery Well #: G109
Site Name: AEG Coffeen Power Station CCB Management Fac	ility Borehole #: G109
State- Plant	ude:°'" Longitude:°'"
Surveyed By: <u>Jeffrey D. Emrick</u>	IL Registration #:035-003507
Drilling Contractor: Layne-Western Co	Driller: T. List
Consulting Firm: Hanson Professional Services Inc.	Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger	Drilling Fluid (Type):
Logged By: Diane M. Lamb	Date Started:2/11/2010 Date Finished:2/11/2010
Report Form Completed By: Diane M. Lamb	Date: 2/19/2010
ANNULAR SPACE DETAILS	Elevations Depths (0.01 ft.) (MSL)* (BGS)
_	630.08 -2.88 Top of Protective Casing
Type of Surface Seal: Concrete	627.20 0.00 Ground Surface
Type of Annular Sealant: High-solids bentonite	
Installation Method:	
Setting Time: _ >24 hr.	$ \underline{\nabla} $ 618.35 8.85 Static Water Level (After Completion) 3/1/2010
Type of Bentonite Seal Granular Pellet Slurry	(Audi Completion) 5/12010
(choose one) Installation Method: Granular	
G Tr. 10 :	
Type of Sand Pack: Quartz sand	
Grain Size: 10/20 (sieve size)	611.81 Top of Screen
Installation Method: <u>Gravity</u>	(07.27 10.02 P.# cg
Type of Backfill Material:n/a	$ \begin{array}{c cccc} \hline & \underline{607.27} & \underline{19.93} & \text{Bottom of Screen} \\ \hline & \underline{606.70} & \underline{20.50} & \text{Bottom of Well} \end{array} $
Installation Method:n/a	606.70 20.50 Bottom of Borehole
	* Referenced to a National Geodetic Datum
	CASING MEASUREMENTS
WELL CONSTRUCTION MATERIALS	Diameter of Borehole (inches) 8.0
(Choose one type of material for each area)	ID of Riser Pipe (inches) 2.0
	Protective Casing Length (feet) 5.0
Protective Casing SS304 SS316 PTFE PVC OTF	Riser Pipe Length (feet) 17.95 IER: (Steel) Rottom of Screen to End Cap (feet) 0.57
	Bottom of Scient to End Cap (leet) 0.57
Riser Pipe Below W.T. SS304 SS316 PTFE PVC OTH	Screen Length (1st slot to last slot) (leet) 4.34

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

(inches)

SS304

Well Completion Form (revised 02/06/02)

SS316

Illinois Environ	mental Protection	Agency				Well	Complet	ion Report
Site #:	Co	ounty: Mon	itgomery	r		W	/ell #:	G110
Site Name:CCB Management	t Facility					В	orehole #:	G110
State Plane Coordinate: X 2,514,057	7.7 Y 875,015.4 (or) Latitude:	39°	4'_	1.800"	Longitud	e: <u>-89°</u> _	24' 2.500"
Surveyed By: <u>Jeffrey D. Emric</u>	ek		IL Regi	stration	#: 035-0	03507		
Drilling Contractor: <u>Layne-We</u>	stern Co		Driller:	T. L	ist			
Consulting Firm: Hanson Profe	essional Services Inc.		Geolog	ist: R	honald W.	Hasenyager	., LPG #196-	000246
Drilling Method: Hollow stem	auger		Drilling	g Fluid (Гуре):	a		
Logged By:			Date St	arted: _	2/11/20	010 Date	e Finished:	2/11/2010
Report Form Completed By:			Date: _	2.	/19/2010			
ANNULAR SPA	CE DETAILS				evations MSL)*	Depths (BGS)	(0.01	ft.)
				`	629.96	2.94	Top of Prote	ctive Casing
					629.65	-2.63	Top of Riser	Pipe
Type of Surface Seal: Concrete				>	627.02	0.00	Ground Surf	ace
Type of Annular Sealant: High-s	solids bentonite			// '	621.86	5.16	Top of Annu	lar Sealant
Installation Method:Tremic		- 9						
Setting Time:>24 hr.		_ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	$\mathbb{Z} \mid \cdot \mid$	_	617.52	9.50	Static Water (After Complete	Level tion) 3/1/2010
Type of Bentonite Seal Gran	ular Pellet Slurry (choose one)							
Installation Method: Granu	,	-	 	_	615.46	_11.56_	Top of Seal	
Setting Time: 10 min		-		_	614.06	12.96	Top of Sand	Pack
Type of Sand Pack:Quartz sand	1	_						
Grain Size: 10/20 (sie	ve size)				611.97	15.05	Top of Scree	n
Installation Method: Gravit	у	-	∄		607.42	10.50		
Type of Backfill Material: <u>n/a</u>	(if applicable)		\exists		607.43 606.86		Bottom of W	
Installation Method: n/a	(п аррисаок)				606.86	20.16	Bottom of Bo	orehole
				*	Referenced to a	National Geodet	ic Datum	
					CAS	SING MEAS	SUREMENT	S.
WELL CONS	STRUCTION MATERIALS			Diamet	ter of Boreho	ole	(inc	ches) 8.0
	e type of material for each area)	,			Riser Pipe		•	ches) 2.0
					ive Casing L	ength		(feet) 5.0 (feet) 17.68
Protective Casing	SS304 SS316 PTFE PV	VC OTHER:			ripe Length of Screen to	n End Can		(feet) 17.68 (feet) 0.57
Riser Pipe Above W.T.	SS304 SS316 PTFE PV	VC OTHER:				st slot to last slo		(feet) 4.54
Riser Pipe Below W.T.	SS304 SS316 PTFE PV	OTHER:			ength of Ca			(feet) 22.79

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

SS304

Well Completion Form (revised 02/06/02)

SS316

Illinois Enviro	onmental Protection Agenc	e y		Well	Completion	Report
Site #:	County: N	Iontgomery	Į.	W	Vell #: G1	.11
Site Name: AEG Coffeen Po	ower Station CCB Management Facili	ty		В	orehole #:	G111
State- Plant Plane Coordinate: X 2,513,98	81.7 Y 875,058.7 (or) Latitud	le:	· ' ''	Longitud	e:°	_'"
Surveyed By:	rick	_ IL Reg	istration #:035-0	03507		
Drilling Contractor: <u>Layne-W</u>	Vestern Co	_ Driller:	T. List			
Consulting Firm: Hanson Pro	ofessional Services Inc.	_ Geolog	ist: Rhonald W.	Hasenyage	r, LPG #196-000	246
Drilling Method: Hollow ster	m auger	_ Drilling	g Fluid (Type):n/a	ı		
Logged By:Diane M. Lamb		_ Date St	tarted: 2/10/20	10 Dat	e Finished: 2/1	1/2010
Report Form Completed By:	Diane M. Lamb	_ Date: _	2/19/2010			
ANNULAR SPA	ACE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
	<u></u>		630.19	2.95	Top of Protective	Casing
			629.90	-2.66	Top of Riser Pipe	
Type of Surface Seal: Concrete			627.24	0.00	Ground Surface	
Type of Annular Sealant: High	e-solids bentonite		622.52	4.72	Top of Annular S	ealant
Installation Method:Trem	ie J					
Setting Time: >24 hr.		$ \nabla $	616.74	10.50	Static Water Leve (After Completion)	
Type of Bentonite Seal Gra						
Installation Method: Grant	(choose one)		616.41	10.83	Top of Seal	
Setting Time: 10 min			614.52	12.72	Top of Sand Pack	
Type of Sand Pack: Quartz sar	nd					
Grain Size:10/20 (s	sieve size)		612.63	_14.61_	Top of Screen	
Installation Method: Gravi	ity					
Type of Backfill Material: <u>n/a</u>			608.09 607.52		Bottom of Screen Bottom of Well	
Installation Method: n/a	(if applicable)		607.52	10.72	Bottom of Boreho	ala.
ilistaliation Method. <u>II/a</u>			* Referenced to a			ЛС
			CAS	ING MEA	SUREMENTS	
WELL CON			Diameter of Boreho	ole	(inches)	8.0
	ISTRUCTION MATERIALS one type of material for each area)		ID of Riser Pipe		(inches)	2.0
			Protective Casing L	ength	(feet)	5.0
Protective Casing	SS304 SS316 PTFE PVC OTHER	R: Steel	Riser Pipe Length Bottom of Screen to	End Can	(feet)	0.57
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER	R:	Screen Length (1s		`	4.54
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER	R:	Total Length of Cas		(feet)	22.38
Screen	SS304 SS316 PTFE PVC OTHER	R:	Screen Slot Size **		(inches)	0.010

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Enviro	onmental Protection Agenc	e y		Well	Completion	Report
Site #:	County: N	Iontgomery	Į.	V	Vell #: G1	19
Site Name: AEG Coffeen Po	ower Station CCB Management Facili	ity		В	orehole #:	G119
State- Plant Plane Coordinate: X 2,513,90	07.7 Y 875,675.0 (or) Latitud	le:	· ' ''	Longitud	e:°	_'"
Surveyed By: <u>Jeffrey D. Emr</u>	rick	_ IL Reg	istration #:035-0	03507		
Drilling Contractor: <u>Layne-W</u>	estern Co	_ Driller:	T. List			
Consulting Firm: Hanson Pro	fessional Services Inc.	_ Geolog	ist: Rhonald W.	Hasenyage	r, LPG #196-000	246
Drilling Method: Hollow ster	n auger	_ Drilling	g Fluid (Type): <u>n/a</u>	ı		
Logged By: Suzanna L. Simp	pson	_ Date St	tarted: 2/9/201	0 Dat	e Finished: 2/	9/2010
Report Form Completed By: S	uzanna L. Simpson	_ Date: _	2/18/2010			
ANNULAR SPA	ACE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
	=		631.85	3.00	Top of Protective	Casing
			631.55	2.70_	Top of Riser Pipe	
Type of Surface Seal: Concrete			628.85	0.00	Ground Surface	
Type of Annular Sealant: High-	-solids bentonite		623.57	5.28	Top of Annular S	ealant
Installation Method:Tremi	ie J					
Setting Time: >24 hr.		$ \underline{\nabla} $	617.59	_11.26_	Static Water Leve (After Completion)	
Type of Bentonite Seal Gra	nular Pellet Slurry					
Installation Method: Gravi	(choose one)		614.87	13.98	Top of Seal	
Setting Time:15 min			613.57		Top of Sand Pack	
Type of Sand Book					•	
Type of Sand Pack: Quartz san Grain Size: 10/20 (s	ieve size)		611.56	17.29	Top of Screen	
Installation Method: Gravi						
Type of Backfill Material: n/a			607.02 606.47	<u>21.83</u> <u>22.38</u>	Bottom of Screen Bottom of Well	
Type of Backini Material. <u>—I/a</u>	(if applicable)		_000.47_		Bottom of wen	
Installation Method: <u>n/a</u>			606.47 * Referenced to a	22.38 National Geodet	Bottom of Boreho	ole
			CAS	ING MEA	SUREMENTS	
			Diameter of Boreho		(inches)	8.0
	STRUCTION MATERIALS one type of material for each area)		ID of Riser Pipe		(inches)	2.0
•	,		Protective Casing L	ength	(feet)	5.0
n	00004 00015 00000		Riser Pipe Length		(feet)	19.99
Protective Casing Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHE SS304 SS316 PTFE PVC OTHE	R: Steel	Bottom of Screen to		(feet)	0.55
Riser Pipe Below W.T.	SS304 SS316 PTFE (PVC) OTHE		Screen Length (1s Total Length of Cas			4.54 25.08
Screen	SS304 SS316 PTFE PVC OTHE		Screen Slot Size **		(feet)	0.010

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environmental Protection		Well Completion Report				
Site #:	County: Mont	tgomery		W	Tell #:G	120
GOD M				Вс	orehole #:	G120
State Plane Coordinate: X 2,513,905.8 Y 875,854.4	(or) Latitude:	39°	<u>4' 10.100"</u>	Longitude	e: <u>-89°</u> <u>2</u> 4	<u>4.400"</u>
Surveyed By:Jeffrey D. Emrick		IL Regis	tration #: <u>035-0</u>	03507		
Drilling Contractor: Layne-Western Co		Driller:	T. List			
Consulting Firm: Hanson Professional Services Inc.	Geologis	t: Rhonald W.	Hasenyager	, LPG #196-000	246	
Drilling Method: Hollow stem auger		Drilling l	Fluid (Type): <u>n/a</u>	a		
Logged By: Suzanna Simpson		Date Sta	rted: <u>2/8/20</u>	10 Date	e Finished:2	/8/2010
Report Form Completed By: Suzanna Simpson		Date: _	2/18/2010			
ANNULAR SPACE DETAILS			Elevations (MSL)*	Depths (BGS)	(0.01 ft.))
			632.18		Top of Protective	Casing
			631.87	2.57	Top of Riser Pipe	•
Type of Surface Seal: Concrete			629.30	0.00	Ground Surface	
There of Annual or Cool and a High collide house wife			624.22	5.08	Top of Annular S	Sealant
Type of Annular Sealant: High-solids bentonite Installation Methods Transis	_					
Installation Method: <u>Tremie</u> Setting Time: >24 hr.		7	615.45	13.85	Static Water Leve	el
					(After Completion)	3/1/2010
Type of Bentonite Seal Granular Pellet Slurry (choose one)	,	YT.				
Installation Method: Gravity			618.14	_11.16_	Top of Seal	
Setting Time: 8 min	—		616.22	_13.08_	Top of Sand Pack	ζ
Type of Sand Pack: Quartz sand						
Grain Size: 10/20 (sieve size)	_		614.20	_15.10_	Top of Screen	
Installation Method: <u>Gravity</u>	_					
Type of Backfill Material: Quartz sand			609.68 609.09	<u>19.62</u> 20.21	Bottom of Screen Bottom of Well	l
(if applicable)						
Installation Method: Gravity			* Referenced to a	22.08 National Geodeti	Bottom of Boreho c Datum	ole
			CAS	UNIC ME A	SUREMENTS	
			Diameter of Boreho		(inches)	8.0
WELL CONSTRUCTION MATERL (Choose one type of material for each area)	ALS		ID of Riser Pipe	J. C	(inches)	2.0
(Choose one type of material for each area)			Protective Casing I	ength	(feet)	5.0
			Riser Pipe Length		(feet)	17.67
Protective Casing SS304 SS316 PTFE	PVC OTHER:	$\overline{}$	Bottom of Screen to	o End Cap	(feet)	0.59
Riser Pipe Above W.T. SS304 SS316 PTFE	PVC OTHER:		Screen Length (1s	st slot to last slot	t) (feet)	4.52
Riser Pipe Below W.T. SS304 SS316 PTFE	PVC OTHER:		Total Length of Ca	sing	(feet)	22.78

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

Illinois Enviro	onmental Protection Agenc	· y		Well	Completion	Report
Site #:	County:M	lontgomery	7	W	Vell #: G1	21
Site Name: AEG Coffeen Po	ower Station CCB Management Facili	ty		В	orehole #:	§121
State- Plant Plane Coordinate: X 2,513,90	04.4 Y 875,964.6 (or) Latitud	e:	· ' "	Longitud	e:°	_'"
Surveyed By:Jeffrey D. Emi	rick	_ IL Regi	istration #:035-0	03507		
Drilling Contractor: <u>Layne-W</u>	Vestern Co	_ Driller:	T. List			
Consulting Firm: Hanson Pro	ofessional Services Inc.	_ Geolog	ist: Rhonald W.	Hasenyage	r, LPG #196-000	246
Drilling Method: Hollow ster	n auger	_ Drilling	g Fluid (Type): <u>n/a</u>	ı		
Logged By: Suzanna L. Sim	pson	_ Date St	arted: 2/4/201	10 Dat	e Finished: 2/	4/2010
Report Form Completed By:S	uzanna L. Simpson	_ Date: _	2/18/2010			
ANNULAR SPA	ACE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			633.14		Top of Protective	Casing
			632.83	3.26_	Top of Riser Pipe	
Type of Surface Seal: <u>Concrete</u>			629.57	0.00	Ground Surface	
Type of Annular Sealant: High	-solids bentonite		624.41	5.16	Top of Annular S	ealant
Installation Method:Trem	ie					
Setting Time: _ >24 hr.		∇	615.13	_14.44	Static Water Leve (After Completion)	
Type of Bentonite Seal Gra	nular Pellet Slurry (choose one)					
Installation Method: Gravi	ity	V V	616.81	12.76	Top of Seal	
Setting Time: 7 min			615.49	_14.08_	Top of Sand Pack	
Type of Sand Pack: Quartz sar	nd					
Grain Size:10/20 (s	sieve size)	≡	612.78	_16.79_	Top of Screen	
Installation Method: Gravi	ity					
Type of Backfill Material:Qua			608.10 607.62	<u>21.47</u> <u>21.95</u>	Bottom of Screen Bottom of Well	
Installation Mathada Cravi	(if applicable)		605.41	24.16	Dattam of Daroha	.lo
Installation Method: <u>Gravi</u>	lty		* Referenced to a		Bottom of Boreho	ne
			CAS	ING MEA	SUREMENTS	
WELL COM	COTPLICATION MATERIAL C		Diameter of Boreho		(inches)	8.0
	STRUCTION MATERIALS one type of material for each area)		ID of Riser Pipe		(inches)	2.0
			Protective Casing L	ength	(feet)	5.0
Protective Casing	SS304 SS316 PTFE PVC OTHER	R: (Steel)	Riser Pipe Length	End C	(feet)	20.05
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER		Bottom of Screen to Screen Length (1s		(feet)	0.48 4.68
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER	₹:	Total Length of Cas		(feet)	25.21
Screen	SS304 SS316 PTFE PVC OTHER	₹:	Screen Slot Size **		(inches)	0.010

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ	nmental Protection Agency	y		Well	Completion	Report
Site #:	County:Mo	ontgomery		W	/ell #:G	122
Site Name: AEG Coffeen Pov	ver Station CCB Management Facilit	у		В	orehole #:	G122
State- Plant	2.8 Y 876,080.1 (or) Latitude				e:°	'
Surveyed By:	ck	IL Registr	ration #:035-0	03507		
Drilling Contractor: <u>Layne-We</u>	stern Co	_ Driller: _	T. List			
Consulting Firm: Hanson Prof	essional Services Inc.	_ Geologist:	Rhonald W.	Hasenyage	r, LPG #196-000	0246
Drilling Method: Hollow stem	auger	_ Drilling F	luid (Type):n/a	1		
Logged By: Diane M. Lamb		_ Date Start	red:2/4/201	10 Date	e Finished: 2	/4/2010
Report Form Completed By:	ane M. Lamb	_ Date:	2/9/2010			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.))
			632.98	-3.12	Top of Protective	e Casing
	T		632.69	2.83	Top of Riser Pipe	e
Type of Surface Seal: Concrete			629.86	0.00	Ground Surface	
Type of Annular Sealant: High-s	olids bentonite		625.01	4.85	Top of Annular S	Sealant
Installation Method:Tremie	7					
Setting Time:>24 hr.		$\overline{\Delta}$	617.02	_12.84_	Static Water Lev	
Type of Bentonite Seal Grant	ılar Pellet Slurry —				(After Completion)	3/1/2010
Installation Method: <u>Gravity</u>	(choose one)		617.01	12.85	Top of Seal	
Setting Time: 10 min			615.41	_14.45	Top of Sand Pac	k
Type of Sand Pack: Quartz sand						
Grain Size:10/20 (sie	ve size)		613.35	_16.51_	Top of Screen	
Installation Method: Gravity	<u>, </u>		600.01	21.05		
Type of Backfill Material: Quart	z sand (if applicable)		608.81 608.20	<u>21.05</u> <u>21.66</u>	Bottom of Screen Bottom of Well	1
Installation Method: <u>Gravity</u>			608.01	21.85	Bottom of Boreh	ole
			* Referenced to a	National Geodeti	ic Datum	
		_	CAS	SING MEAS	SUREMENTS	
WELL COMO	TRUCTION MATERIALS		Diameter of Boreho	ole	(inches)	8.0
	e type of material for each area)		D of Riser Pipe		(inches)	2.0
			rotective Casing L	ength	(feet)	
Protective Casing	SS304 SS316 PTFE PVC OTHER		Riser Pipe Length	- F1C	(feet)	19.34
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER		Sottom of Screen to creen Length (1s	•	(feet)	0.61 4.54
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER		otal Length of Cas		(feet)	24.49

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

(inches)

SS304

Well Completion Form (revised 02/06/02)

SS316

Illinois Enviro	onmental Protection Agend	ey .		Well	Completion	Report
Site #:	County: N	Iontgomery	ý	W	Vell #: G1	23
Site Name: AEG Coffeen Po	ower Station CCB Management Facili	ity		В	orehole #:	3123
State- Plant Plane Coordinate: X 2,513,90	01.5 Y 876,189.6 (or) Latitud	le:	o	Longitud	e:°	_'"
Surveyed By:	rick	IL Reg	istration #:035-0	03507		
Drilling Contractor: <u>Layne-W</u>	Vestern Co	Driller:	T. List			
Consulting Firm: Hanson Pro	ofessional Services Inc.	Geolog	ist: Rhonald W.	Hasenyage	r, LPG #196-000	246
Drilling Method: Hollow ster	n auger	_ Drilling	g Fluid (Type): <u>n/a</u>	ı		
Logged By: <u>Diane M. Lamb</u>		_ Date St	tarted: 2/3/201	10 Dat	e Finished: 2/4	4/2010
Report Form Completed By:	Diane M. Lamb	_ Date: _	2/9/2010			
ANNULAR SPA	ACE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
	<u> </u>		633.29	3.16	Top of Protective	Casing
			632.96	-2.83	Top of Riser Pipe	
Type of Surface Seal: <u>Concrete</u>			630.13	0.00	Ground Surface	
Type of Annular Sealant: High	-solids bentonite		625.06	5.07	Top of Annular S	ealant
Installation Method:Trem	ie J					
Setting Time: _ >24 hr.		$ \Sigma $	614.15	_15.98_	Static Water Leve (After Completion)	
Type of Bentonite Seal Gra	nular Pellet Slurry (choose one)					
Installation Method: Gravi	ity		612.31	17.82	Top of Seal	
Setting Time: 10 min		1 1	611.14	_18.99_	Top of Sand Pack	
Type of Sand Pack: Quartz sar	nd					
Grain Size:10/20 (s	sieve size)		609.19	_20.94_	Top of Screen	
Installation Method: Gravi	ity					
Type of Backfill Material: <u>n/a</u>			604.67 604.06	<u>25.46</u> <u>26.07</u>	Bottom of Screen Bottom of Well	
Installation Method: n/a	(if applicable)		604.06	26.07	Bottom of Boreho	la
ilistaliation Method. <u>II/a</u>			* Referenced to a			ile.
			CAS	SING MEA	SUREMENTS	
WELL CON	CTDI ICTIONI MATERIAL C		Diameter of Boreho	ole	(inches)	8.0
	STRUCTION MATERIALS one type of material for each area)		ID of Riser Pipe		(inches)	2.0
			Protective Casing L	ength	(feet)	5.0
Protective Casing	SS304 SS316 PTFE PVC OTHE	R: Steel	Riser Pipe Length Bottom of Screen to	n End Can	(feet)	0.61
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHE		Screen Length (1s		`	4.52
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHE	R:	Total Length of Cas		(feet)	28.90
Screen	SS304 SS316 PTFE PVC OTHE	R:	Screen Slot Size **		(inches)	0.010

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Enviro	onmental Protection Agenc	· y		Well	Completion	Report
Site #:	County: M	lontgomery	ý	W	Vell #: G1	24
Site Name: <u>AEG Coffeen Po</u>	ower Station CCB Management Facili	ty		В	orehole #:	G124
State- Plant Plane Coordinate: X 2,513,90	00.3 Y 876,304.9 (or) Latitud	e:	o ! !!	Longitud	e:°	
Surveyed By:Jeffrey D. Emr	rick	_ IL Reg	istration #:035-0	03507		
Drilling Contractor: <u>Layne-W</u>	Vestern Co	_ Driller:	T. List			
Consulting Firm: Hanson Pro	ofessional Services Inc.	_ Geolog	ist: Rhonald W.	Hasenyage	r, LPG #196-000	246
Drilling Method: Hollow ster	n auger	_ Drilling	g Fluid (Type): <u>n/a</u>	ì		
Logged By: Suzanna L. Sim	pson	_ Date St	tarted: 2/3/201	10 Dat	e Finished: 2/	3/2010
Report Form Completed By:S	uzanna L. Simpson	_ Date: _	2/5/2010			
ANNULAR SPA	ACE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
	<u> </u>		633.70	, , ,	Top of Protective	Casing
			633.39	-2.97	Top of Riser Pipe	
Type of Surface Seal: <u>Concrete</u>			630.42	0.00	Ground Surface	
Type of Annular Sealant: High	-solids bentonite		625.45	4.97	Top of Annular S	ealant
Installation Method:Trem	ie					
Setting Time: _ >24 hr.		$\overline{\Delta}$	619.43	10.99	Static Water Leve (After Completion)	
Type of Bentonite Seal Gra	nular Pellet Slurry (choose one)					
Installation Method: Gravi	ity	V V	618.34	12.08	Top of Seal	
Setting Time: 10 min			616.50	_13.92_	Top of Sand Pack	
Type of Sand Pack: Quartz san	nd					
Grain Size:10/20 (s	sieve size)	==	614.44	15.98	Top of Screen	
Installation Method: Gravi	ity					
Type of Backfill Material: Qua	artz sand		609.91 609.36	<u>20.51</u> <u>21.06</u>	Bottom of Screen Bottom of Well	
Table Mala C	(if applicable)		(00.45	21.07	D. # CD 1	1
Installation Method: <u>Gravi</u>	ty		* Referenced to a	21.97 National Geodet		oie
			CAS	ING MEA	SUREMENTS	
			Diameter of Boreho		(inches)	8.0
	STRUCTION MATERIALS one type of material for each area)		ID of Riser Pipe		(inches)	2.0
			Protective Casing L	ength	(feet)	5.0
Protective Casing	SS304 SS316 PTFE PVC OTHER	R: (Steel)	Riser Pipe Length	End C	(feet)	18.95
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER		Bottom of Screen to Screen Length (1s		(feet)	0.55 4.53
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER		Total Length of Cas		(feet)	24.03
Screen	SS304 SS316 PTFE PVC OTHER	₹:	Screen Slot Size **		(inches)	0.010

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ		Well Completion Report				
Site #:	County: Mo	ntgomery		W	/ell #:	125
Site Name: CCB Managemen	t Facility			Во	orehole #:	G125
State Plane Coordinate: X 2,513,899	9.1 Y 876,409.5 (or) Latitude:				e: <u>-89°</u> 2	
Surveyed By: <u>Jeffrey D. Emric</u>	ck	IL Registr	ration #:035-00	03507		
Drilling Contractor: <u>Layne-We</u>	estern Co	Driller: _	T. List			
Consulting Firm: Hanson Profe	essional Services Inc.	Geologist:	Rhonald W.	Hasenyager	, LPG #196-000)246
Drilling Method: Hollow stem	auger	Drilling F	luid (Type):n/a	ı		
Logged By: Suzanna Simpson			ted: 2/2/201			
	ızanna Simpson		2/5/2010			
ANNULAR SPA		Date	Elevations		(0.01 ft.	
ANNULAR SFA	CE DETAILS		(MSL)*	Depths (BGS)	(0.01 11.)
			633.82	3.14	Top of Protectiv	e Casing
			633.51	2.83	Top of Riser Pip	e
Type of Surface Seal: Concrete			630.68_	0.00	Ground Surface	
			625.77	4.91	Top of Annular	Sealant
Type of Annular Sealant: High-s	7					
Installation Method: Tremic		_	(22.10	0.50	Cont. Mar. 1	,
Setting Time: >24 hr.		$\overline{\angle}$	622.10	8.58	Static Water Lev (After Completion)	
Type of Bentonite Seal Gran						
Installation Method: <u>Gravit</u>	(choose one)		617.15	_13.53_	Top of Seal	
Setting Time: 10 min			615.05	14.72	Tom of Sand Day	J.
			615.95	14.73	Top of Sand Pac	K
Type of Sand Pack: Quartz sand	<u>d</u>		613.65	17.03	Top of Screen	
	eve size)		013.03		Top of Screen	
Installation Method: <u>Gravit</u>	y		609.12	21.56	Bottom of Scree	n
Type of Backfill Material:n/a_	(if applicable)		608.64	22.04	Bottom of Well	
Installation Method: n/a	(ii application)		608.64	22.04	Bottom of Borel	iole
			* Referenced to a	National Geodeti	ic Datum	
			CAS	ING MEAS	SUREMENTS	
WELL COM		Γ	Diameter of Boreho	ole	(inches)	8.0
	STRUCTION MATERIALS le type of material for each area)		D of Riser Pipe		(inches)	
			rotective Casing L	ength	(feet)	
Protective Casing	SS304 SS316 PTFE PVC OTHER:($\overline{}$	Rottom of Saraan to	End Car	(feet)	0.70
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER:		Sottom of Screen to creen Length (1s		t) (feet)	4.50
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER:		otal Length of Cas		(feet)	2405

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

Illinois Environ	nmental Protection Agend	ey .		Well	Completion	Report
Site #:	County: N	Iontgomery	ý	v	Vell #: G1	26
Site Name: AEG Coffeen Pov	wer Station CCB Management Facili	ity		В	Sorehole #:	G126
State- Plant Plane Coordinate: X 2,513,895	5.4 Y 875,062.4 (or) Latitud	le:	· · · · · · · · · · · · · · · · · · ·	Longitud	e:°	
Surveyed By: <u>Jeffrey D. Emri</u>	ck	IL Reg	istration #:035-0	03507		
Drilling Contractor: <u>Layne-We</u>	estern Co	Driller:	T. List			
Consulting Firm: Hanson Prof	Pessional Services Inc.	Geolog	ist: Rhonald W.	Hasenyage	er, LPG #196-000	246
Drilling Method: Hollow stem	auger	_ Drilling	g Fluid (Type): <u>n/a</u>	i .		
Logged By: Diane M. Lamb		_ Date St	tarted: 2/10/20	10 Dat	e Finished: 2/1	0/2010
Report Form Completed By:	ane M. Lamb	Date: _	2/19/2010			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
	<u></u>		625.69	2.73	Top of Protective	Casing
			625.39	2.43	Top of Riser Pipe	
Type of Surface Seal: Concrete			622.96	0.00	Ground Surface	
Type of Annular Sealant: High-s	solids bentonite		619.96	3.00	Top of Annular S	ealant
Installation Method:Tremie	<u>;</u>					
Setting Time: >24 hr.		$\mid \underline{\nabla} \mid \mid$	614.14	8.82	Static Water Leve (After Completion)	
Type of Bentonite Seal Gran	ular Pellet Slurry (choose one)					
Installation Method: Granul	lar		613.96	9.00	Top of Seal	
Setting Time: 10 min		1 1	612.86	_10.10	Top of Sand Pack	
Type of Sand Pack: Quartz sand	1					
Grain Size:10/20 (sign	eve size)		610.07	12.89	Top of Screen	
Installation Method: Gravity	y		605.52	17.43	Bottom of Screen	
Type of Backfill Material: <u>n/a</u>	(C F H)		605.53 604.96	18.00	Bottom of Well	
Installation Method: <u>n/a</u>	(if applicable)		604.96 * Referenced to a		Bottom of Boreho	ble
			Referenced to a	National Geodet	ic Datum	
					SUREMENTS	0.0
	STRUCTION MATERIALS ne type of material for each area)		Diameter of Boreho ID of Riser Pipe	ole	(inches)	2.0
(Choose of	as the or material for each area)		Protective Casing I	ength	(feet)	5.0
D. e. d. G. d.			Riser Pipe Length		(feet)	15.32
Protective Casing Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHE SS304 SS316 PTFE PVC OTHE	R: Steel	Bottom of Screen to		(feet)	0.57
Riser Pipe Above W.T. Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHE		Screen Length (1s Total Length of Cas			20.43
Screen	SS304 SS316 PTFE PVC OTHE		Screen Slot Size **		(feet)	0.010

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ	nmental Protection Agency	y		Well	Completion	Report
Site #:	County:N	Montgomery		We	ell #: G1	51
Site Name: CCB Managemen	t Facility			Bo	rehole #:	G151
State Plane Coordinate: X 2,513,805	5.9 Y <u>875,023.7</u> (or) Latitu	de:		Longitude	:	
Surveyed By:	ek	IL Registr	ration #: 035-00	03507		
Drilling Contractor: Testing Se	ervice Corp.	Driller: _	B. Williamson			
Consulting Firm: Hanson Profe	essional Services Inc.	Geologist:	Rhonald W.	Hasenyager,	LPG #196-0002	246
Drilling Method: Hollow stem auger Dri			luid (Type):n/a	l		
Logged By: Ryne M. Fiorito		Date Start	red: 12/19/20	011 Date	Finished: 12/	19/2011
Report Form Completed By: R	nonald W. Hasenyager	Date:	12/27/2011			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			626.24		Top of Protective	Casing
			625.93	3.11	Top of Riser Pipe	
Type of Surface Seal: Concrete			622.82	0.00	Ground Surface	
			620.49	2.33	Top of Annular S	ealant
Type of Annular Sealant: High-s	7				1	
Installation Method:Tremio						
Setting Time: >24 hr.			615.03	7.79	Static Water Leve (After Completion)	
Type of Bentonite Seal Gran	ular Pellet Slurry (choose one)					
Installation Method: <u>Gravit</u>	v		611.82	_11.00_	Top of Seal	
Setting Time: 24 min			609.07	13.75	Top of Sand Pack	
Type of Sand Books O	1					
Type of Sand Pack: Quartz sand Grain Size: 10/20 (sie	tve size)		607.48	_15.34_	Top of Screen	
Installation Method: Gravit	,					
instantation Method. Gravit	<u>y</u>		602.98		Bottom of Screen	
Type of Backfill Material: <u>n/a</u>	(if applicable)		602.36	_20.46_	Bottom of Well	
Installation Method:n/a			602.36		Bottom of Boreho	le
			* Referenced to a	National Geodetic	Datum	
			CAS	ING MEAS	UREMENTS	
WELL CONS	STRUCTION MATERIALS		Diameter of Boreho	le	(inches)	8.0
	e type of material for each area)		O of Riser Pipe	41	(inches)	2.0
			rotective Casing L	ength	(feet)	5.0 18.45
Protective Casing	SS304 SS316 PTFE PVC OTHE		tiser Pipe Length Sottom of Screen to	Fnd Can	(feet)	0.62
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHE		creen Length (1s			4.50
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHE		otal Length of Cas		(feet)	23.57

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ	nmental Protection Agency	7		Well (Completion	Report
Site #:	County: <u>N</u>	Iontgomery		Wel	II #: G1	52
Site Name: CCB Managemen	t Facility			Bor	ehole #:	G152
State Plane Coordinate: X 2,513,894	4.5 Y 874,687.5 (or) Latitud	de:		Longitude:		
Surveyed By:	ck	IL Registr	ration #: <u>035-00</u>	03507		
Drilling Contractor: Testing Se	ervice Corp.	_ Driller: _	B. Williamson			
Consulting Firm: Hanson Profe	essional Services Inc.	Geologist:	Rhonald W. I	Hasenyager,	LPG #196-0002	246
Drilling Method: Hollow stem auger Drill			luid (Type): <u>n/a</u>	l .		
Logged By: Ryne M. Fiorito		Date Start	ted: 12/20/20	011 Date I	Finished: <u>12/</u> 2	20/2011
Report Form Completed By: R	nonald W. Hasenyager	Date:	12/27/2011			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			626.67		Γop of Protective	Casing
			626.52	3.46	Гор of Riser Pipe	
Type of Surface Seal: Concrete			623.06	0.00	Ground Surface	
			621.06	2.00	Γop of Annular S	ealant
Type of Annular Sealant: Seal e.	Y					
			60.6.0.6	17.00		
Setting Time:		$\left \begin{array}{c} \overline{\Delta} \end{array}\right $	606.06	17.00 \$	Static Water Leve (After Completion)	
Type of Bentonite Seal Gran	ular Pellet Slurry (choose one)					
Installation Method: Gravit	V		621.06		Γop of Seal	
Setting Time:>24 hr.			611.56	11.50	Гор of Sand Pack	
T. (C. ID.)	_					
Type of Sand Pack: Quartz sand			_609.47_	13.59	Гор of Screen	
Grain Size: 10/20 (sie Installation Method: Gravit	eve size)					
	y		604.97		Bottom of Screen	
Type of Backfill Material:n/a_	(if applicable)		_604.49_	18.57 1	Bottom of Well	
Installation Method:n/a			604.49		Bottom of Boreho	ole
			* Referenced to a l	National Geodetic	Datum	
			CAS	ING MEASU	JREMENTS	
WELL CONS	STRUCTION MATERIALS		Diameter of Boreho	le	(inches)	8.0
	e type of material for each area)		D of Riser Pipe Protective Casing Lo	en ath	(inches)	5.0
			Riser Pipe Length	ongui	(feet)	17.05
Protective Casing	SS304 SS316 PTFE PVC OTHE		Bottom of Screen to	End Cap	(feet)	0.48
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHE		creen Length (1st	-	(feet)	4.50
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHE	_	otal Length of Cas		(feet)	22.03

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ	mental Protection Agency			Well	Completio	n Report
Site #:	County: M	ontgomery		W	ell #:	153
Site Name:CCB Management	t Facility			Во	orehole #:	G153
State Plane Coordinate: X 2,513,532	2.7 Y 874,532.7 (or) Latitud	e:		Longitude	e:	
Surveyed By: <u>Jeffrey D. Emric</u>	k	IL Registr	ation #:035-0	03507		
Drilling Contractor: Testing Se	ervice Corp.	_ Driller: _	B. Williamson			
Consulting Firm: Hanson Profe	essional Services Inc.	_ Geologist:	Rhonald W.	Hasenyager,	, LPG #196-000	0246
Drilling Method: Hollow stem	auger	_ Drilling F	luid (Type):n/a	ì		
Logged By: Ryne M. Fiorito		_ Date Start	ed: 12/15/20	011 Date	Finished: 12	2/15/2011
Report Form Completed By: Rh	nonald W. Hasenyager	Date:	12/27/2011			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
			626.59		Top of Protectiv	e Casing
			626.35		Top of Riser Pip	
Type of Surface Seal: Concrete			623.30_	0.00	Ground Surface	
Type of Annular Sealant: High-s	solids bentonite	H	622.63	0.67	Top of Annular	Sealant
Installation Method:Tremie						
Setting Time: >24 hr.		∇	605.75	_17.55_	Static Water Lev	
					(After Completion)	12/21/2011
Type of Bentonite Seal Grant	ular Pellet Slurry (choose one)					
Installation Method: Gravity	y	\overline{X}	610.30	_13.00_	Top of Seal	
Setting Time: >24 hr.			608.30	_15.00_	Top of Sand Pag	ck
Type of Sand Pack: Quartz sand	1					
	ve size)		607.40	_15.90_	Top of Screen	
Installation Method: Gravit	y					
T			602.96	20.34	Bottom of Scree	n
Type of Backfill Material:n/a	(if applicable)		602.54	20.76	Bottom of Well	
Installation Method:n/a			602.54 * Referenced to a	20.76	Bottom of Borel	nole
			Referenced to a	Ivational Geodetic	Datum	
			CAS	SING MEAS	SUREMENTS	
WELL CONS	TRUCTION MATERIALS		piameter of Boreho	ole	(inches)	
	e type of material for each area)		O of Riser Pipe	an atl-	(inches)	
			rotective Casing L iser Pipe Length	ængtn	(feet)	400-
Protective Casing	SS304 SS316 PTFE PVC OTHER		ottom of Screen to	End Can	(feet)	0.40
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER	. 11	creen Length (1s			4.44
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER		otal Length of Cas		(feet)	22.01

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

Illinois Environ	mental Protection Agency			Well	Completio	n Report
Site #:	County: M	ontgomery		W	ell #:	G154
Site Name:CCB Management	t Facility			Во	orehole #:	G154
State Plane Coordinate: X 2,513,243	3.1 Y 874,978.4 (or) Latitud	e:		Longitude	e:	
Surveyed By: <u>Jeffrey D. Emric</u>	ck	_ IL Registr	ration #:035-0	03507		
Drilling Contractor: Testing Se	ervice Corp.	_ Driller: _	B. Williamson			
Consulting Firm: Hanson Profe	essional Services Inc.	_ Geologist	: _ Rhonald W.	Hasenyager,	, LPG #196-00	00246
Drilling Method: Hollow stem	auger	_ Drilling F	Tluid (Type):n/a	ì		
Logged By: Ryne M. Fiorito		_ Date Star	ted: 12/16/20	011 Date	Finished: 1	2/16/2011
Report Form Completed By: Rh	nonald W. Hasenyager	Date:	12/27/2011			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft	<u></u>)
			626.55		Top of Protecti	ve Casing
			626.35		Top of Riser Pi	_
					Top of rade 11	P
Type of Surface Seal: Concrete			623.52	0.00	Ground Surface	e
Type of Annular Sealant: High-s	solids bentonite		622.44	1.08	Top of Annular	Sealant
Installation Method:Tremic						
		∇	612.42	11.10	Static Water Le	evel
					(After Completion	n) 12/21/2011
Type of Bentonite Seal Gran	Pellet Slurry (choose one)					
Installation Method: Gravit	y	 	613.02	_10.50_	Top of Seal	
Setting Time: 24 min			610.02	_13.50_	Top of Sand Pa	ck
Type of Sand Pack: Quartz sand	1					
	.uve size)		609.26	_14.26_	Top of Screen	
Installation Method: Gravit						
			604.76	18.76	Bottom of Scre	
Type of Backfill Material:n/a_	(if applicable)		604.42	_19.10_	Bottom of Well	
Installation Method:n/a			603.52	20.00	Bottom of Bore	hole
			* Referenced to a	National Geodetic	c Datum	
			CAS	SING MEAS	SUREMENTS	
WELL CONS	STRUCTION MATERIALS		Diameter of Boreho	ole	(inche	
	e type of material for each area)		D of Riser Pipe	41	(inche	
			Protective Casing L	engtn	(fee	
Protective Casing	SS304 SS316 PTFE PVC OTHER		Riser Pipe Length Bottom of Screen to	Fnd Can	(fee	0.24
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER	. 11	Screen Length (1s		`	4.50
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER		Total Length of Cas		(fee	21.02

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

Illinois Environ	mental Protection Agency			Well	Completion	Report
Site #:	County: M	lontgomery		_ We	ell #:G	155
Site Name: CCB Management	t Facility			Во:	rehole #:	G155
State Plane Coordinate: X 2,513,501	.8 Y 875,127.7 (or) Latitud	e:		Longitude	:	
Surveyed By: <u>Jeffrey D. Emric</u>	k	IL Registration	on #: <u>035-00</u>	3507		
Drilling Contractor: Testing Se	ervice Corp.	_ Driller: _B.	. Williamson			
Consulting Firm: Hanson Profe	essional Services Inc.	_ Geologist: _	Rhonald W. H	lasenyager,	LPG #196-000	246
Drilling Method: Hollow stem	auger	_ Drilling Fluid	d (Type): <u>n/a</u>			
Logged By: Ryne M. Fiorito		_ Date Started:	:12/19/201	1 Date	Finished: 12	19/2011
Report Form Completed By: Rh	onald W. Hasenyager	Date:	12/27/2011			
ANNULAR SPA	CE DETAILS]	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			626.07		Top of Protective	Casing
	T		625.86		Top of Riser Pipe	
			023.00	-2.)1	Top of Riser 1 ipo	,
Type of Surface Seal: Concrete			622.89	0.00	Ground Surface	
Type of Annular Sealant: High-s	solids bentonite		621.39	1.50	Top of Annular S	Sealant
Installation Method:Tremie	λ					
Setting Time: >24 hr.		∇	614.99	7.90	Static Water Lev	el
		$\stackrel{-}{\wedge}$			(After Completion)	12/21/2011
Type of Bentonite Seal Grant	ular Pellet Slurry (choose one)					
Installation Method: Gravity	у	\bowtie	611.89	11.00	Top of Seal	
Setting Time: 27 min			609.14	13.75	Top of Sand Pacl	ζ
Time of Sand Dealty O						
Type of Sand Pack: Quartz sand Grain Size: 10/20 (sie	1 ve size)		607.80	15.09	Top of Screen	
Installation Method: Gravity						
instantation Method. Gravit	y		603.31		Bottom of Screen	
Type of Backfill Material: <u>n/a</u>	(if applicable)		602.66	20.23	Bottom of Well	
Installation Method:n/a			602.66		Bottom of Boreh	ole
			* Referenced to a N	ational Geodetic	Datum	
			CASI	NG MEAS	UREMENTS	
WELL CONS	TRUCTION MATERIALS		meter of Borehole	e	(inches)	8.0
	e type of material for each area)		of Riser Pipe	.1	(inches)	2.0
			tective Casing Le	ngth	(feet)	5.0
Protective Casing	SS304 SS316 PTFE PVC OTHEI		er Pipe Length tom of Screen to	End Con	(feet)	18.28 0.43
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER	Bott	een Length (1st s		(feet)	4.49
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER		al Length of Casi		(feet)	23.20

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

Illinois Enviro	onmental Protection Agen	cy	Well Completion Repor
Site #:	County: M	ontgomery	Well #: G200
Site Name: AEG Coffeen Po	ower Station CCB Management Faci	itv	Borehole #: G200
State- Plant	30.6 Y 2,515,650.0 (or) Latitud		
Surveyed By:Jeffrey D. Em	rick	IL Registration #: 035-00	03507
Drilling Contractor: Testing S	Service Corporation	Driller: B. Williamson	
Consulting Firm: Hanson Pro	ofessional Services Inc.	Geologist: Rhonald W I	Hasenyager, LPG #196-000246
Drilling Method: Hollow ster	n auger	Drilling Fluid (Type):	
Logged By: Suzanna L Simp	oson	Date Started: 2/25/200	08 Date Finished: 2/25/2008
Report Form Completed By: St	uzanna L Simpson	Date: 2/29/2008	
ANNULAR SPA	ACE DETAILS	Elevations (MSL)*	Depths (0.01 ft.) (BGS)
		626.54	2.34_ Top of Protective Casing
		625.94	1.74 Top of Riser Pipe
Type of Surface Seal: Concrete		624.20	0.00 Ground Surface
Type of Annular Sealant: Bento	onite chips	620.70	3.50 Top of Annular Sealant
Installation Method: Gravi	tv		
Setting Time: <u>>24 hr.</u>		₫ 621.45	2.75 Static Water Level (After Completion) 3/12/2008
Type of Bentonite Seal Gran	nular Pellet Slurry (choose one)		
Installation Method: Gravit	iv	_620.70	3.50 Top of Seal
Setting Time: >24 hr.		614.20	10.00 Top of Sand Pack
Type of Sand Pack: Quartz san	d		
Grain Size: 10/20 (si	eve size)	612.01	12.19 Top of Screen
Installation Method: Gravit	у		
Type of Backfill Material: Form	nation Sand (if applicable)	607.22 606.84	16.98 Bottom of Screen 17.36 Bottom of Well
Installation Method: Slough	h	606.20	18.00 Bottom of Borehole
			NG MEASUREMENTS
	STRUCTION MATERIALS	Diameter of Boreho	
(Choose of	ne type of material for each area)	ID of Riser Pipe Protective Casing L	(inches) 2.0 ength (feet) 5.0
		Riser Pipe Length	ength (feet) 5.0 (feet) 13.93
Protective Casing	SS304 SS316 PTFE PVC OTHER	Steel Bottom of Screen to	· · · · · · · · · · · · · · · · · · ·
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER		
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER		
Screen	SS304 SS316 PTFE PVC OTHER		(inches) 0.010
Well Completion Form (revised 02/06/02	2)	**Hand-Slotted Well Scre	

Illinois Enviro	nmental Protection Agen	Well Completion Report						
Site #:	County: M	lontgomery		W	/ell #: G2	01		
Site Name: AEG Coffeen Por	wer Station CCB Management Faci	lity		В	orehole #:	G201		
State- Plant Plane Coordinate: X 877,924	4.9 Y 2,514,849.5 (or) Latitud	le:°	<u>"</u>	Longitud	le:°	_'		
Surveyed By: <u>Jeffrey D. Emri</u>	ck	IL Registr	Registration #:035-003507					
Drilling Contractor: Testing Se	ervice Corporation	_ Driller: _	B. Williamson					
Consulting Firm: Hanson Prof	Pessional Services Inc.	_ Geologist:	Rhonald W l	Hasenyage	r, LPG #196-00	0246		
Drilling Method: Hollow stem	auger	_ Drilling Fl	uid (Type):					
Logged By: Suzanna L Simps	son	_ Date Starte	ed: <u>2/25/20</u>	08 Dat	e Finished:2/2	25/2008		
Report Form Completed By: Su	zanna L Simpson	Date:	2/29/2008					
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)			
	-		627.66	3.76	Top of Protective	Casing		
			627.12	-3.22	Top of Riser Pipe	e		
Type of Surface Seal: Concrete			_623.90_	0.00	Ground Surface			
Type of Annular Sealant: Benton	nite chips		620.60	3.30	Top of Annular S	Sealant		
Installation Method: Gravit	y							
Setting Time: >24 hr.		∇	621.73	2.17	Static Water Lev (After Completion)			
Type of Bentonite Seal Gran	ular Pellet Slurry (choose one)				(Atter completion)	3/12/2000		
Installation Method: Gravit	y		620.60	3.30	Top of Seal			
Setting Time: >24 hr.			611.80	_12.10_	Top of Sand Pacl	ζ		
Type of Sand Pack: Quartz sand	1							
Grain Size: 10/20 (sie	eve size)		610.89	_13.01_	Top of Screen			
Installation Method: <u>Gravit</u>	<u>y</u>		(0(10	17.00	D. #			
Type of Backfill Material: n/a	(if applicable)		606.10 605.75	<u>17.80</u> <u>18.15</u>	Bottom of Screen Bottom of Well	1		
Installation Method: <u>n/a</u>			605.75 * Referenced to a	18.15 National Geode	Bottom of Boreh	ole		
			CAS		CLIDEMENTS			
		D	iameter of Boreh		SUREMENTS (inches)	8.0		
	STRUCTION MATERIALS ne type of material for each area)		of Riser Pipe		(inches)	2.0		
(Choose of	ie type of material for each area)		otective Casing I	Length	(feet)	5.0		
			iser Pipe Length		(feet)	16.23		
Protective Casing	SS304 SS316 PTFE PVC OTHER	R: Steel Be	ottom of Screen t	to End Cap	(feet)	0.35		
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER	R: So	ereen Length (1s	st slot to last slo	ot) (feet)	4.79		
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER	R: T	otal Length of Ca	ising	(feet)	21 37		

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

ILLINOIS DEPARTMENT OF PUBLIC HEALTH DIVISION OF ENVIRONMENTAL HEALTH 525 W. JEFFERSON ST. SPRINGFIELD, IL 62761

WATER WELL SEALING FORM

TYPE OR PRESS FIRMLY

RETURN ALL COPIES TO IDPH OR LOCAL HEALTH DEPARTMENT

This form shall be submitted to this Department or the local health department not more than 30 days after a water well, boring or monitoring well is sealed. Such wells are to be sealed not more than 30 days after they are abandoned in accordance with the sealing requirements in the Water Well Construction Code. THE LOCAL HEALTH DEPARTMENT OR REGIONAL PUBLIC HEALTH DEPARTMENT MUST BE NOTIFIED AT LEAST 48 HOURS PRIOR TO SEALING.

Ownership (Name of Controlling Party) AEG C	offeen Power Stat	ion			(G104)	
Well Location 134 CIPS Trail		Co	offeen		Montgome	ry
Address - Lot Number		City			County	
General Description Township 7 (N)	Range_	3	_ (K) (W)		Section _	1
SEQuarter of theNE	Quarter of the	NI	Ξ ()uarter		
Year Drilled 2010						
Drilling Permit Number (and date, if known) n/a						
Type of Well Bored Drilled_ ✓						
Total Depth 20.0 ft. Diamo	eter (inches)	2				
Formation clear of obstructionYes						
DETAILS OF PLUGGING						
		^ -	_			
Filled with Bentonite grout (cement or other materials)	from	0.5	to²	20.0 ft.		
Kind of plug Random soil	from		to).5ft.		
Filled with	from		to	ft.		
Kind of plug	from		to	ft.		
Filled with	from		to	ft.		
Kind of plug	from		to	ft.		
CASING RECORD Upper 2 feet of casing remove	ed <u>√</u> Ye	es	No			
Date well was sealed Month October Da	ay8	Year_	2010	_•		
Licensed water well driller or other person approve	ed by the Depart	ment pe	erforming v	vell sealing		
Rhonald W. Hasenyager, L.P.G.	196-000	246				
Name	Complete	e Licens	se Number			
Hanson Professional Services Inc., 1525 S. 6th St.	Springfi	eld		IL 62	2703	
Address	City			State	e/ZIP	

This state agency is requesting disclosure of information that is necessary to accomplish the statutory purpose as outlined under Public Act 85-0863. Disclosure of this information is mandatory. This form has been approved by the Forms Management Center.

IL 482-0631

Illinois Enviro	7	Well Completion Repo							
Site #:	Cou	ınty: <u>Mon</u>	tgomery	7	W	/ell #:G	3205		
Site Name: AEG Coffeen Por	wer Station CCB Managem	nent Facility	y		В	orehole #:	G205		
State- Plant Plane Coordinate: X 875,550).2 Y 2,515,914.9 (or)	Latitude:			' Longitud	le:°	, "		
Surveyed By:Jeffrey D. Emri	ck		IL Regi	Registration #: 035-003507					
Drilling Contractor: Testing Se	ervice Corporation		Driller:	r: B. Williamson					
Consulting Firm: Hanson Prof	essional Services Inc.		Geolog	gist: Rhonald W Hasenyager, LPG #196-000246					
Drilling Method: Hollow stem auger Drilling			Drilling	g Fluid (Type):					
Logged By: Suzanna L Simps	son		Date St	arted: 2/21/2	008 Dat	e Finished: 2	/21/2008		
Report Form Completed By: Su	zanna L Simpson		Date: _	2/29/2008					
ANNULAR SPA	CE DETAILS			Elevations (MSL)*	Depths (BGS)	(0.01 ft	.)		
				624.87	2.72	Top of Protective	ve Casing		
				624.45	2.30	Top of Riser Pi	pe		
Type of Surface Seal: Concrete			Y I	622.15	0.00	Ground Surface	:		
				619.95_	2.20	Top of Annular	Sealant		
Type of Annular Sealant: Benton									
Installation Method: Gravit			_	(17.00	7.06				
Setting Time: >24 hr.			<u> </u>	617.09	5.06	Static Water Le (After Completion			
Type of Bentonite Seal Gran									
Installation Method: <u>Gravit</u>	(choose one)			_619.95_	2.20	Top of Seal			
Setting Time: >24 hr.				(12.25	0.00	T 60 1D	,		
				613.35	8.80	Top of Sand Pa	ck		
Type of Sand Pack: Quartz sand	1			612.11	10.04	T f C			
Grain Size: 10/20 (sie	eve size)			612.11	_10.04_	Top of Screen			
Installation Method: Gravit	y			607.62	14.52	Bottom of Screen			
Type of Backfill Material: Form				607.62 607.08	<u>14.53</u> <u>15.07</u>	Bottom of Screen			
Lord Het on Medical Character	(if applicable)			606.15	16.00	Bottom of Bore	1 1 -		
Installation Method: Slough	<u> </u>				a National Geode		noie		
				CA	SING MEA	SUREMENTS			
				Diameter of Bore		(inches	0.0		
	STRUCTION MATERIALS the type of material for each area)	S		ID of Riser Pipe		(inches	2.0		
				Protective Casing	Length	(feet	5.0		
				Riser Pipe Length		(feet			
Protective Casing	SS304 SS316 PTFE PVC		Steel	Bottom of Screen	to End Cap	(feet			
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC			Screen Length (ot) (feet			
Riser Pipe Below W.T.	SS304 SS316 PTFE (PVC	C OTHER:		Total Length of C	asing	(feet	17.37		

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

ILLINOIS DEPARTMENT OF PUBLIC HEALTH DIVISION OF ENVIRONMENTAL HEALTH 525 W. JEFFERSON ST. SPRINGFIELD, IL 62761

WATER WELL SEALING FORM

TYPE OR PRESS FIRMLY

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This form shall be submitted to this Department or the local health department not more than 30 days after a water well, boring or monitoring well is sealed. Such wells are to be sealed not more than 30 days after they are abandoned in accordance with the sealing requirements in the Water Well Construction Code. THE LOCAL HEALTH DEPARTMENT OR REGIONAL PUBLIC HEALTH DEPARTMENT MUST BE NOTIFIED AT LEAST 48 HOURS PRIOR TO SEALING.

Well Location 134 CIPS Lane		Coffee	1	Mo	ntgomery
Address - Lot Number		City			ounty
General Description Township 7 (N)	Range	3 (K)	(W)	S	Section
SWQuarter of theNEQ	Quarter of the _	NW	Quart	er	
Year Drilled 2008					
Drilling Permit Number (and date, if known) n/a					
Type of Well Bored Drilled_ ✓	Other				
Total Depth 16 Diamet	er (inches)	8			
Formation clear of obstructionYes	No				
			t 1)		
DETAILS OF PLUGGING (riser pipe and screen	pullea ana anni	ulus was gr	outea)		
Filled with Bentonite grout	from	_		ft.	
Filled with Bentonite grout (cement or other materials)	from	_		ft.	
Filled with Bentonite grout (cement or other materials) Kind of plug		0 1	o16.0		
(cement or other materials)	from	0 t	o 16.0	ft.	
(cement or other materials) Kind of plug	from from	0 t	o16.0	ft. ft.	
(cement or other materials) Kind of plug Filled with	from from	0 t	o 16.0	ft. ft. ft.	
(cement or other materials) Kind of plug Filled with Kind of plug	fromfrom	0 t	0 16.0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	ft. ft. ft. ft.	
(cement or other materials) Kind of plug Filled with Kind of plug Filled with	fromfromfromfrom	0 t	o 16.0 o o o o	ft. ft. ft. ft.	
(cement or other materials) Kind of plug Filled with Filled with Kind of plug	fromfromfrom	0 t	o 16.0 o o o No	ft. ft. ft. ft.	
(cement or other materials) Kind of plug	from	0 t	0 16.0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	ft. ft. ft. ft. ft.	
(cement or other materials) Kind of plug	from	0 to to to the second of the s	0 16.0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	ft. ft. ft. ft. ft.	

This state agency is requesting disclosure of information that is necessary to accomplish the statutory purpose as outlined under Public Act 85-0863. Disclosure of this information is mandatory. This form has been approved by the Forms Management Center.

IL 482-0631

Illinois Environ		Well Completion Report				
Site #:	County: N	1ontgomery		Well	#: <u>G</u>	206
Site Name:CCB Management	t Facility			Bore	hole #:	G206
State Plane Coordinate: X 2,514,669	9.2 Y <u>875,103.9</u> (or) Latitud	de: <u>39°</u> _	4' 2.600"	Longitude: _	-89° 2	<u>3' 54.800"</u>
Surveyed By: <u>Jeffrey D. Emric</u>	ck	IL Regist	ration #:035-00	03507		
Drilling Contractor: <u>Layne-We</u>	stern Co	Driller: _	D. Mahurin			
Consulting Firm: Hanson Profe	essional Services Inc.	Geologist	: Rhonald W.	Hasenyager, L	LPG #196-000	246
Drilling Method: Hollow stem	auger	Drilling F	fluid (Type):n/a	ı		
Logged By: Suzanna Simpson	1	Date Star	ted: 10/14/20	010 Date Fi	inished:10	/14/2010
Report Form Completed By:Su	zanna Simpson	Date:	10/15/2010			
ANNULAR SPA	CE DETAILS		Elevations	Depths	(0.01 ft.))
			(MSL)* _633.07_	(BGS) -2.53 Te	op of Protective	Casina
	T					_
			632.82	2.28_ To	op of Riser Pipe	e
Type of Surface Seal: Concrete			630.54_	0.00 G	bround Surface	
Type of Annular Sealant: High-s	colide bentanite		627.84		op of Annular S	Sealant
Installation Method: Tremie						
		$\left \begin{array}{c} \overline{\Delta} \end{array}\right $	611.96	18.58 St	tatic Water Lev	el
Setting Time		$\left \begin{array}{c} \bot \\ \bot \end{array}\right $	011.70		(After Completion)	
Type of Bentonite Seal Gran	ular Pellet Slurry (choose one)					
Installation Method: Gravit	y		616.24	14.30 T	op of Seal	
Setting Time: 15 min			615.04	15.50 T	op of Sand Pac	le.
	<u> </u>		013.04	13.30 1	op of Sand Fac	K
Type of Sand Pack: Quartz sand	<u>i</u>		613.03	17.51 T	op of Screen	
(4.1	eve size)		013.03		op of Screen	
Installation Method: <u>Gravit</u>	<u>y</u>		608.62	21.92 B	ottom of Screen	1
Type of Backfill Material:n/a	(if applicable)		608.12		ottom of Well	
Installation Method: n/a	(ii applicable)		606.54	24.00 B	ottom of Boreh	ole
			* Referenced to a	National Geodetic D	atum	
			CAS	ING MEASU	REMENTS	
WELL COM		I	Diameter of Boreho	ole	(inches)	8.0
	STRUCTION MATERIALS e type of material for each area)		D of Riser Pipe		(inches)	2.0
			Protective Casing L	ength	(feet)	5.0
Protective Casing	SS304 SS316 PTFE PVC OTHE		Riser Pipe Length	F 10	(feet)	
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHE		Bottom of Screen to		(feet)	0.50
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHE		Screen Length (1s) Fotal Length of Cas		(feet)	24.70
		L'	com Dengui Oi Cas		(ICCL)	- 1.70

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

Illinois Environmental Protection Agency				Well Completion Report			
Site #:	Con	unty: Mon	tgomery		W	/ell #:	3207
Site Name: <u>AEG Coffeen Pov</u>	ver Station CCB Manageme	ent Facility			В	orehole #:	G207
State Plane Coordinate: X 2,514,837	7.9 Y 875,166.4 (or)	Latitude:	39°	4'3.2"	Longitude	e:89 °	23 ' 52.6 "
Surveyed By:	ek		IL Regis	stration #: <u>035-0</u>	03507		
Drilling Contractor: <u>Layne-We</u>	stern Co		Driller:	D. Mahurin			
Consulting Firm: Hanson Professional Services Inc. Ge				st: Rhonald W.	Hasenyager	r, LPG #196-00	0246
Drilling Method: Hollow stem	auger		Drilling	Fluid (Type):n/a	a		
Logged By: Suzanna L. Simps	son		Date Sta	arted: 10/8/20	010 Date	e Finished:1	0/8/2010
Report Form Completed By: Su	zanna L. Simpson		Date: _	10/8/2010			
ANNULAR SPA	CE DETAILS			Elevations (MSL)*	Depths (BGS)	(0.01 ft	.)
				633.37	2.76	Top of Protectiv	e Casing
				633.21	-2.60	Top of Riser Pip	e
Type of Surface Seal: Concrete				630.61	0.00	Ground Surface	
	_			628.61	2.00	Top of Annular	
Type of Annular Sealant: <u>High-s</u>	olids bentonite			020.01		Top of Timulai	South
Installation Method: <u>Tremie</u>		-					
Setting Time: >24 hr.		. _	<u>Z</u>	612.86	17.75	Static Water Lev (After Completion	
Type of Bentonite Seal Grant	alar Pellet Slurry						
Installation Method: Gravity	(choose one)			614.76	15.85	Top of Seal	
Setting Time: 15 min							
				613.63	_16.98_	Top of Sand Pag	ek
Type of Sand Pack: Quartz sand		-		(12.27	10.24		
Grain Size:10/20 (sie	ve size)			612.37	18.24	Top of Screen	
Installation Method: Gravity	7	.		607.94	22.77	D-#	
Type of Backfill Material:n/a			\exists	607.84 607.31	$\frac{22.77}{23.30}$	Bottom of Scree Bottom of Well	en
Installation Method: n/a	(if applicable)			606.61	24.00	Bottom of Borel	nole
installation Method. <u>II/a</u>				* Referenced to a			ioie
				CAS	SING MEAS	SUREMENTS	
WELL GOVE				Diameter of Boreho		(inches	8.0
	TRUCTION MATERIALS e type of material for each area)	•		ID of Riser Pipe		(inches	
				Protective Casing I	ength	(feet	
Protective Cosine	SS304 SS316 PTFE PV	C OTHER.	Steel	Riser Pipe Length		(feet	
Protective Casing Riser Pipe Above W.T.		C OTHER: (Sieel	Bottom of Screen to		(feet	
Riser Pipe Below W.T.	SS304 SS316 PTFE (PV		+	Screen Length (1:			27.00
				Total Length of Ca	anig	(feet	23.90

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ	y	Well Completion Rep				
Site #:	County: <u>M</u>	ontgomery		W	/ell #: <u>G2</u>	208
Site Name: AEG Coffeen Pov	ver Station CCB Management Facilit	ty		В	orehole #:	G208
State Plane Coordinate: X 2,514,993	3.6 Y 875,231.5 (or) Latitude	e: <u>39°</u>	4' 3.9"	Longitud	e:2	3 ' 50.6 "
Surveyed By: <u>Jeffrey D. Emri</u>	ck	_ IL Regis	stration #:035-0	03507		
Drilling Contractor: <u>Layne-We</u>	stern Co	_ Driller:	D. Mahurin			
Consulting Firm: Hanson Prof	essional Services Inc.	_ Geologis	st: Rhonald W.	Hasenyage	r, LPG #196-000	246
Drilling Method: Hollow stem	auger	_ Drilling	Fluid (Type): <u>n/a</u>	ı		
Logged By: Suzanna L. Simps	son	_ Date Sta	arted: 10/7/20	10 Date	e Finished:10	/7/2010
Report Form Completed By: Su	zanna L. Simpson	_ Date: _	10/8/2010			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			633.43	-2.86	Top of Protective	Casing
			633.16	-2.59	Top of Riser Pipe	
Type of Surface Seal: Concrete			630.57	0.00	Ground Surface	
Type of Annular Sealant: High-s	valids bentanite		627.77	2.80	Top of Annular S	ealant
Installation Method: Tremie	7					
Setting Time: >24 hr.		∇	614.76	15.81	Static Water Leve	el
<u> </u>					(After Completion)	11/15/2010
Type of Bentonite Seal Grant	ular Pellet Slurry (choose one)					
Installation Method: Gravity	/ <u> </u>	 	616.07	14.50	Top of Seal	
Setting Time: 9 min			613.90	_16.67_	Top of Sand Pack	
Type of Sand Pack: Quartz sand						
Grain Size:10/20 (sie			613.04	17.53	Top of Screen	
Installation Method: Gravity	/					
Type of Backfill Material:n/a			608.51 607.97	<u>22.06</u> <u>22.60</u>	Bottom of Screen Bottom of Well	
Installation Mathada n/a	(if applicable)		606.57	24.00	Dattam of Daroh	Jo.
Installation Method:n/a			* Referenced to a		Bottom of Boreho	ne
			CAS	ING MEAS	SUREMENTS	
WELL GOVE	TRANSPORTATION AND THE PARTY OF		Diameter of Boreho	ole	(inches)	8.0
	TRUCTION MATERIALS the type of material for each area)		ID of Riser Pipe		(inches)	2.0
		-	Protective Casing L	ength	(feet)	5.0
Protective Casing	SS304 SS316 PTFE PVC OTHER	: (Steel)	Riser Pipe Length	- LC	(feet)	20.12
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER		Bottom of Screen to		(feet)	0.54
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER		Screen Length (1s		t) (feet)	4.53 25.19

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

(inches)

SS304

Well Completion Form (revised 02/06/02)

SS316

Illinois Environ	Well Completion Report					
Site #:	County:	Montgomery		Well	#: <u>G2</u>	.09
Site Name:CCB Management	t Facility			Borel	nole #:	G209
State Plane Coordinate: X 2,515,149	9.6 Y 875,298.2 (or) Latitu	ude: <u>39°</u>	4' 4.500"	Longitude: _	-89° 23	48.700"
Surveyed By: <u>Jeffrey D. Emric</u>	ck	IL Regis	tration #:035-00	03507		
Drilling Contractor: <u>Layne-We</u>	stern Co	Driller:	D. Mahurin			
Consulting Firm: Hanson Profe	essional Services Inc.	Geologis	et: Rhonald W.	Hasenyager, L	PG #196-0002	246
Drilling Method: Hollow stem	auger	Drilling	Fluid (Type):n/a	ı		
Logged By: Suzanna Simpson	l .	Date Sta	rted:10/7/20	10 Date Fi	nished:10	/7/2010
Report Form Completed By:Su	zanna Simpson	Date:	10/8/2010			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			633.16		op of Protective	Casing
			632.91		op of Riser Pipe	_
Type of Surface Seal: Concrete			630.57	0.00 Gı	round Surface	
					op of Annular S	ealant
Type of Annular Sealant: <u>High-s</u>	Y				- P	
Installation Method: Tremie				1505		_
Setting Time: >24 hr.			615.52		atic Water Leve After Completion)	
Type of Bentonite Seal Gran						
Installation Method: Gravit	(choose one)		616.07	14.50 To	op of Seal	
Setting Time:15 min	<u> </u>		614.67	15.90 To	on of Sand Dool	
	<u> </u>		614.67	13.9010	op of Sand Pack	•
Type of Sand Pack: Quartz sand	1		612.83	17.74 To	op of Screen	
(4.1	ve size)		012.03		op of Screen	
Installation Method: <u>Gravity</u>	<u>y</u>		608.29	22.28 Bo	ottom of Screen	
Type of Backfill Material:n/a	(if applicable)		607.76		ottom of Well	
Installation Method: n/a	(ii apprication)		606.57	_24.00 Bo	ottom of Boreho	ole
			* Referenced to a	National Geodetic Da	atum	
		_	CAS	ING MEASUI	REMENTS	
WELL CONS	TDIICTION MATERIAI S		Diameter of Boreho	ole	(inches)	8.0
	STRUCTION MATERIALS e type of material for each area)		ID of Riser Pipe		(inches)	2.0
			Protective Casing L	ength	(feet)	5.0
Protective Casing	SS304 SS316 PTFE PVC OTH		Riser Pipe Length	F 16	(feet)	20.08
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTH		Bottom of Screen to		(feet)	0.53
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTH		Screen Length (1s Total Length of Cas		(feet)	4.54 25.15
-			Loui Longui Oi Cas	,5	(1001)	20.10

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

Illinois Environmental Protection Agency			Well Completion Rep			
Site #:	County: M	ontgomery		Well	1#:G2	210
	ver Station CCB Management Facili				ehole #:	G210
State	9.0 Y 875,359.7 (or) Latitud	•				
Surveyed By:	ck	_ IL Regist	ration #:035-00)3507		
Drilling Contractor: <u>Layne-We</u>	stern Co	_ Driller: _	D. Mahurin			
Consulting Firm: Hanson Prof	essional Services Inc.	_ Geologis	: Rhonald W. l	Hasenyager, l	LPG #196-000)246
Drilling Method: Hollow stem	auger	_ Drilling I	Fluid (Type): <u>n/a</u>	,		
Logged By: Suzanna L. Simps	son	_ Date Star	ted: 10/6/20	10 Date F	inished:10	/6/2010
Report Form Completed By: Su	zanna L. Simpson	_ Date:	10/8/2010			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths	(0.01 ft.)	
			(MSL)* 633.17	(BGS) -2.69 T	Γop of Protective	Casina
	T					
			632.99	2.51_ T	Top of Riser Pipe	;
Type of Surface Seal: Concrete			630.48	0.00	Ground Surface	
Type of Annular Sealant: High-s	colids bentonite		627.48	3.00 T	Γop of Annular S	ealant
Installation Method: Tremie	7					
Setting Time: _ >24 hr.		Ž	615.38	15.10 S	Static Water Leve	- l
Setting Time	_	<u>*</u>			(After Completion)	
Type of Bentonite Seal Grant	ular Pellet Slurry (choose one)					
Installation Method: Gravity	, , , ,	W 24	614.03	16.45 T	Top of Seal	
Setting Time: 15 hrs			612.98	17 50 т	Γop of Sand Pack	,
		T T	012.70		op of Sand Lack	
Type of Sand Pack: Quartz sand	<u> </u>		611.09	19.39 T	Top of Screen	
Grain Size:10/20 (sie			011.07		op of sereen	
Installation Method: <u>Gravity</u>	7		606.55	23.93 B	Bottom of Screen	
Type of Backfill Material: <u>n/a</u>	(Controll)		606.02		Bottom of Well	
Installation Method: n/a	(if applicable)		605.48	25.00 B	Bottom of Boreho	ale
instanction viction. <u>in a</u>				National Geodetic D		710
			CAS	ING MEASU	JREMENTS	
			Diameter of Boreho		(inches)	8.0
	TRUCTION MATERIALS the type of material for each area)	1	D of Riser Pipe		(inches)	2.0
		<u> </u>	Protective Casing L	ength	(feet)	5.0
D t t C :			Riser Pipe Length		(feet)	21.90
Protective Casing			Bottom of Screen to		(feet)	0.53
Riser Pipe Above W.T. Riser Pipe Below W.T.	SS304 SS316 PTFE (PVC) OTHER SS304 SS316 PTFE (PVC) OTHER		Screen Length (1s		(feet)	4.54
MISCI I IPC DEIOW W. I.	55504 55510 TITE (FVC) OTHER		Total Length of Cas	ıng	(feet)	26.97

Well Completion Form (revised 02/06/02)

Screen Slot Size **

 $\hbox{**Hand-Slotted Well Screens Are Unacceptable}\\$

0.010

Illinois Environ	nmental Protection Agenc	y		Well Co	mpletion	Report
Site #:	County: M	ontgomery		Well #	#: <u>G2</u>	11
	ver Station CCB Management Facili				ole#:	G211
State	9.1 Y 875,424.5 (or) Latitude	•				
Surveyed By:	ck	_ IL Regist	ration #: <u>035-00</u>	03507		
Drilling Contractor: <u>Layne-We</u>	stern Co	_ Driller: _	D. Mahurin			
Consulting Firm: Hanson Prof	essional Services Inc.	_ Geologist	: Rhonald W. l	Hasenyager, L	PG #196-000	246
Drilling Method: Hollow stem	auger	_ Drilling F	Fluid (Type): <u>n/a</u>			
Logged By: Suzanna L. Simps	son	_ Date Star	ted: 10/11/20	Date Fir	nished:10/	11/2010
Report Form Completed By: Su	zanna L. Simpson	_ Date:	10/15/2010			
ANNULAR SPA	CE DETAILS		Elevations	Depths	(0.01 ft.)	
			(MSL)* 632.83	(BGS)	p of Protective	Casing
	T					
	l f		632.64	2.33_ To	p of Riser Pipe	
Type of Surface Seal: Concrete			630.31		ound Surface	
Type of Annular Sealant: High-s	colids bentonite		627.31		p of Annular Se	ealant
Installation Method: Tremie						
Setting Time: _ >24 hr.		∇	616.17	14.14 Sta	atic Water Leve	J
Setting Time. 27 III.		~			After Completion)	
Type of Bentonite Seal Grant	ular Pellet Slurry (choose one)					
Installation Method: Gravity			616.01	14.30 To	p of Seal	
Setting Time: 15 min			614.91	15.40 To	p of Sand Pack	
			011.91		p of Sund 1 dek	
Type of Sand Pack: Quartz sand	<u> </u>		_612.97_	17.34 То	p of Screen	
Grain Size:10/20 (sie			012.57		p of Screen	
Installation Method: <u>Gravity</u>	/		608.43	21.88 Во	ottom of Screen	
Type of Backfill Material: <u>n/a</u>	(Continue)		607.90		ottom of Well	
Installation Method: n/a	(if applicable)		606.31	24.00 Bo	ottom of Boreho	ole.
instanction viction. <u>in a</u>				National Geodetic Dat		10
			CAS	ING MEASUF	REMENTS	
		I	Diameter of Boreho		(inches)	8.0
	TRUCTION MATERIALS the type of material for each area)	I	D of Riser Pipe		(inches)	2.0
		I	Protective Casing L	ength	(feet)	5.0
Production C.	GC204 GC217 PEED NYS S		Riser Pipe Length		(feet)	19.67
Protective Casing	SS304 SS316 PTFE PVC OTHER SS304 SS316 PTFE (PVC) OTHER		Bottom of Screen to		(feet)	0.53
Riser Pipe Above W.T. Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER		Screen Length (1s		(feet)	4.54
Table Tipe Below 11.1.	January 1112 (170) SINES		Total Length of Cas	ıng	(feet)	24.74

Well Completion Form (revised 02/06/02)

Screen Slot Size **

 $\hbox{**Hand-Slotted Well Screens Are Unacceptable}\\$

0.010

Illinois Environ	mental Protection Ager	ıcy		Well Co	ompletion	Report
Site #:	County:	Montgomery	7	Well #	±:G2	12
Site Name:CCB Management	t Facility			Boreho	ole #:	G212
State Plane Coordinate: X 2,515,583	3.0 Y 875,486.5 (or) La	titude: 39°	4' 6.300"	Longitude:	-89° 23	43.100"
Surveyed By: <u>Jeffrey D. Emric</u>	ck	IL Regi	stration #: <u>035-0</u>	03507		
Drilling Contractor: <u>Layne-We</u>	stern Co	Driller:	D. Mahurin			
Consulting Firm: Hanson Profe	essional Services Inc.	Geolog	ist: Rhonald W.	Hasenyager, LP	PG #196-0002	246
Drilling Method: Hollow stem	auger	Drilling	g Fluid (Type):n/g	a		
Logged By: Suzanna Simpson		Date St	arted:10/11/20	010 Date Fin	ished:10/	11/2010
Report Form Completed By: Su	zanna Simpson	Date: _	10/19/2010			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			633.12	· · · · ·	o of Protective	Casing
			632.89		o of Riser Pipe	_
Tyma of Cymfaga Coals Compute	=					
Type of Surface Seal: <u>Concrete</u>			630.59	Gro	ound Surface	
Type of Annular Sealant: High-s	solids bentonite		627.59	3.00 Top	o of Annular So	ealant
Installation Method:Tremie	2					
Setting Time: >24 hr.			616.10		tic Water Leve	
Type of Bentonite Seal Gran	ular Pellet Slurry –			(7.1	ites completion)	11/13/2010
	(choose one)					
Installation Method: <u>Gravit</u>	y		616.89	13.70 Top	o of Seal	
Setting Time: 17 min			615.79	14.80 Top	o of Sand Pack	
Type of Sand Pack:Quartz sand	1					
Grain Size: 10/20 (sie	eve size)		613.85	16.74 Top	o of Screen	
Installation Method: <u>Gravit</u>	у					
Type of Backfill Material: n/a			609.30 608.78		ttom of Screen ttom of Well	
	(if applicable)					
Installation Method:n/a			606.59 * Referenced to a	24.00 Bot National Geodetic Date	ttom of Boreho um	le
			CAS	SING MEASUR	EMENITS	
			Diameter of Boreho		(inches)	8.0
	STRUCTION MATERIALS e type of material for each area)		ID of Riser Pipe	- *	(inches)	2.0
(Choose on	,p or minorial for outer area)		Protective Casing I	ength	(feet)	5.0
			Riser Pipe Length		(feet)	19.04
Protective Casing	SS304 SS316 PTFE PVC C	THER:	Bottom of Screen to	o End Cap	(feet)	0.52
Riser Pipe Above W.T.		THER:	Screen Length (1s	st slot to last slot)	(feet)	4.55
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC O	THER:	Total Length of Cas	sing	(feet)	24.11

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

Illinois Environ	nmental Protection Agenc	y		Well Comp	pletion Repor
Site #:	County: M	ontgomery		Well #:	G213
	ver Station CCB Management Facilit				#: G213
State	3.5 Y 875,544.4 (or) Latitude	•			
Surveyed By:	ck	_ IL Regist	ration #: <u>035-00</u>	3507	
Drilling Contractor: <u>Layne-We</u>	stern Co	_ Driller: _	D. Mahurin		
Consulting Firm: Hanson Prof	essional Services Inc.	_ Geologis	t: Rhonald W. I	Hasenyager, LPG	#196-000246
Drilling Method: Hollow stem	auger	_ Drilling I	Fluid (Type): <u>n/a</u>		
Logged By: Suzanna L. Simps	son	_ Date Star	ted: 10/12/20	10 Date Finishe	ed: 10/12/2010
Report Form Completed By: Su	zanna L. Simpson	_ Date:	10/19/2010		
ANNULAR SPA	CE DETAILS		Elevations (MSL)*		(0.01 ft.)
			633.08	(BGS)2.74 Top of	Protective Casing
	T				_
	ļ f		_632.81_	2.47 Top of	Riser Pipe
Type of Surface Seal: Concrete			630.34	Ground	d Surface
Type of Annular Sealant: High-s	colids bentonite		627.64		Annular Sealant
Installation Method: Tremie					
Setting Time: _ >24 hr.		∇	615.01	15.33 Static V	Water Level
Setting Time. 27 III.		<u>*</u>			Completion) 11/15/2010
Type of Bentonite Seal Grant	ular Pellet Slurry (choose one)				
Installation Method: Gravity			616.54	13.80 Top of	Seal
Setting Time: 10 min			615.34	15.00 Top of	Sand Pack
			013.54		Sand I dek
Type of Sand Pack: Quartz sand			613.59	16.75 Top of	Screen
Grain Size:10/20 (sie	ve size)		013.37		Screen
Installation Method: <u>Gravity</u>	/		609.05	21.29 Bottom	n of Screen
Type of Backfill Material:n/a			608.52		of Well
Installation Method: n/a	(if applicable)		606.34	24.00 Bottom	n of Borehole
instanation victiod. <u>IV a</u>				Vational Geodetic Datum	TOT BOTCHOIC
			CASI	NG MEASUREM	MENTS
			Diameter of Borehol		(inches) 8.0
	TRUCTION MATERIALS the type of material for each area)		D of Riser Pipe	<u> </u>	(inches) 2.0
		<u> 1</u>	Protective Casing Le	ength	(feet) 5.0
D t t C :			Riser Pipe Length		(feet) 19.22
Protective Casing			Bottom of Screen to		(feet) 0.53
Riser Pipe Above W.T.	SS304 SS316 PTFE (PVC) OTHER SS304 SS316 PTFE (PVC) OTHER		Screen Length (1st		(feet) 4.54
Riser Pipe Below W.T.	DODON DODIO FIFE FVC OTHER	·	Total Length of Cas	ıng	(feet) 24.29

Well Completion Form (revised 02/06/02)

Screen Slot Size **

 $\hbox{**Hand-Slotted Well Screens Are Unacceptable}\\$

0.010

Illinois Environ	nmental Protection	Agency			Well C	Completion	Report
Site #:	Co	ounty: <u>Mon</u>	tgomery		Well	1#:G2	14
Site Name: AEG Coffeen Pov	ver Station CCB Managem	ent Facility			Bore	ehole #:	G214
State Plane Coordinate: X 2,515,960	0.8 Y 875,668.0 (or) Latitude:	39°	4'_8.1"	Longitude:	89°2	3 ' 38.3 "
Surveyed By:Jeffrey D. Emric	ck		IL Regis	tration #: <u>035-0</u>	03507		
Drilling Contractor: <u>Layne-We</u>	stern Co		Driller:	D. Mahurin			
Consulting Firm: Hanson Profe	essional Services Inc.		Geologis	et: Rhonald W.	Hasenyager,	LPG #196-000	246
Drilling Method: Hollow stem	auger		Drilling	Fluid (Type):n/a	a		
Logged By: Suzanna L. Simps	son		Date Sta	rted: 10/14/20	010 Date F	Finished: 10/	14/2010
Report Form Completed By: Su:	zanna L. Simpson		Date: _	10/19/2010			
ANNULAR SPA	CE DETAILS			Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
				633.08		Top of Protective	Casing
				632.85		Top of Riser Pipe	
				032.03	2.10	top of ruser rape	
Type of Surface Seal: <u>Concrete</u>			Y D	630.39	0.00 0	Ground Surface	
Type of Annular Sealant: High-s	olids bentonite			626.99	3.40 T	Top of Annular S	ealant
Installation Method: Tremie							
Setting Time:>24 hr.		_ <u> </u>	<u>z</u>	609.48		Static Water Leve	
					1	(After Completion)	11/15/2010
Type of Bentonite Seal Grant	Pellet Slurry (choose one)	T_{1}	YT.				
Installation Method: <u>Gravity</u>	r	-	 	615.39	15.00 T	Top of Seal	
Setting Time: 14 min		-		614.34	16.05 T	Top of Sand Pack	:
Type of Sand Pack: Quartz sand							
Grain Size: 10/20 (sie		-		612.64	17.75 T	Top of Screen	
Installation Method: Gravity	•						
			\exists	608.25		Bottom of Screen	
Type of Backfill Material: <u>n/a</u>	(if applicable)	_ _		607.74	22.65 B	Bottom of Well	
Installation Method:n/a		_		606.39		Bottom of Boreho	ole
				* Referenced to a	National Geodetic D	atum	
			Г	CAS	SING MEASU	JREMENTS	
WELL CONS	TRUCTION MATERIALS	S	Γ	Diameter of Boreho	ole	(inches)	8.0
	e type of material for each area)	-		ID of Riser Pipe		(inches)	2.0
				Protective Casing I	ength	(feet)	5.0
Protective Casing	SS304 SS316 PTFE PV	VC OTHER: (Riser Pipe Length Rottom of Screen to	o End Con	(feet)	0.51
Riser Pipe Above W.T.		VC OTHER:		Bottom of Screen to Screen Length (1)		(feet)	4.39
Riser Pipe Below W.T.	SS304 SS316 PTFE PV	VC OTHER:		Total Length of Ca		(feet)	25.11

Well Completion Form (revised 02/06/02)

Screen Slot Size **

 $\hbox{**Hand-Slotted Well Screens Are Unacceptable}\\$

0.010

Illinois Environ	mental Protection Agenc	y		Well Co	mpletion	Report
Site #:	County:	Montgomery		Well #	:G2	15
Site Name: CCB Management	t Facility			Boreho	ole #:C	3215
State Plane Coordinate: X 2,515,971	1.6 Y 875,810.2 (or) Latitu	ude: <u>39°</u>	4' 9.500"	Longitude:	-89° 23	38.200"
Surveyed By: <u>Jeffrey D. Emric</u>	sk	IL Regis	tration #: <u>035-00</u>	03507		
Drilling Contractor: <u>Layne-We</u>	stern Co	Driller:	D. Mahurin			
Consulting Firm: Hanson Profe	essional Services Inc.	Geologis	et: Rhonald W.	Hasenyager, LP	G#196-0002	246
Drilling Method: Hollow stem	auger	Drilling	Fluid (Type):n/a	ı		
Logged By: Suzanna Simpson	l .	Date Sta	rted:10/13/20	010 Date Fin	ished:10/2	13/2010
Report Form Completed By: Su	zanna Simpson	Date:	10/19/2010			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			633.30		o of Protective	Casing
			633.06		of Riser Pipe	
Type of Surface Seal: Concrete			630.48	0.00 Gro	ound Surface	
			627.58		of Annular Se	ealant
Type of Annular Sealant: <u>High-s</u>	Y		027.50	10p	o or runnatur ov	AIIIII
Installation Method:Tremio	2					
Setting Time: >24 hr.			607.64		tic Water Level fter Completion)	
Type of Bentonite Seal Gran						
Installation Method:Gravit	(choose one)		614.08	16.40 Top	o of Seal	
Setting Time: 20 min	<u>*</u>		612.98	17.50 Top	of Sand Pack	
	¥		_012.98_		o or sand rack	
Type of Sand Pack: Quartz sand			_611.07_	19.41 Top	of Screen	
(4.1	ve size)		011.07		of Scient	
Installation Method: Gravit	<u>y</u>		606.68	23.80 Bot	tom of Screen	
Type of Backfill Material:n/a_	(if applicable)		606.17		tom of Well	
Installation Method:n/a	(ii application)		_606.17_	24.31 Bot	tom of Boreho	le
			* Referenced to a	National Geodetic Date	ım	
		_	CAS	ING MEASUR	EMENTS	
WELL CONS	TDIICTION MATERIAI S		Diameter of Boreho	le	(inches)	8.0
	STRUCTION MATERIALS e type of material for each area)		ID of Riser Pipe		(inches)	2.0
			Protective Casing L	ength	(feet)	5.0
Protective Casing	SS304 SS316 PTFE PVC OTH		Riser Pipe Length	F. 1.C	(feet)	21.99
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTH		Bottom of Screen to		(feet)	0.51
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTH		Screen Length (1s Total Length of Cas		(feet)	4.39 26.89
		<u> </u>	. Jun Longui oi Cas	······b	(1001)	20.07

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

Illinois Environ	nmental Protection Agenc	y		Well Con	npletion Repor
Site #:	County: M	ontgomery		Well #:	G216
	ver Station CCB Management Facili				e#:G216
State	3.5 Y 875,976.1 (or) Latitud	-			
Surveyed By:Jeffrey D. Emri	ck	IL Regist	ration #: <u>035-00</u>	03507	
Drilling Contractor: <u>Layne-We</u>	stern Co	_ Driller: _	D. Mahurin		
Consulting Firm: Hanson Prof	essional Services Inc.	_ Geologist	: _ Rhonald W. I	Hasenyager, LPC	G #196-000246
Drilling Method: Hollow stem	_ Drilling F	Fluid (Type): <u>n/a</u>			
Logged By: Suzanna L. Simp	son	_ Date Star	ted: 10/13/20	Date Finis	hed: 10/13/2010
Report Form Completed By:Su	zanna L. Simpson	_ Date:	10/19/2010		
ANNULAR SPA	CE DETAILS		Elevations	Depths	(0.01 ft.)
			(MSL)* 633.02	(BGS)	of Protective Casing
	T				_
			632.76	2.48 Top o	of Riser Pipe
Type of Surface Seal: Concrete			630.28		nd Surface
Type of Annular Sealant: High-s	volids hentonite		627.78		of Annular Sealant
Installation Method: Tremie	λ				
		∇	607.52	22.76 Static	e Water Level
Setting Time: >24 hr.		∇	007.32		r Completion) 11/15/2010
Type of Bentonite Seal Gran	ular Pellet Slurry (choose one)				
Installation Method: Gravity	, , , ,		613.28		of Seal
Setting Time: 15 min			612.08	18.20 Top o	of Sand Daak
			012.00		or Sand Fack
Type of Sand Pack: Quartz sand	<u> </u>		610.24	20.04 Top o	of Screen
Grain Size:10/20 (sie	eve size)		_010.24_	Top 0	or screen
Installation Method: <u>Gravity</u>	<i>I</i>		605.86	24.42 Botto	m of Screen
Type of Backfill Material: <u>n/a</u>	(C. F. H.)		605.35		m of Well
Installation Method: n/a	(if applicable)		604.28	26.00 Botto	m of Borehole
instanation victiod. <u>IV</u> a				National Geodetic Datum	
			CASI	ING MEASURE	MENTS
		I	Diameter of Borehol		(inches) 8.0
	TRUCTION MATERIALS the type of material for each area)	I	D of Riser Pipe		(inches) 2.0
		I	Protective Casing Le	ength	(feet) 5.0
D t t C :			Riser Pipe Length		(feet) 22.52
Protective Casing			Bottom of Screen to		(feet) 0.51
Riser Pipe Above W.T. Riser Pipe Below W.T.	SS304 SS316 PTFE (PVC) OTHER SS304 SS316 PTFE (PVC) OTHER		Screen Length (1st		(feet) 4.38
Tabel Tipe Delow W.1.	55501 55510 TITE (TVC) OTHER	<u> </u>	Total Length of Cas	ing	(feet) 27.41

Well Completion Form (revised 02/06/02)

Screen Slot Size **

 $\hbox{**Hand-Slotted Well Screens Are Unacceptable}\\$

0.010

Illinois Enviror	ımental Pro	tection Agency			Well	Completion	Report
Site #:		County:Mon	ntgomery		We	ell #:G	217
Site Name: <u>AEG Coffeen Pov</u>	ver Station CCB	Management Facility			Во	orehole #:	G217
State Plane Coordinate: X 2,515,963	.0 Y <u>876,1</u>	<u>85.6</u> (or) Latitude:	39°	4'13.2"	Longitude	::89 °2	23 ' 38.3 "
Surveyed By:Jeffrey D. Emric	ck		IL Regis	stration #: <u>035-0</u>	03507		
Drilling Contractor: <u>Layne-Wes</u>	stern Co		Driller:	D. Mahurin			
Consulting Firm: Hanson Profe	essional Services	Inc.	Geologi	st: Rhonald W.	Hasenyager	, LPG #196-000)246
Drilling Method: Hollow stem	auger		Drilling	Fluid (Type):n/a	a		
Logged By: Suzanna L. Simps	son		Date Sta	arted: 10/12/20	010 Date	Finished: 10	/12/2010
Report Form Completed By: Suz	zanna L. Simpson	n	Date: _	10/19/2010			
ANNULAR SPAC	CE DETAILS			Elevations (MSL)*	Depths (BGS)	(0.01 ft.))
				633.34	` ′	Top of Protective	Casing
				633.10	-2.43	Top of Riser Pipe	e
Type of Surface Seal: Concrete				630.67	0.00	Ground Surface	
				628.27	2.40	Top of Annular S	Sealant
Type of Annular Sealant: High-s						•	
Installation Method:Tremie			_	(00.20	21.20		
Setting Time: >24 hr.			$\overline{\triangle}$	609.28	_21.39_	Static Water Lev (After Completion)	
Type of Bentonite Seal Grant		Slurry					
Installation Method: Gravity	(choose one)			612.82	_17.85_	Top of Seal	
Setting Time: 11 min				611.92	10 05	Top of Cond Doo	-
		<u> </u>		611.82	18.83	Top of Sand Pac	X
Type of Sand Pack: Quartz sand				610.18	20.49	Top of Screen	
	ve size)				20.49	Top of Screen	
Installation Method: <u>Gravity</u>	,			605.79	24.88	Bottom of Screen	1
Type of Backfill Material:n/a	(if applicable			605.29	25.38	Bottom of Well	•
Installation Method: n/a	(п аррпсаоте	,		604.67	26.00	Bottom of Boreh	ole
				* Referenced to a	National Geodetic		
				CAS	SING MEAS	SUREMENTS	
WELL CONS	TDI ICTIONI MA	TEDIALC		Diameter of Boreho	ole	(inches)	8.0
	TRUCTION MA e type of material for each			ID of Riser Pipe		(inches)	2.0
				Protective Casing I	Length	(feet)	5.0
Protective Casing	SS304 SS316	PTFE PVC OTHER:	Steel	Riser Pipe Length	- E-1C	(feet)	22.92
Riser Pipe Above W.T.		PTFE PVC OTHER:		Bottom of Screen to	•	(feet)	0.50
Riser Pipe Below W.T.		PTFE PVC OTHER:		Screen Length (1st Total Length of Cast) (feet)	4.39 27.81

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ		Well Completion Report				
Site #:	County: Mo	ontgomery		We	11 #:G2	218
Site Name: CCB Management	t Facility			Bor	ehole #:	G218
State Plane Coordinate: X 2,515,962	2.2 Y 876,380.9 (or) Latitude	: <u>39°</u>	4' 15.200"	Longitude:	89°23	38.200"
Surveyed By: <u>Jeffrey D. Emric</u>	ek	IL Regist	tration #: <u>035-0</u>	03507		
Drilling Contractor: <u>Layne-We</u>	stern Co	Driller:	D. Mahurin			
Consulting Firm: Hanson Profe	essional Services Inc.	Geologis	t: Rhonald W.	Hasenyager,	LPG #196-000	246
Drilling Method: Hollow stem	auger	Drilling l	Fluid (Type):n/a	a		
Logged By: Suzanna Simpson	1	Date Star	rted:10/12/20	010 Date 1	Finished: 10	12/2010
	zanna Simpson		10/19/2010			
ANNULAR SPA	-	Dute	Elevations		(0.01 ft.)	
ANNULAR SFA	CE DETAILS		(MSL)*	Depths (BGS)	(0.01 II.)	
	_		633.34	2.70	Top of Protective	Casing
			633.11	2.47	Top of Riser Pipe	;
Type of Surface Seal: Concrete			630.64	0.00	Ground Surface	
			627.14	3.50	Top of Annular S	Sealant
Type of Annular Sealant: High-s	7					
Installation Method: Tremie			600.00	20.75	C W	,
Setting Time: >24 hr.		∇	_609.89_	20.75	Static Water Lev (After Completion)	
Type of Bentonite Seal Gran						
Installation Method: Gravit	(choose one)		613.14	17.50	Top of Seal	
Setting Time: 17 min						
			612.14	18.50	Top of Sand Pac	
Type of Sand Pack: Quartz sand	1		610.21	20.22	Tf C	
Grain Size:10/20 (sie	ve size)		610.31	20.33	Top of Screen	
Installation Method: Gravit	<u>y</u>		605.87	24.77	Bottom of Screen	
Type of Backfill Material:n/a			605.37		Bottom of Well	
Installation Method:n/a	(if applicable)		604.64	26.00	Bottom of Boreh	ole
instantion (viction).				National Geodetic		,,,,
			CAS	SING MEASU	UREMENTS	
			Diameter of Boreho		(inches)	8.0
	STRUCTION MATERIALS e type of material for each area)		ID of Riser Pipe		(inches)	2.0
		1	Protective Casing I	ength	(feet)	5.0
Protective Ci	CC204 CC214 DTEE DVG OFFER		Riser Pipe Length		(feet)	22.80
Protective Casing Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER: SS304 SS316 PTFE PVC OTHER:		Bottom of Screen to		(feet)	0.50
Riser Pipe Below W.T.	SS304 SS316 PTFE (PVC) OTHER:	——————————————————————————————————————	Screen Length (1s) Total Length of Ca			27.74
1 2 2			Total Leligill Of Ca	əmg	(feet)	41.14

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

Illinois Environ	mental Protection A	Agency			W	ell Compl	letion Report
Site #:	Co	unty: <u>Mon</u>	tgomery			Well #:	G270
Site Name:CCB Management	t Facility					Borehole #:	G270
State Plane Coordinate: X 2,514,996	5.8 Y 874,801.9 (or) Latitude:	39°_	<u>3'</u> <u>59</u> .	.600" Longi	itude:89°	23'50.700"
Surveyed By: <u>Jeffrey D. Emric</u>	ek		IL Regis	stration #:()35-003507		
Drilling Contractor: Testing Se	ervice Corp.		Driller:	B. Willian	nson		
Consulting Firm: Hanson Profe	essional Services Inc.		Geologi	st: Rhonal	d W. Hasenya	nger, LPG #19	96-000246
Drilling Method: Hollow stem	auger		Drilling	Fluid (Type):			
Logged By: Suzanna Simpson	ı		Date Sta	arted:2/2	26/2008	Date Finished:	2/26/2008
Report Form Completed By: Su	zanna Simpson		Date: _	2/29/20	008		
ANNULAR SPA	CE DETAILS			Elevati (MSL)			01 ft.)
				626.4	`		otective Casing
				625.9			
Type of Surface Seal: Concrete				622.9	2 0.00	Ground S	urface
	-			619.9		_	nnular Sealant
Type of Annular Sealant: Benton	nite chips	-			<u> </u>	10p 01 / u	mulai Scalam
Installation Method: <u>Gravit</u>	У	-					
Setting Time: >24 hr.		- \[\breve{\Z}	<u>Z</u>	617.3	0 5.62		ter Level upletion) 3/12/2008
Type of Bentonite Seal Gran							
Installation Method:Gravit	(choose one)			619.9	2 3.00	Top of Se	al
Setting Time:>24 hr.				610.9	2 12.00	Top of Sa	nd Dook
				_010.9		<u> 10</u> 10 01 3a	nu rack
Type of Sand Pack: Quartz sand		-		609.7	9 13.13	3 Top of Sc	reen
Grain Size: 10/20 (sie	,					p	
Installation Method: <u>Gravit</u>	У			605.0	<u>0</u> <u>17.92</u>	2 Bottom of	Screen
Type of Backfill Material:n/a	(if applicable)			604.6	5 18.27	7 Bottom of	fWell
Installation Method:n/a				604.6			Borehole
				* Referen	ced to a National Ge	eodetic Datum	
			ı		CASING MI	EASUREME	
WELL CONS	STRUCTION MATERIALS	,		Diameter of I			(inches) 8.0
(Choose on	e type of material for each area)			ID of Riser P Protective Ca	-		(inches) 2.0 (feet) 5.0
				Riser Pipe Le			(feet) 3.0
Protective Casing	SS304 SS316 PTFE PV	C OTHER:			reen to End Ca	p	(feet) 0.35
Riser Pipe Above W.T.	SS304 SS316 PTFE PV	OTHER:			h (1st slot to las		(feet) 4.79
Riser Pipe Below W.T.	SS304 SS316 PTFE PV	OTHER:	[Total Length	of Casing		(feet) 21.32

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

SS304

Well Completion Form (revised 02/06/02)

SS316

Illinois Enviror	mental Protection Age	ency			Well	Completi	on Report
Site #:	County	. Mont	gomery		W	/ell #:	G271
Site Name:CCB Managemen	t Facility				В	orehole #:	G271
State Plane Coordinate: X 2,515,51	7.1 Y 874,239.4 (or) I	Latitude: _	39°	3' 54.000"	Longitude	e: <u>-89°</u> _	23' 44.100"
Surveyed By: <u>Jeffrey D. Emric</u>	ck		IL Regis	tration #:035-0	03507		
Drilling Contractor: <u>Layne-We</u>	stern Co		Driller:	G. Mills			
Consulting Firm: Hanson Prof	essional Services Inc.		Geologis	t: Rhonald W.	Hasenyager	; LPG #196-0	000246
Drilling Method: Hollow stem	auger		Drilling l	Fluid (Type):			
Logged By: Rhonald W. Hase	enyager		Date Sta	rted:9/9/200)9 Date	e Finished:	9/10/2009
Report Form Completed By: Su	zanna Simpson		Date: _	10/7/2009			
ANNULAR SPA	CE DETAILS			Elevations (MSL)*	Depths (BGS)	(0.01	ft.)
				625.88	2.99	Top of Protec	tive Casing
				625.57	-2.68	Top of Riser I	Pipe
Type of Surface Seal: Concrete				622.89	0.00	Ground Surfa	ce
				619.89	3.00	Top of Annul	
Type of Annular Sealant: High-						- op	
Installation Method:Tremi					12.50		
Setting Time: >24 hr.			-	610.39	_12.50_	Static Water I (After Completi	
Type of Bentonite Seal Gran		#					
Installation Method:Gravit	(choose one)			616.16	6.73	Top of Seal	
Setting Time: 10 min				613.87	9.02	Top of Sand I	Dook
				013.87		Top of Sand I	ack
Type of Sand Pack: Quartz sand				612.93	9.96	Top of Screen	
(a.	eve size)			012.93		Top of Sereen	
Installation Method: <u>Gravit</u>	У			608.58	14.31	Bottom of Scr	reen
Type of Backfill Material: Quar	tz sand (if applicable)			608.10	14.79	Bottom of We	ell
Installation Method: Gravit	у			_606.89_	_16.00_	Bottom of Bo	rehole
				* Referenced to a	National Geodeti	ic Datum	
			_	CAS	ING MEAS	SUREMENTS	S
WELL CONS	STRUCTION MATERIALS			Diameter of Boreho	ole	(inch	
	e type of material for each area)			ID of Riser Pipe	.1	(inch	
				Protective Casing L	ength		eet) 5.0
Protective Casing	SS304 SS316 PTFE PVC	OTHER:		Riser Pipe Length Bottom of Screen to	Fnd Can		eet) 12.64 eet) 0.48
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC	OTHER:		Screen Length (1s			eet) 4.35
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC	OTHER:		Total Length of Cas			eet) 17.47

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

Illinois Enviro	nmental Protection Agency			Well	Completion	Report
Site #:	County: Mor	ntgomery		W	Vell #: <u>G2</u>	72
Site Name: <u>AEG Coffeen Po</u>	wer Station CCB Management Facility			В	orehole #:	3272
State- Plant Plane Coordinate: X 2,515,74	5.0 Y 874,234.8 (or) Latitude:		<u>"</u>	Longitude	e:°	
Surveyed By:Jeffrey D. Emr	ick	IL Regist	ration #: <u>035-00</u>	3507		
Drilling Contractor: <u>Layne-We</u>	estern Co	Driller: _	G. Mills			
Consulting Firm: Hanson Pro-	Geologist	: Rhonald W. H	Hasenyage	r, LPG #196-0002	246	
Drilling Method: Hollow stem	n auger	Drilling I	Fluid (Type):			
Logged By: Rhonald W. Has	enyager	Date Star	ted: 9/10/200	9 Date	e Finished: 9/1	0/2009
Report Form Completed By: Su	ızanna L. Simpson	Date:	10/7/2009			
ANNULAR SPA	CE DETAILS		Elevations	Depths	(0.01 ft.)	
			(MSL)*	(BGS)	(*** **)	
			624.11	-3.39	Top of Protective	Casing
			623.81	-3.09	Top of Riser Pipe	
Type of Surface Seal: Concrete			620.72	0.00	Ground Surface	
T CA LOLL WIL			617.72	3.00	Top of Annular Se	ealant
Type of Annular Sealant: High-	7					
Installation Method: Tremi		_	611.23	9.49	Static Water Level	I
Setting Time. 224 III.		☑	011.23		(After Completion)	
Type of Bentonite Seal Gran	nular Pellet Slurry (choose one)					
Installation Method: <u>Gravit</u>	` '	X X	614.55	6.17	Top of Seal	
Setting Time: 10 min			612.74	7.98	Top of Sand Pack	
Type of Sand Pack: Quartz sand	d					
	eve size)	:	611.61	9.11	Top of Screen	
Installation Method: Gravit	<u>y</u>					
Type of Backfill Material:n/a_			606.74 606.40	13.98 14.32	Bottom of Screen Bottom of Well	
Type of Backfill Material. <u>11/a</u>	(if applicable)		_000.40_	17.32	Bottom of wen	
Installation Method:n/a			606.40 * Referenced to a N	14.32 Tational Geodeti	Bottom of Boreho	le
			CAST			
		Γ,			SUREMENTS	0.0
	STRUCTION MATERIALS		Diameter of Borehol D of Riser Pipe	<u> </u>	(inches)	2.0
(Choose o	ne type of material for each area)		Protective Casing Le	ength	(feet)	5.0
			Riser Pipe Length		(feet)	12.20
Protective Casing	SS304 SS316 PTFE PVC OTHER:	Steel	Bottom of Screen to	End Cap	(feet)	0.34
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER:		Screen Length (1st		t) (feet)	4.87
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER:	-	Total Length of Casi	nα	(feet)	17 41

Screen

Well Completion Form (revised 02/06/02)

SS316

PTFE PVC OTHER:

**Hand-Slotted Well Screens Are Unacceptable

0.010

(inches)

Screen Slot Size **

Illinois Environ		Well Completion Report				
Site #:	County: Mo	ontgomery		W	ell #:	G273
Site Name: CCB Management	: Facility			Во	orehole #:	G273
State Plane Coordinate: X 2,515,975	5.5 Y 874,235.2 (or) Latitude	e: <u>39°</u>	3' 53.900"	Longitude	e: <u>-89°</u> _	23' 38.300"
Surveyed By:	k	IL Registr	ration #: <u>035-00</u>	03507		
Drilling Contractor: <u>Layne-We</u>	stern Co	_ Driller: _	G. Mills			
Consulting Firm: Hanson Profe	essional Services Inc.	Geologist	: Rhonald W.	Hasenyager	, LPG #196-0	00246
Drilling Method: Hollow stem	auger	_ Drilling F	luid (Type):			
Logged By: Rhonald W. Hase	nyager	_ Date Start	ted:9/10/20	09 Date	Finished:	9/10/2009
Report Form Completed By: Su	zanna Simpson	Date:	10/7/2009			
ANNULAR SPA			Elevations (MSL)*	Depths (BGS)	(0.01	ft.)
			623.33	3.16	Top of Protect	tive Casing
	T		623.02	-2.85	Top of Riser I	_
Type of Surface Seal: Concrete			620.17	0.00	Ground Surfa	ce
Type of Annular Sealant: High-s	solids bentonite		617.17	3.00	Top of Annula	ar Sealant
Installation Method:Tremie	7					
Setting Time: _ >24 hr.		∇	610.28	9.89	Static Water I (After Completion	
Type of Bentonite Seal Grant					(_F	,
Installation Method: <u>Gravit</u>	(choose one)	KX	614.07	6.10	Top of Seal	
Setting Time: 10 min			612.45	7.72	Top of Sand F	ack
Type of Sand Pack: Quartz sand	<u> </u>					
Grain Size:10/20 (sie	ve size)		611.09	9.08	Top of Screen	
Installation Method: Gravity	<u>y</u>					
Type of Backfill Material: Quart	z sand (if applicable)		605.61 605.07	14.56 15.10	Bottom of Scr Bottom of We	
Installation Method: Gravit			604.17	16.00	Bottom of Bor	rehole
			* Referenced to a	National Geodeti	c Datum	
		_	CAS	ING MEAS	SUREMENTS	3
WELL CONS	TRUCTION MATERIALS	Г	Diameter of Boreho	le	(inch	es) 8.0
	e type of material for each area)		D of Riser Pipe		(inch	
			Protective Casing L	ength		eet) 5.0
Protective Casing	SS304 SS316 PTFE PVC OTHER		Riser Pipe Length Bottom of Screen to	End Con	•	eet) 11.93 eet) 0.54
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER		Screen Length (1s	•	•	eet) 0.54 eet) 5.48
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER		Cotal Length of Cas			eet) 17.95

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

Illinois Enviro	nmental Protection	Agency			Well	Completion	Report
Site #:	C	ounty: Mon	tgomery	Ī	W	/ell #:G	274
Site Name: AEG Coffeen Por	wer Station CCB Manager	nent Facility			В	orehole #:	G274
State- Plant Plane Coordinate: X 2,516,19	5.6 Y <u>874,239.2</u> (o	r) Latitude:	C	· ' "	Longitud	e:°	
Surveyed By: <u>Jeffrey D. Emri</u>	ck		IL Regi	istration #: <u>035-0</u>	03507		
Drilling Contractor: <u>Layne-We</u>	estern Co		Driller:	G. Mills			
Consulting Firm: Hanson Prof	Fessional Services Inc.		Geolog	ist: Rhonald W.	Hasenyage	r, LPG #196-000)246
Drilling Method: Hollow stem	ı auger		Drilling	g Fluid (Type):			
Logged By: Rhonald W. Hase	enyager		Date St	arted: 9/16/20	009 Dat	e Finished: 9/	16/2009
Report Form Completed By: Su	zanna L. Simpson		Date: _	10/7/2009			
ANNULAR SPA	CE DETAILS			Elevations (MSL)*	Depths (BGS)	(0.01 ft.))
				624.32	-2.65	Top of Protective	Casing
		T		624.04		Top of Riser Pipe	
Type of Surface Seal: <u>Concrete</u>				621.67	0.00	Ground Surface	
Type of Annular Sealant: High-	solids bentonite			618.67	3.00	Top of Annular S	Sealant
Installation Method: Tremic							
Setting Time:>24 hr.		_ \	7_	608.55	13.12	Static Water Lev (After Completion)	
Type of Bentonite Seal Gran	ular Pellet Slurry					(i iidi compiduoii)	<i>3</i> /21/2003
Installation Method: <u>Gravit</u>	(choose one)	_	XX	611.93	9.74	Top of Seal	
Setting Time: 15 min		-		610.15	11.52	Top of Sand Pac	ζ
Type of Sand Pack: Quartz sand	1						
	eve size)	- 		_608.77_	12.90	Top of Screen	
Installation Method: <u>Gravit</u>	y	-					
Type of Backfill Material:n/a	(if applicable)	_ 🗏		604.00 603.61	17.67 18.06	Bottom of Screen Bottom of Well	1
Installation Method:n/a	(п арупсаоге)			_603.61_	_18.06_	Bottom of Boreh	ole
				* Referenced to a	National Geodeti	ic Datum	
				CAS	SING MEA	SUREMENTS	
WELL CONS	STRUCTION MATERIAL	S		Diameter of Boreho	ole	(inches)	8.0
	ne type of material for each area)	_		ID of Riser Pipe	1	(inches)	2.0
				Protective Casing I	Length	(feet)	5.0
Protective Casing	SS304 SS316 PTFE P	VC OTHER: (Steel	Riser Pipe Length Bottom of Screen t	o End Con	(feet)	0.39
Riser Pipe Above W.T.		VC OTHER:		Screen Length (1	-		4.77
Riser Pipe Below W.T.	SS304 SS316 PTFE P	VC OTHER:		Total Length of Ca		(feet)	20.43

Well Completion Form (revised 02/06/02)

SS316

PTFE PVC OTHER:

**Hand-Slotted Well Screens Are Unacceptable

Screen Slot Size **

0.010

Illinois Environ	mental Protection Agency	y		Well	Completion	Report
Site #:	County: M	ontgomery		W	/ell #: <u>G2</u>	275
Site Name: AEG Coffeen Pow	ver Station CCB Management Facilit	V		Ве	orehole #:	G275
State- Plant	.9 Y 874,298.9 (or) Latitude	-				
Surveyed By: <u>Jeffrey D. Emric</u>	k	_ IL Registr	ration #:035-0	03507		
Drilling Contractor: <u>Layne-Wes</u>	stern Co	_ Driller: _	G. Mills			
Consulting Firm: Hanson Profe	essional Services Inc.	_ Geologist:	Rhonald W.	Hasenyager	r, LPG #196-000)246
Drilling Method: Hollow stem	auger	Drilling Fluid (Type):				
Logged By: Rhonald W. Hase:	nyager	_ Date Start	ed: 9/16/20	09 Date	e Finished: 9/2	16/2009
Report Form Completed By: Suz	zanna L. Simpson	Date:	10/7/2009			
ANNULAR SPAC	CE DETAILS		Elevations		(0.01 ft.))
			(MSL)*	(BGS)		
	T		618.53	2.39	Top of Protective	Casing
	l f.		618.26	2.12	Top of Riser Pipe	e
Type of Surface Seal: Concrete			616.14	0.00	Ground Surface	
			613.14	3.00	Top of Annular S	Sealant
Type of Annular Sealant: High-se	71					
Installation Method: Tremie			(05.90	10.25	Cont. What I	1
Setting Time: >24 hr.		Ž	_605.89_	10.25	Static Water Leve (After Completion)	
Type of Bentonite Seal Granu						
Installation Method: Gravity	(choose one)		_610.42_	5.72	Top of Seal	
Setting Time: 15 min			600 16	6.98	Ton of Cond Dool	-
			609.16	0.98	Top of Sand Pack	
Type of Sand Pack: Quartz sand			607.92	8.22	Top of Screen	
Grain Size:10/20 (siev	ve size)		_007.92_	0.22	Top of Screen	
Installation Method: <u>Gravity</u>			603.52	12.62	Bottom of Screen	
Type of Backfill Material:n/a		==	602.95	13.19	Bottom of Well	L
T (U.C. M.A. I.)	(if applicable)		(02.05	12.10	D # CD 1	
Installation Method:n/a			602.95 * Referenced to a	13.19 National Geodeti		ole
			CAS	SING MEAS	SUREMENTS	
		D	Diameter of Boreho		(inches)	8.0
	TRUCTION MATERIALS etype of material for each area)	П	O of Riser Pipe		(inches)	2.0
		P	rotective Casing I	ength	(feet)	5.0
	99994		iser Pipe Length		(feet)	10.34
Protective Casing Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER SS304 SS316 PTFE PVC OTHER		ottom of Screen to	-		0.52
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER		creen Length (1storal Length of Cas		t) (feet)	4.45 15.31

Screen

Well Completion Form (revised 02/06/02)

SS316

PTFE

PVC OTHER:

**Hand-Slotted Well Screens Are Unacceptable

Screen Slot Size **

0.010

Illinois Environ	mental Protection Ag	gency			Well	Completi	on Report	
Site #:	Coun	ty: <u>Mon</u>	tgomery	7	W	/ell #:	G276	
Site Name:CCB Management	t Facility				Во	orehole #:	G276	
State Plane Coordinate: X 2,516,358	3.8 Y 874,438.6 (or)	Latitude:	39°	3' 55.900'	Longitude	e: <u>-89°</u>	23' 33.400"	
Surveyed By: <u>Jeffrey D. Emric</u>	ck		IL Regi	stration #:035-0	003507			
Drilling Contractor: <u>Layne-We</u>	stern Co		Driller:	G. Mills				
Consulting Firm: Hanson Profe	essional Services Inc.		Geolog	ist: Rhonald W.	Hasenyager	, LPG #196-0	00246	
Drilling Method: Hollow stem	auger		Drilling	ng Fluid (Type):				
Logged By: Rhonald W. Hase	nyager		Date St	arted: 9/16/20	009 Date	e Finished:	9/16/2009	
Report Form Completed By: Su	zanna Simpson		Date:	10/7/2009				
ANNULAR SPA	CE DETAILS			Elevations (MSL)*	Depths (BGS)	(0.01	ft.)	
				632.40		Top of Protect	ive Casing	
				632.00				
				_032.00		Top of raser i	TPC	
Type of Surface Seal: <u>Concrete</u>			Ý 🕅	629.14	0.00	Ground Surfac	ce	
Type of Annular Sealant: High-s	solids bentonite			626.14	3.00	Top of Annula	ır Sealant	
Installation Method:Tremie								
			<u>z</u>	603.59	25.55	Static Water I		
						(After Completic	on) 9/21/2009	
Type of Bentonite Seal Grant	Pellet Slurry (choose one)	T1	YT.					
Installation Method: Gravity	у	×	\bowtie	610.06	19.08	Top of Seal		
Setting Time: 15 min				608.11	21.03	Top of Sand P	ack	
Type of Sand Pack: Quartz sanc	1							
	ve size)			606.73	_22.41_	Top of Screen		
Installation Method: Gravit	,							
			∄	601.92	27.22	Bottom of Scr		
Type of Backfill Material: Quart	(if applicable)			601.49	27.65	Bottom of We	II	
Installation Method: Gravity	у			601.14 * Referenced to	28.00 a National Geodeti	Bottom of Bor	rehole	
				Referenced to	a reational ocodeti	c Datum		
						SUREMENTS		
	TRUCTION MATERIALS			Diameter of Boreh	iole	(inch	2.0	
(Choose on	e type of material for each area)			ID of Riser Pipe Protective Casing	I enoth	(inch	es) 2.0 et) 5.0	
				Riser Pipe Length		•	et) 3.0 et) 25.27	
Protective Casing	SS304 SS316 PTFE PVC	OTHER:		Bottom of Screen			et) 0.43	
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC	OTHER:		Screen Length (1		`	et) 4.81	
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC	OTHER:		Total Length of Ca	asing	(fe	et) 30.51	

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

Illinois Environ	nmental Protection Agency			Well	Completion	Report
Site #:	County: Mo	ntgomery		W	/ell #: <u>G2</u>	77
Site Name: AEG Coffeen Pov	wer Station CCB Management Facility			Be	orehole #:	277
State- Plant Plane Coordinate: X 2,516,370	0.5 Y 874,581.8 (or) Latitude:			Longitude	e:°	'
Surveyed By:Jeffrey D. Emri	ck	IL Registra	ation #:035-00)3507		
Drilling Contractor: <u>Layne-We</u>	estern Co	Driller:	G. Mills			
Consulting Firm: Hanson Prof	Sessional Services Inc.	Geologist:	Rhonald W. 1	Hasenyager	r, LPG #196-0002	246
Drilling Method: Hollow stem	auger	Drilling Flo	uid (Type):			
Logged By: Rhonald W. Hase	enyager	Date Starte	ed:9/14/200	09 Date	e Finished: 9/1	4/2009
Report Form Completed By: Su	zanna L. Simpson	Date:	10/7/2009			
ANNULAR SPA	-		Elevations	Depths	(0.01 ft.)	
	C2 2211120		(MSL)*	(BGS)	(0.01 10.)	
			623.35	-2.56	Top of Protective	Casing
		=	623.08	2.29	Top of Riser Pipe	
Type of Surface Seal: Concrete			620.79	0.00	Ground Surface	
Type of Annular Sealant: High-s	solids bentonite		617.79	3.00	Top of Annular Se	ealant
Installation Method:Tremie	7					
Setting Time: >24 hr.		abla	_602.56_	18.23	Static Water Level	
					(After Completion)	9/21/2009
Type of Bentonite Seal Gran	ular Pellet Slurry (choose one)					
Installation Method: Gravity	——————————————————————————————————————		_608.00_	_12.79_	Top of Seal	
Setting Time: 18 min			607.00	13.79	Top of Sand Pack	
Type of Sand Pack: Quartz sand	<u> </u>					
Grain Size:10/20 (sign	eve size)		_606.50_	_14.29_	Top of Screen	
Installation Method: Gravity	y		602.02	10.77		
Type of Backfill Material: Quar			602.02 601.55	18.77 19.24	Bottom of Screen Bottom of Well	
	(if applicable)					
Installation Method: <u>Gravity</u>	<i>y</i>		600.79 * Referenced to a N	20.00 National Geodeti	Bottom of Boreho	le
			CAS	ING MEAS	SUREMENTS	
		Di	iameter of Boreho		(inches)	8.0
	TRUCTION MATERIALS ne type of material for each area)		of Riser Pipe		(inches)	2.0
		Pr	otective Casing Lo	ength	(feet)	5.0
n	GG004 GG016 PTTT		iser Pipe Length		(feet)	16.58
Protective Casing Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER: SS304 SS316 PTFE (PVC) OTHER:		ottom of Screen to	-	(feet)	0.47
Riser Pipe Below W.T.	SS304 SS316 PTFE (PVC) OTHER:		ereen Length (1st		t) (feet)	21.53

Screen

Well Completion Form (revised 02/06/02)

SS316

PTFE PVC OTHER:

**Hand-Slotted Well Screens Are Unacceptable

0.010

(inches)

Screen Slot Size **

Illinois Environ	nmental Protection Agency	7		Well	Completion	Report
Site #:	County: <u>Mo</u>	ntgomery		W	/ell #: <u>G2</u>	278
Site Name: AEG Coffeen Pov	wer Station CCB Management Facility	<i>I</i>		В	orehole #:	G278
State- Plant Plane Coordinate: X 2,516,200).7 Y 874,875.4 (or) Latitude:			Longitude	e:°	
Surveyed By: <u>Jeffrey D. Emri</u>	ck	IL Registra	ation #:035-0	03507		
Drilling Contractor: <u>Layne-We</u>	estern Co	Driller: _	G. Mills			
Consulting Firm: Hanson Prof	essional Services Inc.	Geologist:	Rhonald W.	Hasenyager	r, LPG #196-000)246
Drilling Method: Hollow stem	ng Method: Hollow stem auger Drilling Fluid (Type):					
Logged By: Rhonald W. Hase	enyager	Date Starte	ed: 9/11/20	09 Date	e Finished: 9/	11/2009
Report Form Completed By: Su	zanna L. Simpson	Date:	10/7/2009			
ANNULAR SPA	CE DETAILS		Elevations		(0.01 ft.))
			(MSL)*	(BGS)	T (D ()	<i>C</i> :
			631.49	2.04	Top of Protective	Casing
			631.17	2.32	Top of Riser Pipe	:
Type of Surface Seal: Concrete			_628.85_	0.00	Ground Surface	
T CA L C L 4 HT L			625.85	3.00	Top of Annular S	Sealant
Type of Annular Sealant: High-s	71					
Installation Method: Tremie			604.05	22.00		
Setting Time: >24 hr.		$\overline{\Delta}$	_604.87_	23.98	Static Water Leve (After Completion)	
Type of Bentonite Seal Gran						
Installation Method: Gravity	(choose one)		613.74	15 11	Ton of Seal	
Setting Time: 22 min					Top of Scar	
Setting Time. 22 min	<u> </u>	X	611.90	16.95	Top of Sand Pack	ζ
Type of Sand Pack: Quartz sand	<u> </u>					
Grain Size:10/20 (sign	eve size)	≣ │	609.92	18.93	Top of Screen	
Installation Method: Gravity						
Type of Backfill Material: n/a			605.15 604.79	23.70 24.06	Bottom of Screen Bottom of Well	ı
1)pe of Buomm 11	(if applicable)				Bowell of Well	
Installation Method: <u>n/a</u>			604.79 * Referenced to a	24.06 National Geodeti	Bottom of Boreho	ole
			CAS			
		n	riameter of Boreho		SUREMENTS (inches)	8.0
	TRUCTION MATERIALS ue type of material for each area)		O of Riser Pipe	-	(inches)	2.0
(2.13000 0.1	• • • • • • • • • • • • • • • • • • • •		rotective Casing L	ength	(feet)	5.0
	I		iser Pipe Length		(feet)	21.25
Protective Casing	SS304 SS316 PTFE PVC OTHER:	Steel	ottom of Screen to	o End Cap	(feet)	0.36
Riser Pipe Above W.T. Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER: SS304 SS316 PTFE PVC OTHER:		creen Length (1s			4.77
Most Tipe Delow W.1.	DOSOT DOSTO TITE (FVC) OTHER.	1 T	otal Length of Cas	sing	(feet)	26.38

Screen

Well Completion Form (revised 02/06/02)

SS316

PTFE

PVC OTHER:

**Hand-Slotted Well Screens Are Unacceptable

Screen Slot Size **

0.010

Illinois Environmental Protection Agency	Well Completion Report
Site #: County: M	Contgomery Well #: G279
Site Name: CCB Management Facility	Borehole #: G279
State Plane Coordinate: X 2,516,245.6 Y 875,028.1 (or) Latitude	e: <u>39° 4' 1.800"</u> Longitude: <u>-89° 23' 34.800'</u>
Surveyed By:	IL Registration #: 035-003507
Drilling Contractor: Layne-Western Co	Driller: G. Mills
Consulting Firm: Hanson Professional Services Inc.	Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger	Drilling Fluid (Type):
Logged By: Rhonald W. Hasenyager	Date Started:9/10/2009 Date Finished:9/10/2009
Report Form Completed By: Suzanna Simpson	Date:10/7/2009
ANNULAR SPACE DETAILS	Elevations Depths (0.01 ft.) (MSL)* (BGS)
Type of Surface Seal: Concrete	629.19 0.00 Ground Surface
Type of Annular Sealant: High-solids bentonite	
Installation Method: <u>Tremie</u>	
Setting Time:>24 hr.	<u>√</u> 601.66 27.53 Static Water Level (After Completion) 9/21/2009
Type of Bentonite Seal Granular Pellet Slurry	
(choose one) Installation Method: Gravity	
Setting Time:18 min	
Type of Sand Pack: Quartz sand	
Grain Size: 10/20 (sieve size)	
Installation Method: <u>Gravity</u>	
Type of Backfill Material: Quartz Sand	602.40 26.79 Bottom of Screen 604.51 24.68 Bottom of Well
(if applicable)	27.3
Installation Method: Gravity	* Referenced to a National Geodetic Datum
	CASING MEASUREMENTS
	Diameter of Borehole (inches) 8.0
WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)	ID of Riser Pipe (inches) 2.0
	Protective Casing Length (feet) 5.0
Protective Casing SS304 SS316 PTFE PVC OTHER	Riser Pipe Length (feet) 25.25
Riser Pipe Above W.T. SS304 SS316 PTFE PVC OTHER	Bottom of Screen to End Cap (feet) 0.33
Riser Pipe Below W.T. SS304 SS316 PTFE PVC OTHER	Selecti Length (1st slot to last slot) (leet) 4.57

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

SS304

Well Completion Form (revised 02/06/02)

SS316

Illinois Environ	mental Protection	Agency				Well	Complet	tion Report
Site #:	Со	ounty: <u>Mon</u>	tgomery	ī		W	/ell #:	G280
Site Name: CCB Management	t Facility					В	orehole #:	G280
State Plane Coordinate: X 2,515,679	9.5 Y 875,045.1 (or	r) Latitude:	39°	4'_	2.000"	Longitud	e: <u>-89°</u> _	23' 42.000"
Surveyed By: <u>Jeffrey D. Emric</u>	ek		IL Regi	istration	#: 035-0	03507		
Drilling Contractor: Testing Se	ervice Corp.		Driller:	B. V	Williamson			
Consulting Firm: Hanson Profe	essional Services Inc.		Geolog	ist: R	Rhonald W.	Hasenyager	., LPG #196-	-000246
Drilling Method: Hollow stem	auger		Drilling	g Fluid ((Type):			
Logged By: Suzanna Simpson	ı		Date St	arted:	2/26/20	08 Date	e Finished:	2/26/2008
Report Form Completed By: Su	zanna Simpson		Date: _	2	2/29/2008			
ANNULAR SPA	CE DETAILS				levations (MSL)*	Depths (BGS)	(0.01	ft.)
					625.79	-2.84	Top of Prote	ective Casing
		T			625.30	-2.35		_
Type of Surface Seal: Concrete				»— -	622.95	0.00	Ground Surf	face
Type of Annular Sealant: Benton	nite chins			/ -	620.85	2.10	Top of Annu	ılar Sealant
Installation Method: Gravit	•	- 9						
Setting Time: _ >24 hr.		_ \\\\\\\\\\	<u>z</u>	_	618.61	4.34	Static Water	Level (stion) 3/12/2008
Type of Bentonite Seal Gram	ular Pellet Slurry						(After Comple	210H) 3/12/2008
Installation Method: Gravit	(choose one)				620.85	2.10	Top of Seal	
Setting Time: >24 hr.		_ 🖁					•	D 1
				-	611.75	11.20	Top of Sand	Pack
Type of Sand Pack: Quartz sand	1	_			610.16	12.79	Top of Scree	an .
Grain Size: 10/20 (sie	,			_	010.10		Top of Scied	211
Installation Method: <u>Gravit</u>	У	-			605.32	17.63	Bottom of Se	creen
Type of Backfill Material:n/a_	(if applicable)	_ \square			604.97	17.98	Bottom of W	/ell
Installation Method:n/a	, ,	_		_	604.97	17.98	Bottom of B	orehole
				*	Referenced to a	National Geodet	ic Datum	
					CAS	SING MEAS	SUREMENT	TS .
WELL CONS	STRUCTION MATERIALS	S			eter of Boreho	ole	(in	ches) 8.0
	e type of material for each area)	_			Riser Pipe	41		ches) 2.0
					tive Casing L	ength		(feet) 5.0
Protective Casing	SS304 SS316 PTFE P	VC OTHER:			Pipe Length n of Screen to	o End Can		(feet) 15.14 (feet) 0.35
Riser Pipe Above W.T.	SS304 SS316 PTFE P	VC OTHER:			Length (1s			(feet) 4.84
Riser Pipe Below W.T.	SS304 SS316 PTFE P	VC OTHER:			Length of Cas			(feet) 20.33

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

SS304

Well Completion Form (revised 02/06/02)

SS316

Illinois Enviror	nmental Protection Agency			Well	Completion	Report
Site #:	County: _ Me	ontgomery		W	Vell #: G2	281
Site Name: Natural Resource	Technology, Inc. Coffeen Energy Cen	ter		В	orehole #:	G281
State Plane Coordinate: X 874,37	5.4 Y 2,514,455.5 (or) Latitude	e:		Longitud	e:	
Surveyed By: Gary C. Rogers		IL Registr	ration #:035-0	02957		
Drilling Contractor: Ramsey G	eotechnical Engineering, LLC	_ Driller: _	D. Crump			
Consulting Firm: Hanson Prof	essional Services Inc.	_ Geologist:	Rhonald W.	Hasenyager	<u>-, LPG #196-000</u>	246
Drilling Method: Hollow stem auger Drill			luid (Type): <u>no</u>	ne		
Logged By: Kristen L. Theest	îeld	_ Date Start	ed: 9/8/201	15 Date	e Finished:9	/8/2015
Report Form Completed By: Su	ızanna L. Keim	Date:	10/6/2015			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
	_		626.64	2.82	Top of Protective	Casing
	F		626.36	-2.54	Top of Riser Pipe	;
Type of Surface Seal: Concrete			623.82	0.00	Ground Surface	
			622.82	1.00	Top of Annular S	Sealant
Type of Annular Sealant: High-	7					
Installation Method:Tremi	e					
Setting Time: >24 hours		$\overline{\Delta}$			Static Water Leve (After Completion)	el
Type of Bentonite Seal Gran						
Installation Method: Gravit	(choose one)		612.59	_11.23_	Top of Seal	
Setting Time: 25 minutes			610.37	13.45	Top of Sand Pac	ζ.
T. 40 ID I					1	
Type of Sand Pack: Quartz San	_		608.31	15.51	Top of Screen	
Grain Size: 10-20 (sie					1	
Installation Method: <u>Gravit</u>	<u>y</u>		603.66	20.16	Bottom of Screen	ı
Type of Backfill Material:n/a_	(if applicable)		603.53	20.29	Bottom of Well	
Installation Method:	(= apparation)		603.53	20.29	Bottom of Boreh	ole
			* Referenced to a	National Geodet	ic Datum	
			CAS	SING MEAS	SUREMENTS	
WELL COM	CTDLICTION MATERIAL C	Е	iameter of Boreho	ole	(inches)	8.0
	STRUCTION MATERIALS ne type of material for each area)		O of Riser Pipe		(inches)	2.0
			rotective Casing L	ength	(feet)	5.0
Protective Casing	SS304 SS316 PTFE PVC OTHER	G: 1	iser Pipe Length ottom of Screen to	n End Can	(feet)	17.80 0.38
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER		creen Length (1s	•		4.65
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER		otal Length of Ca		(feet)	22.83

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

Illinois Enviror	nmental Protection Agency			Well	Completion	Report	
Site #:	County: _ Mo	ontgomery		W	Vell #:G	301	
Site Name: Natural Resource	Technology, Inc. Coffeen Energy Cen	ter		В	orehole #:	G301	
State Plane Coordinate: X 872,23	4.8 Y 2,515,583.0 (or) Latitude	e:		Longitud	e:		
Surveyed By: Gary C. Rogers		IL Registi	ration #:035-0	02957			
Drilling Contractor: Ramsey G	eotechnical Engineering, LLC	_ Driller: _	D. Crump				
Consulting Firm: Hanson Prof	essional Services Inc.	Geologist	Rhonald W.	Hasenyager	<u>, LPG #196-000</u>	246	
Drilling Method: Hollow stem auger Dri			ing Fluid (Type):				
Logged By: Kristen L. Theest	îeld	Date Start	ted: 9/4/201	15 Date	e Finished: 9/	/4/2015	
Report Form Completed By: Su	ızanna L. Keim	Date:	10/6/2015				
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)		
			622.98	2.71	Top of Protective	Casing	
	l E		622.65	2.38	Top of Riser Pipe	•	
Type of Surface Seal: Concrete			620.27	0.00	Ground Surface		
Type of Annular Sealant: Bento	nite Chips		618.20	2.07	Top of Annular S	Sealant	
Installation Method: <u>Gravit</u>	<u>y</u>						
Setting Time: _ >24 hours		∇			Static Water Leve (After Completion)	el	
Type of Bentonite Seal Gran	Pellet Slurry (choose one)						
Installation Method: <u>Gravit</u>	y		n/a	n/a	Top of Seal		
Setting Time: 25 minutes			612.75	7.52	Top of Sand Pack	ζ	
Type of Sand Pack: Quartz San	d						
Grain Size:10-20 (side	eve size)		608.96	11.31	Top of Screen		
Installation Method: <u>Gravit</u>	y		604.31	15.96	Bottom of Screen		
Type of Backfill Material:n/a	(if applicable)		604.06	16.21	Bottom of Well		
Installation Method:	(п присток)		604.06	16.21	Bottom of Boreho	ole	
			* Referenced to a	National Geodet	ic Datum		
			CAS	SING MEAS	SUREMENTS		
WELL CONS	STRUCTION MATERIALS		Diameter of Boreho	ole	(inches)	8.0	
	ne type of material for each area)		D of Riser Pipe		(inches)	2.0	
			rotective Casing L	ength	(feet)	5.0 13.56	
Protective Casing	SS304 SS316 PTFE PVC OTHER	a. 1	Riser Pipe Length Bottom of Screen to	o End Can	(feet)	0.38	
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER:		creen Length (1s	•		4.65	
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER		otal Length of Ca		(feet)	18.59	

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

Illinois Environ	mental Protection Agend	cy		Well	Completion	Report
Site #:	County:	Montgomery		W	/ell #:G	302
Site Name: Natural Resource	Technology, Inc. Coffeen Energy (Center		В	orehole #:	G302
State Plane Coordinate: X 872,253	3.0 Y 2,516,214.2 (or) Latit	tude:		Longitud	e:	
Surveyed By: Gary C. Rogers		IL Regi	stration #: <u>035-0</u>	002957		
Drilling Contractor: Ramsey G	eotechnical Engineering, LLC	Driller:	D. Crump			
Consulting Firm: Hanson Profe	essional Services Inc.	Geologi	st: Rhonald W.	Hasenyager	, LPG #196-000	246
Drilling Method: Hollow stem	auger	Drilling	Fluid (Type):nc	one		
Logged By: Kristen L. Theesf	eld	Date St	arted:9/3/20	15 Date	e Finished: 9	/4/2015
Report Form Completed By: Su	zanna L. Keim	Date: _	10/7/2015			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.))
	_		620.34	-2.39	Top of Protective	Casing
	_		620.04	-2.09	Top of Riser Pipe	2
Type of Surface Seal: Concrete			617.95	0.00	Ground Surface	
Type of Annular Sealant: High-s	solids bentonite		615.95	2.00	Top of Annular S	Sealant
Installation Method:Tremic	2					
Setting Time: >24 hours					Static Water Lev (After Completion)	el
Type of Bentonite Seal Gran	ular Pellet Slurry (choose one)					
Installation Method: Gravit	y		607.78	_10.17_	Top of Seal	
Setting Time: 25 minutes			605.88	_12.07_	Top of Sand Pac	ζ
Type of Sand Pack: Quartz Sand	d					
Grain Size: 10-20 (sie	ve size)		604.74	13.21	Top of Screen	
Installation Method: <u>Gravit</u>	y		(00.00	17.06	D. #	
Type of Backfill Material:n/a	(Controll)		<u>600.09</u> <u>599.56</u>	17.86 18.39	Bottom of Screen Bottom of Well	l
Installation Method:	(if applicable)		599.56	18.39	Bottom of Boreh	ole
			* Referenced to a	a National Geodet	ic Datum	
			CAS	SING MEAS	SUREMENTS	
WELL COM	TDIICTION MATERIAI C		Diameter of Boreh	ole	(inches)	8.0
	TRUCTION MATERIALS e type of material for each area)		ID of Riser Pipe		(inches)	2.0
			Protective Casing I		(feet)	5.0
Protective Casing	SS304 SS316 PTFE PVC OTI	HER: Steel	Riser Pipe Length Bottom of Screen t		(feet)	0.53
Riser Pipe Above W.T.		HER:	Screen Length (1			4.65
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTI	HER:	Total Length of Ca		(feet)	20.48

Screen Slot Size **

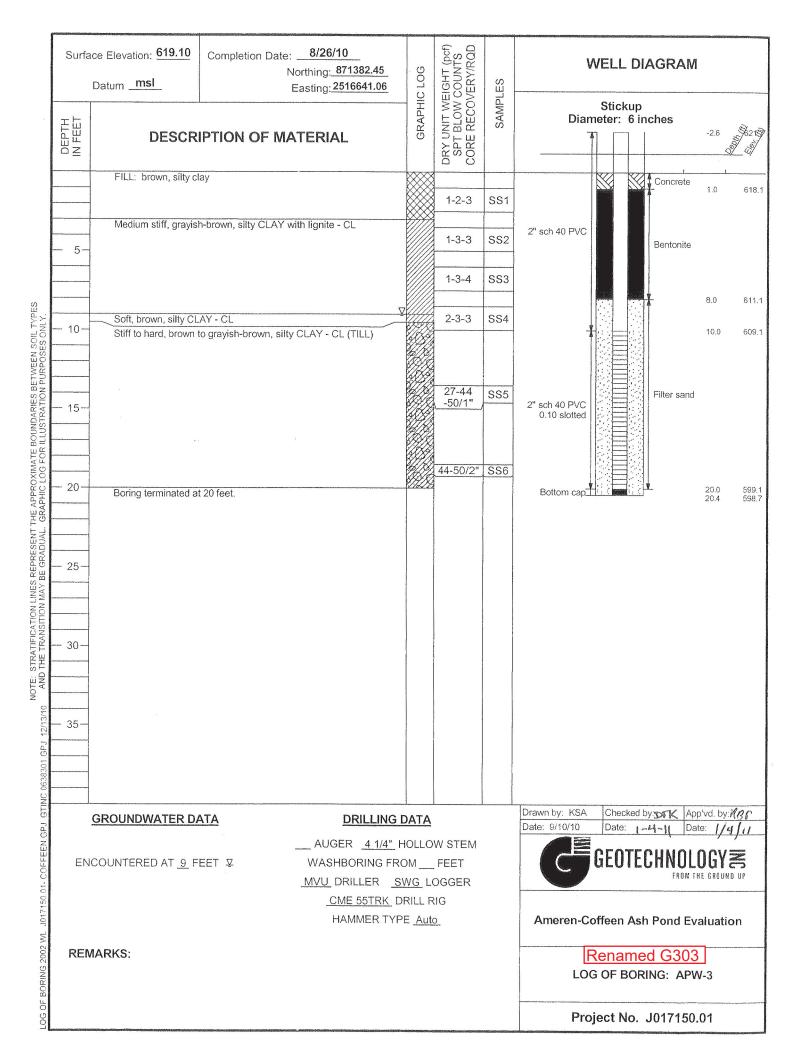
**Hand-Slotted Well Screens Are Unacceptable

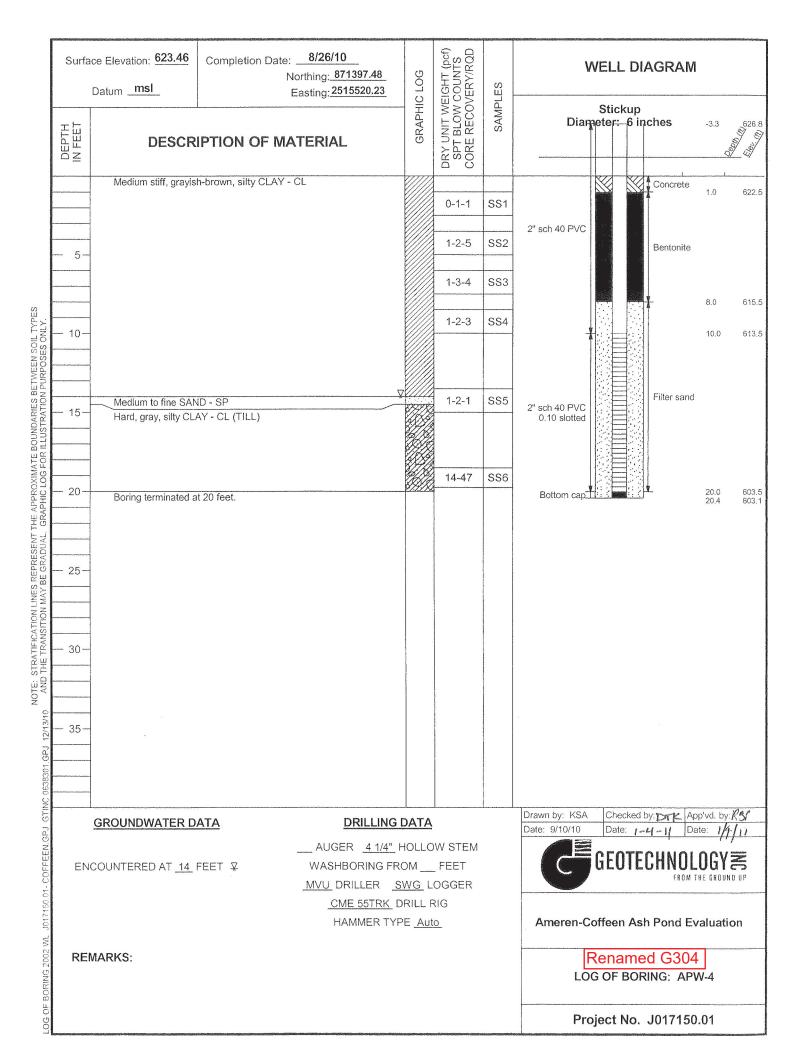
0.010

SS304

Well Completion Form (revised 02/06/02)

SS316





ILLINOIS DEPARTMENT OF PUBLIC HEALTH DIVISION OF ENVIRONMENTAL HEALTH 525 W. JEFFERSON ST. SPRINGFIELD, IL 62761

WATER WELL SEALING FORM

TYPE OR PRESS FIRMLY

RETURN ALL COPIES TO IDPH OR LOCAL HEALTH DEPARTMENT

This form shall be submitted to this Department or the local health department not more than 30 days after a water well, boring or monitoring well is sealed. Such wells are to be sealed not more than 30 days after they are abandoned in accordance with the sealing requirements in the Water Well Construction Code. THE LOCAL HEALTH DEPARTMENT OR REGIONAL PUBLIC HEALTH DEPARTMENT MUST BE NOTIFIED AT LEAST 48 HOURS PRIOR TO SEALING.

Ownership (Name of Controlling Party) AEG	Coffeen Power Stat	ion		(G104)	
Well Location 134 CIPS Trail		Co	ffeen	Montgome	ery
Address - Lot Number			City	County	
General Description Township 7 (N))(%) Range	3	_(K)(W)	Section _	1
SEQuarter of theNE	Quarter of the	NE	Qua	arter	
Year Drilled2010					
Drilling Permit Number (and date, if known)n/a					
Type of Well Bored Drilled✓					
Total Depth 20.0 ft. Diar	neter (inches)	2			
Formation clear of obstructionYes					
	110				
DETAILS OF PLUGGING					
Filled with Bentonite grout (cement or other materials)	from	0.5	to20.	<u>0ft.</u>	
(cement or other materials)					
Kind of plug_Random soil	from	0	to0.5	ft.	
Filled with	from		to	ft.	
Kind of plug	from		to	ft.	
Filled with	from		to	ft.	
Kind of plug	from		to	ft.	
CASING RECORD Upper 2 feet of casing remove	ved <u>√</u> Ye	s	No		
Date well was sealed Month October I	Day8	Year_	2010		
Licensed water well driller or other person appro-	ved by the Departi	ment pe	rforming wel	ll sealing.	
Rhonald W. Hasenyager, L.P.G.	196-000	246			
Name	Complete	License	Number		-
Hanson Professional Services Inc., 1525 S. 6th St.	Springfi	eld		IL 62703	
Address	— City			State/ZIP	

This state agency is requesting disclosure of information that is necessary to accomplish the statutory purpose as outlined under Public Act 85-0863. Disclosure of this information is mandatory. This form has been approved by the Forms Management Center.

IL 482-0631

Illinois Environ	mental Protection Agend	ey .		Well	Completio	n Report
Site #:	County:	Montgomery		W	ell #:	3305
Site Name: Coffeen Power Sta	ation			Во	orehole #:	G305
State Plane Coordinate: X 871,156	5.3 Y 2,515,199.4 (or) Latit	ude:		Longitude	e:	
Surveyed By: Gary C. Rogers		IL Regis	stration #:035-0	02957		
Drilling Contractor: Ramsey G	eotechnical Engineering LLC	Driller:	B. Williamson			
Consulting Firm: Hanson Profe	essional Services Inc.	Geologi	st: Rhonald W.	Hasenyager	, LPG #196-00	0246
Drilling Method: Hollow stem	auger	Drilling	Fluid (Type): <u>no</u>	ne		
Logged By: Suzanna L. Keim		Date Sta	arted:5/3/201	16 Date	Finished:	5/3/2016
Report Form Completed By: Su	zanna L. Keim	Date: _	5/19/2016			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft)
	_		625.88	· · ·	Top of Protectiv	e Casing
			625.55	3.01	Top of Riser Pip	
Type of Surface Seal: Concrete			622.54	0.00	Ground Surface	
Type of Annular Sealant: Benton	nite Chips		_621.54_	1.00	Top of Annular	Sealant
Installation Method: Gravit						
Setting Time: _ >12 hours		$\left \begin{array}{c} \overline{\Delta} \end{array} \right $			Static Water Le (After Completion	
Type of Bentonite Seal Grant	ular Pellet Slurry (choose one)					
Installation Method:n/a			n/a	n/a	Top of Seal	
Setting Time: <u>n/a</u>			611.04	_11.50_	Top of Sand Pa	ek
Type of Sand Pack: Quartz Sand	d					
Grain Size: 10-40 (sie	ve size)		609.10	_13.44_	Top of Screen	
Installation Method: <u>Gravit</u>	у					
Type of Backfill Material:n/a			604.27 604.09	<u>18.27</u> <u>18.45</u>	Bottom of Scree Bottom of Well	en
Installation Method:	(if applicable)		604.00	10 45	D-#	1 .
installation Method.			604.09 * Referenced to a	18.45 National Geodeti	Bottom of Borel	ioie
			CAS	SING MEAS	SUREMENTS	
		[Diameter of Boreho		(inches	8.0
	TRUCTION MATERIALS e type of material for each area)		ID of Riser Pipe		(inches	2.0
			Protective Casing L	ength	(feet	
Protective Casing	SS304 SS316 PTFE PVC OTF	HER: Steel	Riser Pipe Length	T. 1.C.		
Riser Pipe Above W.T.		HER:	Bottom of Screen to			
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTF		Screen Length (1s Total Length of Cas		(feet	

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

SS304

Well Completion Form (revised 02/06/02)

SS316

Illinois Environ	nmental Protection Agency	у		Well	Completio	n Report
Site #:	County:N	Montgomery		W	/ell #:	306
Site Name: Coffeen Power Sta	ation			В	orehole #:	G306
State Plane Coordinate: X 871,14	1.0 Y 2,516,120.4 (or) Latitu	de:		Longitud	e:	
Surveyed By: Gary C. Rogers		IL Regist	ration #: <u>035-00</u>	02957		
Drilling Contractor: Ramsey G	eotechnical Engineering LLC	Driller: _	B. Williamson			
Consulting Firm: Hanson Profe	essional Services Inc.	Geologist	: Rhonald W.	Hasenyager	, LPG #196-00	0246
Drilling Method: Hollow stem auger Dri			Fluid (Type): <u>nor</u>	ne		
Logged By: Suzanna L. Keim		Date Star	ted: <u>5/3/201</u>	.6 Date	e Finished:5	5/3/2016
Report Form Completed By:Su	zanna L. Keim	Date:	5/19/2016			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
			626.12	3.28	Top of Protectiv	e Casing
			625.72	2.88_	Top of Riser Pip	e
Type of Surface Seal: Concrete			622.84	0.00	Ground Surface	
			621.84	1.00	Top of Annular	Sealant
Type of Annular Sealant: Benton	A				-	
Installation Method: Gravit	<u>y</u>				Static Water Le	val
Setting Time:>12 hours		$\left \begin{array}{c} \overline{\Delta} \end{array}\right $			(After Completion)	
Type of Bentonite Seal Gran	ular Pellet Slurry					
Installation Method:n/a			n/a	n/a	Top of Seal	
Setting Time:n/a			611.24	11.60	Top of Sand Pag	ck
T. CC. ID. I					· · · · · · · ·	
Type of Sand Pack: Quartz San			609.77	_13.07_	Top of Screen	
Grain Size: 10-40 (sie Installation Method: Gravit						
	,		605.16	17.68	Bottom of Scree	n
Type of Backfill Material: <u>n/a</u>	(if applicable)		604.98	17.86	Bottom of Well	
Installation Method:			604.84 * Referenced to a	18.00	Bottom of Borel	nole
			· Referenced to a	National Geodet	ic Datum	
					SUREMENTS	
	STRUCTION MATERIALS		Diameter of Boreho		(inches)	
(Choose on	e type of material for each area)		D of Riser Pipe Protective Casing L		(inches)	
			Riser Pipe Length	viigui	(feet	1
Protective Casing	SS304 SS316 PTFE PVC OTHE	n a 1	Bottom of Screen to	End Cap	(feet	
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHE		Screen Length (1s		`	4.61
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHE	ER:	Total Length of Cas	sing	(feet	20.74

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

Illinois Enviror	nmental Protectio	n Agency			Well	Completion	n Report
Site #:		County: Mon	tgomery		W	/ell #:	307
Site Name: Coffeen Power Sta	ation Ash Pond 1				В	orehole #:	G307
State Plane Coordinate: X 2,515,553	.3 Y 871,398.6	(or) Latitude:			Longitud	e:	
Surveyed By:			IL Regis	stration #:			
Drilling Contractor: Bulldog D	rilling, Inc.		Driller:	J. Gates			
Consulting Firm: Hanson Profe	essional Services Inc.		Geologis	st: Rhonald W.	Hasenyage	r, LPG #196-00	0246
Drilling Method: Hollow Stem	Auger		Drilling	Fluid (Type): <u>no</u>	ne		
Logged By: Rhonald W. Hase	nyager		Date Sta	arted: 7/26/20	16 Dat	e Finished:7	/27/2016
Report Form Completed By: Rh	onald W. Hasenyager		Date: _	7/28/2016			
ANNULAR SPA	CE DETAILS			Elevations (MSL)*	Depths (BGS)	(0.01 ft	.)
				624.72	-2.64	Top of Protectiv	e Casing
				624.47	2.39	Top of Riser Pip	oe
Type of Surface Seal: Concrete			Y Town	622.08	0.00	Ground Surface	
Type of Annular Sealant: Benton	ita China			620.08	2.00	Top of Annular	Sealant
Installation Method: Gravity		_					
Setting Time: 18 hrs.		\[\sum_{\bullet} \]	z I I	623.84	-1.76	Static Water Lev	vel .
						(After Completion) 7/27/2016
Type of Bentonite Seal Grant	llar Pellet Slurry (choose one)		YTT.				
Installation Method:			$\overline{\mathbf{x}}$	n/a	<u>n/a</u>	Top of Seal	
Setting Time:		— X		610.10	11.98	Top of Sand Pag	ek
Type of Sand Pack: Quartz sand							
Grain Size: 10/20 (sie	ve size)			609.12	_12.96_	Top of Screen	
Installation Method: <u>Gravity</u>	,	-		604.20	17.00	D	
Type of Backfill Material:none			\exists	604.28 603.86	<u>17.80</u> <u>18.22</u>	Bottom of Scree Bottom of Well	n
Installation Method: n/a	(if applicable)			603.86	18.22	Bottom of Borel	nole
				* Referenced to a			
				CAS	SING MEA	SUREMENTS	
WELL CONG	TRUCTION MATERIA	10		Diameter of Boreho	ole	(inches	8.0
	e type of material for each area)	ഥാ	Г	ID of Riser Pipe		(inches	
				Protective Casing I	ength	(feet	
Protective Casing	SS304 SS316 PTFE	PVC OTHER: S		Riser Pipe Length Pottom of Serson to	o End C	(feet)	0.40
Riser Pipe Above W.T.		PVC OTHER:		Bottom of Screen to Screen Length (1s	•		4.04
Riser Pipe Below W.T.		PVC OTHER:		Total Length of Car		t) (feet) (feet)	20.61

Well Completion Form (revised 02/06/02)

Screen Slot Size **

 $\hbox{**Hand-Slotted Well Screens Are Unacceptable}\\$

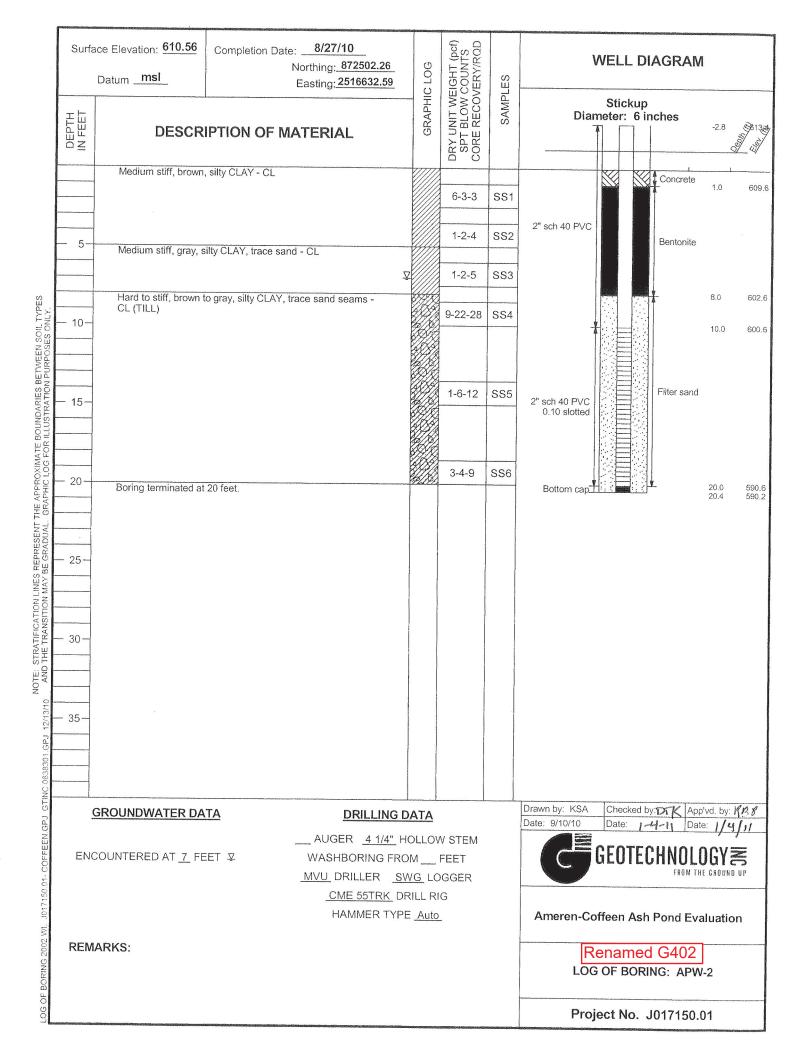
0.010

Illinois Environ	mental Protection Agen	cy		Well	Completion	Report		
Site #:	County: _	Montgomery	7	W	Vell #:G	101		
Site Name: Natural Resource	Technology, Inc. Coffeen Energy (Center		В	orehole #:	G401		
State Plane Coordinate: X 872,510	0.6 Y 2,515,614.8 (or) Lati	itude:		Longitude	e:			
Surveyed By: Gary C. Rogers		IL Regi	stration #:035-	002957				
Drilling Contractor: Ramsey G	eotechnical Engineering, LLC	Driller:	iller: D. Crump					
Consulting Firm: Hanson Professional Services Inc. Geo			ist: Rhonald W	. Hasenyager	<u>, LPG #196-000</u>	246		
Drilling Method: Hollow stem auger Drill			g Fluid (Type): <u>n</u>	one				
Logged By: Rhonald W. Hase	nyager	Date St	arted: 9/14/2	015 Date	e Finished: 9/1	4/2015		
Report Form Completed By:Su	zanna L. Keim	Date: _	10/7/2015					
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)			
	г		625.84	2.81	Top of Protective	Casing		
			625.57	2.54	Top of Riser Pipe			
Type of Surface Seal: Concrete			623.03	0.00	Ground Surface			
Type of Annular Sealant: Benton	nite Chins		621.33	1.70	Top of Annular S	ealant		
Installation Method: Gravit	-							
Setting Time: _ >24 hours					Static Water Leve (After Completion)	el		
Type of Bentonite Seal Gran	ılar Pellet Slurry —				(Tittel Completion)			
Installation Method: Gravit	(choose one)		n/a	n/a	Top of Seal			
Setting Time: 25 minutes								
<u></u>			610.12	12.91	Top of Sand Pack			
Type of Sand Pack: Quartz Sand	1		600.67	1426				
Grain Size: 10-20 (sie	ve size)		608.67	14.36	Top of Screen			
Installation Method: Gravit	y		604.24	10.70	D 00			
Type of Backfill Material: <u>n/a</u>			604.24 603.74	18.79 19.29	Bottom of Screen Bottom of Well			
	(if applicable)		(02.72	10.20				
Installation Method:			603.73 * Referenced to	19.30 a National Geodeti	Bottom of Boreho	ole		
			C.A.	CDICAGA				
			Diameter of Borel		SUREMENTS	8.0		
	TRUCTION MATERIALS e type of material for each area)		ID of Riser Pipe		(inches)	2.0		
(Choose on	type of material for each area)		Protective Casing		(feet)	5.0		
			Riser Pipe Length		(feet)	16.70		
Protective Casing		HER: Steel	Bottom of Screen	to End Cap	(feet)	0.50		
Riser Pipe Above W.T.		THER:	Screen Length (1st slot to last slot	t) (feet)	4.63		
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OT	THER:	Total Length of C	asing	(feet)	21.83		

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable



Illinois Environ	nmental Protection	Agency				Well	Completio	n Report
Site #:	Co	ounty: <u>Mon</u>	tgomery	,		W	/ell #:	6403
Site Name: Natural Resource	Technology, Inc. Coffeen E	Energy Center	r			В	orehole #:	G403
State Plane Coordinate: X 873,56	1.3 Y 2,514,616.6 (or	r) Latitude:				Longitud	e:	
Surveyed By: Gary C. Rogers			IL Regi	stration	#:035-0	02957		
Drilling Contractor: Ramsey Geotechnical Engineering, LLC Dri				D. C	Crump			
Consulting Firm: <u>Hanson Professional Services Inc.</u> Geo				ist: R	honald W.	Hasenyager	<u>, LPG #196-00</u>	0246
Drilling Method: Hollow stem auger Drilli				g Fluid (Туре):no	one		
Logged By: Kristen L. Theest	feld		Date Sta	arted: _	9/11/20)15 Date	e Finished: 9	/11/2015
Report Form Completed By: Su	ızanna L. Keim		Date: _	1	0/7/2015			
ANNULAR SPA	CE DETAILS				evations (MSL)*	Depths (BGS)	(0.01 ft	.)
				_	626.72	2.91	Top of Protectiv	e Casing
			\exists	_	626.47	2.66	Top of Riser Pip	pe
Type of Surface Seal: Concrete		-		>	623.81	0.00	Ground Surface	
Type of Annular Sealant: Bento	onita Chine			/ _	621.81	2.00	Top of Annular	Sealant
Installation Method: Gravit		- 9						
Setting Time:>24 hours	.)	_ _	<u>z</u>	_			Static Water Le	
Type of Bentonite Seal Grar	nular Pellet Slurry						(After Completion))
Installation Method: n/a	(choose one)				n/a	n/a	Top of Seal	
		-		-	11/a	II/a	Top of Seaf	
Setting Time: <u>n/a</u>		- 🔀		_	612.64	11.17	Top of Sand Pa	ck
Type of Sand Pack: Quartz San	d	_						
Grain Size:10-20 (si	eve size)			_	610.70_	13.11	Top of Screen	
Installation Method: Gravit	ty	-			606.03	17.78	Bottom of Scree	an.
Type of Backfill Material:n/a	(if applicable)		$\exists \mid$	_	605.66	18.15	Bottom of Well	311
Installation Method:	· 11				605.66	18.15	Bottom of Borel	hole
				*	Referenced to a	National Geodet	ic Datum	
			·		CAS	SING MEAS	SUREMENTS	
WELL CONS	STRUCTION MATERIALS	S		Diame	ter of Boreho	ole	(inches	
	ne type of material for each area)	,			Riser Pipe		(inches	
					tive Casing I	_ength	(feet	
Protective Casing	SS304 SS316 PTFE P	VC OTHER: S	teel		Pipe Length n of Screen to	o End Can	(feet	0.25
Riser Pipe Above W.T.	SS304 SS316 PTFE P	VC OTHER:				st slot to last slo	`	4.67
Riser Pipe Below W.T.	SS304 SS316 PTFE P	VC OTHER:			ength of Ca		(feet	20.01

SS304

Well Completion Form (revised 02/06/02)

SS316

Total Length of Casing

**Hand-Slotted Well Screens Are Unacceptable

Screen Slot Size **

Illinois Environmental Protection Agency			Well	Completion	Report
Site #: County: _Mon	ntgomery		W	vell #:G4	.04
Site Name: Coffeen Energy Center				Former	y MW22S SB22
State					
Plane Coordinate: X 2,516,397.9 Y 873,999.8 (or) Latitude:				e:	
Surveyed By: Darren E. Forgy	IL Registration	n#: <u>035-00</u>	03637		
Drilling Contractor: Reynolds Drilling Corp.	Driller: A.	Rachford			
Consulting Firm: Hanson Professional Services Inc.	Geologist:,	LPG#			
Drilling Method: Hollow stem auger	Drilling Fluid	(Type): <u>no</u>	ne		
Logged By: Rhonald W. Hasenyager	Date Started:	5/1/200	07 Date	e Finished:5/	1/2007
Report Form Completed By: Rhonald W. Hasenyager	Date:	5/2/2007			
ANNULAR SPACE DETAILS		levations (MSL)*	Depths (BGS)	(0.01 ft.)	
		616.02	2.92	Top of Protective	Casing
	<u> </u>	615.77	-2.67	Top of Riser Pipe	
Type of Surface Seal: Concrete	-	613.10	0.00	Ground Surface	
Type of Annular Sealant: Bentonite chips	-	613.10	0.00	Top of Annular S	ealant
Installation Method:gravity					
Setting Time:>12 hours \	<u>z</u>	611.03	2.07	Static Water Leve	
Type of Bentonite Seal Granular Pellet Slurry				(After Completion)	5/10/2007
Installation Method:	-	n/a	n/a	Top of Seal	
Setting Time:	-	608.05	5.05	Top of Sand Pack	
Type of Sand Pack: Quartz sand					
Grain Size: 10/20 (sieve size)	- ∃	606.68	6.42	Top of Screen	
Installation Method: gravity	≣				
Type of Backfill Material: Formation sand	- □	601.93 601.48	<u>11.17</u> 11.62	Bottom of Screen Bottom of Well	
(if applicable)		001.48	11.02	Bottom of Wen	
Installation Method: slough		601.10 Referenced to a	12.00 National Geodeti	Bottom of Boreho	le
		CAS	ING MEAS	SUREMENTS	
	Diame	eter of Boreho		(inches)	8.0
WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)	ID of	Riser Pipe		(inches)	2.0
	Protec	ctive Casing L	ength	(feet)	5.0
Protective Cosing SCOM SCOM PEER IN/O CONTROL		Pipe Length		()	9.09
Protective Casing SS304 SS316 PTFE PVC OTHER: S Riser Pipe Above W.T. SS304 SS316 PTFE PVC OTHER:	Dotto	m of Screen to		(feet)	0.45
Riser Pipe Below W.T. SS304 SS316 PTFE PVC OTHER:		n Length (1s Length of Cas		t) (feet)	4.75 14 29

Well Completion Form (revised 02/06/02)

SS316

PTFE PVC OTHER:

Total Length of Casing

**Hand-Slotted Well Screens Are Unacceptable

0.010

Screen Slot Size **

Illinois Environmental Protection Agency	Well Completion Repor
Site #: County:Mon	ntgomery Well #: G405
Site Name: Coffeen Energy Center	Borehole #: SB21
State Plane Coordinate: X 2,515,335.7 Y 873,996.8 (or) Latitude:	Longitude:
Surveyed By: Darren E. Forgy	IL Registration #:035-003637
Drilling Contractor: Reynolds Drilling Corp.	Driller: A. Rachford
Consulting Firm: Hanson Professional Services Inc.	Geologist:, LPG#
Drilling Method: Hollow stem auger	Drilling Fluid (Type):none
Logged By: Rhonald W. Hasenyager	Date Started:5/1/2007 Date Finished:5/1/2007
Report Form Completed By: Rhonald W. Hasenyager	Date: 5/2/2007
ANNULAR SPACE DETAILS	Elevations Depths (0.01 ft.) (MSL)* (BGS)
	624.043.14 Top of Protective Casing
Type of Surface Seal: Concrete	
Type of Annular Sealant: Bentonite chips	620.90 0.00 Top of Annular Sealant
Installation Method: gravity	
	\triangle 619.67 1.23 Static Water Level
	(After Completion) 5/10/2007
Type of Bentonite Seal Granular Pellet Slurry (choose one)	
Installation Method:	n/aTop of Seal
Setting Time:	613.19 7.71 Top of Sand Pack
Type of Sand Pack: Quartz sand	
Grain Size: 10/20 (sieve size)	611.89 9.01 Top of Screen
Installation Method:gravity	(07.14 13.77 P. 11.00
Type of Backfill Material: (if applicable)	607.14 13.76 Bottom of Screen 606.69 14.21 Bottom of Well
(if applicable) Installation Method:	606.69 14.21 Bottom of Borehole
	* Referenced to a National Geodetic Datum
	CASING MEASUREMENTS
WELL CONCTRUCTION MATERIALS	Diameter of Borehole (inches) 8.0
WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)	ID of Riser Pipe (inches) 2.0
	Protective Casing Length (feet) 5.0
Protective Casing SS304 SS316 PTFE PVC OTHER: S	Riser Pipe Length (feet) 11.89
Riser Pipe Above W.T. SS304 SS316 PTFE PVC OTHER:	Bottom of Screen to End Cap (feet) 0.45 Screen Length (1st slot to last slot) (feet) 4.75
Riser Pipe Below W.T. SS304 SS316 PTFE PVC OTHER:	Total Length of Casing (feet) 17.09

Total Length of Casing

**Hand-Slotted Well Screens Are Unacceptable

0.010

Screen Slot Size **

SS316

PTFE PVC OTHER:

SS304

Well Completion Form (revised 02/06/02)

Illinois Environmental Protection Agency					Wel	l Completi	on Report
Site #:	Co	ounty: <u>Mor</u>	itgomery		v	Vell #:	G406
Site Name: Coffeen Power Sta					E	Borehole #:	G406
State Plane Coordinate: X 2,514,702	2.4 Y 872,521.3 (or	r) Latitude:	39°	<u>3'</u> <u>37.1</u>	14" Longitud	le:89°	23' 54.628"
Surveyed By: Gary C. Rogers			IL Regi	stration #: 03	5-002957		
Drilling Contractor: Bulldog Drilling, Inc. Dr				J. Dittmaier			
Consulting Firm: Hanson Professional Services Inc. Geol				ist: Rhonald	W. Hasenyage	r, LPG #196-0	00246
Drilling Method: Hollow stem	auger		Drilling	; Fluid (Type): _	none		
Logged By: Kristen L. Theesfe	eld		Date St	arted: 8/19	9/2016 Dat	te Finished:	8/19/2016
Report Form Completed By: Su	zanna L. Keim		Date: _	8/24/201	6		
ANNULAR SPA	CE DETAILS			Elevation (MSL)*		(0.01	ft.)
				_625.70	, ,	Top of Protect	ive Casing
		T		625.36		-	
Type of Surface Seal: Concrete				621.86	0.00	Ground Surfa	ne.
				619.86			
Type of Annular Sealant: High-s	solids bentonite	-		017.00		Top of Funda	ii Scarant
Installation Method: Tremis		-					
Setting Time: >24 hours		_ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	$\mathbb{Z} \mid \cdot \mid$			Static Water I (After Completic	
Type of Bentonite Seal Grant							
Installation Method: Gravit	(choose one)			610.74	11.12	Top of Seal	
Setting Time: 30 minutes		_ \		609.65	12.21	Top of Sand P	'ack
						Top of Sunu i	
Type of Sand Pack: Quartz Sand		-		608.30	13.56	Top of Screen	
Grain Size: 10-20 (sie	,					•	
Installation Method: <u>Gravit</u>	У	-		603.49	18.37	Bottom of Scr	een
Type of Backfill Material:n/a	(if applicable)	_ _		603.11	18.75	Bottom of We	11
Installation Method:				603.11		Bottom of Bor	ehole
				* Reference	d to a National Geode	tic Datum	
				(CASING MEA	SUREMENTS	;
WELL CONS	STRUCTION MATERIALS	S		Diameter of Bo		(inch	
	e type of material for each area)	_		ID of Riser Pip		(inch	
				Protective Casi			et) 5.0 et) 17.06
Protective Casing	SS304 SS316 PTFE P	VC OTHER:	Steel	Riser Pipe Leng Bottom of Scre		•	et) 17.06 et) 0.38
Riser Pipe Above W.T.	SS304 SS316 PTFE P	VC OTHER:			(1st slot to last slo		et) 4.81
Riser Pipe Below W.T.	SS304 SS316 PTFE P	VC OTHER:		Total Length of			eet) 22.25

Well Completion Form (revised 02/06/02)

SS316

PTFE PVC OTHER:

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

Illinois Environmental Protection A			Well	l Completio	n Report	
Site #: County: Montgomery				W	Vell #:	6407
Site Name: Coffeen Power Station - Ash Pond 2				В	orehole #:	G407
State Plane Coordinate: X 2,513,705.9 Y 872,973.4 (or)	Latitude:	39°	3' 41.665"	Longitud	le: <u>-89°</u> 2	<u>7.213"</u>
Surveyed By: Gary C. Rogers	II	L Registra	ation #:035-0	02957		
Drilling Contractor: Bulldog Drilling, Inc.	Г	Oriller: _	J. Dittmaier			
Consulting Firm: Hanson Professional Services Inc.		Geologist:	Rhonald W.	Hasenyage	r, LPG #196-00	0246
Drilling Method: Hollow stem auger	Orilling Fl	uid (Type): <u>no</u>	ne			
Logged By: Kristen L. Theesfeld	Г	Date Start	ed: 8/16/20	16 Dat	e Finished: 8	/16/2016
Report Form Completed By: Suzanna L. Keim	Γ	Date:	8/24/2016			
ANNULAR SPACE DETAILS			Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
			621.70	3.35	Top of Protectiv	e Casing
			621.32	-2.97	Top of Riser Pip	
Type of Surface Seal: Concrete			618.35	0.00	Ground Surface	
_			616.35	2.00		
Type of Annular Sealant: High-solids bentonite					Top of Filmaur	Soulant
Installation Method: <u>Tremie</u>						
Setting Time: _>24 hours					Static Water Legarda (After Completion)	
Type of Bentonite Seal Granular Pellet Slurry						
Installation Method: Gravity	× ×	KX	607.50	_10.85_	Top of Seal	
Setting Time:15 minutes			605.50	12.85	Top of Sand Pag	ck
Type of Sand Pack: Quartz Sand						
Grain Size: 10-20 (sieve size)			604.57	13.78	Top of Screen	
Installation Method: <u>Gravity</u>						
Type of Backfill Material: Quartz Sand			599.74 599.31	<u>18.61</u> 19.04	Bottom of Scree Bottom of Well	n
(if applicable)		_				
Installation Method: <u>Gravity</u>			598.35 * Referenced to a	20.00 National Geodet	Bottom of Borel tic Datum	nole
			CAS		CLIDEMENITO	
		Гр	iameter of Boreho		SUREMENTS (inches	8.0
WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)			of Riser Pipe		(inches	2.0
(-11111 - 1111 -			rotective Casing L	ength	(feet	
D +		$\overline{}$	iser Pipe Length		(feet)	
Protective Casing SS304 SS316 PTFE PVC Riser Pipe Above W.T. SS304 SS316 PTFE PVC		P	ottom of Screen to		(feet	
	OTHER:		creen Length (1st) otal Length of Cas		ot) (feet) (feet)	22.01

SS304

Well Completion Form (revised 02/06/02)

SS316

Total Length of Casing

**Hand-Slotted Well Screens Are Unacceptable

Screen Slot Size **

Illinois Environmental Protection Agency			Well Completion Report			
Site #:	County: <u>Montg</u>	gomery	V	Vell #: G4	10	
Site Name:Coffeen Power Station - Ash Pond	12		B	orehole #:	G410	
State Plan€oordinate: X 2,513,206.3 Y 872,968	3.5_ (or) Latitude:	39° 3' 4	11.658" Longitu	de: <u>-89°</u> 24	<u>l'</u> 13.546"	
Surveyed By: <u>Matthew H. Schrader</u>	II	L Registration #:	035-003487			
Drilling Contractor: Bulldog Drilling, Inc.	D	Oriller: <u>C. Dut</u> t	con			
Consulting Firm: <u>Hanson Professional Service</u>	es Inc. G	Geologist: <u>Rho</u>	nald W. Hasenya	ger, LPG #196-0	000246	
Drilling Method: <u>Hollow stem auger</u>	D	Orilling Fluid (Ty	pe): <u>none</u>			
Logged By: Rhonald W. Hasenyager	D	Date Started:2	2/23/2018 Dat	te Finished: 2/2	23/2018	
Report Form Completed By: Suzanna L. Keim	В	Date:2/26	/2018			
ANNULAR SPACE DETAILS		Eleva	tions Depths	(0.01 ft.))	
		(MS	, ,	Top of Protectiv	ro Cooina	
			.18 -2.97	rop of Protectiv	e Casing	
		619	.79 -2.58	Top of Riser Pip	e	
Type of Surface Seal: <u>Concrete</u>		617	0.00	Ground Surface		
Type of Annular Sealant:			n/a	Top of Annular	Sealant	
Installation Method:	7					
Setting Time:		612	39 4.82	Static Water Lev	zel	
Setting Time.				(After Completion)		
Type of Bentonite Seal Granular Pellet S	Slurry					
Installation Method: <u>Gravity</u>	<u>~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ </u>	616	.71 0.50	Top of Seal		
Setting Time: >24 hours			.81 7.40	Top of Sand Pac	k	
	V V		7110	Top of build I de		
Type of Sand Pack: Filter Sand		608	3.32 8.89	Top of Screen		
Grain Size: 20/40 (sieve size)				1		
Installation Method: <u>Gravity</u>		_603	.53_ 13.68_	Bottom of Scree	n	
Type of Backfill Material: <u>n/a</u> (if applicable)		603	14.09	Bottom of Well		
Installation Method: n/a		603	.12 14.09	Bottom of Borel	ıole	
		* Refe	renced to a National Geod	detic Datum		
			CASING MEA	SUREMENTS		
WELL CONSTRUCTION MATE	EDIALC	Diameter	of Borehole	(inches)	8.0	
(Choose one type of material for each a		ID of Rise	•	(inches)	2.0	
			Casing Length	(feet)	5.0	
Protective Casing SS304 SS316 PT	TFE PVC OTHER: (Steel	Riser Pipe	Length Screen to End Cap	(feet)	11.47 0.41	
	TFE PVC OTHER:	Dottom of	ngth (1st slot to last s		4.79	
	TFE PVC OTHER:		gth of Casing	(feet)	16.67	

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

(inches)

SS304

Well Completion Form (revised 02/06/02)

SS316

Illinois Environmental Protection Agen	ncy Well Completion Report
Site #: County:Mon	ntgomery Well #: G411
Site Name: Coffeen Power Station - Ash Pond 2	Borehole #:G411
State Plan€oordinate: X <u>2,513,122.4</u> Y <u>873,844.8</u> (or) Latitude:	39°3'50.326" Longitude:89°24'14.517"
Surveyed By: <u>Matthew H. Schrader</u>	IL Registration #:035-003487
Drilling Contractor: Bulldog Drilling, Inc.	Driller: <u>C. Dutton</u>
Consulting Firm: Hanson Professional Services Inc.	Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: <u>Hollow stem auger</u>	Drilling Fluid (Type):none
Logged By:Rhonald W. Hasenyager	Date Started: 2/22/2018 Date Finished: 2/22/2018
Report Form Completed By: Suzanna L. Keim	Date:2/26/2018
ANNULAR SPACE DETAILS	Elevations Depths (0.01 ft.) (MSL)* (BGS)
Type of Surface Seal: Concrete	620.49 0.00 Ground Surface
Type of Annular Sealant:	rop or Ammanar Seamant
Installation Method:	
Setting Time: \(\sum_{\bullet}	<u>617.84</u> <u>2.65</u> Static Water Level (After Completion) 3/1/2018
Type of Bentonite Seal Granular Pellet Slurry (choose one)	
Installation Method: Gravity	
Setting Time: >24 hours	610.42 10.07 Top of Sand Pack
	Top of sund rack
Type of Sand Pack: Filter Sand	609.28 11.21 Top of Screen
Grain Size: 20/40 (sieve size)	
Installation Method: <u>Gravity</u>	
Type of Backfill Material:(if applicable)	604.02 16.47 Bottom of Well
Installation Method: <u>n/a</u>	604.02 16.47 Bottom of Borehole
	* Referenced to a National Geodetic Datum
	CASING MEASUREMENTS
WELL CONSTRUCTION MATERIALS	Diameter of Borehole (inches) 8.0
(Choose one type of material for each area)	ID of Riser Pipe (inches) 2.0
	Protective Casing Length (feet) 5.0 Riser Pipe Length (feet) 13.97
Protective Casing SS304 SS316 PTFE PVC OTHER: St	
Riser Pipe Above W.T. SS304 SS316 PTFE PVC OTHER:	Screen Length (1st slot to last slot) (feet) 4.86
Riser Pipe Below W.T. SS304 SS316 PTFE PVC OTHER:	Total Length of Casing (feet) 19.23

PTFE PVC OTHER:

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

(inches)

SS304

Well Completion Form (revised 02/06/02)

SS316

Illinois Environ	nmental Protection Age	ncy		Well Completio	n Report
Site #:	County:]	Montgomery		Well #:M	W1D
Site Name: AEG Coffeen Por	wer Station CCB Management Fa	cility		Borehole #:	SB-01
State Plane Coordinate: X 874,972	2.6 Y 2,513,478.0 (or) Latitu	ıde:°	'" I	Longitude:°	
Surveyed By:Darren E. Forg	y	IL Regis	stration #: <u>035-0036</u>	637	
Drilling Contractor: Testing So	ervice Corporation	Driller:	B. Williamson		
Consulting Firm: Hanson Prof	Sessional Services Inc.	Geologi	st: <u>Rhonald W Has</u>	senyager, LPG #196-0	000246
Drilling Method: Hollow stem	auger	Drilling	Fluid (Type): Potab	ole water	
Logged By: <u>Testing Services</u>	Corp.	Date Sta	arted: 5/3/2006	Date Finished:	5/3/2006
Report Form Completed By: R	nonald W Hasenyager	Date:	6/7/2006	_	
ANNULAR SPA	CE DETAILS			epths (0.01 ft	<u></u>)
			` / /	-1.89 Top of Protecti	ve Casing
			609.01	-1.61 Top of Riser Pi	pe
Type of Surface Seal: Concrete				0.00 Ground Surface	e
Type of Annular Sealant: Bento	nite grout		604.80	2.60 Top of Annula	Sealant
Installation Method: Tremio	7				
Setting Time: +24 hr.			571.12 _ 3	36.28 Static Water Le	evel
				(After Completio	n) 6/1/2006
Type of Bentonite Seal Grant	Pellet Slurry (choose one)	1 1			
Installation Method: <u>Gravit</u>	<u>y</u>		578.90	28.50 Top of Seal	
Setting Time: 21 min.			575.82	31.58 Top of Sand Pa	ck
Type of Sand Pack: Quartz sand	1				
Grain Size: #5 (sie	ve size)		574.11 3	Top of Screen	
Installation Method: Gravit	y		560.25	38.05 Bottom of Scre	
Type of Backfill Material:n/a	(if applicable)			Bottom of Wel	
Installation Method:n/a	(п аррпсавіе)		567.40	40.00 Bottom of Bore	ehole
			* Referenced to a Nati	ional Geodetic Datum	
		_	CASINO	G MEASUREMENTS	
WELL COMO	TDIICTION MATERIAI C		Diameter of Borehole	(inches	7.3
	TRUCTION MATERIALS e type of material for each area)		ID of Riser Pipe	(inches	2.0
		-	Protective Casing Leng	gth (feet	
Protective Cosine	SS304 SS316 PTFE PVC OTH	TR C(1)	Riser Pipe Length	(feet	
Protective Casing Riser Pipe Above W.T.	\$\$304 \$\$316 PTFE PVC OTH \$\$304 \$\$316 PTFE PVC OTH	ED	Bottom of Screen to E	-	
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTH		Screen Length (1st slo Total Length of Casins		40.00

SS316 PTFE PVC OTHER:

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

(inches)

SS304

Well Completion Form (revised 02/06/02)

ILLINOIS DEPARTMENT OF PUBLIC HEALTH DIVISION OF ENVIRONMENTAL HEALTH 525 W. JEFFERSON ST. SPRINGFIELD, IL 62761

WATER WELL SEALING FORM

TYPE OR PRESS FIRMLY

RETURN ALL COPIES TO IDPH OR LOCAL HEALTH DEPARTMENT

This form shall be submitted to this Department or the local health department not more than 30 days after a water well, boring or monitoring well is sealed. Such wells are to be sealed not more than 30 days after they are abandoned in accordance with the sealing requirements in the Water Well Construction Code. THE LOCAL HEALTH DEPARTMENT OR REGIONAL PUBLIC HEALTH DEPARTMENT MUST BE NOTIFIED AT LEAST 48 HOURS PRIOR TO SEALING.

Ownership (Name of Controlling Party) AEG C	offeen Power Stat	ion			(G104)	
Well Location 134 CIPS Trail		Co	offeen		Montgome	ry
Address - Lot Number			City		County	
General Description Township 7 (N)	Range_	3	_ (K) (W)		Section _	1
SEQuarter of theNE	Quarter of the	NI	Ξ ()uarter		
Year Drilled 2010						
Drilling Permit Number (and date, if known) n/a						
Type of Well Bored Drilled_ ✓						
Total Depth 20.0 ft. Diamo	eter (inches)	2				
Formation clear of obstructionYes						
DETAILS OF PLUGGING						
		^ -	_			
Filled with Bentonite grout (cement or other materials)	from	0.5	to²	20.0 ft.		
Kind of plug Random soil	from		to).5ft.		
Filled with	from		to	ft.		
Kind of plug	from		to	ft.		
Filled with	from		to	ft.		
Kind of plug	from		to	ft.		
CASING RECORD Upper 2 feet of casing remove	ed <u>√</u> Ye	es	No			
Date well was sealed Month_October Da	ay8	Year_	2010	_•		
Licensed water well driller or other person approve	ed by the Depart	ment pe	erforming v	vell sealing		
Rhonald W. Hasenyager, L.P.G.	196-000	246				
Name	Complete	e Licens	se Number			
Hanson Professional Services Inc., 1525 S. 6th St.	Springfi	eld		IL 62	2703	
Address	City			State	e/ZIP	

This state agency is requesting disclosure of information that is necessary to accomplish the statutory purpose as outlined under Public Act 85-0863. Disclosure of this information is mandatory. This form has been approved by the Forms Management Center.

IL 482-0631

Illinois Environ	nmental Protecti	on Agency	y		Well	Completion	ı Report
Site #:		County: <u>Mor</u>	itgomer	у	W	ell #:M\	W2D
Site Name: AEG Coffeen Pov	wer Station CCB Manag	gement Facilit	у		Во	orehole #:S	B-02b
State Plane Coordinate: X 876,414	H.O Y 2,513,209.7 (or) Latitude:		0 !	Longitude	e:°	
Surveyed By: <u>Darren E. Forg</u>	y		IL Reg	istration #: <u>035</u> -	003637		
Drilling Contractor: Testing Se	ervice Corporation		Driller	: _B. Williamson	n		
Consulting Firm: Hanson Prof	Sessional Services Inc.		Geolog	gist: <u>Rhonald W</u>	/ Hasenyage	r, LPG #196-00	00246
Drilling Method: Hollow stem	n auger		Drillin	g Fluid (Type): <u>F</u>	Potable water	r	
Logged By: <u>Testing Services</u>	Corp.		Date S	tarted: 5/5/20	006 Date	Finished: 5	/5/2006
Report Form Completed By: R	nonald W Hasenyager		Date:	6/7/2006			
ANNULAR SPA	CE DETAILS			Elevations (MSL)*	Depths (BGS)	(0.01 ft.))
				627.52	3.42	Top of Protectiv	e Casing
				627.07	2.97	Top of Riser Pip	e
Type of Surface Seal: Concrete				624.10	0.00	Ground Surface	
Town of America Contact. Donto.				621.00	3.10	Top of Annular	Sealant
Type of Annular Sealant: Benton							
Installation Method:Tremie		_ -	7	616.74	7.36	Static Water Lev	, a1
Setting Time:+24 hr.		_ \[\frac{1}{2}	_	_010.74		(After Completion)	
Type of Bentonite Seal Grant	Pellet Slurry (choose one)			•			
Installation Method: Gravit	` '			606.00	_18.10	Top of Seal	
Setting Time: 20 min.				603.92	20.18	Top of Sand Pac	k
						rop or sumor we	
Type of Sand Pack: Quartz sand		_		602.07	_22.03_	Top of Screen	
Grain Size: #5 (sie						1	
Installation Method: <u>Gravit</u>	у			_597.27_	_26.83_	Bottom of Scree	n
Type of Backfill Material: <u>n/a</u>	(if applicable)	_ [596.88	27.22	Bottom of Well	
Installation Method: Re-dri				596.88	27.22	Bottom of Borel	iole
				* Referenced to	a National Geode	tic Datum	
				CA	SING MEAS	SUREMENTS	
WELL CONG	TRUCTION MATERIA	1.0		Diameter of Bore	hole	(inches)	8.0
	TRUCTION MATERIA e type of material for each area)	IL)		ID of Riser Pipe		(inches)	2.0
				Protective Casing	_	(feet)	5.0
Protective Casing	SS304 SS316 PTFE	PVC OTHER: (Steel	Riser Pipe Length		(feet)	25.00
Riser Pipe Above W.T.	_	PVC OTHER:		Bottom of Screen	-		0.39 4.80
Riser Pipe Below W.T.		PVC OTHER:		Screen Length (Total Length of C		t) (feet) (feet)	30.19

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

ILLINOIS DEPARTMENT OF PUBLIC HEALTH DIVISION OF ENVIRONMENTAL HEALTH 525 W. JEFFERSON ST. SPRINGFIELD, IL 62761

WATER WELL SEALING FORM

TYPE OR PRESS FIRMLY

RETURN ALL COPIES TO IDPH OR LOCAL HEALTH DEPARTMENT

This form shall be submitted to this Department or the local health department not more than 30 days after a water well, boring or monitoring well is sealed. Such wells are to be sealed not more than 30 days after they are abandoned in accordance with the sealing requirements in the Water Well Construction Code. THE LOCAL HEALTH DEPARTMENT OR REGIONAL PUBLIC HEALTH DEPARTMENT MUST BE NOTIFIED AT LEAST 48 HOURS PRIOR TO SEALING.

Ownership (Name of Controlling Party) AEG C	offeen Power Stat	ion			(G104)	
Well Location 134 CIPS Trail		Co	offeen		Montgome	ry
Address - Lot Number			City		County	
General Description Township 7 (N)	Range_	3	_ (K) (W)		Section _	1
SEQuarter of theNE	Quarter of the	NI	Ξ ()uarter		
Year Drilled 2010						
Drilling Permit Number (and date, if known) n/a						
Type of Well Bored Drilled_ ✓						
Total Depth 20.0 ft. Diamo	eter (inches)	2				
Formation clear of obstructionYes						
DETAILS OF PLUGGING						
		^ -	_			
Filled with Bentonite grout (cement or other materials)	from	0.5	to²	20.0 ft.		
Kind of plug Random soil	from		to).5ft.		
Filled with	from		to	ft.		
Kind of plug	from		to	ft.		
Filled with	from		to	ft.		
Kind of plug	from		to	ft.		
CASING RECORD Upper 2 feet of casing remove	ed <u>√</u> Ye	es	No			
Date well was sealed Month_October Da	ay8	Year_	2010	_•		
Licensed water well driller or other person approve	ed by the Depart	ment pe	erforming v	vell sealing		
Rhonald W. Hasenyager, L.P.G.	196-000	246				
Name	Complete	e Licens	se Number			
Hanson Professional Services Inc., 1525 S. 6th St.	Springfi	eld		IL 62	2703	
Address	City			State	e/ZIP	

This state agency is requesting disclosure of information that is necessary to accomplish the statutory purpose as outlined under Public Act 85-0863. Disclosure of this information is mandatory. This form has been approved by the Forms Management Center.

IL 482-0631

Illinois Enviro	nmental Protection Agen	cy		Well (Completion	Report
Site #:	County: M	ontgomery	7	We	ell #:MW	72S
Site Name: <u>AEG Coffeen Po</u>	wer Station CCB Management Facil	ity		Во	rehole #:SE	3-02a
State Plane Coordinate: X 876,403	8.9 Y 2,513,210.0 (or) Latitude	e:°		Longitude	::°	
Surveyed By:Darren E. Forg	у	IL Regi	stration #: <u>035-0</u>	03637		
Drilling Contractor: Testing S	ervice Corporation	_ Driller:	B. Williamson			
Consulting Firm: Hanson Pro	fessional Services Inc.	Geolog	ist: Rhonald W	Hasenyager	·, LPG #196-000	0246
Drilling Method: Hollow sten	n auger	Drilling	; Fluid (Type): Po	otable water		
Logged By: <u>Testing Services</u>	Corp.	_ Date St	arted: 5/5/200	06 Date	Finished: 5/5	5/2006
Report Form Completed By: R	honald W Hasenyager	Date: _	6/7/2006			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			627.55		Top of Protective	Casing
	F		_627.07	2.97	Top of Riser Pipe	
Type of Surface Seal: Concrete			624.10	0.00	Cussual Symfoss	
			<i></i>		Ground Surface	4
Type of Annular Sealant: Bento	onite chips		619.72	4.38	Top of Annular So	ealant
Installation Method: Gravit	ty					
Setting Time: +24 hr.		∇	616.68	7.42	Static Water Leve (After Completion)	
Town of Doubouits Cost					(After Completion)	772000
Type of Bentonite Seal Gran	ular Pellet Slurry (choose one)					
Installation Method: Gravit	ty		619.72	4.38	Top of Seal	
Setting Time: +24 hr.			614.60	9.50	Top of Sand Pack	
Type of Sand Pack: Quartz sand	d					
Grain Size: #5 (sie		==	613.76	_10.34_	Top of Screen	
Installation Method: Gravit						
			608.98		Bottom of Screen	
Type of Backfill Material: <u>n/a</u>	(if applicable)		608.59	15.51	Bottom of Well	
Installation Method:n/a			608.59		Bottom of Boreho	le
			* Referenced to a	National Geodet	ic Datum	
			CAS	ING MEAS	SUREMENTS	
WELL COME			Diameter of Boreh	ole	(inches)	7.3
	TRUCTION MATERIALS the type of material for each area)		ID of Riser Pipe		(inches)	2.0
		-	Protective Casing 1	Length	(feet)	5.0
Protective Casing	SS304 SS316 PTFE PVC OTHER	: (Steel)	Riser Pipe Length		(feet)	13.31
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER		Bottom of Screen t	_	(feet)	0.39
Riser Pipe Below W.T.	SS304 SS316 PTFE (PVC) OTHER		Screen Length (1s			4.78
- Delon III.			Total Length of Ca	ısıng	(feet)	18.48

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

ILLINOIS DEPARTMENT OF PUBLIC HEALTH DIVISION OF ENVIRONMENTAL HEALTH 525 W. JEFFERSON ST. SPRINGFIELD, IL 62761

WATER WELL SEALING FORM

TYPE OR PRESS FIRMLY

RETURN ALL COPIES TO IDPH OR LOCAL HEALTH DEPARTMENT

This form shall be submitted to this Department or the local health department not more than 30 days after a water well, boring or monitoring well is sealed. Such wells are to be sealed not more than 30 days after they are abandoned in accordance with the sealing requirements in the Water Well Construction Code. THE LOCAL HEALTH DEPARTMENT OR REGIONAL PUBLIC HEALTH DEPARTMENT MUST BE NOTIFIED AT LEAST 48 HOURS PRIOR TO SEALING.

Ownership (Name of Controlling Party) AEG C	offeen Power Stat	ion			(G104)	
Well Location 134 CIPS Trail		Co	offeen		Montgome	ry
Address - Lot Number			City		County	
General Description Township 7 (N)	Range_	3	_ (K) (W)		Section _	1
SEQuarter of theNE	Quarter of the	NI	Ξ ()uarter		
Year Drilled 2010						
Drilling Permit Number (and date, if known) n/a						
Type of Well Bored Drilled_ ✓						
Total Depth 20.0 ft. Diamo	eter (inches)	2				
Formation clear of obstructionYes						
DETAILS OF PLUGGING						
		^ -	_			
Filled with Bentonite grout (cement or other materials)	from	0.5	to²	20.0 ft.		
Kind of plug Random soil	from		to).5ft.		
Filled with	from		to	ft.		
Kind of plug	from		to	ft.		
Filled with	from		to	ft.		
Kind of plug	from		to	ft.		
CASING RECORD Upper 2 feet of casing remove	ed <u>√</u> Ye	es	No			
Date well was sealed Month_October Da	ay8	Year_	2010	_•		
Licensed water well driller or other person approve	ed by the Depart	ment pe	erforming v	vell sealing		
Rhonald W. Hasenyager, L.P.G.	196-000	246				
Name	Complete	e Licens	se Number			
Hanson Professional Services Inc., 1525 S. 6th St.	Springfi	eld		IL 62	2703	
Address	City			State	e/ZIP	

This state agency is requesting disclosure of information that is necessary to accomplish the statutory purpose as outlined under Public Act 85-0863. Disclosure of this information is mandatory. This form has been approved by the Forms Management Center.

IL 482-0631

Illinois Enviro	nmental Protection Agen	cy		Well C	completion	Report
Site #:	County: M	ontgomery		Wel	1#:MW	/3D
Site Name: <u>AEG Coffeen Po</u>	wer Station CCB Management Faci	lity		Bore	ehole #: S	B-03
State Plane Coordinate: X 876,554	4.5 Y 2,514,535.3 (or) Latitud	e:°		Longitude:	<u> </u>	
Surveyed By:Darren E. Forg	у	_ IL Registra	ation #: <u>035-00</u>	3637		
Drilling Contractor: Testing S	ervice Corporation	_ Driller: _	B. Williamson			
Consulting Firm: Hanson Pro	fessional Services Inc.	_ Geologist:	Rhonald W H	Iasenyager,	LPG #196-00	0246
Drilling Method: Hollow stem	n auger	_ Drilling Fl	uid (Type): Pota	able water		
Logged By: <u>Testing Services</u>	Corp.	_ Date Starte	ed: 4/27/200	6 Date F	Finished: 4/2	27/2006
Report Form Completed By: R	honald W Hasenyager	Date:	6/7/2006			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			629.37	-3.67 T	op of Protective	Casing
			628.94	-3.24 T	Cop of Riser Pipe	:
Type of Surface Seal: Concrete			625.70	0.00	Ground Surface	
			623.00	2.70 T	Top of Annular S	ealant
Type of Annular Sealant: Bento	7					
Installation Method: <u>Tremi</u>				77. 40.		
Setting Time: +24 hr.		Σ	570.30		Static Water Leve (After Completion)	
Type of Bentonite Seal Gran						
Installation Method: Gravit	(choose one)		576.70	49.00 T	op of Seal	
Setting Time: 25 min.			575.60	50.10 		
<u> </u>			575.60	<u> </u>	Op of Sand Pack	(
Type of Sand Pack: Quartz sand	d		572.41	52.20 T		
Grain Size: #5 (sie	eve size)		573.41	_52.29 T	Cop of Screen	
Installation Method: Gravit	<u>y</u>		5 (0)(4	<i>57.06</i> F		
Type of Backfill Material: Cutti			568.64 568.30		Bottom of Screen Bottom of Well	l
Table Mala O	(if applicable)		5(7.70	50.00 F		1
Installation Method: Over-	drill borenole				Bottom of Boreho Datum	oie
			CASIN	NG MEASI	JREMENTS	
		Di	iameter of Borehol		(inches)	8.0
	TRUCTION MATERIALS the type of material for each area)		of Riser Pipe		(inches)	2.0
		Pr	otective Casing Le	ength	(feet)	5.0
D			iser Pipe Length		(feet)	55.51
Protective Casing			ottom of Screen to	-	(feet)	0.36
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER		ereen Length (1st s		(feet)	4.77
Riser Pipe Below W.T.	SS304 SS316 PTFE (PVC) OTHER	· To	otal Length of Casi	ıng	(feet)	60.64

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Enviro	nmental Protection Agen	cy		Well (Completion Report
Site #:	County: M	ontgomery	7	We	ılı #:MW4S
Site Name: <u>AEG Coffeen Po</u>	wer Station CCB Management Facil	ity		Box	rehole #:SB-04a
State Plane Coordinate: X 877,999	9.7 Y 2,514,450.6 (or) Latitude	e:°		Longitude	
Surveyed By:Darren E. Forg	у	_ IL Regi	stration #:035-0	03637	
Drilling Contractor: <u>Testing S</u>	ervice Corporation	_ Driller:	B. Williamson		
Consulting Firm: Hanson Pro	fessional Services Inc.	Geolog	ist: Rhonald W	Hasenyager	, LPG #196-000246
Drilling Method: Hollow sten	n auger	_ Drilling	Fluid (Type): Po	otable water	
Logged By:Testing Services	Corp.	_ Date St	arted:5/11/20	06 Date	Finished: <u>5/11/2006</u>
Report Form Completed By: R	honald W Hasenyager	_ Date: _	6/7/2006		
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
			626.07	` ′	Top of Protective Casing
	F		625.60	3.20_	Top of Riser Pipe
Type of Surface Seal: Concrete			622.40	0.00	Ground Surface
			619.57		Top of Annular Sealant
Type of Annular Sealant: Bento	onite chips		017.57		Top of Amilian Scalam
Installation Method: Gravit	ty				
Setting Time: +24 hr.		$\bar{\Delta}$	616.73	5.67	Static Water Level (After Completion) 6/1/2006
Type of Bentonite Seal Gran	- 1				
Installation Method: Gravit	(choose one)		619.57	2.83	Top of Seal
Setting Time: +24 hr.			614.15	8.25	Top of Sand Pack
			014.13		Top of Sand Fack
Type of Sand Pack: Quartz sand			612.57	9.83	Top of Screen
Grain Size: #5 (sie			012.37		Top of Scient
Installation Method: <u>Gravit</u>	ty		608.14	14.26	Bottom of Screen
Type of Backfill Material:n/a	(if applicable)		607.63		Bottom of Well
Installation Method: n/a	(ii applicatie)		607.63	14.77	Bottom of Borehole
			* Referenced to a		
			CAS	ING MEAS	UREMENTS
WELL CONG	TRUCTION MATERIAL C		Diameter of Boreh	ole	(inches) 7.3
	TRUCTION MATERIALS ne type of material for each area)		ID of Riser Pipe		(inches) 2.0
		-	Protective Casing 1	Length	(feet) 5.0
Destruction Coni	CC204 CC216 DTEE DVG COVER	(St. 1)	Riser Pipe Length		(feet) 14.25
Protective Casing Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER SS304 SS316 PTFE PVC OTHER	: Steel	Bottom of Screen t		(feet) 0.51
Riser Pipe Above W.T. Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER		Screen Length (1s		
Misci I ipe Delow W.1.	SSOUT SSOID TIPE (FVC) OTHER		Total Length of Ca	ısıng	(feet) 19.19

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Enviro	nmental Protection Agen	cy		Well (Completion Repor
Site #:	County: M	ontgomery	7	We	11 #:MW5D
Site Name: AEG Coffeen Po	ower Station CCB Management Facil	ity		Bor	rehole #: SB-05
State Plane Coordinate: X 878,17	4.8 Y 2,513,290.3 (or) Latitude	e:°	' "	Longitude:	
Surveyed By: Darren E. Forg	SV.	IL Regi	stration #: <u>035-0</u>	03637	
	Service Corporation	_	B. Williamson		
Consulting Firm: Hanson Pro	fessional Services Inc.	Geologi	st: Rhonald W	Hasenyager,	, LPG #196-000246
Drilling Method: Hollow ster	n auger	_ Drilling	Fluid (Type): Po	otable water	
Logged By:Testing Services	s Corp.	_ Date Sta	arted: 5/12/20	06 Date	Finished: 5/17/2006
Report Form Completed By: R	honald W Hasenyager	Date: _	6/7/2006		
ANNULAR SPA	ACE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
			626.21	` ′	Top of Protective Casing
			625.78		Top of Riser Pipe
			023.76		Top of Riser Tipe
Type of Surface Seal: Concrete		Y	622.60	0.00	Ground Surface
Type of Annular Sealant: Bento	onite grout		620.77	1.83	Top of Annular Sealant
Installation Method:Trem:	7				
Setting Time: +24 hr.		∇	572.16	50.44	Static Water Level
Setting Time		<u>*</u>	372.10		(After Completion) 6/1/2006
Type of Bentonite Seal Gran	Pellet Slurry (choose one)				
Installation Method: Gravi	, ,		582.29	40.31	Top of Seal
Setting Time: 18 min.			570.14	12.16	T CC ID I
			579.14	43.40	Top of Sand Pack
Type of Sand Pack: Quartz san	<u>d</u>		577.02	45 57	T. 60
Grain Size: #5 (si	eve size)		577.03	43.37	Top of Screen
Installation Method: Gravi	ty		572 27	50.22	D.44 of C
Type of Backfill Material: <u>n/a</u>			<u>572.27</u> <u>571.88</u>		Bottom of Screen Bottom of Well
T all a Mail 1 and	(if applicable)		560.60	54.00	D CD. 1.1
Installation Method: <u>n/a</u>					Bottom of Borehole c Datum
			CAS	ING MEAS	UREMENTS
			Diameter of Boreh		(inches) 8.0
	STRUCTION MATERIALS ne type of material for each area)	ļ	ID of Riser Pipe		(inches) 2.0
(Choose of		Ţ	Protective Casing I	Length	(feet) 5.0
		[Riser Pipe Length	-	(feet) 48.74
Protective Casing	SS304 SS316 PTFE PVC OTHER	: Steel	Bottom of Screen t	o End Cap	(feet) 0.39
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER		Screen Length (1s	st slot to last slot)	(feet) 4.76
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER	:	Total Length of Ca	sing	(feet) 53.89

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Enviro	nmental Protection Age	ncy		Well C	ompletion	Report
Site #:	County:	Montgomer	y	Well	#:MV	V5S
Site Name: <u>AEG Coffeen Po</u>	wer Station CCB Management Fa	cility		Bore	hole #:S]	B-05a
State Plane Coordinate: X 878,175	5.6 Y 2,513,285.5 (or) Latit	ude:		Longitude: _	°	_'"
Surveyed By: Darren E. Forg	у	IL Reg	istration #: <u>035-0</u>	03637		
Drilling Contractor: <u>Testing S</u>	ervice Corporation	Driller	B. Williamson			
Consulting Firm: Hanson Pro	fessional Services Inc.	Geolog	ist: Rhonald W	Hasenyager,	LPG #196-00	0246
Drilling Method: Hollow sten	n auger	Drilling	g Fluid (Type): <u>Po</u>	table water		
Logged By: <u>Testing Services</u>	Corp.	Date S	earted: 5/17/20	06 Date Fi	inished: <u>5/1</u>	7/2006
Report Form Completed By:Rl	honald W Hasenyager	Date:	6/7/2006			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
	_		626.14	` ′	op of Protective	Casing
			625.73	3.13 To	op of Riser Pipe	•
Type of Surface Seal: Concrete			622.60	0.00 G	round Surface	
			620.49		op of Annular S	ealant
Type of Annular Sealant: Bento	nite chips				op of Ammuna e	cuiunt
Installation Method: Gravit	<u>y</u>					
Setting Time: +24 hr.		$ \nabla $	615.86		atic Water Leve After Completion)	
Type of Bentonite Seal Gran	ular Pellet Slurry —					
Installation Method: Gravit	(choose one)		620.49	2.11 To	op of Seal	
	<u>y</u>		020.47		op or sear	
Setting Time:+24 hr.			611.06	11.54 To	op of Sand Pack	ζ
Type of Sand Pack: Quartz sand	d					
Grain Size: #5 (sie	eve size)		609.94	12.66 To	op of Screen	
Installation Method: Gravit	<u>y</u>					
Type of Backfill Material:Cutt	ings		605.19 604.89		ottom of Screen ottom of Well	l
	(if applicable)					
Installation Method: Over-	drill borehole		604.89 * Referenced to a	17.71 Be National Geodetic I	ottom of Boreh Datum	ole
			G A G			
			Diameter of Boreho	ING MEASU	(inches)	8.0
	TRUCTION MATERIALS te type of material for each area)		ID of Riser Pipe	oie	(inches)	2.0
(Choose on	o ope of material for each area)		Protective Casing I	Length	(feet)	5.0
			Riser Pipe Length		(feet)	15.69
Protective Casing		ER: Steel	Bottom of Screen t	o End Cap	(feet)	0.40
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTH		Screen Length (1s	t slot to last slot)	(feet)	4.75
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTH	ER:	Total Length of Ca	sing	(feet)	20.84

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environmo	ental Protection A	gency			Well	Completio	n Report
Site #:	County:	: _ Montgom	nery		W	ell #:N	MW6S
Site Name: <u>AEG Coffeen Power S</u>	Station CCB Management	Facility			Во	orehole #:	SB-06a
State Plane Coordinate: X 879,021.2	Y <u>2,513,189.4</u> (or) L	atitude:	o	<u>'</u> "	Longitude	e:°	
Surveyed By: Darren E. Forgy		IL F	Registratio	n#: <u>035-0</u>	03637		
Drilling Contractor: Testing Service	e Corporation	Dril	ler: B.	Williamson			
Consulting Firm: Hanson Profession	onal Services Inc.	Geo	logist:	Rhonald W	Hasenyage	r, LPG #196-	000246
Drilling Method: Hollow stem aug	er	Dril	ling Fluid	(Type): <u>Po</u>	table water	r	
Logged By:Testing Services Corp).	Date	e Started:	5/4/200	06 Date	Finished:	5/4/2006
Report Form Completed By: Rhonal	ld W Hasenyager	Date	e:	6/7/2006			
ANNULAR SPACE	DETAILS		E	levations (MSL)*	Depths (BGS)	(0.01 f	t.)
				626.67	3.57	Top of Protect	ive Casing
			-	626.21	3.11	Top of Riser P	ipe
Type of Surface Seal: Concrete			<u> </u>	623.10	0.00	Ground Surfac	e
Type of Amerylan Contents Doutemite of	hima		// .	620.10	3.00	Top of Annula	r Sealant
Type of Annular Sealant: Bentonite c	mps		7				
Installation Method: <u>Gravity</u> Setting Time: <u>+24 hr.</u>				616.89	6.21	Static Water L	evel
			-			(After Completic	
Type of Bentonite Seal Granular	Pellet Slurry (choose one)						
Installation Method: Gravity			-	620.10	3.00	Top of Seal	
Setting Time: +24 hr.			_	613.34	9.76	Top of Sand Pa	ack
Type of Sand Pack: Quartz sand							
Grain Size: #5 (sieve size))		-	612.06	_11.04_	Top of Screen	
Installation Method: Gravity	,						
			-	607.48	15.62	Bottom of Scre	
Type of Backfill Material:n/a	(if applicable)		-	607.02	16.08	Bottom of Wel	1
Installation Method:n/a			_	607.02 Referenced to a	16.08 National Geodet	Bottom of Bor	ehole
				11010101000 10 10	Transfer George	ar David	
						SUREMENTS	
	CTION MATERIALS			Picer Pine	ole	(inche	
(Choose one type of	of material for each area)			Riser Pipe ctive Casing I	ength	(inche	
				Pipe Length	zengui	(fee	
Protective Casing SS30	04 SS316 PTFE PVC	OTHER: Steel		m of Screen t	o End Can	`	
Riser Pipe Above W.T. SS30	04 SS316 PTFE PVC	OTHER:		n Length (1s	-		4.70
Riser Pipe Below W.T. SS30	04 SS316 PTFE PVC	OTHER:		Length of Ca		(fee	10.10

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Enviro	nmental Protection Agend	e y		Well C	completion	Report
Site #:	County: Mo	ontgomery	У	Wel	1#:MW	7S
Site Name: <u>AEG Coffeen Po</u>	wer Station CCB Management Facil	ity		Bore	ehole #:SB	8-07a
State Plane Coordinate: X 879,183	1.1 Y 2,514,397.5 (or) Latitude	::°		Longitude:	·	_'
Surveyed By:Darren E. Forg	у	IL Regi	istration #: <u>035-0</u>	03637		
Drilling Contractor: Reynolds	Drilling Corp.	Driller:	P. McIntire			
Consulting Firm: Hanson Pro	fessional Services Inc.	Geolog	ist: Rhonald W	Hasenyager,	LPG #196-000)246
Drilling Method: Hollow sten	ı auger	Drilling	g Fluid (Type): <u>Po</u>	otable water		
Logged By: Rhonald W Hase	enyager	Date St	arted: 5/9/200	06 Date F	Finished: <u>5/9</u>	2/2006
Report Form Completed By: R	honald W Hasenyager	Date: _	6/7/2006			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			627.71	, ,	op of Protective	Casing
	T		627.56		op of Riser Pipe	C
			027.30	<u></u>	op of Riser 1 ipe	
Type of Surface Seal: Concrete			624.50	0.00	Ground Surface	
Type of Annular Sealant: Bento	nite chips		621.70	2.80 T	op of Annular Se	ealant
Installation Method: Gravit	7					
Setting Time: +24 hr.		∇	619.60	4.90 S	tatic Water Leve	1
		_			(After Completion) 6	
Type of Bentonite Seal Gran	ular Pellet Slurry (choose one)	WIT .				
Installation Method: Gravit	` ' '		621.70	2.80 T	op of Seal	
Setting Time: +24 hr.			616.23	8.27 T	op of Sand Pack	
			010.23		op of Sand Lack	
Type of Sand Pack: Quartz sand			614.59	9.91 т	op of Screen	
Grain Size: #JC50FS (sie			014.37		op of Screen	
Installation Method: Gravit	<u>y</u>		610.71	13.79 E	Bottom of Screen	
Type of Backfill Material:n/a	(if applicable)		610.11		Bottom of Well	
Installation Method: n/a	(if applicable)		610.11	14.39 B	Bottom of Boreho	le
instantion (retired:iru				National Geodetic		
			CAS	ING MEASU	JREMENTS	
			Diameter of Boreh	ole	(inches)	8.0
	TRUCTION MATERIALS ne type of material for each area)		ID of Riser Pipe		(inches)	2.0
			Protective Casing	Length	(feet)	5.0
D			Riser Pipe Length		(feet)	12.37
Protective Casing		Steel	Bottom of Screen t	-	(feet)	0.60
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER		Screen Length (1s		(feet)	4.48
Riser Pipe Below W.T.	SS304 SS316 PTFE (PVC) OTHER		Total Length of Ca	sing	(feet)	17.45

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Enviro	nmental Protectio	n Agency	7		Well (Completion	Report
Site #:	Co	ounty: <u>Mon</u>	itgomer	у	We	ell #:M\	W8S
Site Name: <u>AEG Coffeen Po</u>	wer Station CCB Manage	ment Facility	у		Во	rehole #: S	B-08a
State Plane Coordinate: X 879,776	5.6 Y 2,514,478.8 (or	r) Latitude:		o ' ' ''	Longitude	:°	
Surveyed By: <u>Darren E. Forg</u>	у		IL Reg	gistration #: <u>035-0</u>	003637		
Drilling Contractor: Reynolds	Drilling Corp.		Driller	: K. Doetzel			
Consulting Firm: Hanson Pro	fessional Services Inc.		Geolog	gist: Rhonald W	Hasenyager	, LPG #196-00	00246
Drilling Method: Hollow sten	n auger		Drillin	g Fluid (Type): Po	otable water		
Logged By: Reynolds Drillin	g Corp.		Date S	tarted: 5/10/20	006 Date	Finished: 5/	10/2006
Report Form Completed By: R	honald W Hasenyager		Date:	6/7/2006			
ANNULAR SPA	CE DETAILS			Elevations (MSL)*	Depths (BGS)	(0.01 ft.))
				628.26	-3.56	Top of Protective	e Casing
				627.92	3.22	Top of Riser Pip	e
Type of Surface Seal: Concrete				624.70	0.00	Ground Surface	
				622.20	2.50	Top of Annular S	Sealant
Type of Annular Sealant: Bento		-					
Installation Method: <u>Gravit</u>	ту	-					
Setting Time: +24 hr.		_ \[\sum_{\sum}	<u> </u>	619.37	5.33	Static Water Lev (After Completion)	
Type of Bentonite Seal Gran		+		-			
Installation Method: Gravit	(choose one)			622.20	2.50	Top of Seal	
Setting Time: +24 hr.	*	_				•	
		-		614.72	9.98	Top of Sand Pac	k
Type of Sand Pack: Quartz sand	d	_					
Grain Size: #JC50FS (sie	eve size)			613.19	11.51	Top of Screen	
Installation Method: Gravit	zy	-			4.5.00		
Type of Backfill Material: Quan	rtz sand		=	608.70 608.10		Bottom of Screen Bottom of Well	n
	(if applicable)						
Installation Method: <u>Gravit</u>	zy			607.62 * Referenced to a	<u>17.08</u> a National Geodeti	Bottom of Boreh	ole
				G L G	DICAGA	LIDENCENTEC	
				Diameter of Boreh		UREMENTS	8.0
	TRUCTION MATERIAL type of material for each area)	LS		ID of Riser Pipe	IOIC	(inches)	2.0
(Choose on	or the or material for each area)			Protective Casing	Length	(feet)	5.0
	I			Riser Pipe Length		(feet)	14.73
Protective Casing		VC OTHER: (Steel	Bottom of Screen	to End Cap	(feet)	0.60
Riser Pipe Above W.T.		VC OTHER:		Screen Length (1s	st slot to last slot) (feet)	4.49
Riser Pipe Below W.T.	SS304 SS316 PTFE P	VC OTHER:		Total Length of Ca	asing	(feet)	19.82

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Enviro	nmental Protection	Agency			Well (Completio	n Report
Site #:	Cou	nty: Mont	gomery	7	We	ell #:M	W9D
Site Name: <u>AEG Coffeen Po</u>	wer Station CCB Managem	ent Facility			Во	rehole #:	SB-09
State Plane Coordinate: X 879,679	9.7 Y 2,515,666.3 (or)	Latitude: _			Longitude	::°	
Surveyed By:Darren E. Forg	у		IL Regi	stration #:035-0	03637		
Drilling Contractor: Reynolds	Drilling Corp.		Driller:	K. Doetzel			
Consulting Firm: Hanson Pro	fessional Services Inc.		Geologi	ist: Rhonald W	Hasenyager	r, LPG #196-0	00246
Drilling Method: Hollow sten	n auger		Drilling	Fluid (Type): Po	table water		
Logged By: Reynolds Drillin	g Corp.		Date Sta	arted: 5/3/200	06 Date	Finished: 5	/3/2006
Report Form Completed By: R	honald W Hasenyager		Date: _	6/7/2006			
ANNULAR SPA	CE DETAILS			Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
				627.84	, ,	Top of Protectiv	e Casing
			7	627.52	-2.92	Top of Riser Pip	oe
Type of Surface Seal: Concrete				624.60	0.00	Ground Surface	
	_			621.70		Top of Annular	
Type of Annular Sealant: Bento	nite grout					Top of / initialar	Scarait
Installation Method:Tremi	e						
Setting Time: +24 hr.				572.14	_52.46_	Static Water Le	
Type of Bentonite Seal Gran	ular Pellet Slurry						
Installation Method: Gravit	(choose one)			582.60	42.00	Top of Seal	
Setting Time: 15 min.	·y			302.00	12.00	Top of Scar	
Setting Time. 13 mm.				_580.80_	_43.80_	Top of Sand Pag	ek
Type of Sand Pack: Quartz sand	d						
Grain Size: #JC50FS (sie	eve size)			578.79	45.81	Top of Screen	
Installation Method: Gravit	y						
Type of Backfill Material: Quan	rtz sand			<u>574.03</u> <u>573.60</u>		Bottom of Scree Bottom of Well	en
	(if applicable)						
Installation Method: <u>Gravit</u>	<u>y</u>			<u>570.60</u> * Referenced to a		Bottom of Bore	hole
				CAG		TIDEMENTS	
				Diameter of Boreho		SUREMENTS (inches)	8.0
	TRUCTION MATERIALS te type of material for each area)	\$		ID of Riser Pipe		(inches)	2.0
(2.1.2000 0.1				Protective Casing I	Length	(feet)	
D	G0204 G0216 T			Riser Pipe Length		(feet)	
Protective Casing	SS304 SS316 PTFE PVC		Steel	Bottom of Screen to	-	(feet)	
Riser Pipe Above W.T. Riser Pipe Below W.T.	SS304 SS316 PTFE PVC SS304 SS316 PTFE PVC	OTHER:		Screen Length (1s			
Riser i ipe below W.1.	DOUG DOUGH TITE (FVC	OIIIER.		Total Length of Ca	sıng	(feet)	57.44

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ	nmental Protectio	n Agency	7		Well (Completio	n Report
Site #:	Co	ounty: <u>Mor</u>	itgomer	у	We	ell #:M	W9S
Site Name: AEG Coffeen Pov	wer Station CCB Manage	ment Facilit	У		Во	rehole #:	SB-09a
State Plane Coordinate: X 879,684	4.9 Y 2,515,666.2 (o	r) Latitude:		o ' ' ''	Longitude	:°	
Surveyed By:Darren E. Forgy	y		IL Reg	gistration #: <u>035-0</u>	03637		
Drilling Contractor: Reynolds	Drilling Corp.		Driller	: K. Doetzel			
Consulting Firm: Hanson Prof	essional Services Inc.		Geolog	gist: Rhonald W	Hasenyager	, LPG #196-0	00246
Drilling Method: Hollow stem	auger		Drillin	g Fluid (Type): Po	otable water		
Logged By: Reynolds Drilling	g Corp.		Date S	tarted: 5/3/200	06 Date	Finished:5	7/3/2006
Report Form Completed By: Rh	nonald W Hasenyager		Date:	6/7/2006			
ANNULAR SPA	CE DETAILS			Elevations (MSL)*	Depths (BGS)	(0.01 ft	.)
				627.84	3.24	Top of Protectiv	ve Casing
				627.51	2.91	Top of Riser Pi	pe
Type of Surface Seal: Concrete				624.60	0.00	Ground Surface	
T. CA. I. C. I. A. D. A.	9 1:			621.35	3.25	Top of Annular	Sealant
Type of Annular Sealant: Benton	-	-					
Installation Method: Gravity		_ _	7	619.37	5.23	Static Water Le	1
Setting Time:+24 hr.		_ \[\sqrt{\bar{Z}}	<u> </u>	_019.37_		(After Completion	
Type of Bentonite Seal Grant	Pellet Slurry (choose one)			-			
Installation Method: <u>Gravit</u>	,			621.35	3.25	Top of Seal	
Setting Time: +24 hr.		_		615.49	9.11	Top of Sand Pa	ek
						rop or sumaru	
Type of Sand Pack: Quartz sand		-		613.39	11.21	Top of Screen	
Grain Size: #JC50FS (sie						1	
Installation Method: <u>Gravit</u>	y	-		_608.98_	_15.62_	Bottom of Scree	en
Type of Backfill Material: <u>n/a</u>	(if applicable)	_ 🗀		608.40	16.20	Bottom of Well	
Installation Method:n/a	(ir apprioacie)			608.40	16.20	Bottom of Bore	hole
				* Referenced to a	National Geodeti	ic Datum	
				CAS	ING MEAS	UREMENTS	
WELL CONG	TRUCTION MATERIAL			Diameter of Boreh	ole	(inches)	8.0
	TRUCTION MATERIAI e type of material for each area)	LO		ID of Riser Pipe		(inches)	
				Protective Casing I	Length	(feet)	
Protective Casing	SS304 SS316 PTFE P	VC OTHER: (Steel	Riser Pipe Length	E 16	(feet)	
Riser Pipe Above W.T.		VC OTHER:		Bottom of Screen t	-	(feet)	4.44
Riser Pipe Below W.T.		VC OTHER:		Screen Length (1s Total Length of Ca		(feet)	10.11

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environmental Protection Agency	y Well Completion Report
Site #: County: _Mon	ntgomery Well #: MW10D
Site Name: AEG Coffeen Power Station CCB Management Facilit	Borehole #: SB-10
State Plane Coordinate: X 878,245.1 Y 2,515,914.0 (or) Latitude:	°'" Longitude:°'"
Surveyed By: Darren E. Forgy	IL Registration #: <u>035-003637</u>
Drilling Contractor: Reynolds Drilling Corp.	Driller: K. Doetzel
Consulting Firm: Hanson Professional Services Inc.	Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger	Drilling Fluid (Type): Potable water
Logged By: Reynolds Drilling Corp.	Date Started:5/1/2006 Date Finished:5/1/2006
Report Form Completed By: Rhonald W Hasenyager	Date:6/7/2006
ANNULAR SPACE DETAILS	Elevations Depths (0.01 ft.) (MSL)* (BGS)
Type of Surface Seal: Concrete	621.20 0.00 Ground Surface
Time of America Scalenty, Posterite energy	619.77 1.43 Top of Annular Sealant
Type of Annular Sealant: Bentonite grout Installation Method: Tremie	
	(After Completion) 6/1/2006
Type of Bentonite Seal Granular Pellet Slurry (choose one)	
Installation Method:Tremie	619.77 1.43 Top of Seal
Setting Time: 22 min.	
Type of Sand Pack: Quartz sand	
Grain Size: #JC50FS (sieve size)	
Installation Method: Gravity	
Type of Backfill Material: Quartz sand	$ \begin{array}{c ccccc} \hline & 574.63 & 46.57 & Bottom of Screen \\ \hline & 574.18 & 47.02 & Bottom of Well \end{array} $
(if applicable)	572.45 48.75 Bottom of Borehole
Installation Method: Gravity	* Referenced to a National Geodetic Datum
	CASING MEASUREMENTS
WELL CONCEDUCTION MATERIALS	Diameter of Borehole (inches) 8.0
WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)	ID of Riser Pipe (inches) 2.0
	Protective Casing Length (feet) 5.0
Protective Casing SS304 SS316 PTFE PVC OTHER:	Riser Pipe Length (feet) 45.06
Riser Pipe Above W.T. SS304 SS316 PTFE PVC OTHER:	Bottom of Screen to End Cap (feet) 0.43
Riser Pipe Below W.T. SS304 SS316 PTFE PVC OTHER:	Screen Length (1st slot to last slot) (feet) 4.73 Total Length of Casing (feet) 50.24

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Enviro	nmental Protection Agen	cy		Well C	ompletion	Report
Site #:	County: M	ontgomery		Well	#:MW	10S
Site Name: <u>AEG Coffeen Po</u>	wer Station CCB Management Faci	lity		Bore	ehole #:S]	B-10a
State Plane Coordinate: X 878,250	0.5 Y 2,515,914.4 (or) Latitud	e:°		Longitude:	0	
Surveyed By: Darren E. Forg	у	_ IL Regis	stration #: <u>035-0</u>	03637		
Drilling Contractor: Reynolds	Drilling Corp.	_ Driller:	K. Doetzel			
Consulting Firm: Hanson Pro	fessional Services Inc.	_ Geologis	st: Rhonald W l	Hasenyager,	LPG #196-00	0246
Drilling Method: Hollow ster	n auger	_ Drilling	Fluid (Type): Po	table water		
Logged By: Reynolds Drillin	ıg Corp.	_ Date Sta	rted: 5/2/200	6 Date F	inished: 5/2	2/2006
Report Form Completed By: Report Form Comple	honald W Hasenyager	_ Date: _	6/7/2006			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			624.55	· ·	op of Protective	Casing
			624.24	3.04 T	op of Riser Pipe	e
Type of Surface Seal: Concrete			621.20	0.00G	Fround Surface	
			618.83	2.37 T	op of Annular S	Sealant
Type of Annular Sealant: Bento	7				1	
Installation Method: <u>Gravi</u>	ty					
Setting Time: +24 hr.		$\overline{\Delta}$	616.29		tatic Water Leve (After Completion)	
Type of Bentonite Seal Gran	ular Pellet Slurry					
Installation Method: Gravi	(choose one)		618.83	2.37 T	op of Seal	
	ty		010.03		op of Scar	
Setting Time: +24 hr.	<u> </u>		611.90	9.30 T	op of Sand Pack	ζ.
Type of Sand Pack: Quartz san	d					
Grain Size: #JC50FS (sid	eve size)		609.92	11.28 T	op of Screen	
Installation Method: <u>Gravi</u>	ty					
Type of Backfill Material: n/a			605.44 604.90		ottom of Screen	1
	(if applicable)					
Installation Method: <u>n/a</u>			604.90 * Referenced to a	16.30 B National Geodetic l	ottom of Boreho Datum	ole
		[-		NG MEASU		8.0
	TRUCTION MATERIALS ne type of material for each area)		Diameter of Boreho ID of Riser Pipe	ne	(inches)	2.0
(Choose of	to type of material for each area)		Protective Casing I	ength	(feet)	5.0
			Riser Pipe Length		(feet)	14.32
Protective Casing			Bottom of Screen to	End Cap	(feet)	0.54
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER		Screen Length (1st	slot to last slot)	(feet)	4.48
Riser Pipe Below W.T.	SS304 SS316 PTFE (PVC) OTHER	L:	Total Length of Ca	sing	(feet)	19.34

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Enviro	nmental Protection Agen	cy		Well (Completion	Report
Site #:	County: M	ontgomer	ý	We	II #: <u>MW</u>	11D
Site Name: <u>AEG Coffeen Po</u>	wer Station CCB Management Facil	ity		Bor	rehole #:S	B-11
State Plane Coordinate: X 876,749	9.6 Y 2,515,976.7 (or) Latitude	e:º		Longitude:	o	_'
Surveyed By: _ Darren E. Forg	у	IL Regi	istration #: <u>035-0</u>	03637		
Drilling Contractor: Reynolds	Drilling Corp.	_ Driller:	K. Doetzel			
Consulting Firm: Hanson Pro	fessional Services Inc.	_ Geolog	ist: Rhonald W	Hasenyager,	LPG #196-00	0246
Drilling Method: Hollow sten	ı auger	_ Drilling	g Fluid (Type): Po	otable water		
Logged By: Reynolds Drillin	g Corp.	_ Date St	arted: 4/27/20	06 Date 1	Finished: <u>4/2</u>	8/2006
Report Form Completed By: R	honald W Hasenyager	Date: _	6/7/2006			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			625.70	` /	Γop of Protective	Casing
	T		625.36		Γορ of Riser Pipe	
				3.30_	rop of Kiser ripe	
Type of Surface Seal: Concrete		YM	622.00	0.00	Ground Surface	
Type of Annular Sealant: Bento	nite grout		618.89	3.11	Γop of Annular S	ealant
Installation Method:Tremi	7					
Setting Time: +24 hr.		∇	615.97	6.03	Static Water Leve	:1
setting Time. ————————————————————————————————————		<u>-</u>			(After Completion)	
Type of Bentonite Seal Gran	ular Pellet Slurry (choose one)					
Installation Method: Gravit	, , ,		597.20	_24.80_	Γop of Seal	
Setting Time: 18 min.			595.59	26.41	Гор of Sand Pack	
			<u> </u>		rop or Sand rack	
Type of Sand Pack: Quartz sand			593.69	_28.31	Γop of Screen	
Grain Size: #JC50FS (sie					rop or sereen	
Installation Method: Gravit	<u>y</u>		588.96	33.04	Bottom of Screen	
Type of Backfill Material: <u>n/a</u>	(if applicable)		588.50		Bottom of Well	
Installation Method: n/a	(if applicable)		585.67	36.33	Bottom of Boreho	ale
instantation Method. <u>II/a</u>				National Geodetic		nc .
			CAS	ING MEAS	UREMENTS	
			Diameter of Boreh		(inches)	8.0
	TRUCTION MATERIALS the type of material for each area)		ID of Riser Pipe		(inches)	2.0
			Protective Casing	Length	(feet)	5.0
D	00004 00016 77777		Riser Pipe Length		(feet)	31.67
Protective Casing		: Steel	Bottom of Screen t	-	(feet)	0.46
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER		Screen Length (1s			4.73
Riser Pipe Below W.T.	SS304 SS316 PTFE (PVC) OTHER		Total Length of Ca	sing	(feet)	36.86

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Enviro	nmental Protection Agend	cy		Well	Completion	Report
Site #:	County: Mo	ontgomery	,	_ W	ell #:MW	′11S
Site Name: <u>AEG Coffeen Po</u>	wer Station CCB Management Facil	ity		Вс	orehole #:S	B-11a
State	9.4 Y 2,515,971.2 (or) Latitude				»:°	
Surveyed By: Darren E. Forg	у	IL Regis	stration #:035-003	3637		
Drilling Contractor: Reynolds	Drilling Corp.	Driller:	K. Doetzel			
Consulting Firm: Hanson Pro	fessional Services Inc.	Geologi	st: Rhonald W H	asenyageı	; LPG #196-00	0246
Drilling Method: Hollow sten	n auger	Drilling	Fluid (Type): Pota	able water		
Logged By: Reynolds Drillin	g Corp.	Date Sta	arted: 4/28/2000	6 Date	Finished: 4/2	28/2006
Report Form Completed By: R	honald W Hasenyager	Date: _	6/7/2006			
ANNULAR SPA	CE DETAILS			Depths (BGS)	(0.01 ft.)	
			625.47	-3.47	Top of Protective	Casing
			625.16	-3.16	Top of Riser Pipe	•
Type of Surface Seal: Concrete			622.00	0.00	Ground Surface	
Type of Annular Sealant: Bento	nito china		620.00	2.00	Top of Annular S	ealant
Installation Method: Gravit	7					
Setting Time:+24 hr.		∇	616.58	5.42	Static Water Lev	el
setting Time. ————————————————————————————————————		<u>-</u>			(After Completion)	
Type of Bentonite Seal Gran	ular Pellet Slurry (choose one)					
Installation Method: <u>Gravi</u>	ty 🔛		_620.00_	2.00	Top of Seal	
Setting Time: 26 min.			615.25	6.75	Top of Sand Pack	ζ.
Type of Sand Pack: Quartz san-	4					
Grain Size: #JC50FS (sic		_	613.11	8.89	Top of Screen	
Installation Method: Gravit						
Type of Poelrfill Materials n/a			608.37 607.92	13.63	Bottom of Screen	l
Type of Backfill Material:n/a_	(if applicable)		_007.92_	14.08	Bottom of Well	
Installation Method:n/a			607.92 * Referenced to a N	14.08 ational Geodet	Bottom of Boreh	ole
			G A GD			
			Diameter of Borehol		SUREMENTS (inches)	8.0
	TRUCTION MATERIALS ne type of material for each area)	Г	ID of Riser Pipe		(inches)	2.0
(5.1.5050 01	· · · · · · · · · · · · · · · · · · ·	Γ	Protective Casing Le	ength	(feet)	5.0
			Riser Pipe Length		(feet)	12.04
Protective Casing	SS304 SS316 PTFE PVC OTHER	: Steel	Bottom of Screen to	End Cap	(feet)	0.46
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER		Screen Length (1st s	slot to last slot	t) (feet)	4.74
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER	:	Total Length of Casi	ng	(feet)	17.24

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Enviro	nmental Protection Agen	cy	Well	Completion	Report
Site #:	County: <u>M</u>	ontgomery	W	Vell #:MW	12D
Site Name: <u>AEG Coffeen Po</u>	wer Station CCB Management Faci	lity	В	orehole #:S	B-12
State Plane Coordinate: X 875,515	5.1 Y 2,515,900.6 (or) Latitud	e:'_	" Longitud	e:°	<u>'</u>
Surveyed By: <u>Darren E. Forg</u>	у	IL Registration #:	035-003637		
Drilling Contractor: <u>Testing S</u>	ervice Corporation	Driller: B. Wil	liamson		
Consulting Firm: Hanson Pro	fessional Services Inc.	Geologist: Rho	nald W Hasenyage	er, LPG #196-00	0246
Drilling Method: Hollow sten	n auger	_ Drilling Fluid (Ty	pe): Potable wate	r	
Logged By: Reynolds Drillin	g Corp.	_ Date Started:	5/10/2006 Date	e Finished: 5/1	0/2006
Report Form Completed By: R	honald W Hasenyager	Date:6/7.	/2006		
ANNULAR SPA	CE DETAILS		ations Depths	(0.01 ft.)	
		62	5.49 -3.29	Top of Protective	Casing
			5.03 -2.83	Top of Riser Pipe	;
Type of Surface Seal: Concrete			2.20 0.00	Ground Surface	
		61	9.43 2.77	Top of Annular S	ealant
Type of Annular Sealant: Bento	7			•	
Installation Method:Tremi	<u>e</u>				
Setting Time: +24 hr.		∑ _573	5.30 46.90	Static Water Leve (After Completion)	
Type of Bentonite Seal Gran					
Installation Method: Gravit	(choose one)	58	5.50 36.70	Top of Seal	
Setting Time: +24 hr.				•	
		_58	1.69 40.51	Top of Sand Pack	C
Type of Sand Pack: Quartz sand	d		0.74		
Grain Size: #5 (sie	eve size)	<u></u>	9.74 42.46	Top of Screen	
Installation Method: Gravit	<u> </u>		7.01	_	
Type of Backfill Material: n/a			5.21 4.73 46.99 47.47	Bottom of Screen Bottom of Well	l
	(if applicable)				
Installation Method: Over-	drill borehole		2.20 50.00 Cerenced to a National Geode	Bottom of Boreho	ole
			CACDIC MEA	CLIDEN ENTE	
		Diameter	of Borehole		8.0
	TRUCTION MATERIALS te type of material for each area)	ID of Rise		(inches)	2.0
(Choose on	o type of material for each area)		Casing Length	(feet)	5.0
		Riser Pipe	e Length	(feet)	45.29
Protective Casing			f Screen to End Cap	(feet)	0.48
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER	Screen Le	ength (1st slot to last slo	ot) (feet)	4.53
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER	Total Len	gth of Casing	(feet)	50.30

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Enviro	nmental Protection Agen	cy	Well Comp	letion Report
Site #:	County: M	ontgomery	Well #:	MW12S
Site Name: AEG Coffeen Por	wer Station CCB Management Faci	lity	Borehole #:	SB-12a
State Plane Coordinate: X 875,520	0.1 Y 2,515,900.5 (or) Latitud	e:'	" Longitude:	o ! "
Surveyed By: <u>Darren E. Forg</u>	y	IL Registration #:0	35-003637	
Drilling Contractor: Testing S	ervice Corporation	Driller:B. Willian	ison	
Consulting Firm: Hanson Prot	Sessional Services Inc.	Geologist: Rhonald	d W Hasenyager, LPG #	196-000246
Drilling Method: Hollow stem	auger	_ Drilling Fluid (Type):	Potable water	
Logged By: Reynolds Drillin	g Corp.	Date Started:5/1	0/2006 Date Finished	5/10/2006
Report Form Completed By: R1	nonald W Hasenyager	Date:6/7/200	06	
ANNULAR SPA	CE DETAILS	Elevatio (MSL)		.01 ft.)
			`	rotective Casing
		625.10		iser Pipe
Type of Surface Seal: Concrete		622.20	0.00 Ground S	Surface
Type of Annular Sealant: Bento	nite chins	_619.20	3.00 Top of A	nnular Sealant
Installation Method: Gravit	7			
Setting Time:+24 hr.		☑ 615.44	6.76 Static Wa	ater Level
			(After Con	mpletion) 6/1/2006
Type of Bentonite Seal Grant	Pellet Slurry (choose one)			
Installation Method: Gravit	y	619.20	3.00 Top of So	eal
Setting Time: 18 min.		613.95	8.25 Top of Sa	and Pack
Type of Sand Pack:Quartz sand	1			
Grain Size: #5 (sie	ve size)			creen
Installation Method: <u>Gravit</u>	y		45.40	
Type of Backfill Material: <u>n/a</u>		$==$ $\frac{607.02}{606.59}$		
Installation Method: n/a	(if applicable)	606.59) 15.61 Bottom o	of Borehole
installation Method. <u>II/a</u>			ed to a National Geodetic Datum	1 Borenoie
		(CASING MEASUREME	ENTS
WELL CONG	TRUCTION MATERIAL C	Diameter of B	Borehole	(inches) 7.3
	TRUCTION MATERIALS e type of material for each area)	ID of Riser Pi	pe	(inches) 2.0
		Protective Cas	sing Length	(feet) 5.0
Protective Corine	SS304 SS316 PTFE PVC OTHER	Riser Pipe Len		(feet) 13.51
Protective Casing Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER SS304 SS316 PTFE (PVC) OTHER		reen to End Cap	(feet) 0.43
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER	Screen Lengu	of Casing	(feet) 4.57

SS316 PTFE PVC OTHER:

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

(inches)

SS304

Well Completion Form (revised 02/06/02)

Illinois Environ	mental Protection Agend	e y	Well Co	mpletion	Report
Site #:	County: Mo	ontgomery	Well #	#:MW1	3D
Site Name: AEG Coffeen Pow	ver Station CCB Management Facili	ity	Boreh	ole #:SE	B-13
State Plane Coordinate: X 874,694.	3 Y 2,513,929.9 (or) Latitude	:'	" Longitude:	·	<u>'</u>
Surveyed By: <u>Darren E. Forgy</u>		IL Registration #:0	35-003637		
Drilling Contractor: Testing Ser	rvice Corporation	Driller: B. Willian	nson		
Consulting Firm: Hanson Profe	essional Services Inc.	Geologist: Rhonal	d W Hasenyager, I	LPG #196-000	246
Drilling Method: Hollow stem	auger	Drilling Fluid (Type):	Potable water		
Logged By: Reynolds Drilling	Corp.	Date Started:5/9	9/2006 Date Fin	nished:5/9/	/2006
Report Form Completed By: Rho	onald W Hasenyager	Date:6/7/200	06		
ANNULAR SPAC	CE DETAILS	Elevatio (MSL)		(0.01 ft.)	
	_	626.33	3	of Protective (Casing
	T	_625.87	7	p of Riser Pipe	
Type of Surface Seal: Concrete		622.70)0.00 Gro	ound Surface	
		619.64	4 3.06 Top	o of Annular Se	alant
Type of Annular Sealant: Benton	7				
Installation Method: Tremie					
Setting Time: +24 hr.		∑		tic Water Level fter Completion) 6/	
Type of Bentonite Seal Granul					
Installation Method: Gravity	(choose one)	577.48	3 45.22 Top	o of Seal	
Setting Time: +24 hr.		574.76	47.04		
<u> </u>		574.76	<u>47.94</u> 10	o of Sand Pack	
Type of Sand Pack: Quartz sand		572.90) 40.01 T	CC	
Grain Size: #5 (sieve	e size)	572.89	<u>9 49.81 </u>	o of Screen	
Installation Method: <u>Gravity</u>		568.10) 54.60 Bo	ttom of Screen	
Type of Backfill Material:n/a		567.70		ttom of Well	
Installation Method: n/a	(if applicable)	567.70) 55.00 Bo	ttom of Borehol	le
insulation victiod. <u>II/u</u>			eed to a National Geodetic Da		
			CASING MEASUF	REMENTS	
WELL CONCE	EDITOTION MATTERIAL C	Diameter of E	Borehole	(inches)	8.0
	FRUCTION MATERIALS type of material for each area)	ID of Riser P		(inches)	2.0
		Protective Ca		(feet)	5.0
Protective Casing	SS304 SS316 PTFE PVC OTHER:	Riser Pipe Le		(feet)	52.98
	SS304 SS316 PTFE PVC OTHER:	Bottom of Sc	reen to End Cap (1st slot to last slot)	(feet)	<u>0.40</u> <u>4.79</u>
-	SS304 SS316 PTFE PVC OTHER:			(feet)	58.17

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Enviro	nmental Protection Agen	cy		Well Co	ompletion	Report
Site #:	County: M	ontgomery		Well	#:MW	13S
Site Name: AEG Coffeen Por	wer Station CCB Management Facil	ity		Borel	nole #:SI	B-13a
State Plane Coordinate: X 874,695	5.7 Y 2,513,925.3 (or) Latitude	e:°		Longitude: _	<u> </u>	_'"
Surveyed By: <u>Darren E. Forg</u>	y	IL Registr	ration #:035-00)3637		
Drilling Contractor: Testing Se	ervice Corporation	_ Driller: _	B. Williamson			
Consulting Firm: Hanson Prot	Pessional Services Inc.	Geologist	: Rhonald W F	Hasenyager,	LPG #196-00	0246
Drilling Method: Hollow stem	auger	_ Drilling F	luid (Type): Pot	table water		
Logged By: Reynolds Drillin	g Corp.	_ Date Start	sed: 5/9/200	6 Date Fi	nished: 5/9	9/2006
Report Form Completed By: R	nonald W Hasenyager	Date:	6/7/2006			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			626.42	-3.72 To	op of Protective	Casing
			625.92	-3.22 To	op of Riser Pipe	
Type of Surface Seal: Concrete			622.70_	0.00 Gı	round Surface	
			619.35	3.35 To	op of Annular S	ealant
Type of Annular Sealant: <u>Bento</u>	7				•	
Installation Method: <u>Gravit</u>			61446	0.24		
Setting Time: +24 hr.		$\overline{\Delta}$	614.46		atic Water Leve After Completion)	
Type of Bentonite Seal Gran	-					
Installation Method: Gravit	(choose one)		619.35	3.35 To	op of Seal	
Setting Time: 21 min.			(12.65	10.05	CC 1D 1	
			612.65	10.03 16	op of Sand Pack	
Type of Sand Pack: Quartz sand	1		611.27	11 /2 т.	on of Conson	
Grain Size: #5 (sie	ve size)		611.27	11.43To	op of Screen	
Installation Method: <u>Gravit</u>	<u>y</u>		606.47	16.23 Bo	ottom of Screen	
Type of Backfill Material:n/a	(if applicable)		606.08		ottom of Well	
Installation Method: n/a	(п аррисаоте)		606.08	16.62 Bo	ottom of Boreho	ole
			* Referenced to a N			
			CASI	NG MEASU	REMENTS	
WELL CONS	TRUCTION MATERIALS	D	iameter of Boreho	le	(inches)	7.3
	e type of material for each area)		O of Riser Pipe		(inches)	2.0
			rotective Casing L	ength	(feet)	5.0
Protective Casing	SS304 SS316 PTFE PVC OTHER	(C) 1	iser Pipe Length ottom of Screen to	Fnd Can	(feet)	0.39
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER	B	creen Length (1st	-	(feet)	4.80
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER		otal Length of Cas		(feet)	19.84

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Enviro	nmental Protection Age	ncy		Well C	ompletion	Report
Site #:	County:!	Montgomery	7	Well	#:MW	′14S
Site Name: AEG Coffeen Po	ower Station CCB Management Fac	ility		Bore	hole #:S	b-14a
State Plane Coordinate: X 875,73	7.8 Y 2,514,125.9 (or) Latitu	de:º		Longitude: _	°	_'"
Surveyed By: Darren E. Forg	<u>xy</u>	IL Regi	stration #:035-0	03637		
Drilling Contractor: Testing S	Service Corporation	Driller:	B. Williamson			
Consulting Firm: Hanson Pro	fessional Services Inc.	Geolog	ist: Rhonald W	Hasenyager,	LPG #196-00	0246
Drilling Method: Hollow ster	n auger	Drilling	g Fluid (Type): Po	table water		
Logged By: Reynolds Drillin	ng Corp.	Date St	arted: 5/2/200	06 Date Fi	inished: 5/2	2/2006
Report Form Completed By: Report Form Comple	honald W Hasenyager	Date: _	6/7/2006			
ANNULAR SPA	ACE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			627.23	2.63_ To	op of Protective	Casing
			626.82	2.22_ To	op of Riser Pipe	:
Type of Surface Seal: Concrete			624.60	0.00 G	round Surface	
Town of America Coolema. Donat			622.20		op of Annular S	ealant
Type of Annular Sealant: Bento						
Installation Method: <u>Gravi</u> Setting Time: +24 hr.		Ţ	620.11	4.49 St	atic Water Lev	a1
Setting Time	_	\			After Completion)	
Type of Bentonite Seal Gran	Pellet Slurry (choose one)					
Installation Method: <u>Gravi</u>	, ,		622.20		op of Seal	
Setting Time: 23 min.			613.67	10.93 To	op of Sand Pack	ζ.
Tyme of Sand Books						
Type of Sand Pack: Quartz san Grain Size: #5 (si			612.34	12.26	op of Screen	
Installation Method: Gravi						
			607.58		ottom of Screen	ı
Type of Backfill Material: <u>n/a</u>	(if applicable)		607.22	17.38 Be	ottom of Well	
Installation Method:n/a			_607.22		ottom of Boreh	ole
			* Referenced to a	National Geodetic I	Jatum	
		Г	CAS	ING MEASU	REMENTS	
WELL CONS	STRUCTION MATERIALS		Diameter of Boreho	ole	(inches)	8.0
	ne type of material for each area)		ID of Riser Pipe Protective Cosing I	anath	(inches)	5.0
			Protective Casing I Riser Pipe Length	zengui	(feet)	14.48
Protective Casing	SS304 SS316 PTFE PVC OTH	ER: Steel	Bottom of Screen to	o End Can	<u> </u>	0.36
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTH	ER:	Screen Length (1s		(feet)	4.76
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTH	ER:	Total Length of Ca		(feet)	19.60

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

ILLINOIS DEPARTMENT OF PUBLIC HEALTH DIVISION OF ENVIRONMENTAL HEALTH 525 W. JEFFERSON ST. SPRINGFIELD, IL 62761

WATER WELL SEALING FORM

TYPE OR PRESS FIRMLY

RETURN ALL COPIES TO IDPH OR LOCAL HEALTH DEPARTMENT

This form shall be submitted to this Department or the local health department not more than 30 days after a water well, boring or monitoring well is sealed. Such wells are to be sealed not more than 30 days after they are abandoned in accordance with the sealing requirements in the Water Well Construction Code. THE LOCAL HEALTH DEPARTMENT OR REGIONAL PUBLIC HEALTH DEPARTMENT MUST BE NOTIFIED AT LEAST 48 HOURS PRIOR TO SEALING.

Ownership (Name of Controlling Party) AEG C	offeen Power Stat	ion			(G104)	
Well Location 134 CIPS Trail		Co	offeen		Montgome	ry
Address - Lot Number			City		County	
General Description Township 7 (N)	Range_	3	_ (K) (W)		Section _	1
SEQuarter of theNE	Quarter of the	NI	Ξ ()uarter		
Year Drilled 2010						
Drilling Permit Number (and date, if known) n/a						
Type of Well Bored Drilled_ ✓						
Total Depth 20.0 ft. Diamo	eter (inches)	2				
Formation clear of obstructionYes						
DETAILS OF PLUGGING						
		^ -	_			
Filled with Bentonite grout (cement or other materials)	from	0.5	to²	20.0 ft.		
Kind of plug Random soil	from		to).5ft.		
Filled with	from		to	ft.		
Kind of plug	from		to	ft.		
Filled with	from		to	ft.		
Kind of plug	from		to	ft.		
CASING RECORD Upper 2 feet of casing remove	ed <u>√</u> Ye	es	No			
Date well was sealed Month_October Da	ay8	Year_	2010	_•		
Licensed water well driller or other person approve	ed by the Depart	ment pe	erforming v	vell sealing		
Rhonald W. Hasenyager, L.P.G.	196-000	246				
Name	Complete	e Licens	se Number			
Hanson Professional Services Inc., 1525 S. 6th St.	Springfi	eld		IL 62	2703	
Address	City			State	e/ZIP	

This state agency is requesting disclosure of information that is necessary to accomplish the statutory purpose as outlined under Public Act 85-0863. Disclosure of this information is mandatory. This form has been approved by the Forms Management Center.

IL 482-0631

Illinois Enviro	nmental Protection Age	ncy		Well C	Completion	Report
Site #:	County:!	Montgomer	У	Wel	1#: MW	15D
Site Name: <u>AEG Coffeen Po</u>	wer Station CCB Management Fac	cility		Bore	ehole #:Sl	B-15b
State Plane Coordinate: X 875,970	0.5 Y 2,515,080.7 (or) Latitu	de:		Longitude:	<u> </u>	
Surveyed By:Darren E. Forg	у	IL Reg	istration #: <u>035-0</u>	03637		
Drilling Contractor: <u>Testing S</u>	ervice Corporation	Driller:	B. Williamson			
Consulting Firm: Hanson Proj	fessional Services Inc.	Geolog	ist: Rhonald W	Hasenyager,	LPG #196-00	0246
Drilling Method: Hollow stem	n auger	Drilling	g Fluid (Type): <u>Po</u>	table water		
Logged By: Reynolds Drillin	g Corp.	Date St	arted: 4/24/20	06 Date F	Finished: <u>4/2</u>	25/2006
Report Form Completed By: R	nonald W Hasenyager	Date: _	6/7/2006			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			626.93	` /	Top of Protective	Casing
			626.45	2.65 T	Top of Riser Pipe	;
Type of Surface Seal: Concrete			623.80_	0.00 0	Ground Surface	
			620.55	3.25	Top of Annular S	ealant
Type of Annular Sealant: Bento	nite grout				1	
Installation Method: <u>Tremi</u>	<u>e</u>					
Setting Time: +24 hr.		$ \overline{\Delta} $	618.56		Static Water Leve (After Completion)	
Type of Bentonite Seal Gran	ular Pellet Slurry —					
Installation Method: Gravit	(choose one)		594.80	29.00 Т	Top of Seal	
Setting Time: +24 hr.	· ·	() ()			op of Sear	
Setting Time	2		591.80	32.00 T	Top of Sand Pack	ζ
Type of Sand Pack:Quartz sand	<u>d</u>					
Grain Size: #5 (sie	eve size)		590.12	<u>33.68</u> T	Top of Screen	
Installation Method: <u>Gravit</u>	у		505.25	20.45		
Type of Backfill Material: <u>n/a</u>			<u>585.35</u> <u>585.00</u>		Bottom of Screen Bottom of Well	l
Table Mala Date	(if applicable)		505.00	20.00	S # 6D 1	
Installation Method: <u>Re-dri</u>	II borehole				Bottom of Boreho Datum	ole
			CASI	NG MEASI	JREMENTS	
			Diameter of Boreho		(inches)	8.0
	TRUCTION MATERIALS e type of material for each area)		ID of Riser Pipe		(inches)	2.0
			Protective Casing I	ength	(feet)	5.0
Protective Coning	SS304 SS316 PTFE PVC OTH	ED. Ctarl	Riser Pipe Length		(feet)	36.32
Protective Casing Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTH SS304 SS316 PTFE PVC OTH	ER: Steel	Bottom of Screen to	-		0.36
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTH		Screen Length (1s Total Length of Ca		(feet)	4.77
*			Total Length Of Ca	omg	(leet)	71.TJ

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

ILLINOIS DEPARTMENT OF PUBLIC HEALTH DIVISION OF ENVIRONMENTAL HEALTH

525 W. JEFFERSON ST. SPRINGFIELD, IL 62761

Sealing Form for MW15D - Original Mislabelled as MW15S

WATER WELL SEALING FORM

TYPE OR PRESS FIRMLY

RETURN ALL COPIES TO IDPH OR LOCAL HEALTH DEPARTMENT

This form shall be submitted to this Department or the local health department not more than 30 days after a water well, boring or monitoring well is sealed. Such wells are to be sealed not more than 30 days after they are abandoned in accordance with the sealing requirements in the Water Well Construction Code. THE LOCAL HEALTH DEPARTMENT OR REGIONAL PUBLIC HEALTH DEPARTMENT MUST BE NOTIFIED AT LEAST 48 HOURS PRIOR TO SEALING.

Well Location 134 CIFS Hall				Co	ffeen			Montgome	ry
Well Location 134 CIPS Trail Address - Lot Number	r				City	7		County	
General Description Township	7 (N))(%)	Range	3	_ (K)(W))		Section _	1
SEQuarter of the _	NE	Ouar	ter of the	NI	3	Quarte	er		
Year Drilled 2010		_ (_ •			
Drilling Permit Number (and date, if k	nown) ^{n/a}								
Type of Well Bored Dri									
Total Depth20.0 ft.	Diar	meter (i	nches)	2					
Formation clear of obstruction									
DETAILS OF PLUGGING									
	rials)		from	0.5	to	20.0	ft.		
· ·	,			0		0.5	a.		
Kind of plug Random soil			from		to	0.5	ft.		
Filled with			from		to		ft.		
Kind of plug			from		to		ft.		
Filled with			from		to		ft.		
Kind of plug			from		to		ft.		
	asing remo	ved _	✓ Y	es	N	0			
CASING RECORD Upper 2 feet of ca	_				2010				
	bber I	Day		Year_		·			
	ober I					 g well se	ealing.		
Date well was sealed Month Octo	ober I			ment pe		· g well se	ealing.		

This state agency is requesting disclosure of information that is necessary to accomplish the statutory purpose as outlined under Public Act 85-0863. Disclosure of this information is mandatory. This form has been approved by the Forms Management Center.

IL 482-0631

Illinois Enviro	nmental Protection Agen	cy		Well (Completion	Report
Site #:	County: M	ontgomery	7	Wo	ell #:MW	/15S
Site Name: <u>AEG Coffeen Po</u>	wer Station CCB Management Facil	ity		Во	orehole #:S	B-15a
State Plane Coordinate: X 875,97	1.1 Y 2,515,076.3 (or) Latitude	e:°		Longitude	o:	
Surveyed By: Darren E. Forg	xy	IL Regi	stration #: <u>035-0</u>	03637		
Drilling Contractor: Testing S	ervice Corporation	_ Driller:	B. Williamson			
Consulting Firm: Hanson Pro	fessional Services Inc.	Geologi	ist: Rhonald W	Hasenyager	c, LPG #196-00	00246
Drilling Method: Hollow ster	n auger	Drilling	Fluid (Type): Po	otable water	•	
Logged By: Reynolds Drillin	ng Corp.	_ Date Sta	arted: 4/25/20	06 Date	Finished: 4/2	25/2006
Report Form Completed By: Report Form Comple	honald W Hasenyager	_ Date: _	6/7/2006			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.))
			627.06	` /	Top of Protective	e Casing
			626.60		Top of Riser Pip	_
						-
Type of Surface Seal: Concrete			623.80	0.00	Ground Surface	
Type of Annular Sealant: Bento	onite chips		623.30	0.50	Top of Annular S	Sealant
Installation Method: Gravi	7					
Setting Time: +24 hr.		∇	618.81	4.99	Static Water Lev	el
Setting Time. 124 iii.		*			(After Completion)	
Type of Bentonite Seal Gran	nular Pellet Slurry (choose one)					
Installation Method: Gravi	ty		614.30	9.50	Top of Seal	
Setting Time: 20 min.			611.60	10.00		
·			611.60	12.20	Top of Sand Pac	k
Type of Sand Pack: Quartz san	d					
Grain Size: #5 (si	eve size)		609.39	14.41	Top of Screen	
Installation Method: <u>Gravi</u>	ty					
Type of Backfill Material: <u>n/a</u>			604.64 604.18		Bottom of Screen Bottom of Well	1
	(if applicable)					
Installation Method: <u>n/a</u>			604.18 * Referenced to a		Bottom of Boreh ic Datum	ole
					SUREMENTS	9.0
	STRUCTION MATERIALS	Γ	Diameter of Boreh ID of Riser Pipe	oie	(inches)	2.0
(Choose of	ne type of material for each area)	T I	Protective Casing 1	Length	(feet)	5.0
		Γ	Riser Pipe Length		(feet)	17.28
Protective Casing	SS304 SS316 PTFE PVC OTHER	: Steel	Bottom of Screen t	to End Cap	(feet)	19.62
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER	:	Screen Length (1s		t) (feet)	4.77
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER	:	Total Length of Ca	sing	(feet)	41.67

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

ILLINOIS DEPARTMENT OF PUBLIC HEALTH DIVISION OF ENVIRONMENTAL HEALTH 525 W. JEFFERSON ST. SPRINGFIELD, IL 62761

WATER WELL SEALING FORM

TYPE OR PRESS FIRMLY

RETURN ALL COPIES TO IDPH OR LOCAL HEALTH DEPARTMENT

This form shall be submitted to this Department or the local health department not more than 30 days after a water well, boring or monitoring well is sealed. Such wells are to be sealed not more than 30 days after they are abandoned in accordance with the sealing requirements in the Water Well Construction Code. THE LOCAL HEALTH DEPARTMENT OR REGIONAL PUBLIC HEALTH DEPARTMENT MUST BE NOTIFIED AT LEAST 48 HOURS PRIOR TO SEALING.

Ownership (Name of Controlling Party) AEG C	offeen Power Stat	ion		(G104)	
Well Location 134 CIPS Trail		Coffeen		Montgomer	У
Address - Lot Number		City	y	County	
General Description Township 7 (N)	Range	3 (K)(W)	Section	10
SEQuarter of theNE	Quarter of the	NE	_Quarter		
Year Drilled 2010					
Drilling Permit Number (and date, if known) n/a					
Type of Well Bored Drilled					
Total Depth 20.0 ft. Diame	eter (inches)	2			
Formation clear of obstruction Yes					
DETAILS OF PLUGGING					
Filled with Bentonite grout (cement or other materials)	from	0.5 to	20.0 ft.		
Kind of plug_Random soil			0.5 ft.		
Filled with					
Kind of plug	from	to	ft.		
Filled with	from	to	ft.		
Kind of plug	from	to	ft.		
CASING RECORD Upper 2 feet of casing remove	ed <u>√</u> Ye	sN	0		
Date well was sealed Month October Da	ay8	Year 2010	·		
Licensed water well driller or other person approve	ed by the Departi	ment performin	g well sealing.		
Rhonald W. Hasenyager, L.P.G.	196-0002	246			
Name	Complete	License Numb	er		
Hanson Professional Services Inc., 1525 S. 6th St.	Springfie	eld	IL 62	2703	
Address	City		State	e/ZIP	

This state agency is requesting disclosure of information that is necessary to accomplish the statutory purpose as outlined under Public Act 85-0863. Disclosure of this information is mandatory. This form has been approved by the Forms Management Center.

IL 482-0631

Illinois Enviro	nmental Protection Agend	ey		Well	Completion	Report
Site #:	County: Mo	ontgomery		_ W	ell #:MW	16D
Site Name: <u>AEG Coffeen Po</u>	wer Station CCB Management Facil	ity		Во	orehole #:S	B-16b
State Plane Coordinate: X 877,354	4.9 Y 2,515,079.4 (or) Latitude	::°		Longitude	e:°	_'"
Surveyed By: Darren E. Forg	у	IL Regis	stration #: <u>035-00</u>	3637		
Drilling Contractor: Reynolds	Drilling Corp.	Driller:	K. Doetzel			
Consulting Firm: Hanson Pro	fessional Services Inc.	Geologi	st: <u>Rhonald W H</u>	Iasenyage	r, LPG #196-00	0246
Drilling Method: Hollow sten	ı auger	Drilling	Fluid (Type): Pot	able water	•	
Logged By: Rhonald W Hase	enyager	Date Sta	arted: 4/21/200	6 Date	Finished: 4/2	25/2006
Report Form Completed By: R	honald W Hasenyager	Date: _	6/7/2006			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			629.68	-3.58	Top of Protective	Casing
			629.33	-3.23	Top of Riser Pipe	e
Type of Surface Seal: Concrete			626.10	0.00	Ground Surface	
Type of America Cooleans Donto	aits amout		623.77	2.33	Top of Annular S	Sealant
Type of Annular Sealant: Bento Installation Method: Tremi						
Setting Time:+24 hr.		∇	574.73	51.37	Static Water Lev	el
setting Time. ————————————————————————————————————		<u>-</u>			(After Completion)	
Type of Bentonite Seal Gran	ular Pellet Slurry (choose one)					
Installation Method: <u>Gravi</u>	xy 💮		584.65	41.45	Top of Seal	
Setting Time: +24 hr.			582.65	43.45	Top of Sand Pack	ζ
Type of Sand Pack: Quartz san-	d					
Grain Size: #JC50FS (sic		==	580.20	45.90	Top of Screen	
Installation Method: Gravit						
Type of Backfill Material: n/a			<u>575.76</u> <u>575.32</u>	50.34	Bottom of Screen Bottom of Well	1
Type of Backfill Materialii/a_	(if applicable)		_373.32_	30.78	Bottom of Wen	
Installation Method: Re-dri	Ill borehole		575.10 * Referenced to a N	51.00 National Geodet	Bottom of Boreh	ole
			CACD		NIDEMENTO	
			Diameter of Borehol		SUREMENTS (inches)	8.0
	TRUCTION MATERIALS te type of material for each area)	Г	ID of Riser Pipe		(inches)	2.0
(5.1.5050 01			Protective Casing Lo	ength	(feet)	5.0
			Riser Pipe Length		(feet)	48.83
Protective Casing			Bottom of Screen to	End Cap	(feet)	0.44
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER	——————————————————————————————————————	Screen Length (1st	slot to last slo	t) (feet)	4.74
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER		Total Length of Cas	ing	(feet)	54.01

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Enviro	nmental Protection	Agency			Well (Completion	n Report
Site #:	Cour	nty: <u>Mont</u> g	gomery		We	ell #:MV	V16S
Site Name: <u>AEG Coffeen Po</u>	wer Station CCB Manageme	ent Facility			Во	orehole #:S	SB-16a
State Plane Coordinate: X 877,355	5.1 Y 2,515,088.0 (or)	Latitude:			Longitude	::°	
Surveyed By:Darren E. Forg	у		IL Regis	stration #: <u>035-0</u>	03637		
Drilling Contractor: Reynolds	Drilling Corp.	1	Driller:	K. Doetzel			
Consulting Firm: Hanson Pro	fessional Services Inc.		Geologi	st: Rhonald W	Hasenyager	., LPG #196-0	00246
Drilling Method: Hollow sten	n auger		Drilling	Fluid (Type): Po	table water		
Logged By: Rhonald W Hase	enyager		Date Sta	rted: 4/25/20	06 Date	Finished: 4/	25/2006
Report Form Completed By: R	honald W Hasenyager		Date: _	6/7/2006			
ANNULAR SPA	CE DETAILS			Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
				629.62	` ′	Top of Protectiv	e Casing
			7	629.28	-3.18	Top of Riser Pip	e
Type of Surface Seal: Concrete				626.10	0.00	Ground Surface	
				624.66		Top of Annular	Sealant
Type of Annular Sealant: Bento	nite chips			021.00		rop or rumum	Scaraine
Installation Method: <u>Gravit</u>	ży						
Setting Time: +24 hr.				620.36	5.74	Static Water Lev (After Completion)	
Type of Bentonite Seal Gran	ular Pellet Slurry						
Installation Method: Gravit	(choose one)			624.66	1.44	Top of Seal	
Setting Time: 17 min.						1	
				612.40	_13.70_	Top of Sand Pac	ŀk
Type of Sand Pack: Quartz sand	d						
Grain Size: #JC50FS (sie	eve size)			611.51	14.59	Top of Screen	
Installation Method: Gravit	ży			60.6.60	10.44	_	
Type of Backfill Material: Quan	rtz sand			606.69 606.34		Bottom of Scree Bottom of Well	n
	(if applicable)			606.00	10.00		
Installation Method: <u>Gravit</u>	Ty					Bottom of Borel	iole
				CAS	DIC MEAS	TIDEMENTS	
			Γ	Diameter of Boreho		SUREMENTS (inches)	8.0
	TRUCTION MATERIALS type of material for each area)		Г	ID of Riser Pipe		(inches)	2.0
(2				Protective Casing I	Length	(feet)	
	I			Riser Pipe Length		(feet)	17.74
Protective Casing	SS304 SS316 PTFE PVC			Bottom of Screen t	-	(feet)	0.38
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC		11	Screen Length (1s			4.82
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC	OTHER:		Total Length of Ca	sing	(feet)	22.94

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ	nmental Protection Agend	cy	V	Well Completi	on Report
Site #:	County: Mo	ontgomery		Well #:N	<u>//W17D</u>
Site Name: AEG Coffeen Pov	wer Station CCB Management Facil	ity		Borehole #:	SB-17
State Plane Coordinate: X 878,659	1.0 Y 2,515,090.4 (or) Latitude	e:°	'" Lo	ongitude:°	
Surveyed By: <u>Darren E. Forgy</u>	ÿ	IL Regis	tration #: <u>035-00363</u>	37	
Drilling Contractor: Reynolds	Drilling Corp.	Driller:	K. Doetzel		
Consulting Firm: Hanson Prof	Sessional Services Inc.	Geologis	st: Rhonald W Hase	enyager, LPG #196	-000246
Drilling Method: Hollow stem	auger	Drilling	Fluid (Type): <u>Potable</u>	e water	
Logged By: Reynolds Drilling	g Corp.	Date Sta	rted:5/4/2006	Date Finished:	5/4/2006
Report Form Completed By: Rh	onald W Hasenyager	Date: _	6/7/2006		
ANNULAR SPA	CE DETAILS			pths (0.01 :	ft.)
			630.62 -3	Top of Protec	tive Casing
			_630.293	.19 Top of Riser l	Pipe
Type of Surface Seal: Concrete			627.10 0	.00 Ground Surfa	ce
Type of Annular Sealant: Benton	aita araut		624.92 2	.18 Top of Annul	ar Sealant
Installation Method: Tremie	7				
Setting Time:+24 hr.		∇	572.65 54	4.45 Static Water I	Level
c <u> </u>				(After Complete	on) 6/1/2006
Type of Bentonite Seal Granu	Pellet Slurry (choose one)				
Installation Method: <u>Gravity</u>	y		581.55 45	5.55 Top of Seal	
Setting Time: +24 hr.			580.25 46	5.85 Top of Sand I	ack
Type of Sand Pack: Quartz sand	1				
Grain Size: #JC50FS (siev	ve size)		578.28 48	3.82 Top of Screen	ı
Installation Method: Gravity	<u>y</u>				
Type of Backfill Material:n/a				Bottom of Sci Bottom of We	
Installation Method: n/a	(if applicable)		573.23 53	3.87 Bottom of Bo	rehole
installation victiod. <u>II/a</u>			* Referenced to a Nation		renote
			CASING	MEASUREMENT	S
WELL CONS	TRUCTION MATERIALS		Diameter of Borehole	(inch	es) 8.0
	e type of material for each area)		ID of Riser Pipe	(inch	
			Protective Casing Lengt		
Protective Casing	SS304 SS316 PTFE PVC OTHER	(C) 1	Riser Pipe Length Bottom of Screen to End	d Cap (fe	
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER		Screen Length (1st slot t	-	4.50
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER		Total Length of Casing	(fe	

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Enviro	nmental Protection Agen	cy		Well	Completion	Report
Site #:	County: M	ontgomery	7	W	ell #: MW	717S
Site Name: <u>AEG Coffeen Po</u>	wer Station CCB Management Faci	lity		Во	orehole #:S	B-17a
State Plane Coordinate: X 878,658	8.5 Y 2,515,084.8 (or) Latitud	e:°		Longitude	e:°	
Surveyed By:Darren E. Forg	y	_ IL Regi	stration #:035-00)3637		
Drilling Contractor: Reynolds	Drilling Corp.	_ Driller:	K. Doetzel			
Consulting Firm: Hanson Pro	fessional Services Inc.	_ Geologi	st: Rhonald W I	Hasenyager	r, LPG #196-00	0246
Drilling Method: Hollow stem	n auger	_ Drilling	Fluid (Type): Pot	table water	•	
Logged By: Reynolds Drillin	g Corp.	_ Date Sta	arted: 5/4/200	6 Date	Finished: 5/	4/2006
Report Form Completed By: RI	nonald W Hasenyager	_ Date: _	6/7/2006			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			630.68	-3.58	Top of Protective	e Casing
	f		630.34	-3.24	Top of Riser Pipe	e
Type of Surface Seal: Concrete			627.10	0.00	Ground Surface	
Town of America Contract Donto			626.40	0.70	Top of Annular S	Sealant
Type of Annular Sealant: Bento Installation Method: Gravit	7					
Setting Time:+24 hr.		∇	620.21	6.89	Static Water Lev	el
setting Time. ————————————————————————————————————		<u>*</u>			(After Completion)	
Type of Bentonite Seal Gran	ular Pellet Slurry (choose one)					
Installation Method: <u>Gravit</u>	у	\bowtie	617.33	9.77	Top of Seal	
Setting Time: 22 min.			614.80	12.30	Top of Sand Pacl	ζ
Type of Sand Pack: Quartz sand	4					
Grain Size: #JC50FS (sie		==	613.08	14.02	Top of Screen	
Installation Method: Gravit	y					
Type of Backfill Material: n/a			603.54 602.99	23.56 24.11	Bottom of Screen Bottom of Well	1
	(if applicable)					
Installation Method: <u>n/a</u>			602.99 * Referenced to a l	24.11 National Geodet	Bottom of Boreh	ole
			CASI	NG MFAS	SUREMENTS	
			Diameter of Boreho		(inches)	8.0
	TRUCTION MATERIALS e type of material for each area)		ID of Riser Pipe		(inches)	2.0
			Protective Casing L	ength	(feet)	5.0
D			Riser Pipe Length		(feet)	17.26
Protective Casing		R: Steel	Bottom of Screen to	-	(feet)	0.55
Riser Pipe Above W.T.	SS304 SS316 PTFE (PVC) OTHER SS304 SS316 PTFE (PVC) OTHER		Screen Length (1st			9.54
Riser Pipe Below W.T.	SS304 SS316 PTFE (PVC) OTHER	·.	Total Length of Cas	sing	(feet)	27.35

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Enviro	nmental Protection Agen	cy		Well C	Completion	Report
Site #:	County: N	Iontgomery		Wel	II #:MW	718S
Site Name: <u>AEG Coffeen Po</u>	wer Station CCB Management Fac	ility		Bor	ehole #:S	B-18a
State Plane Coordinate: X 878,604	4.7 Y 2,513,745.2 (or) Latitud	le:°		Longitude:	o	
Surveyed By:Darren E. Forg	у	_ IL Regis	stration #: <u>035-0</u>	03637		
Drilling Contractor: Reynolds	Drilling Corp.	_ Driller:	B. Williamson			
Consulting Firm: Hanson Proj	fessional Services Inc.	_ Geologis	st: Rhonald W	Hasenyager,	LPG #196-00	0246
Drilling Method: Hollow stem	n auger	_ Drilling	Fluid (Type): Po	otable water		
Logged By: Reynolds Drillin	g Corp.	_ Date Sta	arted: 5/11/20	06 Date I	Finished:5/1	1/2006
Report Form Completed By: R	honald W Hasenyager	_ Date: _	6/7/2006			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			629.02	3.42_	Γop of Protective	Casing
			628.71	3.11 7	Γop of Riser Pipe	e
Type of Surface Seal: Concrete			625.60	0.00	Ground Surface	
T. CA. I. C. I. (D.)			622.13	3.47	Гор of Annular S	Sealant
Type of Annular Sealant: Bento						
Installation Method: Gravit			(19.72	(97)	N. 1. 117 . I	1
Setting Time: +24 hr.		Σ	618.73		Static Water Lev (After Completion)	
Type of Bentonite Seal Gran	-					
Installation Method: <u>Gravit</u>	(choose one)		622.13	3.47	Γop of Seal	
Setting Time: 25 min.			615.79	9.81	Гор of Sand Pacl	,
			013.79		top of Sand Faci	X.
Type of Sand Pack: Quartz sand	<u>d</u>		614.29	_11.31_ 7	Γop of Screen	
Grain Size: #JC50FS (sie	eve size)		014.29	11.51	rop or screen	
Installation Method: <u>Gravit</u>	<u>y</u>		609.81	15.79 H	Bottom of Screer	
Type of Backfill Material:n/a_			609.20		Bottom of Well	1
To stall at the Mathed to 10 /s	(if applicable)		600.20	16.40 т	D-44	-1-
Installation Method: <u>n/a</u>			609.20 * Referenced to a	16.40 I National Geodetic	Bottom of Boreh Datum	oie
			CAS	ING MEASI	UREMENTS	
			Diameter of Boreh		(inches)	8.0
	TRUCTION MATERIALS te type of material for each area)		ID of Riser Pipe		(inches)	2.0
`			Protective Casing I	Length	(feet)	5.0
D			Riser Pipe Length		(feet)	14.42
Protective Casing			Bottom of Screen t	-	(feet)	0.61
Riser Pipe Above W.T. Riser Pipe Below W.T.	SS304 SS316 PTFE (PVC) OTHE SS304 SS316 PTFE (PVC) OTHE		Screen Length (1s			4.48
Kisci i ipe Beiow W.1.	SSSOT SSSTO TIPE (FVC) OTHE		Total Length of Ca	sing	(feet)	19.51

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Site#: County: Montgomery Well #: Site Name: Ash Pond Investigation Borehole #: State Plane Coordinate: X 2,515,867.9 Y 874,226.4 (or) Latitude: Longitude:	SB20
State	
Surveyed By: Darren E. Forgy IL Registration #: 035-003637	
Drilling Contractor: Reynolds Drilling Corp. Driller: A. Rachford	
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #19	6-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): none	
Logged By: Rhonald W. Hasenyager Date Started: 5/1/2007 Date Finished:	5/1/2007
Report Form Completed By: Rhonald W. Hasenyager Date: 5/2/2007	
ANNULAR SPACE DETAILS Elevations Depths (MSL)* (BGS)	1 ft.)
	ective Casing
	er Pipe
Type of Surface Seal: Concrete 620.30 0.00 Ground Sur	face
Type of Annular Sealant: Bentonite chips 620.30 0.00 Top of Ann	nular Sealant
Installation Method: gravity	
Setting Time: _ >12 hours	er Level
	letion) 5/10/2007
Type of Bentonite Seal Granular Pellet Slurry (choose one)	
Installation Method: n/a n/a Top of Seal	
Setting Time: 613.05 7.25 Top of San	d Pack
Type of Sand Pack: Quartz sand	
Grain Size:	en
Installation Method:gravity	_
Type of Backfill Material:	
(if applicable)	Rorcholo
* Referenced to a National Geodetic Datum	on a lore
CASING MEASUREMEN	TS
	iches) 8.0
WELL CONSTRUCTION MATERIALS (Choose one type of material for each area) ID of Riser Pipe (in	iches) 2.0
Protective Casing Length	(feet) 5.0
Protective Coging 90004 90004 DTEE DVC OTUED (See	(feet) 11.11
Riser Pine A hove W.T. SS304 SS316 PTEE CPXTC OTHER	(feet) 0.45
Riser Pine Below W.T. SS304 SS316 PTEF (PVC) OTHER	(feet) 4.81 (feet) 16.37

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

(inches)

Screen

Well Completion Form (revised 02/06/02)

Illinois Environmental Protection Agency	Well Completion Report
Site #: County: Mor	ntgomery Well #: R104
Site Name: CCB Management Facility	Borehole #: R104
State Plane Coordinate: X 2,514,503.4 Y 875,857.8 (or) Latitude:	39° 4' 10.000" Longitude: -89° 23' 56.800"
Surveyed By: <u>Jeffrey D. Emrick</u>	IL Registration #:035-003507
Drilling Contractor: Layne-Western Co	Driller: D. Mahurin
Consulting Firm: Hanson Professional Services Inc.	Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger	Drilling Fluid (Type):n/a
Logged By: Rhonald W. Hasenyager	Date Started:10/8/2010 Date Finished:10/8/2010
Report Form Completed By: Suzanna Simpson	Date:10/19/2010
ANNULAR SPACE DETAILS	Elevations Depths (0.01 ft.) (MSL)* (BGS)
	632.02 -2.99 Top of Protective Casing
	631.84 -2.81 Top of Riser Pipe
Type of Surface Seal: Concrete	
Type of Samuel Seam	629.03 0.00 Ground Surface
Type of Annular Sealant: High-solids bentonite	
Installation Method:	
Setting Time: >24 hr.	
Type of Bentonite Seal Granular Pellet Slurry	(vite completion) 17/13/2010
(choose one)	
Installation Method: Gravity	
Setting Time: 15 min	
Type of Sand Pack: Quartz sand	
Grain Size: 10/20 (sieve size)	
Installation Method: Gravity	
Type of Backfill Material: n/a	609.71 19.32 Bottom of Screen 609.18 19.85 Bottom of Well
(if applicable)	
Installation Method:n/a	609.18 19.85 Bottom of Borehole * Referenced to a National Geodetic Datum
	CASING MEASUREMENTS
WELL CONSTRUCTION MATERIALS	Diameter of Borehole (inches) 8.0
(Choose one type of material for each area)	ID of Riser Pipe (inches) 2.0 Protective Casing Length (feet) 5.0
	Riser Pipe Length (feet) 17.60
Protective Casing SS304 SS316 PTFE PVC OTHER: (
Riser Pipe Above W.T. SS304 SS316 PTFE PVC OTHER:	Screen Length (1st slot to last slot) (feet) 4.53
Riser Pipe Below W.T. SS304 SS316 PTFE PVC OTHER:	Total Length of Casing (feet) 22.66

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ	Illinois Environmental Protection Agency				Completion	Report
Site #:	County: _	Montgomery		Well	#: <u>R2</u>	.01
Site Name:CCB Managemen	t Facility			Bore	hole #:I	R201
State Plane Coordinate: X 2,514,842	2.0 Y <u>877,925.3</u> (or) Lati	tude:39°_	<u>4'</u> <u>30.500"</u>	Longitude: _	-89° 23	52.300"
Surveyed By: <u>Jeffrey D. Emric</u>	ck	IL Regi	stration #:035-0	03507		
Drilling Contractor: <u>Layne-We</u>	stern Co	Driller:	D. Mahurin			
Consulting Firm: Hanson Profe	essional Services Inc.	Geologi	st: Rhonald W.	Hasenyager, L	PG #196-0002	246
Drilling Method: Hollow stem	auger	Drilling	Fluid (Type): <u>n/a</u>	a		
Logged By: Rhonald W. Hase	nyager	Date St	arted:10/15/20	010 Date F	inished:10/	15/2010
Report Form Completed By: Su	zanna Simpson	Date: _	10/19/2010			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
			626.51		op of Protective	Casing
	c		626.34		op of Riser Pipe	
Type of Surface Seal: Concrete	_		624.02	0.00 G	Fround Surface	
			621.52		op of Annular S	aalant
Type of Annular Sealant: High-s	solids bentonite		021.32		op of Affilular S	zaiaiii
Installation Method:Tremio	2					
Setting Time:>24 hr.			618.70		tatic Water Leve (After Completion)	
Type of Bentonite Seal Gran	ular Pellet Slurry —					
Installation Method: Gravit	(choose one)		614.47	9.55 T	op of Seal	
Setting Time: 48 min	y		014.47	<u></u>	op of Scar	
Setting Time. 48 min			612.90	11.12 Te	op of Sand Pack	
Type of Sand Pack:Quartz sand	1					
Grain Size: 10/20 (sie	eve size)		611.75	12.27 To	op of Screen	
Installation Method: <u>Gravit</u>	у		60 m.a .6	16.66 -		
Type of Backfill Material:n/a			607.36 606.80		ottom of Screen ottom of Well	
	(if applicable)		606.00	17.00		
Installation Method:n/a			606.80 * Referenced to a	17.22 B National Geodetic D	ottom of Boreho	ле
			CAS	SING MEASU	DEMENITS	
			Diameter of Boreho		(inches)	8.0
	STRUCTION MATERIALS e type of material for each area)		ID of Riser Pipe		(inches)	2.0
(2			Protective Casing I	ength	(feet)	5.0
	I		Riser Pipe Length		(feet)	14.59
Protective Casing		THER:	Bottom of Screen to	o End Cap	(feet)	0.56
Riser Pipe Above W.T.		HER:	Screen Length (1s		(feet)	4.39
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OT	HER:	Total Length of Cas	sing	(feet)	19.54

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ	Illinois Environmental Protection Agency					Well	Comple	tion Report
Site #:	Co	ounty: <u>Mor</u>	ntgomery			W	/ell #:	R205
Site Name: Coffeen Power Sta	ation - Gypsum Mgmt Facil	lity				В	orehole #:	R205
State Plane Coordinate: X 2,515,910).1 Y <u>875,548.8</u> (or	r) Latitude:	39°_	4'	6.935"	Longitud	e: <u>-89°</u> _	23' 38.991"
Surveyed By: Gary C. Rogers			IL Regis	stration #	#: <u>035-0</u>	2957		
Drilling Contractor: Bulldog D	rilling		Driller:		tes			
Consulting Firm: Hanson Profe	essional Services Inc.		Geologi	st: Rh	onald W.	Hasenyager	:, LPG #196	-000246
Drilling Method: Hollow Stem	Auger		Drilling	Fluid (T	ype):			
Logged By: Rhonald W. Hase	nyager		Date Sta	arted:	3/20/20	017 Date	e Finished: _	3/20/2017
Report Form Completed By: Su	zanna L. Keim		Date: _	3/2	27/2017			
ANNULAR SPA	CE DETAILS				evations MSL)*	Depths (BGS)	(0.01	ft.)
				`	24.94	3.03_	Top of Prote	ective Casing
				_6	524.52	2.61	Top of Riser	Pipe
Type of Surface Seal: Concrete				<u> </u>	521.91	0.00	Ground Sur	face
Type of Annular Sealant: High-s	colids bentonite			// _6	19.91	2.00	Top of Annu	ılar Sealant
Installation Method: Tremie		- 7						
Setting Time: _ >24 hr.		- <u> </u>	\mathbb{Z}	_6	19.41_	2.50	Static Water	Level
							(After Comple	etion) 3/21/2017
Type of Bentonite Seal Gran	Pellet Slurry (choose one)		Y					
Installation Method: <u>Gravit</u>	У	- 😾	\bowtie	_6	14.26	7.65	Top of Seal	
Setting Time: 95 minutes		-		_6	11.99	9.92	Top of Sand	Pack
Type of Sand Pack: Quartz sand	i							
Grain Size: #0 (sie				_6	10.59	_11.32_	Top of Scree	en
Installation Method: Gravit	У	_						
Type of Backfill Material:n/a					05.90 05.49	<u>16.01</u> 16.42	Bottom of S Bottom of V	
	(if applicable)	_ _						
Installation Method:n/a					05.49 Referenced to a	16.42 National Geodet	Bottom of B ic Datum	orehole
					CAS	SING MEAS	SUREMENT	ΓS
				Diamete	er of Boreho			ches) 8.0
	STRUCTION MATERIALS e type of material for each area)	S			iser Pipe	-		ches) 2.0
•	,			Protecti	ve Casing I	ength		(feet) 5.0
D					pe Length			(feet) 13.93
Protective Casing Riser Pipe Above W.T.		VC OTHER:	steel		of Screen to			(feet) 0.41
Riser Pipe Above W.T. Riser Pipe Below W.T.		VC OTHER:				st slot to last slo		(feet) 4.69
raser i pe below w.i.	55507 55510 TIFE (F	, C) OTHER.	[]	Total Le	ength of Ca	sıng		(feet) 19.03

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

SS304

Well Completion Form (revised 02/06/02)

SS316

Illinois Environ	mental Protection A	Agency				Well	Comple	tion Report
Site #:	Co	unty: <u>Mon</u>	itgomery			W	/ell #:	T127
Site Name: CCB Management	t Facility					В	orehole #:	T127
State Plane Coordinate: X 2,513,911	1.0 Y 875,359.2 (or) Latitude:	39°_	4'	5.200"	Longitud	e: <u>-89°</u> _	24' 4.400"
Surveyed By:			IL Regi	stration #:				
Drilling Contractor: <u>Layne-We</u>	stern Co		Driller:	T. List	-			
Consulting Firm: Hanson Profe	essional Services Inc.		Geologi	ist: Rho	onald W.	Hasenyager	<u>, LPG #196</u>	-000246
Drilling Method: Hollow stem	auger		Drilling	; Fluid (Ty	pe): <u>n/a</u>	a		
Logged By:			Date St	arted:	2/10/20	010 Date	e Finished: _	2/10/2010
Report Form Completed By:			Date: _	2/1	9/2010			
ANNULAR SPA	CE DETAILS				v ations ISL)*	Depths (BGS)	(0.0)	1 ft.)
					31.29	· · ·	Top of Prote	ective Casing
				_63	80.96	-2.89	Top of Rise	r Pipe
Type of Surface Seal: Concrete	-			> <u>62</u>	28.07	0.00	Ground Sur	face
Type of Annular Sealant: High-s	solids bentonite			62	22.43	5.64	Top of Ann	ular Sealant
Installation Method:Tremic		_]						
Setting Time: _ >24 hr.		_ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	$\underline{z} \mid \cdot \mid$	_61	6.81	_11.26_	Static Water (After Comple	r Level etion) 3/1/2010
Type of Bentonite Seal Gran								
Installation Method: Granu	(choose one)	-	\bowtie	_61	3.43	_14.64_	Top of Seal	
Setting Time: 10 min		-		_61	2.32	15.75	Top of Sand	l Pack
Type of Sand Pack: Quartz sand	1	_						
Grain Size:10/20 (sie	ve size)			_61	0.54	17.53	Top of Scree	en
Installation Method: Gravit	У	-		-		22.05	_	
Type of Backfill Material: <u>n/a</u>	(if applicable)		\exists		06.00 05.43	<u>22.07</u> <u>22.64</u>	Bottom of V	
Installation Method: n/a	(ii applicable)			60	05.43	22.64	Bottom of E	Borehole
				* Re	eferenced to a	National Geodet	ic Datum	
					CAS	SING MEAS	SUREMEN	ΓS
WELL CONG	TDUCTION MATERIAL C	,		Diameter	of Boreho	ole	(ir	nches) 8.0
	STRUCTION MATERIALS e type of material for each area)	•		ID of Ris	-			nches) 2.0
					e Casing I	Length		(feet) 5.0
Protective Casing	SS304 SS316 PTFE PV	/C OTHER:		Riser Pip	-	o End Cap		(feet) 20.42 (feet) 0.57
Riser Pipe Above W.T.	SS304 SS316 PTFE PV	C OTHER:				st slot to last slo		(feet) 0.57 (feet) 4.54
Riser Pipe Below W.T.	SS304 SS316 PTFE PV	C OTHER:			ngth of Ca			(feet) 25.53

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

SS304

Well Completion Form (revised 02/06/02)

SS316

Illinois Enviro	onmental Protection Agend	e y		Well	Completion	Report
Site #:	County: N	Iontgomery	V	W	Vell #:T1	28
Site Name: <u>AEG Coffeen Po</u>	ower Station CCB Management Facili	ity		В	Forehole #:	128
State- Plant Plane Coordinate: X 2,513,90	09.5 Y 875,509.7 (or) Latitud	le:	·	Longitud	e:°	'"
Surveyed By:		_ IL Reg	istration #:			
Drilling Contractor: <u>Layne-W</u>	estern Co	_ Driller:	T. List			
Consulting Firm: Hanson Pro	ofessional Services Inc.	_ Geolog	ist: Rhonald W.	Hasenyage	er, LPG #196-000	246
Drilling Method: Hollow ster	n auger	_ Drilling	g Fluid (Type): <u>n/a</u>	ı		
Logged By: Suzanna L. Sim	pson	_ Date St	tarted: 2/9/201	0 Dat	e Finished: 2/	9/2010
Report Form Completed By: S	uzanna L. Simpson	_ Date: _	2/18/2010			
ANNULAR SPA	ACE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
	<u></u>		631.23	2.79_	Top of Protective	Casing
			630.93	2.49	Top of Riser Pipe	
Type of Surface Seal: Concrete			628.44	0.00	Ground Surface	
Type of Annular Sealant: High	-solids bentonite		623.38	5.06	Top of Annular S	ealant
Installation Method:Trem	ie J					
Setting Time: >24 hr.		$ \overline{\Delta} $	616.09	12.35	Static Water Leve (After Completion)	
Type of Bentonite Seal Gra	nular Pellet Slurry					
Installation Method: Granu	(choose one)		615.53	12.91	Top of Seal	
Setting Time: 10 min			614.13	_14.31_	Top of Sand Pack	:
Type of Sand Pack: Quartz san	nd					
	sieve size)		611.91	16.53	Top of Screen	
Installation Method: Gravi	ity					
Type of Backfill Material: Qua			607.40 606.80	<u>21.04</u> <u>21.64</u>	Bottom of Screen Bottom of Well	
Installation Method: Gravi	(if applicable)		604.38	24.06	Bottom of Boreho	ale.
ilistaliation friction. Oravi			* Referenced to a			'IC
			CAS	ING MEA	SUREMENTS	
WELL CON	STRUCTION MATERIALS		Diameter of Boreho	ole	(inches)	8.0
	one type of material for each area)		ID of Riser Pipe		(inches)	2.0
			Protective Casing L	ength	(feet)	5.0
Protective Casing	SS304 SS316 PTFE PVC OTHE	R: Steel	Riser Pipe Length Bottom of Screen to	End Can	(feet)	0.60
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHE	R:	Screen Length (1s		`	4.51
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHE	R:	Total Length of Cas		(feet)	25.13
Screen	SS304 SS316 PTFE PVC OTHE	R:	Screen Slot Size **		(inches)	0.010

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ	y		Well Cor	npletion	Report			
Site #:	County: M	ontgomery		Well #:	T20)2		
Site Name: AEG Coffeen Pov	ver Station CCB Management Facili	ty		Borehol	e#:T	202		
State	5.0 Y 876,699.4 (or) Latitud	-						
Surveyed By:	ck	_ IL Regist	ration #:035-00	03507				
Drilling Contractor: <u>Layne-We</u>	stern Co	_ Driller: _	D. Mahurin					
Consulting Firm: Hanson Prof	essional Services Inc.	_ Geologist	Geologist: Rhonald W. Hasenyager, LPG #196-000246					
Drilling Method: Hollow stem	auger	_ Drilling F	Fluid (Type): <u>n/a</u>	,				
Logged By: Suzanna L. Simps	son	_ Date Star	ted: 10/15/20	10 Date Finis	shed:10/1	5/2010		
Report Form Completed By: Su	zanna L. Simpson	_ Date:	10/19/2010					
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)			
			628.79		of Protective (Casing		
			628.63		of Riser Pipe			
Type of Surface Seal: Concrete			(2(22	0.00	10.0			
- J. P. C. S.			626.22		and Surface	1		
Type of Annular Sealant: High-s	solids bentonite		623.42		of Annular Se	alant		
Installation Method:Tremie	·							
Setting Time: >24 hr.		$\bar{\Delta}$	613.72		c Water Level er Completion) 1			
Type of Bentonite Seal Grand	ular Pellet Slurry				•			
Installation Method: Gravity	(choose one)		616.50	9.72 Top	of Seal			
			010.30	<u>9.72</u> 10p	oi seai			
Setting Time: 15 min			615.27	10.95 Top	of Sand Pack			
Type of Sand Pack: Quartz sand	<u> </u>							
Grain Size: 10/20 (sie	ve size)		613.95	12.27 Top	of Screen			
Installation Method: Gravity	<i>I</i>							
Type of Backfill Material:n/a			609.57 609.01		om of Screen om of Well			
	(if applicable)		600.00	10.00				
Installation Method:n/a			608.22 * Referenced to a N	18.00 Botto National Geodetic Datum	om of Borehol	e		
			CAS	ING MEASURE	EMENTS			
		Г	Diameter of Boreho		(inches)	8.0		
	TRUCTION MATERIALS te type of material for each area)		D of Riser Pipe		(inches)	2.0		
(2			Protective Casing L	ength	(feet)	5.0		
			Riser Pipe Length		(feet)	14.68		
Protective Casing			Bottom of Screen to	End Cap	(feet)	0.56		
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER		Screen Length (1s		(feet)	4.38		
Riser Pipe Below W.T.	SS304 SS316 PTFE (PVC) OTHER	₹:	Total Length of Cas	ing	(feet)	19.62		

Well Completion Form (revised 02/06/02)

Screen Slot Size **

 $\hbox{**Hand-Slotted Well Screens Are Unacceptable}\\$

0.010

Illinois Enviror	nmental Protection Ag	gency				Well	Complet	ion Report
Site #:	Count	ty: Mon	tgomery			W	/ell #:	T408
Site Name: Coffeen Power Sta	ation - Ash Pond 2					В	orehole #:	T408
State Plane Coordinate: X 2,515,314	4.9 Y 873,999.4 (or)	Latitude:	39°	<u>3'</u> <u>5</u>	1.671"	Longitud	e: <u>-89°</u> _	23' 46.704"
Surveyed By: Gary C. Rogers			IL Regi	stration #: _	035-0	02957		
Drilling Contractor: Bulldog D	Orilling, Inc.		Driller:	_ J. Dittma	iier			
Consulting Firm: Hanson Prof	essional Services Inc.		Geolog	ist: Rhon	ald W.	Hasenyager	., LPG #196-	000246
Drilling Method: Hollow stem	auger		Drilling	; Fluid (Type): <u>no</u>	one		
Logged By: Kristen L. Theesf	îeld		Date St	arted:8	3/17/20	016 Date	e Finished:	8/17/2016
Report Form Completed By:Su	ızanna L. Keim		Date: _	8/24/	2016			
ANNULAR SPA	CE DETAILS			Eleva (MS		Depths (BGS)	(0.01	ft.)
				_624.		3.35	Top of Prote	ctive Casing
				624.	08_	-2.99	Top of Riser	Pipe
Type of Surface Seal: Concrete				621.	09	0.00	Ground Surf	ace
				619.		2.00	Top of Annu	
Type of Annular Sealant: High-							- P	
Installation Method:Tremi	e							
Setting Time: >24 hours			7				Static Water (After Complex	
Type of Bentonite Seal Gran	Pellet Slurry (choose one)	+						
Installation Method: Gravit	,		V V	602.	99_	_18.10_	Top of Seal	
Setting Time: 30 minutes				602.	04	19.05	Top of Sand	Pack
Town of Could Barby 10 11 C							-	
Type of Sand Pack: Quartz San Grain Size: 10-20 (six				600.	43_	20.66	Top of Scree	n
Installation Method: Gravit	,							
				595.	60	25.49	Bottom of So	ereen
Type of Backfill Material:	(if applicable)			_595.	17_	_25.92_	Bottom of W	Vell (
Installation Method:				595.		25.92		orehole
				* Refer	enced to a	National Geodet	ic Datum	
					CAS	SING MEAS	SUREMENT	S
WELL CONS	STRUCTION MATERIALS			Diameter of		ole	(inc	ches) 8.0
	ne type of material for each area)			ID of Riser	_			ches) 2.0
						ength		(feet) 5.0
Protective Casing	SS304 SS316 PTFE PVC	OTHER: St	teel	Riser Pipe I		o End Cap		(feet) 23.65 (feet) 0.43
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC					st slot to last slo		feet) 4.83
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC	OTHER:		Total Lengt				(feet) 28.91

SS304

Well Completion Form (revised 02/06/02)

SS316

PTFE PVC OTHER:

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ	ıcy		Well	Completion	Report	
Site #:	County: _	Montgomery		W	ell #:T4	09
Site Name: Coffeen Power Sta	ation - Ash Pond 2			Во	orehole #:T	<u>`409</u>
State Plane Coordinate: X 2,514,693	8.9 Y 872,517.8 (or) Lati	itude: 39°	3' 37.079"	Longitude	e:89°23	54.736"
Surveyed By: Gary C. Rogers		IL Regis	tration #:035-00	2957		
Drilling Contractor: Bulldog D	rilling, Inc.	Driller:	J. Dittmaier			
Consulting Firm: Hanson Profe	essional Services Inc.	Geologis	t: <u>Rhonald W. I</u>	Hasenyager.	, LPG #196-0002	46
Drilling Method: Hollow stem	auger	Drilling	Fluid (Type): <u>nor</u>	ne		
Logged By: Kristen L. Theesf	eld	Date Sta	rted: 8/19/201	Date	Finished: 8/1	9/2016
Report Form Completed By:Su	zanna L. Keim	Date:	8/24/2016			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
	ŗ		625.35		Top of Protective	Casing
			625.01	-3.16	Top of Riser Pipe	
Type of Surface Seal: Concrete		1	621.85	0.00	Ground Surface	
Type of Annular Sealant: High-s	solids bentonite		619.85	2.00	Top of Annular Se	ealant
Installation Method: Tremie						
Setting Time: _ >24 hours		$ \nabla $			Static Water Leve (After Completion)	I
Type of Bentonite Seal Gran	ular Pellet Slurry — (choose one)					
Installation Method: Gravit	<u>y</u>		602.65	19.20	Top of Seal	
Setting Time: 30 minutes			601.30	20.55	Top of Sand Pack	
Type of Sand Pack: Quartz Sand	d					
Grain Size: 10-20 (sie	ve size)		600.06	21.79	Top of Screen	
Installation Method: <u>Gravit</u>	y		505.26	26.50	D 00	
Type of Backfill Material:			595.26 594.86	26.59 26.99	Bottom of Screen Bottom of Well	
	(if applicable)		594.86	26.99	Bottom of Boreho	le.
instanction viction.			* Referenced to a N			
			CASI	NG MEAS	SUREMENTS	
WELL COM	TDIICTIONI MATERIAI C		Diameter of Borehol	le	(inches)	8.0
	TRUCTION MATERIALS e type of material for each area)		ID of Riser Pipe		(inches)	2.0
			Protective Casing Le			5.0
Protective Casing	SS304 SS316 PTFE PVC OT		Riser Pipe Length	F 16		24.95
Riser Pipe Above W.T.			Bottom of Screen to Screen Length (1st		(feet)	0.40 4.80
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OT		Total Length of Casi		(feet)	30.15

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

SS304

Well Completion Form (revised 02/06/02)

SS316

Illinois Environ			Well C	Completion	Report	
Site #:	County:			Well	#: <u>TA</u>	31
Site Name:Coffeen Power Sta	ation			Bore	hole #:T	A31
State Plane Coordinate: X 2,513,856	5.8 Y 876,542.2 (or) Latitude	: <u>39°</u> _	4' 16.930"	Longitude: _	89° <u>24</u>	4.920"
Surveyed By: Gary C. Rogers		IL Registr	ration #:035-00	02957		
Drilling Contractor: Ramsey		Driller: _	B. Williamson			
Consulting Firm: Hanson Profe	essional Services Inc.	Geologist:	Rhonald W.	Hasenyager, L	.PG #196-0002	246
Drilling Method: Hollow Stem	Auger	Drilling F	luid (Type):no	ne		
Logged By: Rhonald W. Hase	enyager	Date Start	ted:10/28/20)14 Date Fi	inished: 10/2	28/2014
	nonald W. Hasenyager		11/5/2014			
ANNULAR SPA			Elevations	Depths	(0.01 ft.)	
			(MSL)* _626.90_	(BGS) -3.01 To	op of Protective	Casino
						Cusing
			626.55	2.66 To	op of Riser Pipe	
Type of Surface Seal: Concrete			623.89	0.00 G	round Surface	
T CA LC L D A			620.89	3.00 To	op of Annular Se	ealant
Type of Annular Sealant: Bento	7	19				
Installation Method: <u>tremie</u>			615.78	8.11 St	tatic Water Leve	ı
Setting Time: 45 min.		$\overline{\Delta}$	_013.78_		After Completion) 1	
Type of Bentonite Seal Gran						
Installation Method: _ Gravit	(choose one)		613.56	10.33 To	op of Seal	
Setting Time: 30 min.			610.01	12.00 T	f.Cd Dd-	
			610.81	13.08 To	op of Sand Pack	
Type of Sand Pack: Quartz Sand	d		608.80	15.09 To	on of Caraon	
Grain Size:10/20 (sie			_000.80_	13.09 10	op of Screen	
Installation Method: <u>Gravit</u>	y & Surge		604.32	19.57 В	ottom of Screen	
Type of Backfill Material: <u>n/a</u>	(if applicable)	=	603.70		ottom of Well	
Installation Method:	, ,		603.70	20.19 B	ottom of Boreho	le
			* Referenced to a	National Geodetic Da	atum	
			CAS	ING MEASU	REMENTS	
WELL CONS	STRUCTION MATERIALS	Б	Diameter of Boreho	ole	(inches)	8.0
	e type of material for each area)		D of Riser Pipe	41-		2.0
			rotective Casing L tiser Pipe Length	ength	(feet)	5.0 17.82
Protective Casing	SS304 SS316 PTFE PVC OTHER:	G: 1	Bottom of Screen to	End Cap	(feet)	0.55
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER:		creen Length (1s	-	(feet)	4.48
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER:		otal Length of Cas		(feet)	22.85

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ			Well C	Completion	Report	
Site #:	County:			Well	#: <u>TA</u>	32
Site Name: Coffeen Power Sta	ation			Bore	ehole#:T	A32
State Plane Coordinate: X 2,513,605	5.2 Y 877,532.6 (or) Latitude	39°	4' 26.730"	Longitude:	89° 24	8.000"
Surveyed By: Gary C. Rogers		IL Registr	ation #:035-00	02957		
Drilling Contractor: Ramsey		Driller: _	B. Williamson			
Consulting Firm: Hanson Profe	essional Services Inc.	Geologist:	Rhonald W.	Hasenyager, I	LPG #196-0002	246
Drilling Method: Hollow Stem	ı Auger	Drilling F	luid (Type):no	ne		
Logged By: Rhonald W. Hase	enyager	Date Start	ed:10/27/20)14 Date F	inished: 10/2	27/2014
	nonald W. Hasenyager		11/5/2014			
ANNULAR SPA			Elevations	Depths	(0.01 ft.)	
			(MSL)* _621.76_	(BGS) -2.83 T	op of Protective	Casina
	T				•	
			621.42	2.49 T	op of Riser Pipe	
Type of Surface Seal: Concrete			618.93	0.00 G	Fround Surface	
Town of Associate Contract.			616.03		op of Annular So	ealant
Type of Annular Sealant: Bento	J.					
Installation Method: <u>tremie</u>		∇	604.88	14.05 S	tatic Water Leve	1
Setting Time: 30 min.		∇	_004.88		(After Completion)	
Type of Bentonite Seal Gran	ular Pellet Slurry (choose one)					
Installation Method: <u>Gravit</u>	` ' '	V V	611.09	7.84 T	op of Seal	
Setting Time: 35 min.			609.18	9.75 T	op of Sand Pack	
	V	V V		1	op of Sund Fuck	
Type of Sand Pack: Quartz San			607.62	11.31 т	op of Screen	
Grain Size: 10/20 (sign					ър от жето	
Installation Method: <u>Gravit</u>	<u>y</u>		603.25	15.68 B	Sottom of Screen	
Type of Backfill Material:n/a_	(if applicable)		602.46	16.47 B	Bottom of Well	
Installation Method:			602.46	16.47 B	Sottom of Boreho	le
			* Referenced to a	National Geodetic D	atum	
			CAS	ING MEASU	REMENTS	
WELL CONS	STRUCTION MATERIALS		viameter of Boreho	ole	(inches)	8.0
	the type of material for each area)		O of Riser Pipe	anath		5.0
			rotective Casing L iser Pipe Length	ength	(feet)	13.94
Protective Casing	SS304 SS316 PTFE PVC OTHER:	a. 1	ottom of Screen to	End Cap	(feet)	0.65
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC OTHER:		creen Length (1s		(feet)	4.37
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OTHER:		otal Length of Cas		(feet)	18.96

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ	mental Protection Age	ency			Well	Complet	ion Report	
Site #:	County	: Montgome	ry		W	/ell #:	TA33	
Site Name: Coffeen Well Seal	ing & Assmt Well Install				Во	orehole #:	TA33b	
State Plane Coordinate: X 2,513,248	3.7 Y 876,605.4 (or) I	Latitude: 39°	4'	17.500"	Longitude	e: <u>89°</u> _	24' 12.700"	
Surveyed By: Gary C. Rogers		IL Re	gistration #	035-00)2957			
Drilling Contractor: Ramsey		Drille	Driller: B. Williamson					
Consulting Firm: Hanson Profe	essional Services Inc.	Geold	gist: Rho	onald W. 1	Hasenyager	; LPG #196-	000246	
Drilling Method: Hollow Stem	Auger	Drillin	ng Fluid (Ty	rpe): <u>No</u>	ne			
Logged By: Suzanna L. Keim		Date	Started:	6/2/201	5 Date	e Finished:	6/2/2015	
Report Form Completed By: Su	zanna L. Keim	Date:	6/4	4/2015				
ANNULAR SPA	CE DETAILS			vations ISL)*	Depths (BGS)	(0.01	ft.)	
			`	25.05	2.54	Top of Protec	ctive Casing	
			62	25.27	2.76	Top of Riser	Pipe	
Type of Surface Seal: Concrete			= >> <u>62</u>	22.51	0.00	Ground Surfa	ace	
Type of Annular Sealant: High-s	solids bentonite		_62	20.51	2.00	Top of Annu	lar Sealant	
Installation Method:Tremie	>							
Setting Time:>48 hrs.			6.	15.51	7.00	Static Water (After Complet	Level ion) 6/16/2015	
Type of Bentonite Seal Gran			=					
Installation Method: Gravit	(choose one)		6.	14.51_	8.00	Top of Seal		
Setting Time: 30 minutes			_6.	12.11	_10.40_	Top of Sand	Pack	
Type of Sand Pack: Quartz sand	<u> </u>							
Grain Size: 10-20 (sie	ve size)		_63	10.28	12.23	Top of Scree	1	
Installation Method: Gravit	у		-		4.5.00			
Type of Backfill Material:n/a	(if applicable)			05.62 05.07	16.89 17.44	Bottom of Sc Bottom of W		
Installation Method:			60	05.07	17.44	Bottom of Bo	orehole	
			* R	eferenced to a	National Geodeti			
				CAS	ING MEAS	SUREMENT	S	
WELL CONG	TDIICTIONI MATERIAI C		Diameter	of Boreho	le	(inc	hes) 7.5	
	TRUCTION MATERIALS e type of material for each area)		ID of Ris	-			hes) 2.0	
				e Casing L			feet) 5.0	
Protective Casing	SS304 SS316 PTFE PVC	OTHER: Steel		e Length	F., 1 C.,	•	feet) 14.99	
Riser Pipe Above W.T.	SS304 SS316 PTFE (PVC)	OTHER: GLECT			End Cap t slot to last slot		feet) 0.55 feet) 4.66	
Riser Pipe Below W.T.		OTHER:		ngth of Cas		•	feet) 4.66 feet) 20.20	

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

SS304

Well Completion Form (revised 02/06/02)

SS316

Illinois Environ	mental Protection	Agency				Well	Completi	on Report
Site #:	C	ounty: Mor	ntgomery	Į.		W	/ell #:	TA34
Site Name: Coffeen Well Seal	ing & Assmt Well Install					В	orehole #:	TA34
State Plane Coordinate: X 2,513,466	6.7 Y <u>875,906.1</u> (o	or) Latitude:	39°		10.500"	Longitud	e: <u>89°</u> _	24' 10.000"
Surveyed By: Gary C. Rogers			IL Regi	istrati	on #: <u>035-0</u>	02957		
Drilling Contractor: Ramsey			Driller:	B	. Williamson			
Consulting Firm: Hanson Profe	essional Services Inc.		Geolog	gist: _	Rhonald W.	Hasenyager	c, LPG #196-0	000246
Drilling Method: Hollow Stem	Auger		Drilling	g Flui	d (Type): <u>No</u>	one		
Logged By: Suzanna L. Keim			Date St	tarted	:6/3/201	15 Date	e Finished:	6/3/2015
Report Form Completed By: Su	zanna L. Keim		Date: _		6/4/2015			
ANNULAR SPA	CE DETAILS				Elevations (MSL)*	Depths (BGS)	(0.01	ft.)
					626.77	2.67	Top of Protec	tive Casing
		T			626.52	-2.42	-	_
Type of Surface Seal: Concrete				>>	624.10	0.00	Ground Surfa	ce
Type of Annular Sealant: Benton	nite chips				623.10	1.00	Top of Annul	ar Sealant
Installation Method: <u>Gravit</u>	y	_ 7						
Setting Time: 1 hour		_ ½	$\mathbb{Z} \mid \cdot \mid$		616.00	8.10	Static Water 1 (After Completi	
Type of Bentonite Seal Gran	~							
Installation Method:n/a	(choose one)	_			n/a	n/a	Top of Seal	
Setting Time:n/a		-			615.10	9.00	Top of Sand l	Pack
Type of Sand Pack: Quartz sand	1	_						
Grain Size: 10-20 (sie	eve size)				613.18	10.92	Top of Screen	1
Installation Method: <u>Gravit</u>	у	_			600.60	1.7.41	D 69	
Type of Backfill Material:n/a	(if applicable)				608.69	15.41 16.10	Bottom of Sci Bottom of We	
Installation Method:					608.00	16.10	Bottom of Bo	rehole
					* Referenced to a	National Geodet	ic Datum	
					CAS	SING MEAS	SUREMENT	S
WELL CONS	STRUCTION MATERIAL	S			meter of Boreho	ole	(incl	
	e type of material for each area)				of Riser Pipe	41	(incl	
					tective Casing L			(eet) 5.0
Protective Casing	SS304 SS316 PTFE P	VC OTHER:	Steel		er Pipe Length tom of Screen to		•	(eet) 13.34 (eet) 0.69
Riser Pipe Above W.T.	SS304 SS316 PTFE P	VC OTHER:			en Length (1s		•	eet) 0.09
Riser Pipe Below W.T.	SS304 SS316 PTFE P	OTHER:			al Length of Cas			eet) 18.52

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

SS304

Well Completion Form (revised 02/06/02)

SS316

Illinois Enviro	onmental Protecti	on Agen	cy		Well (Completion	n Report
Site #:	Cou	unty:				Well #:Т	`R32
Site Name: Coffeen Power S	Station				E	Borehole #:	TR32
State Plan&oordinate: X_2,513,605	. <u>0</u> Y <u>877,523.7</u> (or	·) Latitude:	39°	<u>4'</u> 266	53.000"Longitu	de: 89° 2	24' 8.070"
Surveyed By: Kyle J. Nolan			IL Reg	stration #:	035-003919		
Drilling Contractor: Ramsey	Geotechnical Engineerin	ıg, LLC	Driller	: <u>B. Willian</u>	nson		
Consulting Firm: Hanson Pr	ofessional Services Inc.		Geolog	ist: <u>Rhonal</u>	d W. Hasenya	nger, LPG #196	-000246
Drilling Method: Hollow Ste	m Auger		Drillin	g Fluid (Type)	: _ none		
Logged By: Rhonald W. Has	enyager		Date S	arted:7/2	2/2019 Da	te Finished:7	//2/2019
Report Form Completed By:I	Rhonald W. Hasenyager		Date:	7/3/20	19		
ANNULAR SPA	CE DETAILS			Elevatio	ons Depths * (BGS)	(0.01 f	t.)
				621.97	,	Top of Protect	ive Casing
			\top		_	-	
				621.68	3 -2.40	Top of Riser Pi	pe
Type of Surface Seal: <u>Concrete</u>	<u> </u>		Y D	619.28	<u>0.00</u>	Ground Surfac	e
Type of Annular Sealant: Bent	ronite			616.28	3.00	Top of Annula	r Sealant
Installation Method: Grav		.	14				
Setting Time: 30 min.		· <u>\</u>	7			Static Water L	evel
Setting Time		- -	-		_	(After Completio	
Type of Bentonite Seal Gran	ular Pellet Slurry						
Installation Method:		- V V	××	n/a	n/a	Top of Seal	
Setting Time:				609.77	7 9.51	Top of Sand Pa	ıck
		V		<u></u>		_ rop or build re	
Type of Sand Pack: Quartz Sa		-		608.28	3 11.00	Top of Screen	
Grain Size: 10/20 (sie	ve size)			000.20	<u> </u>	_ Top of sereen	
Installation Method: <u>Grav</u>	ity	·		603.60) 15.68	Bottom of Scre	oen.
Type of Backfill Material:n/a		. 🗏		603.11		-	
Y . H .: W.I I	(if applicable)			602.11	1617	D (D	
Installation Method:				603.11 * Reference	$\frac{16.17}{160}$ ed to a National Geo	-	enoie
					CASING MEA	SUREMENTS	
				Diameter of F		(inches	8.0
	TRUCTION MATERIALS type of material for each area)			ID of Riser Pi			
				Protective Ca	sing Length	(feet	5.0
D				Riser Pipe Le		(feet	
Protective Casing	SS304 SS316 PTFE PV		eel		reen to End Cap		
Riser Pipe Above W.T. Riser Pipe Below W.T.		OTHER:			h (1st slot to last		
Maci Tipe Delow W.I.	SUSUT SUSTO TIFE (FV	U DITIER.		Total Length	of Casing	(feet	18.57

SS304

Well Completion Form (revised 02/06/02)

SS316

Total Length of Casing

**Hand-Slotted Well Screens Are Unacceptable

Screen Slot Size **

0.010

Illinois Environ	nmental Protection A	Agency				Well	Comple	etion !	Report
Site #:	Coun	nty: <u>Mon</u>	tgomery	у			Well #:	G30′	7D
Site Name: Coffeen Part 845	Groundwater]	Borehole #: _	G3	07D
State Plane Coordinate: X 2,515,560							de:		
Surveyed By: Kyle J. Nolan					on #:035-00				
Drilling Contractor: Roberts En	nv. Drilling Inc.		Driller:	: <u>N</u>	S att				
Consulting Firm: Hanson Prof	essional Services Inc.		Geolog	gist: _	Rhonald W.	Hasenyag	er, LPG #19	06-0002	46
Drilling Method: Hollow stem	auger		Drilling Fluid (Type): None						
Logged By: Colin Winter			Date St	tarted	:2/9/202	21 Da	te Finished:	2/9	/2021
Report Form Completed By:Co	lin Winter		Date:		5/3/2021				
ANNULAR SPA	CE DETAILS				Elevations (MSL)*	Depths (BGS)	(0.0	01 ft.)	
					625.29	2.78	Top of Pro	tective (Casing
			=		624.88	-2.37	Top of Ris	er Pipe	
Type of Surface Seal: Concrete				-	622.51	0.00	Ground Su	rface	
	_				621.51	1.00	_		alant
Type of Annular Sealant: <u>High-s</u>							- 1		
Installation Method: Tremie	:								
Setting Time: >24 hours			<u>Z</u>				Static Wate (After Com		
Type of Bentonite Seal Gran				-					
Installation Method: Gravity	(choose one)				575.51	47.00	Top of Sea	ıl	
Setting Time: 30 minutes					574.51	40.00	T CC	10.1	
-					574.51	48.00	_ Top of San	id Pack	
Type of Sand Pack: Quartz sand	<u> </u>				572 52	10.00	T		
Grain Size:10-20 (sie	ve size)				573.53	48.98	Top of Scr	een	
Installation Method: Gravity	7				562.76	58.75	Bottom of	Compan	
Type of Backfill Material: N/A					563.76 562.91	59.60	Bottom of		
Table Mala	(if applicable)				5(2.51	(0.00	D. ()	D 1.1	
Installation Method:						60.00 National Geode	Bottom of Etic Datum	Borehole	2
					CAS		ASUREMEN	JTC	
				Dia	meter of Boreho			inches)	8.0
	TRUCTION MATERIALS e type of material for each area)				of Riser Pipe		,	inches)	2.0
(,			Prot	tective Casing L	ength		(feet)	5.0
		_		Rise	er Pipe Length			(feet)	51.85
Protective Casing	SS304 SS316 PTFE PVC		teel	Bott	tom of Screen to	End Cap		(feet)	0.35
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC				een Length (1s		lot)	(feet)	9.77
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC	OTHER:		Tota	al Length of Cas	sing		(feet)	61.97

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ	nmental Protectio	n Agency				Well	Comple	tion Report
Site #:		County: <u>Mon</u>	tgomery	у			Well #:	G308
Site Name: Coffeen Part 845	Groundwater]	Borehole #:	G308
State Plane Coordinate: X 2,515,101	4 Y <u>871,454.7</u>	(or) Latitude:						
Surveyed By: Kyle J. Nolan			IL Regi	istrati	on #: <u>035-0</u>	03919		
Drilling Contractor: Roberts En	ıv. Drilling Inc.		Driller:	N	latt			
Consulting Firm: Hanson Profe	essional Services Inc.		Geolog	gist: _	Rhonald W.	Hasenyag	er, LPG #196	6-000246
Drilling Method: Hollow stem	auger		Drilling Fluid (Type): None					
Logged By: Colin Winter			Date St	tarted	:1/13/20	<u>21</u> Da	te Finished: _	1/13/2021
Report Form Completed By:Co	lin Winter		Date: _		5/3/2021			
ANNULAR SPA	CE DETAILS				Elevations (MSL)*	Depths (BGS)	(0.0)	1 ft.)
					624.96	-3.37	Top of Prot	ective Casing
					624.59	-3.00	Top of Rise	r Pipe
Type of Surface Seal: Concrete					621.59	0.00	Ground Sur	face
					621.39	0.20	_	
Type of Annular Sealant: Benton		— Ŋ					- 1	
Installation Method: Gravity	7	_ _						
Setting Time: >24 hours		\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	<u>Z</u>				Static Water (After Comp.)	
Type of Bentonite Seal Grant	ular Pellet Slurry	+		-				
Installation Method:	` '				n/a	n/a	Top of Seal	
Setting Time:					612.59	9.00	Top of Sano	l Pack
					012.57		_ Top of Sun	a r dok
Type of Sand Pack: Quartz Sand		_			611.49	10.10	Top of Scre	en
Grain Size: 10-20 (sie							- 1	
Installation Method: <u>Gravity</u>	7				606.70	14.89	Bottom of S	Screen
Type of Backfill Material: N/A	(if applicable)	_ [606.35	15.24	Bottom of V	Well
Installation Method:					605.79	15.80	Bottom of I	Borehole
					* Referenced to a	National Geode	etic Datum	
					CAS	SING MEA	ASUREMEN	TS
WELL CONG	TRUCTION MATERIA	I C		Dia	meter of Boreho	ole	(ii	nches) 8.0
	TRUCTION MATERIA e type of material for each area)	LS		ID o	of Riser Pipe		(ii	nches) 2.0
					ective Casing L	ength		(feet) 5.0
Protective Casing	SS304 SS316 PTFE	PVC OTHER: (S	teel		er Pipe Length			(feet) 13.10
Riser Pipe Above W.T.		PVC OTHER:			tom of Screen to			(feet) 0.35
Riser Pipe Below W.T.		PVC OTHER:			een Length (1s) al Length of Cas			(feet) 4.79 (feet) 18.24
_				100	Length of Car	8		10.21

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ	nmental Protection	n Agency				Well	Comple	tion Report
Site #:	C	County: Mon	itgomer	У			Well #:	G309
Site Name: Coffeen Part 845	Groundwater					E	Borehole #:	G309
State Plane Coordinate: X 2,515,067	7.1 Y 871,865.8 (c	or) Latitude:				Longitud	le:	
Surveyed By: Kyle J. Nolan			IL Reg	istration	#:035-0	03919		
Drilling Contractor: Roberts E	nv. Drilling Inc.		Driller:	Ma	tt			
Consulting Firm: Hanson Prof	essional Services Inc.		Geolog	ist:F	Chonald W.	Hasenyage	er, LPG #196	6-000246
Drilling Method: Hollow stem	auger		Drilling Fluid (Type): None					
Logged By: Colin Winter			Date S	tarted:	1/12/20	21 Dat	te Finished: _	1/12/2021
Report Form Completed By:Co	olin Winter		Date:		5/3/2021			
ANNULAR SPA	CE DETAILS				levations (MSL)*	Depths (BGS)	(0.0)	1 ft.)
					626.20	3.43	Top of Prote	ective Casing
				_	625.88	-3.11	Top of Rise	r Pipe
Type of Surface Seal: Concrete				: >	622.77	0.00	Ground Sur	face
				_	622.27	0.50	Top of Ann	ular Sealant
Type of Annular Sealant: Benton		- 🎵						
Installation Method: Gravity	ý.	_	_					
Setting Time: >24 hours		_ \[\frac{1}{2} \]	∠	-		-	Static Water (After Compl	
Type of Bentonite Seal Gran	ular Pellet Slurry (choose one)							
Installation Method:		_		-	n/a	n/a	Top of Seal	
Setting Time:		_		_	610.77	_12.00	Top of Sano	d Pack
Type of Sand Pack: Quartz sand								
Grain Size: 10-20 (sie		_ _		_	609.80	12.97	Top of Scre	en
Installation Method: Gravity								
				_	605.02	17.75		
Type of Backfill Material: N/A	(if applicable)	_ _		-	604.67	18.10	Bottom of V	Vell
Installation Method:				_	604.67	<u> 18.10</u>	Bottom of E	Borehole
				*	Referenced to a	National Geode	tic Datum	
					CAS	ING MEA	SUREMEN	TS
WELL CONS	TRUCTION MATERIAL	LS			eter of Boreho		,	nches) 8.0
	ne type of material for each area)				Riser Pipe	anath	`	(feet) 2.0
					tive Casing L Pipe Length	_		(feet) 5.0 (feet) 16.08
Protective Casing	SS304 SS316 PTFE I	PVC OTHER: S	teel		n of Screen to			(feet) 0.35
Riser Pipe Above W.T.	SS304 SS316 PTFE I	PVC OTHER:			Length (1s			(feet) 4.78
Riser Pipe Below W.T.	SS304 SS316 PTFE I	PVC OTHER:			Length of Cas			(feet) 21.21

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ	nmental Protectio	n Agency				Well	Complet	ion Report
Site #:		County: <u>Mon</u>	tgomery	у			Well #:	G310
Site Name: Coffeen Part 845	Groundwater					F	Borehole #:	G310
State Plane Coordinate: X 2,515,159	9.4 Y <u>872,239.4</u> ((or) Latitude:				Longitud	le:	
Surveyed By: Kyle J. Nolan			IL Reg	istratio	on #: <u>035-0</u>	03919		
Drilling Contractor: Roberts E	nv. Drilling Inc.		Driller:	: <u>M</u>	att			
Consulting Firm: Hanson Prof	essional Services Inc.		Geolog	gist: _	Rhonald W.	Hasenyage	er, LPG #196	-000246
Drilling Method: Hollow stem	auger		Drilling Fluid (Type): None					
Logged By: Colin Winter			Date St	tarted:	1/15/20	21 Da	te Finished:	1/15/2021
Report Form Completed By:Co	olin Winter		Date: _		5/3/2021			
ANNULAR SPA	CE DETAILS]	Elevations (MSL)*	Depths (BGS)	(0.01	ft.)
					623.32	3.43	Top of Prote	ctive Casing
			$=$ $\begin{bmatrix} & & & & & & & & & & & & & & & & & & &$		622.87	-2.98	Top of Riser	Pipe
Type of Surface Seal: Concrete					619.89_	0.00	Ground Surf	ace
					619.39	0.50	Top of Annu	ılar Sealant
Type of Annular Sealant: Benton		_ }						
Installation Method: Gravity	У	_	7				Static Water	I aval
Setting Time:>24 hours		_ _	_				(After Comple	
Type of Bentonite Seal Gran	ular Pellet Slurry			-				
Installation Method:	` '	_	\boxtimes		n/a	n/a	Top of Seal	
Setting Time:		_ 🐰			610.69	9.20	Top of Sand	Pack
Type of Sand Pack: Quartz sand								
Grain Size: 10-20 (sie					609.65	10.24	Top of Scree	n
Installation Method: Gravity								
			≣		604.86	15.03	•	
Type of Backfill Material: N/A	(if applicable)	_ _			604.51	15.38	Bottom of W	/ell
Installation Method:					603.99 * Referenced to a	15.90 National Geode	Bottom of B	orehole
							SUREMENT	
WELL CONS	TRUCTION MATERIA	LS			neter of Boreho			ehes) 8.0
(Choose or	ne type of material for each area)				f Riser Pipe	enoth	`	thes) 2.0 feet) 5.0
					ective Casing L r Pipe Length	-		feet) 5.0 feet) 13.22
Protective Casing	SS304 SS316 PTFE	PVC OTHER: S	teel		om of Screen to			feet) 13.22 feet) 0.35
Riser Pipe Above W.T.	SS304 SS316 PTFE	PVC OTHER:			en Length (1s	•		feet) 4.79
Riser Pipe Below W.T.	SS304 SS316 PTFE	PVC OTHER:			l Length of Cas			feet) 18.36

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ	nmental Protection	n Agency				Well	Complet	ion Report
Site #:		County: Mon	tgomer	<u>y</u>			Well #:	G311
Site Name: Coffeen Part 845	Groundwater					I	Borehole #:	G311
State Plane Coordinate: X 2,515,881	8 Y 872,238.7	(or) Latitude:				Longitue	de:	
Surveyed By: Kyle J. Nolan			IL Reg	gistrati	ion #:035-0	03919		
Drilling Contractor: Roberts En	nv. Drilling Inc.		Driller:	: <u>N</u>	<u> latt</u>			
Consulting Firm: Hanson Prof	essional Services Inc.		Geolog	gist: _	Rhonald W.	Hasenyage	er, LPG #196	-000246
Drilling Method: Hollow stem	auger		Drilling Fluid (Type): None					
Logged By: Colin Winter			Date S	tarted	:2/5/202	21 Da	te Finished: _	2/5/2021
Report Form Completed By:Co	lin Winter		Date:		5/3/2021			
ANNULAR SPA	CE DETAILS				Elevations (MSL)*	Depths (BGS)	(0.01	ft.)
					621.55	3.23	Top of Prote	ctive Casing
					621.04	-2.72	Top of Riser	Pipe
Type of Surface Seal: Concrete					618.32	0.00	Ground Surf	ace
					617.52	0.80	Top of Annu	ılar Sealant
Type of Annular Sealant: Bentor		_						
Installation Method: Gravity	7		7				Static Water	I1
Setting Time:		_	_				(After Comple	
Type of Bentonite Seal Grant	alar Pellet Slurry		Y	-				
Installation Method:	,		X X		n/a	n/a	Top of Seal	
Setting Time:					610.12	8.20	Top of Sand	Pack
Town of Court Pools		<u> </u>					- 1	
Type of Sand Pack: Quartz sand Grain Size: 10-20 (sie		- $ $ $ $			609.05	9.27	Top of Scree	en
Installation Method: Gravity								
					604.28	14.04	Bottom of S	creen
Type of Backfill Material: N/A	(if applicable)	L			603.92	14.40	Bottom of W	/ell
Installation Method:					603.92	14.40	Bottom of B	orehole
					* Referenced to a	National Geode	tic Datum	
					CAS	SING MEA	SUREMEN	ΓS
WELL CONS	TRUCTION MATERIA	LS			meter of Boreho	ole	,	ches) 8.0
	e type of material for each area)				of Riser Pipe	au atla		ches) 2.0
					tective Casing L er Pipe Length	-		(feet) 5.0 (feet) 11.99
Protective Casing	SS304 SS316 PTFE	PVC OTHER: S	teel		tom of Screen to			(feet) 11.99 (feet) 0.36
Riser Pipe Above W.T.	SS304 SS316 PTFE (PVC OTHER:	-		een Length (1s			(feet) 4.77
Riser Pipe Below W.T.	SS304 SS316 PTFE (PVC OTHER:			al Length of Cas		•	(feet) 17.12

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ	nmental Protection Ag	gency				Well	Comple	tion]	Report
Site #:	County	: Mont	tgomery	y			Well #:	G311	D
Site Name: Coffeen Part 845	Groundwater]	Borehole #: _	G3	11D
State Plane Coordinate: X 2,515,881							de:		
Surveyed By: Kyle J. Nolan			IL Reg	istrat	ion #:035-00	03919			
Drilling Contractor: Roberts En	nv. Drilling Inc.		Driller:	: <u>N</u>	<u>latt</u>				
Consulting Firm: Hanson Prof	essional Services Inc.		Geolog	gist: _	Rhonald W.	Hasenyag	er, LPG #19	6-0002	46
Drilling Method: Hollow stem	auger		Drilling Fluid (Type): None						
Logged By: Colin Winter			Date St	tarted	l: <u>2/5/202</u>	21 Da	te Finished: _	2/5/	2021
Report Form Completed By:Co	lin Winter		Date:		5/3/2021				
ANNULAR SPA	CE DETAILS				Elevations (MSL)*	Depths (BGS)	(0.0)	01 ft.)	
					621.75	3.36	Top of Pro	tective C	Casing
					621.24	-2.85	Top of Rise	er Pipe	
Type of Surface Seal: Concrete				-	618.39	0.00	Ground Su	rface	
					617.09	1.30	Top of Anr	nular Sea	ılant
Type of Annular Sealant: High-s			R						
Installation Method:Tremie			,					· .	
Setting Time: 1 hour	_		_				Static Water (After Comp		
Type of Bentonite Seal Gran				-					
Installation Method: Gravity	(choose one)	V V	V. V		572.39	46.00	Top of Sea	1	
Setting Time: 30 minutes					569.39	49.00	Top of San	d Pack	
T. (G. 1D.)							_ Top or Sun	a r uen	
Type of Sand Pack: Quartz sand					_568.23_	50.16	Top of Scr	een	
Grain Size: 10-20 (sie Installation Method: Tremie									
instanation Method. Tremie					558.29	60.10	Bottom of	Screen	
Type of Backfill Material: N/A	(if applicable)		_		557.81	60.58	Bottom of	Well	
Installation Method:					557.81	60.58	Bottom of	Borehole	e
					* Referenced to a l	National Geode	etic Datum		
					CAS	ING MEA	SUREMEN	NTS	
WELL CONS	TRUCTION MATERIALS			Dia	meter of Boreho	ole	(i	inches)	6.0
	type of material for each area)				of Riser Pipe			inches)	2.0
					tective Casing L	ength		(feet)	5.0
Protective Casing	SS304 SS316 PTFE PVC	OTHER: (St	eel		er Pipe Length tom of Screen to	End Com		(feet)	53.01 0.48
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC	OTHER:			een Length (1s			(feet)	9.94
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC	OTHER:			al Length of Cas		/	(feet)	63.43

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ	nmental Protection	n Agency				Well	Complet	tion Report
Site #:		County: <u>Mon</u>	tgomer	у			Well #:	G312
Site Name: Coffeen Part 845	Groundwater					I	Borehole #:	G312
State Plane Coordinate: X 2,516,557	7.4 Y <u>872,260.9</u> (or) Latitude:				Longitue	de:	
Surveyed By: Kyle J. Nolan			IL Reg	gistratio	on #:035-0	03919		
Drilling Contractor: Roberts Ex	nv. Drilling Inc.		Driller:	: <u>M</u>	att			
Consulting Firm: Hanson Prof	essional Services Inc.		Geolog	gist: _	Rhonald W.	Hasenyage	er, LPG #196	5-000246
Drilling Method: Hollow stem	auger		Drilling Fluid (Type): None					
Logged By: Colin Winter			Date S	tarted:	2/4/202	21 Da	te Finished: _	2/4/2021
Report Form Completed By:Co	lin Winter		Date:		5/3/2021			
ANNULAR SPA	CE DETAILS]	Elevations (MSL)*	Depths (BGS)	(0.0)	1 ft.)
					620.11	3.19	_ Top of Prote	ective Casing
					619.78	-2.86	_ Top of Rise	r Pipe
Type of Surface Seal: Concrete				-	616.92	0.00	Ground Sur	face
					615.92	1.00	_	
Type of Annular Sealant: Benton							- 1	
Installation Method: Gravity	/	_ _						
Setting Time: >24 hours		_ \[\sum_{\sum}	<u>Z</u>				_ Static Water (After Compl	
Type of Bentonite Seal Gran	ular Pellet Slurry (choose one)			-				
Installation Method:		_	\boxtimes		n/a	n/a	Top of Seal	
Setting Time:		_ 🐰			608.92	8.00	Top of Sand	l Pack
Type of Sand Books							-	
Type of Sand Pack: Quartz sand Grain Size: 10-20 (sie		_ _			607.13	9.79	Top of Scree	en
Installation Method: Gravity								
instanation victiod. Oravity			∄│		602.34	14.58	Bottom of S	creen
Type of Backfill Material: N/A	(if applicable)	_ _			601.99	14.93	_ Bottom of V	Vell
Installation Method:					601.67	15.25	Bottom of B	Sorehole
					* Referenced to a	National Geode	etic Datum	
					CAS	SING MEA	ASUREMEN'	ΓS
WELL CONS	TRUCTION MATERIA	1.5		Diar	neter of Boreho	ole	(in	ches) 8.0
	type of material for each area)	L.S			of Riser Pipe		`	ches) 2.0
					ective Casing L	<u>-</u>		(feet) 5.0
Protective Casing	SS304 SS316 PTFE	PVC OTHER: S	teel		er Pipe Length om of Screen to			(feet) 12.65 (feet) 0.35
Riser Pipe Above W.T.		PVC OTHER:			en Length (1s			(feet) 0.35 (feet) 4.79
Riser Pipe Below W.T.	SS304 SS316 PTFE	PVC OTHER:			l Length of Cas			(feet) 4.79 (feet) 17.79

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Enviro	nmental Protection A	gency				Well	Complet	tion 1	Report
Site #:	Count	y: <u>Mont</u>	gomery	7		v	Vell #:	G31	3
Site Name: Coffeen Part 845	Groundwater					E	Sorehole #:	G3	313
State Plane Coordinate: X 2,516,80	3.7 Y 871,976.8 (or)	Latitude: _				Longitud	le:		
Drilling Contractor: Roberts I	Env. Drilling Inc.		Driller:	Mat	t				
	fessional Services Inc.						er, LPG #196		
	n auger								
_									
Report Form Completed By:C					5/3/2021		_		
ANNULAR SPA			Date: _	El	evations	Depths	(0.0)	1 ft.)	
					(MSL)*	(BGS)			
		\Box	$\overline{}$	_	614.62	3.11_	Top of Prote	ective C	asing
			\exists	_	614.30	2.79	Top of Rise	r Pipe	
Type of Surface Seal: Concrete				>	611.51	0.00	Ground Sur	face	
	,			/ _	611.11	0.40	Top of Ann	ular Sea	ılant
Type of Annular Sealant: Bento			19						
Installation Method: <u>Gravi</u>									
Setting Time: >24 hours			-	_			Static Water (After Compl		
Type of Bentonite Seal Gran	nular Pellet Slurry								
Installation Method:	(choose one)				n/a	n/a	Top of Seal		
				_	II u		rop or sear		
Setting Time:				_	606.51	5.00	Top of Sand	d Pack	
Type of Sand Pack: Quartz san	d								
Grain Size: <u>10-20</u> (s	ieve size)			-	605.21	6.30	Top of Scre	en	
Installation Method: Gravi	ty								
T CD 1CHM : 1 N/A				_	600.40	11.11			
Type of Backfill Material: N/A	(if applicable)			_	600.05	11.46	Bottom of V	Vell	
Installation Method:				_	599.51	12.00		Borehole	;
				*	Referenced to a	National Geodet	ic Datum		
					CAS	SING MEA	SUREMEN	TS	
*****	OFFICE ON A CONTRACT OF			Diame	ter of Boreho	ole	(ir	nches)	8.0
	STRUCTION MATERIALS one type of material for each area)			ID of I	Riser Pipe		(ir	nches)	2.0
					tive Casing I	ength		(feet)	5.0
Protective Cosine	SS304 SS316 PTFE PVC	OTHER: (Sto	aal		Pipe Length			(feet)	9.09
Protective Casing Riser Pipe Above W.T.	SS304 SS316 PTFE PVC SS304 SS316 PTFE PVC				of Screen to			(feet)	0.35
Riser Pipe Below W.T.		OTHER:	-+		Length (1:		•	(feet)	4.81 14.25
	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1			1 Otal I	ength of Ca	sing		(feet)	14.43

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ	nmental Protection	Agency				Well	Comple	tion Report		
Site #:	Cc	ounty: <u>Mon</u>	tgomery	y			Well #:	G314		
Site Name: Coffeen Part 845	Groundwater					I	Borehole #:	G314		
State Plane Coordinate: X 2,516,852	2.1 Y 871,630.2 (or	·) Latitude:				Longitue	de:			
Surveyed By: Kyle J. Nolan			IL Registration #:035-003919							
Drilling Contractor: Holcomb	Foundation Engineering C	0.	Driller:	Ste	eve					
Consulting Firm: Hanson Prof	essional Services Inc.		Geolog	ist:]	Rhonald W.	Hasenyage	er, LPG #196	6-000246		
Drilling Method: Hollow stem	auger		Drilling Fluid (Type): None							
Logged By: Colin Winter			Date St	tarted:	2/26/20	21 Da	te Finished: _	2/26/2021		
Report Form Completed By:Co	lin Winter		Date: _		5/3/2021					
ANNULAR SPA	CE DETAILS			E	levations (MSL)*	Depths (BGS)	(0.0)	1 ft.)		
					614.28	-3.17	Top of Prot	ective Casing		
		T			613.88	2.77	Top of Rise	er Pipe		
Type of Surface Seal: Concrete					611.11	0.00	Ground Sur	face		
				7	609.11	2.00	-	ular Sealant		
Type of Annular Sealant: Benton	nite chips	-			009.11		_ 100 01 711111	ulai Scalaili		
Installation Method: Gravity	<i>I</i>	-								
Setting Time: 1 hour		_ \[\breez	<u> </u>				Static Water (After Comp			
Type of Bentonite Seal Gran	,									
Installation Method:	(choose one)	_			n/a	n/a	Top of Seal			
Setting Time:					507.61	12.50	Ton of Son	d Dools		
					597.61	13.30_	Top of Sand	1 Раск		
Type of Sand Pack: Quartz sand	<u> </u>	-			596.55	14.56	Top of Scre	an		
Grain Size: 10-20 (sie					370.33	14.50_	_ 10p of sere	CII		
Installation Method: <u>Gravity</u>	I	-			591.53	19.58	Bottom of S	Screen		
Type of Backfill Material: N/A	(if applicable)	_ E			591.09	20.02	Bottom of V			
Installation Method:					591.06	20.05	Bottom of I	Rorehole		
instantation Wethod.		_			* Referenced to a		-	Jorenoie		
					CAS	SING MEA	SUREMEN	TS		
				Diam	eter of Boreho			nches) 8.0		
	TRUCTION MATERIALS e type of material for each area)	S		ID of	Riser Pipe		(ii	nches) 2.0		
				Prote	ctive Casing L	ength		(feet) 5.0		
D + +: - C :	ggaga ggaga				Pipe Length			(feet) 17.30		
Protective Casing		VC OTHER: S	teel		m of Screen to			(feet) 0.47		
Riser Pipe Above W.T. Riser Pipe Below W.T.		VC OTHER:			n Length (1s			(feet) 5.02		
Misci I the Delow W.1.	55507 55510 FIFE (P	OTHER.		Lotal	Length of Cas	sıng		(feet) 22.79		

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ	nmental Protection A	Agency				Well	Comple	tion	Report
Site #:	Cour	nty: <u>Mon</u>	tgomery	у			Well #:	G314	4D
Site Name: Coffeen Part 845	Groundwater					I	Borehole #: _	G3	14D
State Plane Coordinate: X 2,516,853	8.9 Y 871,642.0 (or)	Latitude:				Longitue	de:		
Surveyed By: Kyle J. Nolan			IL Reg	gistrat	ion #:035-00	03919			
Drilling Contractor: Roberts En	nv. Drilling Inc.		Driller:	: <u>N</u>	<u> latt</u>				
Consulting Firm: Hanson Prof	essional Services Inc.		Geolog	gist: _	Rhonald W.	Hasenyag	er, LPG #19	6-0002	46
Drilling Method: Hollow stem	auger		Drilling Fluid (Type): None						
Logged By: Colin Winter			Date St	tarted	1:2/10/20	21 Da	te Finished: _	2/12	2/2021
Report Form Completed By:Co	lin Winter		Date: _		5/3/2021				
ANNULAR SPA	CE DETAILS				Elevations (MSL)*	Depths (BGS)	(0.0)	01 ft.)	
					614.10	-3.23	Top of Pro	tective (Casing
		T			613.70	-2.83	Top of Rise	er Pipe	
Type of Surface Seal: Concrete				- - ~	610.87	0.00	Ground Su	rface	
	_				608.87	2.00	-		alant
Type of Annular Sealant: High-s	_						- 1		
Installation Method: Tremie									
Setting Time: >24 hours			<u> </u>				Static Water (After Comp		
Type of Bentonite Seal Grant				-					
Installation Method: Gravity	(choose one)				574.07	36.80	Top of Sea	1	
Setting Time: 30 minutes					572.97	37 90	Top of San	d Dack	
							_ Top or Sun	u i ack	
Type of Sand Pack: Quartz sand					571.53	39.34	Top of Scr	een	
Grain Size: 10-20 (sie							F		
Installation Method: <u>Gravity</u>					_561.76_	49.11	Bottom of	Screen	
Type of Backfill Material: N/A	(if applicable)				561.40	49.47	-	Well	
Installation Method:					510.57	100.30	Bottom of 1	Borehol	e
					* Referenced to a		-		
					CAS	ING MEA	SUREMEN	NTS	
WELL COM	TRUCTION MATERIAL C			Dia	meter of Boreho	ole	(i	inches)	8.0
	TRUCTION MATERIALS e type of material for each area)			ID	of Riser Pipe		(i	inches)	2.0
					tective Casing L	ength		(feet)	5.0
Protective Cosing	SS304 SS316 PTFE PVC	OTHER: (S	taal		er Pipe Length			(feet)	42.17
Protective Casing Riser Pipe Above W.T.	SS304 SS316 PTFE PVC SS304 SS316 PTFE PVC				tom of Screen to			(feet)	0.36
Riser Pipe Below W.T.		OTHER:			een Length (1s al Length of Cas		lot)	(feet)	9.77 52.30
-				101	ai Longin oi Cas	,111 <u>5</u>		(1001)	J2.JU

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ	nmental Protectio	n Agency				Well	Complet	tion Report
Site #:		County: <u>Mon</u>	tgomery	у			Well #:	G315
Site Name: Coffeen Part 845	Groundwater					I	Borehole #:	G315
State Plane Coordinate: X 2,516,086	5.6 Y <u>871,385.0</u>	(or) Latitude:				Longitue	de:	
Surveyed By: Kyle J. Nolan			IL Regi	istrati	on #: <u>035-0</u>	03919		
Drilling Contractor: Roberts En	nv. Drilling Inc.		Driller:	: <u>N</u>	latt			
Consulting Firm: Hanson Prof	essional Services Inc.		Geolog	gist: _	Rhonald W.	Hasenyag	er, LPG #196	6-000246
Drilling Method: Hollow stem	auger		Drilling Fluid (Type): None					
Logged By: Colin Winter			Date St	tarted	:1/13/20	21 Da	te Finished: _	1/13/2021
Report Form Completed By:Co	lin Winter		Date: _		5/3/2021			
ANNULAR SPA	CE DETAILS				Elevations (MSL)*	Depths (BGS)	(0.0)	1 ft.)
					623.89	2.95	Top of Prote	ective Casing
					623.52	-2.58	Top of Rise	r Pipe
Type of Surface Seal: Concrete					620.94	0.00	Ground Sur	face
					620.54	0.40	•	
Type of Annular Sealant: Benton	nite chips				020.31		_ 10p 0171mi	aidi Sedidiri
Installation Method: <u>Gravity</u>	7	_						
Setting Time: >24 hours		\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	<u>Z</u>				Static Water (After Compl	
Type of Bentonite Seal Grant	ular Pellet Slurry			-				
Installation Method:	,				n/a	n/a	Top of Seal	
Setting Time:					612.64	8.30	Top of Sand	l Pack
		V					_ Top of Suite	- 1 4441
Type of Sand Pack: Quartz sand		_			611.25	9.69	Top of Scre	en
Grain Size: 10-20 (sie							. 1	
Installation Method: <u>Gravity</u>	7				606.46	14.48	Bottom of S	Screen
Type of Backfill Material: N/A	(if applicable)	_ [606.09	14.85	Bottom of V	Well
Installation Method:					605.04	15.90	Bottom of E	Borehole
					* Referenced to a	National Geode	tic Datum	
					CAS	SING MEA	SUREMEN	TS
WELL CONG	TRUCTION MATERIA	I C		Dia	meter of Boreho	ole	(ir	nches) 8.0
	TRUCTION MATERIA e type of material for each area)	LS		ID o	of Riser Pipe		(ir	nches) 2.0
					ective Casing L	ength		(feet) 5.0
Protective Casing	SS304 SS316 PTFE	PVC OTHER: (S	teel		er Pipe Length			(feet) 12.27
Riser Pipe Above W.T.		PVC OTHER:			tom of Screen to			(feet) 0.37
Riser Pipe Below W.T.		PVC OTHER:			een Length (1s) al Length of Cas			(feet) 4.79 (feet) 17.43
-				100	Longin or Cas	5		11.73

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ	nmental Protection	n Agency				Well	Complet	tion Report
Site #:		County: <u>Mon</u>	tgomery	У		v	Vell #:	G316
Site Name: Coffeen Part 845	Groundwater					E	Borehole #:	G316
State Plane Coordinate: X 2,517,211	.6 Y 871,643.1 (or) Latitude:				Longitud	le:	
Surveyed By: Kyle J. Nolan			IL Reg	istration	#:035-0	03919		
Drilling Contractor: Roberts En	nv. Drilling Inc.		Driller:	Mat	tt			
Consulting Firm: Hanson Prof	essional Services Inc.		Geolog	ist: R	Chonald W.	Hasenyage	er, LPG #196	5-000246
Drilling Method: Hollow stem	auger		Drilling	g Fluid ((Type): <u>No</u>	one		
Logged By: Colin Winter			Date St	tarted:	1/14/20	21 Dat	te Finished: _	1/14/2021
Report Form Completed By:Co	lin Winter		Date:		5/3/2021			
ANNULAR SPA	CE DETAILS				levations (MSL)*	Depths (BGS)	(0.01	1 ft.)
					603.06	3.42	Top of Prote	ective Casing
				_	602.59	-2.95	Top of Riser	r Pipe
Type of Surface Seal: Concrete				· ·	599.64	0.00	Ground Sur	face
				_	598.84	0.80	Top of Ann	ular Sealant
Type of Annular Sealant: Benton								
Installation Method: Gravity	7	_	_				C W.	T 1
Setting Time:		_ \[\frac{1}{2}	_	-			Static Water (After Compl	
Type of Bentonite Seal Grant	ılar Pellet Slurry							
Installation Method:	, ,		VV	_	n/a	n/a	Top of Seal	
Setting Time:					590.64	9.00	Top of Sand	l Pack
T. (0. 1D.)				_				
Type of Sand Pack: Quartz sand		- <u> </u>		_	589.62	_10.02_	Top of Scree	en
Grain Size: 10-20 (sie Installation Method: Gravity								
nistanation Method. Oravity				_	584.82	_14.82_	Bottom of S	creen
Type of Backfill Material: N/A	(if applicable)	_ _		-	584.48	15.16	Bottom of V	Vell
Installation Method:				_	583.89	15.75	Bottom of B	Borehole
				*	Referenced to a	National Geode	tic Datum	
					CAS	ING MEA	SUREMEN	TS
WELL CONS	TRUCTION MATERIAL	LS			eter of Boreho	ole	(in	aches) 8.0
	e type of material for each area)				Riser Pipe		`	nches) 2.0
					tive Casing L	_		(feet) 5.0
Protective Casing	SS304 SS316 PTFE	PVC OTHER: S	teel		Pipe Length n of Screen to			(feet) 12.97 (feet) 0.34
Riser Pipe Above W.T.		PVC OTHER:			n Length (1s			(feet) 0.34 (feet) 4.80
Riser Pipe Below W.T.	SS304 SS316 PTFE	PVC OTHER:			Length of Cas		•	(feet) 4.80 (feet) 18.11

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environ	nmental Protection Age	ency			Well	Completi	on Report
Site #:	County:	Montgor	nery			Vell #:	G317
Site Name: Coffeen Part 845	Groundwater				E	Borehole #:	G317
State Plane Coordinate: X 2,517,087	7.4 Y 871,234.2 (or) La	titude:			Longitud	le:	
Surveyed By: Kyle J. Nolan		IL	Registra	tion #:035-00	03919		
Drilling Contractor: Roberts Ex	nv. Drilling Inc.	Dr	iller: <u>l</u>	Matt			
Consulting Firm: Hanson Prof	essional Services Inc.	Ge	ologist:	Rhonald W.	Hasenyage	er, LPG #196-	000246
Drilling Method: Hollow stem	auger	Dr	illing Flu	id (Type): No	one		
Logged By: Colin Winter		Da	te Starte	d:1/13/20	21 Dat	te Finished:	1/14/2021
Report Form Completed By:Co	lin Winter	Da	te:	5/3/2021			
ANNULAR SPA	CE DETAILS			Elevations (MSL)*	Depths (BGS)	(0.01	ft.)
			_	642.26	-3.41	Top of Protec	tive Casing
				641.93	-3.08	Top of Riser	Pipe
Type of Surface Seal: Concrete		4		638.85	0.00	Ground Surfa	ce
				638.05	0.80		
Type of Annular Sealant: High-s			4			. 1	
Installation Method: Tremie	:						
Setting Time: >24 hours						Static Water I (After Complet	
Type of Bentonite Seal Gran		#					
Installation Method: Gravity	(choose one)			612.55	26.30	Top of Seal	
Setting Time: 24 hours			\1	609.75	20.10	Top of Sand	Dools
				009.73		Top of Sand	Tack
Type of Sand Pack: Quartz sand	<u> </u>			608.71	30.14	Top of Screen	1
Grain Size: 10-20 (sie						100 01 00100	-
Installation Method: <u>Gravity</u>	<i>I</i>			603.92	34.93	Bottom of Sc	reen
Type of Backfill Material: N/A	(if applicable)			603.57	35.28	Bottom of Wo	ell
Installation Method:				602.85	36.00	Bottom of Bo	rehole
				* Referenced to a l	National Geode	tic Datum	
				CAS	ING MEA	SUREMENT	S
WELL CONS	TRUCTION MATERIALS		Di	ameter of Boreho	ole	(incl	nes) 8.0
	type of material for each area)			of Riser Pipe		(incl	
				otective Casing L	ength		Seet) 5.0
Protective Casing	SS304 SS316 PTFE PVC O	THER: Steel	\neg	ser Pipe Length ttom of Screen to	End Con		Geet) 33.22 Geet) 0.35
Riser Pipe Above W.T.		THER:		reen Length (1s			(eet) 0.35 (eet) 4.79
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC O	THER:		tal Length of Cas			eet) 4.79

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Enviror	mental Prote	ection Agency				Well	Comple	tion Report
Site #:		County: Mor	itgomery	y		v	Vell #:	XPW01
Site Name: Coffeen Part 845	Groundwater					E	Borehole #:	XPW01
State Plane Coordinate: X 2,515,366								
Surveyed By: Kyle J. Nolan								
Drilling Contractor: Roberts Er	nv. Drilling Inc.		Driller:	Matt				
Consulting Firm: Hanson Profe	essional Services Ir	nc.	Geolog	ist: Rho	onald W.	Hasenyage	er, LPG #19	6-000246
Drilling Method: Hollow stem	auger		Drilling	g Fluid (Ty	pe): <u>No</u>	one		
Logged By: Colin Winter			Date St	tarted:	2/8/20	21 Dat	te Finished: _	2/8/2021
Report Form Completed By: <u>Co</u>	lin Winter		Date:	5/3	3/2021			
ANNULAR SPA	CE DETAILS				vations (SL)*	Depths (BGS)	(0.0)	1 ft.)
				_63	34.92	3.07	Top of Prot	ective Casing
				_63	34.57	-2.72	Top of Rise	er Pipe
Type of Surface Seal: Concrete					31.85	0.00	Ground Sur	face
Trans of American Contents - Doutson	ita ahina			_63	31.15	0.70	Top of Ann	ular Sealant
Type of Annular Sealant: Benton								
Installation Method: <u>Gravity</u> Setting Time: <u>>24 hours</u>	<u></u>		7				Static Wate	r I evel
Setting Time			_				(After Comp	
Type of Bentonite Seal Grant	ılar Pellet S	Slurry						
Installation Method:	` '		$\overline{\mathbf{x}}$		n/a	n/a	Top of Seal	Į.
Setting Time:				62	24.85	7.00	Top of San	d Pack
Town of Court Pools							•	
Type of Sand Pack: Quartz sand Grain Size: 10-20 (sie				_62	23.64	8.21	Top of Scre	een
Installation Method: Gravity			∄│					
instanction viction. <u>Gravity</u>					18.87	12.98	Bottom of S	Screen
Type of Backfill Material: N/A	(if applicable)	L	_	_61	18.49	13.36	Bottom of V	Well
Installation Method:					17.85	14.00	Bottom of I	Borehole
				* Re	eferenced to a	National Geode	tic Datum	
					CAS	SING MEA	SUREMEN	TS
WELL CONG	TRUCTION MAT	EDIALC		Diameter	of Boreh	ole	(i	nches) 8.0
	e type of material for each are			ID of Ris				nches) 2.0
					_	Length		(feet) 5.0
Protective Casing	SS304 SS316 PT	FE PVC OTHER: (S	teel		e Length			(feet) 10.93
Riser Pipe Above W.T.		FE PVC OTHER:	$\overline{}$			o End Cap st slot to last slo		(feet) 0.38 (feet) 4.77
Riser Pipe Below W.T.		FE PVC OTHER:			engun (1 ngth of Ca		JI)	(feet) 4.77 (feet) 16.08

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Illinois Environmental Protection Agency	Well Completion Report
Site #: County:Mor	ntgomery Well #: XPW02
Site Name: Coffeen Part 845 Groundwater	Borehole #: XPW02
State Plane Coordinate: X 2,515,627.3 Y 871,987.1 (or) Latitude:	
Surveyed By: Kyle J. Nolan	IL Registration #: 035-003919
Drilling Contractor: Roberts Env. Drilling Inc.	Driller: Matt
Consulting Firm: Hanson Professional Services Inc.	Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger	Drilling Fluid (Type): None
Logged By: Colin Winter	Date Started: 2/8/2021 Date Finished: 2/8/2021
Report Form Completed By: Colin Winter	Date: 5/3/2021
ANNULAR SPACE DETAILS	Elevations Depths (0.01 ft.) (MSL)* (BGS)
Type of Surface Seal: Concrete	
Type of Annular Sealant: Bentonite chips	
Installation Method: <u>Gravity</u> Setting Time: _ >24 hours	Z Static Water Level
Setting Time: 42 Thous	(After Completion)
Type of Bentonite Seal Granular Pellet Slurry (choose one)	
Installation Method:	n/a Top of Seal
Setting Time:	
Type of Sand Pack: Quartz sand	
Grain Size: 10-20 (sieve size)	
Installation Method: Gravity	
Type of Backfill Material: N/A	618.79 17.85 Bottom of Screen 618.44 18.20 Bottom of Well
(if applicable)	(19.44 19.20 P. # CD. 1.1
Installation Method:	
	CASING MEASUREMENTS
WELL CONCEDITION AS TERMS	Diameter of Borehole (inches) 8.0
WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)	ID of Riser Pipe (inches) 2.0
	Protective Casing Length (feet) 5.0
Dutatin Cain	Riser Pipe Length (feet) 11.10
Protective Casing SS304 SS316 PTFE PVC OTHER: SR304 SS316 PTFE PVC OTHER: SR304 SS316 PTFE PVC OTHER:	Bottom of Screen to End Cap (leet) 0.53
Riser Pipe Below W.T. SS304 SS316 PTFE PVC OTHER:	Screen Length (1st slot to last slot) (feet) 9.80 Total Length of Casing (feet) 21.25

SS304

Well Completion Form (revised 02/06/02)

SS316

Total Length of Casing

**Hand-Slotted Well Screens Are Unacceptable

Screen Slot Size **

0.010

🔷 Depitpn WiGhlti	Nli me Alt ml.mt i	v Sli LT			h læ	gtNrelmpti	PlrtU
Site #:	Co	ounty: <u>Mon</u>	tgomery	I	W	/ell #: <u>G27</u>	75D
Site Name: Coffeen Part 845	Groundwater				В	orehole #:G	275D
State Plane Coordinate: X 2,516,366	5.5 Y 874,285.3 (or	r) Latitude:			Longitud	e:	
Surveyed By: Michael J. Gran				stration #:035-0			
			Driller:	Matt			
Consulting Firm: Hanson Profe				ist: Rhonald W.	Hasenyage	r, LPG #196-000	246
Drilling Method: Hollow stem	auger		Drilling	g Fluid (Type): No	one		
				arted: 1/28/20			
Report Form Completed By: Co			Date:				
v a a s I v P c Av				Wel Comptin	RlrnEn	(0.01 ft.)	
				(MSL)*	(BGS)	T CD:	<i>c</i> :
		\Box	$\overline{}$	620.69	3.17	Top of Protective	Casing
				620.31	2.79	Top of Riser Pipe	
Type of Surface Seal: Concrete				617.52_	0.00	Ground Surface	
				617.02	0.50	Top of Annular S	oolont
Type of Annular Sealant: High-s	olids bentonite	- 🕅		017.02		Top of Affiliata S	caiaiii
Installation Method:Tremie		_					
Setting Time: >24 hours		_ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	<u>z</u>			Static Water Leve	1
						(After Completion)	
Type of Bentonite Seal Gran	Pellet Slurry (choose one)		Ϋ́Τ				
Installation Method: Gravity		_		572.02	45.50	Top of Seal	
Setting Time: 30 minutes				551 0 0	46.50		
<u> </u>		- 🔀	X	571.02	_46.50_	Top of Sand Pack	
Type of Sand Pack: Quartz sand		_					
Grain Size: 10-20 (sie	ve size)			567.76	49.76	Top of Screen	
Installation Method: Gravity	,						
			∄	557.97	_59.55	Bottom of Screen	
Type of Backfill Material: N/A	(if applicable)	_ _	_	557.63	_59.89	Bottom of Well	
Installation Method:				517.82	99.70	Bottom of Boreho	Ja
instanation Method.		_		* Referenced to a			ne .
				CAS	SING MEA	SUREMENTS	
							8.0
	TRUCTION MATERIAL	S		Diameter of Boreh ID of Riser Pipe	oie	(inches)	2.0
(Choose on	e type of material for each area)			Protective Casing I		` ´	5.0
				Riser Pipe Length	_	(feet)	52.55
Protective Casing	SS304 SS316 PTFE P	VC OTHER: S	teel	Bottom of Screen t		`	0.34
Riser Pipe Above W.T.	SS304 SS316 PTFE P	VC OTHER:		Screen Length (1			9.79
Riser Pipe Below W.T.	SS304 SS316 PTFE P	VC OTHER:		Total Length of Ca	sing	(feet)	62.68

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

(inches)

SS304

Well Completion Form (revised 02/06/02)

SS316

Depit pr Wi Cplt i Nli me Alt nh Lnpt i v Sli	IT		h læ	gtNrelmpti	PlrtU
Site #: County: _ !	Montgomery		W	Vell #: <u>G2</u>	06D
Site Name: Coffeen Part 845 Groundwater			Bo	orehole #:	6282D
State Plane Coordinate: $X_{2,514,684.6}$ $Y_{875,111.4}$ (or) Latitu	ıde:		Longitude	e:	
Surveyed By: Michael J. Graminski					
Drilling Contractor: Roberts		Matt			
Consulting Firm: Hanson Professional Services Inc.	Geologist:	Rhonald W.	Hasenyageı	r, LPG #196-000)246
Drilling Method: Hollow stem auger	Drilling Fl	uid (Type): <u>No</u>	one		
Logged By: Colin Winter	Date Starte	ed:1/25/20	21 Date	e Finished:1/2	25/2021
Report Form Completed By: Colin Winter		5/3/2021			
v a a s I v P c Av g WRWy v II c		Wel Compt i n (MSL)*		(0.01 ft.))
_		634.35	2.94	Top of Protective	e Casing
		634.14	2.73	Top of Riser Pipe	e
Type of Surface Seal: Concrete		631.41	0.00	Ground Surface	
		631.11	0.30		Saalaut
Type of Annular Sealant: High-solids bentonite		_031.11		Top of Annular S	seaiani
Installation Method:					
Setting Time:	$ \overline{\Delta} $			Static Water Lev (After Completion)	
Type of Bentonite Seal Granular Pellet Slurry					
(choose one) Installation Method: Gravity	\	585.81	45.60	Top of Seal	
		363.61	_ 43.00	rop or sear	
Setting Time:15 hours		_583.41_	_48.00_	Top of Sand Pack	k
Type of Sand Pack: Quartz sand					
Grain Size: 10-20 (sieve size)		_582.21_	_49.20_	Top of Screen	
Installation Method: <u>Gravity</u>					
Type of Backfill Material: N/A		<u>572.41</u> 572.02	<u>59.00</u> 59.39	Bottom of Screen Bottom of Well	n
(if applicable)					
Installation Method:		571.41 * Referenced to a	60.00 National Geodetic		ole
		G 1 G	DIG ME L		
				SUREMENTS	0.0
WELL CONSTRUCTION MATERIALS		iameter of Boreho O of Riser Pipe		(inches)	2.0
(Choose one type of material for each area)		otective Casing L	enoth		5.0
		iser Pipe Length	-	(feet)	51.93
Protective Casing SS304 SS316 PTFE PVC OTH	()	ottom of Screen to		`	0.39
Riser Pipe Above W.T. SS304 SS316 PTFE PVC OTHER	T.D.	creen Length (1s			9.77
Riser Pipe Below W.T. SS304 SS316 PTFE PVC OTHER	ED.	otal Length of Cas		(feet)	62.09

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

🔷 DepitpnWiGLti	Nli me Alt ml.mt	i v Sli LT			h læ	gtNrdmpti	PlrtU
Site #:		County: Mor	itgomery		W	Vell #: G2	283
Site Name: Coffeen Part 845	Groundwater				В	orehole #:	G283
State Plane Coordinate: X 2,516,503	3.0 Y 874,113.0	(or) Latitude:			Longitud	e:	
Surveyed By: Michael J. Gran				stration #:035-0			
-			Driller:	Matt			
Consulting Firm: <u>Hanson Prof</u>	essional Services Inc.		Geologi	st: <u>Rhonald W.</u>			
Orilling Method: Hollow stem	auger		Drilling	Fluid (Type): No	one		
Logged By: Colin Winter	_		Date Sta	arted: 1/18/20)21 Dat	e Finished: 1/1	18/2021
Report Form Completed By:Co			Date:	5/3/2021			
v a a s I v P cAv				Wel Compt i n (MSL)*		(0.01 ft.)	
				611.07	2.77	Top of Protective	Casing
				610.75	-2.45	Top of Riser Pipe	;
Type of Surface Seal: Concrete				608.30	0.00	Ground Surface	
				607.70	0.60		ealant
Type of Annular Sealant: Benton	nite chips	_ 🕅				Top of Aimulai S	calant
Installation Method: Gravity	У						
Setting Time: >24 hours		\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	$\mathbb{Z} \mid \cdot \mid$			Static Water Leve (After Completion)	el
Type of Bentonite Seal Gran	ular Pellet Slurry						
Installation Method:	, ,			n/a	n/a	Top of Seal	
Setting Time:				600.80	7.50	Top of Sand Pack	ζ.
		v v				1	
Type of Sand Pack: Quartz Sand				599.91	8.39	Top of Screen	
Grain Size: 10-20 (sign	,		∄			•	
Installation Method: Gravity	y.			_590.13_	18.17	Bottom of Screen	
Type of Backfill Material: N/A	(if applicable)	_ [589.94	18.36	Bottom of Well	
Installation Method:				589.94	18.36	Bottom of Boreho	ole
				* Referenced to a	National Geodet		
				CAS	SING MEA	SUREMENTS	
******	TENTION AND THE	I.C.	[Diameter of Boreho	ole	(inches)	8.0
	TRUCTION MATERIA ne type of material for each area)	LS		ID of Riser Pipe		(inches)	2.0
				Protective Casing I	ength	(feet)	5.0
	I			Riser Pipe Length		(feet)	10.84
Protective Casing	SS304 SS316 PTFE	PVC OTHER:	Steel)	Bottom of Screen to	o End Cap	(feet)	0.38
Riser Pipe Above W.T.		PVC OTHER:		Screen Length (1s	st slot to last slo	ot) (feet)	9.78
Riser Pipe Below W.T.	SS304 SS316 PTFE (PVC OTHER:		Total Length of Cas	sing	(feet)	21.00

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

(inches)

SS304

Well Completion Form (revised 02/06/02)

SS316

Depit pn WiCpLti	iNlimeAU:nhLnptiv	Sli LT			h læ	gtNrelmpti	PlrtU
Site #:	Count	ty: <u>Montg</u>	gomery		W	/ell #: G2	284
Site Name: Coffeen Part 845	Groundwater				В	orehole #:	G284
State Plane Coordinate: X 2,516,922	2.9 Y 874,423.6 (or)	Latitude:			Longitude	e:	
Surveyed By: Michael J. Gran							
Drilling Contractor: Roberts			Driller: <u>1</u>	Matt			
Consulting Firm: Hanson Prof	Pessional Services Inc.		Geologist:			r, LPG #196-000	
Drilling Method: <u>Hollow stem</u>	ı auger	1	Drilling Flu	iid (Type): <u>No</u>	one		
Logged By: Colin Winter		1	Date Starte	d:1/20/20	21 Date	e Finished:1/2	20/2021
Report Form Completed By: Co			Date:	5/3/2021			
v a a s I v P cAv				Wel Compt in (MSL)*		(0.01 ft.)	
				618.66	3.33	Top of Protective	Casing
				618.42	3.09	Top of Riser Pipe	;
Type of Surface Seal: Concrete				615.33	0.00	Ground Surface	
				614.33	1.00	Top of Annular S	ealant
Type of Annular Sealant: Bento							
Installation Method: Gravit	<u>y</u>					G W.	
Setting Time: >24 hours						Static Water Leve (After Completion)	el
Type of Bentonite Seal Gran	•						
Installation Method:	(choose one)			n/a	n/a	Top of Seal	
Setting Time:				608.33	7.00	Top of Sand Pacl	,
				008.33		Top of Sand Faci	
Type of Sand Pack: Quartz San	d			607.25	8.08	Top of Screen	
Grain Size:10-20 (signal of the size is a size in the size in the size is a size in the size in	,			007.23		Top of Screen	
Installation Method: <u>Gravit</u>	y			602.48	12.85	Bottom of Screen	
Type of Backfill Material: N/A	(2 1 11)]	602.10	13.23	Bottom of Well	
Installation Method	(if applicable)			601.33	14.00	Bottom of Boreho	ale.
instanation wethod.	_			* Referenced to a			ЛС
				CAS	SING MEAS	SUREMENTS	
			Dia	ameter of Boreho		(inches)	8.0
	STRUCTION MATERIALS ne type of material for each area)			of Riser Pipe			2.0
`	,		Pro	otective Casing L	ength	(feet)	5.0
	1		Ris	ser Pipe Length		(feet)	11.17
Protective Casing	SS304 SS316 PTFE PVC	OTHER: Stee	Bo	ttom of Screen to	o End Cap	(feet)	0.38
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC	OTHER:	Se	reen Length (1s	st slot to last slo	t) (feet)	4.77
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC	OTHER:	То	tal Length of Cas	sing	(feet)	16.32

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Depit pn Wi Cplti	i Nli me Altıhlıqti	v Sl i LT			h læ	gtNrelmpti	PlrtU
Site #:	Cou	unty: <u>Mon</u>	tgomery		W	Vell #: G2	285
Site Name: Coffeen Part 845	Groundwater				В	orehole #:	G285
State Plane Coordinate: X 2,516,680	0.4 Y 874,795.0 (or)	Latitude:			Longitude	e:	
Surveyed By: Michael J. Gran							
Drilling Contractor: Roberts			Driller: _	Matt			
Consulting Firm: Hanson Prof						r, LPG #196-000	
Drilling Method: Hollow stem	ı auger		Drilling F	luid (Type): No	one		
Logged By: Colin Winter	_		Date Start	red: 1/19/20)21 Date	e Finished: 1/2	19/2021
Report Form Completed By: Co				5/3/2021			
v a a s I v P cAv				WelComptin	RlrnEn	(0.01 ft.))
				(MSL)* 613.90	(BGS) -3.36	Top of Protective	Casino
		T					
				613.52	2.98	Top of Riser Pipe	2
ype of Surface Seal: Concrete				610.54	0.00	Ground Surface	
				610.24	0.30	Top of Annular S	Sealant
ype of Annular Sealant: Bento							
Installation Method: <u>Gravit</u>	<u>y</u>	.					
Setting Time: >24 hours		. _	7			Static Water Leve (After Completion)	el
Type of Bentonite Seal Gran	nular Pellet Slurry						
T all a Maria	(choose one)			1	1	T 60 1	
Installation Method:				n/a	n/a	Top of Seal	
Setting Time:				598.24	_12.30_	Top of Sand Pacl	ζ.
Type of Sand Pack: Quartz San	d						
Grain Size: 10-20 (si				596.86	_13.68_	Top of Screen	
Installation Method: Gravit	v						
				587.09	_23.45_	Bottom of Screen	ı
Type of Backfill Material: N/A	(if applicable)	. L		_586.71_	23.83	Bottom of Well	
Installation Method:				_584.54_	26.00	Bottom of Boreho	ole
				* Referenced to a	National Geodeti	c Datum	
				CAS	SING MEAS	SUREMENTS	
WELL COM	CEDITORION A CEDA : X C		Б	Diameter of Boreho	ole	(inches)	8.0
	STRUCTION MATERIALS ne type of material for each area)		П	D of Riser Pipe		(inches)	2.0
			P	rotective Casing I	ength	(feet)	5.0
D to the Control			$\overline{}$	Liser Pipe Length		(feet)	16.66
Protective Casing	SS304 SS316 PTFE PV			Sottom of Screen to			0.38
Riser Pipe Above W.T.	SS304 SS316 PTFE PV			creen Length (1			9.77
Riser Pipe Below W.T.	SS304 SS316 PTFE PV	C OTHER:	T	otal Length of Ca	sing	(feet)	26.81

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Depit pn Wi Cplti	i Nli meAlt mlLnt i	v Sl i LT			h læ	gtNrelmpti	PlrtU
Site #:	Cc	ounty: <u>Mon</u>	tgomery		W	/ell #: G2	286
Site Name: Coffeen Part 845	Groundwater				В	orehole #:	G286
State Plane Coordinate: X 2,516,56	1.8 Y 875,072.2 (or	r) Latitude:			Longitud	e:	
Surveyed By: Michael J. Gran							
			Driller:	Matt			
Consulting Firm: Hanson Prof						r, LPG #196-000	
Drilling Method: Hollow stem	auger		Drilling I	Fluid (Type): No	one		
- 44						e Finished: 1/2	
Report Form Completed By: Co				5/3/2021			
						(0.01.0.)	
v a a s I v P cAv	g wkwy v II c			Wel Compt i n (MSL)*	(BGS)	(0.01 ft.)	
				613.57	-3.60	Top of Protective	Casing
				613.13	3.16	Top of Riser Pipe	e
Type of Surface Seal: Concrete				600.07	0.00	Constant	
Type of Surface Sean. — Somerece				609.97	0.00		
Гуре of Annular Sealant:Benton	nite chips	-		609.47	0.50	Top of Annular S	Sealant
Installation Method: Gravit	у	_					
Setting Time: >24 hours		_ \[\bright\]	<u>z</u>			Static Water Leve (After Completion)	el
Type of Pontonite Scal Com	ular Pellet Slurry					(riter completion)	
Type of Bentonite Seal Gran	nular Pellet Slurry (choose one)						
Installation Method:		_		n/a	n/a	Top of Seal	
Setting Time:		- 🕅		607.27	2.70	Top of Sand Pacl	ζ
Type of Sand Pack: Quartz San							
Grain Size: 10-20 (signature)		- _		606.60	3.37	Top of Screen	
Installation Method: Gravit	•						
instanation Method. Oravit		-		601.81	8.16	Bottom of Screen	l
Type of Backfill Material: N/A	(if applicable)	_ _		601.47	8.50	Bottom of Well	
Installation Method:				599.97	10.00	Bottom of Boreho	ole
				* Referenced to a	National Geodeti	ic Datum	
				CAS	SING MEAS	SUREMENTS	
		_		Diameter of Boreho		(inches)	8.0
	STRUCTION MATERIALS the type of material for each area)	S		D of Riser Pipe		(inches)	2.0
			<u> </u>	Protective Casing I	ength	(feet)	5.0
	Т	_		Riser Pipe Length		(feet)	6.53
Protective Casing		VC OTHER: S	teel I	Bottom of Screen to	o End Cap	(feet)	0.34
Riser Pipe Above W.T.		VC OTHER:		Screen Length (1	st slot to last slo	t) (feet)	4.79
Riser Pipe Below W.T.	SS304 SS316 PTFE (P	VC OTHER:		Total Length of Ca	sing	(feet)	11.66

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Depit pn Wi Cplt	i Nli me AU: nhLnpti v S	li LT			h læ	gtNrelmpti	PlrtUn
Site #:	County:	Montg	gomery		W	Vell #:G	287
Site Name: Coffeen Part 845	Groundwater				В	orehole #:	G287
State Plane Coordinate: X 2,516,41	.5.5 Y 875,442.8 (or) L	atitude:			Longitude	e:	
	minski			ration #:035-0			
Drilling Contractor: Roberts			Driller: _	Matt			
	fessional Services Inc.			: Rhonald W.			
_	n auger			luid (Type): No			
_			_				
				ted: 1/18/20		e Finished:1/	18/2021
Report Form Completed By:C	olin Winter		Date:	5/3/2021			
v a a s I v P cAv	g WRWy v II c			Wel Compt in (MSL)*	RlrnEn (BGS)	(0.01 ft.)
				617.88	3.54	Top of Protective	e Casing
			7	617.45	3.11	Top of Riser Pip	e
Type of Surface Seal: Concrete	:			614.34	0.00	Ground Surface	
				613.74	0.60	Top of Annular	Sealant
Type of Annular Sealant: Bento	onite chips			013.74		Top of Amilular	Sealant
Installation Method: <u>Gravi</u>	ty						
Setting Time: >24 hours						Static Water Lev (After Completion)	
True of Doutonite Seel Co-	nular Pellet Slurry					(ritter completion)	
Type of Bentonite Seal Gra	nular Pellet Slurry (choose one)						
Installation Method:		$\overline{\mathbf{x}}$		n/a	n/a	Top of Seal	
Setting Time:				609.84	4.50	Top of Sand Pac	k
T (C 1D 1							
Type of Sand Pack: Quartz San				608.91	5.43	Top of Screen	
Grain Size: 10-20 (s						1	
Installation Method: <u>Gravi</u>	ty			604.09	10.25	Bottom of Screen	n
Type of Backfill Material: N/A]	603.75	10.59	Bottom of Well	11
	(if applicable)			602.54	11.00		
Installation Method:				602.54 * Referenced to a	11.80 National Geodeti	Bottom of Boreh c Datum	ole
				G.L.G	DIC ME L		
			Г			SUREMENTS	0.0
	STRUCTION MATERIALS			Diameter of Boreho D of Riser Pipe	ole	(inches)	
(Choose o	one type of material for each area)			Protective Casing L		` `	
				Riser Pipe Length	_	(feet)	0.4
Protective Casing	SS304 SS316 PTFE PVC	OTHER: Stee	$\overline{}$	Bottom of Screen to		•	
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC	OTHER:	S	Screen Length (1s	st slot to last slo	t) (feet)	4.82
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC	OTHER:	T	Total Length of Cas	sing	(feet)	13.70

Well Completion Form (revised 02/06/02)

Screen Slot Size **

**Hand-Slotted Well Screens Are Unacceptable

0.010

Depit po Wi Cplt:	iNlimeAU:nhLnptiv	Sli LT			h læ	gtNrelmpti	PlrtU
Site #:	Coun	ty: Mont	gomery		W	Vell #:G	288
Site Name: Coffeen Part 845	Groundwater				B	orehole #:	G288
State Plane Coordinate: X 2,517,07	1.4 Y 875,279.6 (or)	Latitude:			Longitude	e:	
Surveyed By: Michael J. Gran							
- 4			Driller:]	Matt			
Consulting Firm: Hanson Prof	fessional Services Inc.		Geologist:			r, LPG #196-00	
Drilling Method: Hollow stem	ı auger		Drilling Flu	ıid (Type): No	one		
Logged By: Colin Winter	•		Date Starte	d: 1/21/20	21 Date	e Finished:1/	21/2021
Report Form Completed By: Co				5/3/2021			
vaas I v P cAv				Wel Compt in		(0.01 ft.)
				(MSL)*	(BGS)		
		—		620.37	3.29	Top of Protective	e Casing
			a	620.07	2.99	Top of Riser Pip	e
Type of Surface Seal: Concrete				617.08	0.00	Ground Surface	
	_			616.78	0.30	Top of Annular S	Sealant
Type of Annular Sealant: Bento	nite chips					Top of Financial	
Installation Method: Gravit	<u>y</u>						
Setting Time: _ >24 hours						Static Water Lev (After Completion)	
Type of Bentonite Seal Gran	nular Pellet Slurry						
	(choose one)		1				
Installation Method:				<u>n/a</u>	n/a	Top of Seal	
Setting Time:				611.08	6.00	Top of Sand Pac	k
Type of Sand Pack: Quartz San	d						
Grain Size: 10-20 (si				609.49	7.59	Top of Screen	
Installation Method: Gravit	v						
	,			604.82	12.26	Bottom of Screen	1
Type of Backfill Material: N/A	(if applicable)		_	604.33	12.75	Bottom of Well	
Installation Method:	· · · · · · · · · · · · · · · · · · ·			603.08	14.00	Bottom of Boreh	ole
				* Referenced to a	National Geodeti	c Datum	
				CAS	SING MEAS	SUREMENTS	
WPW - 003			Di	ameter of Boreho	ole	(inches)	8.0
	STRUCTION MATERIALS ne type of material for each area)		ID	of Riser Pipe		(inches)	2.0
			Pre	otective Casing L	ength	(feet)	5.0
	T			ser Pipe Length		(feet)	10.58
Protective Casing	SS304 SS316 PTFE PVC			ottom of Screen to			0.49
Riser Pipe Above W.T.	SS304 SS316 PTFE PVC			reen Length (1s		t) (feet)	
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC	OTHER:	To	tal Length of Cas	sing	(feet)	15.74

Well Completion Form (revised 02/06/02)

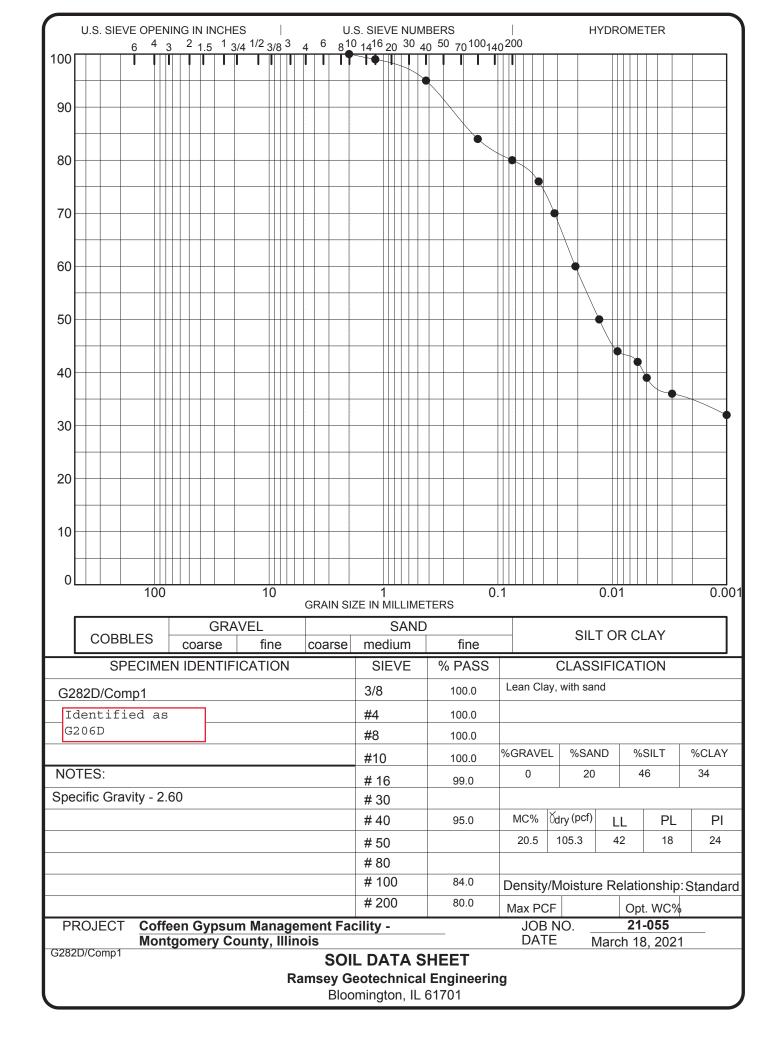
Screen Slot Size **

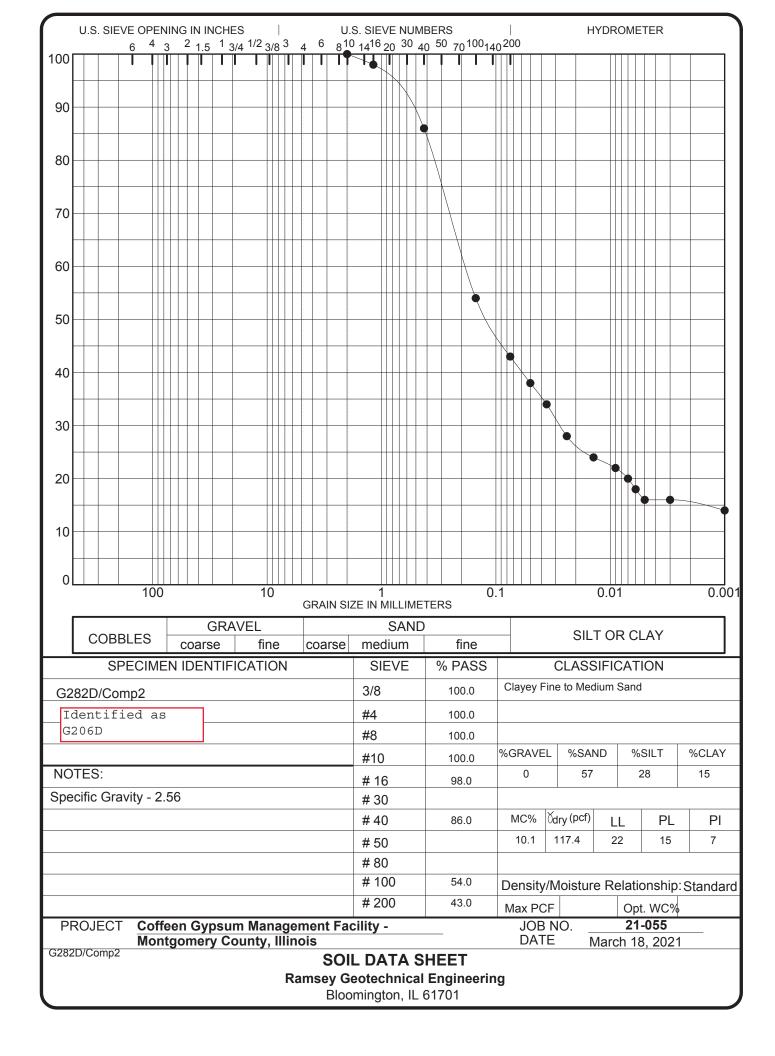
**Hand-Slotted Well Screens Are Unacceptable

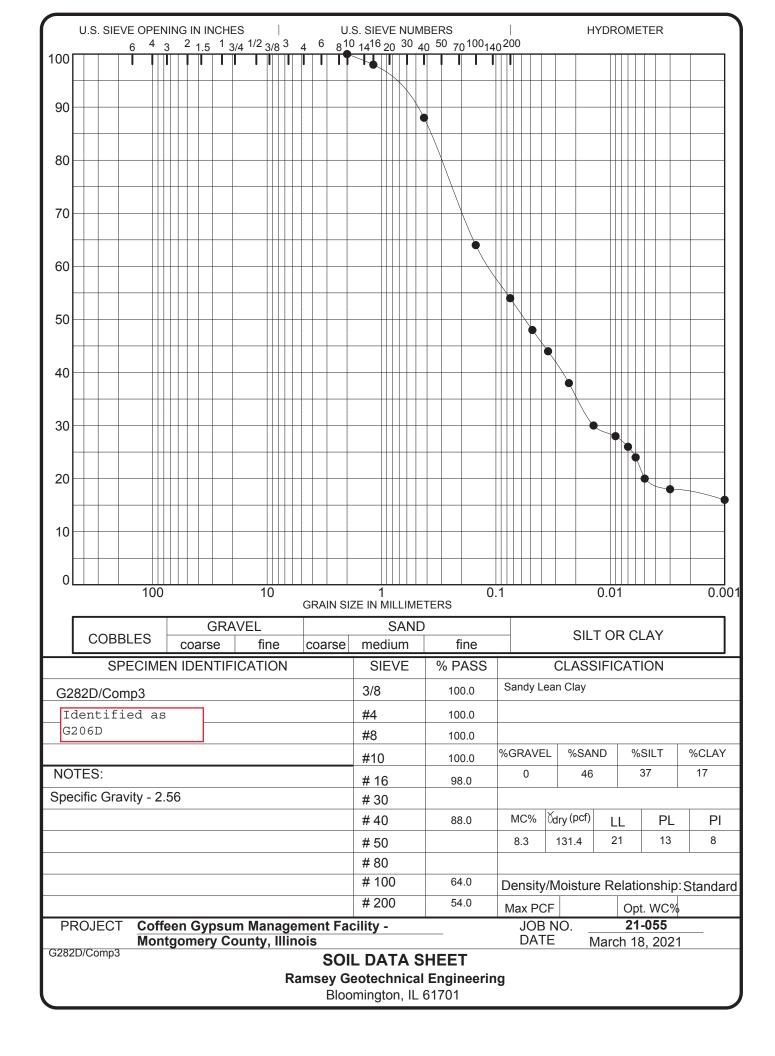
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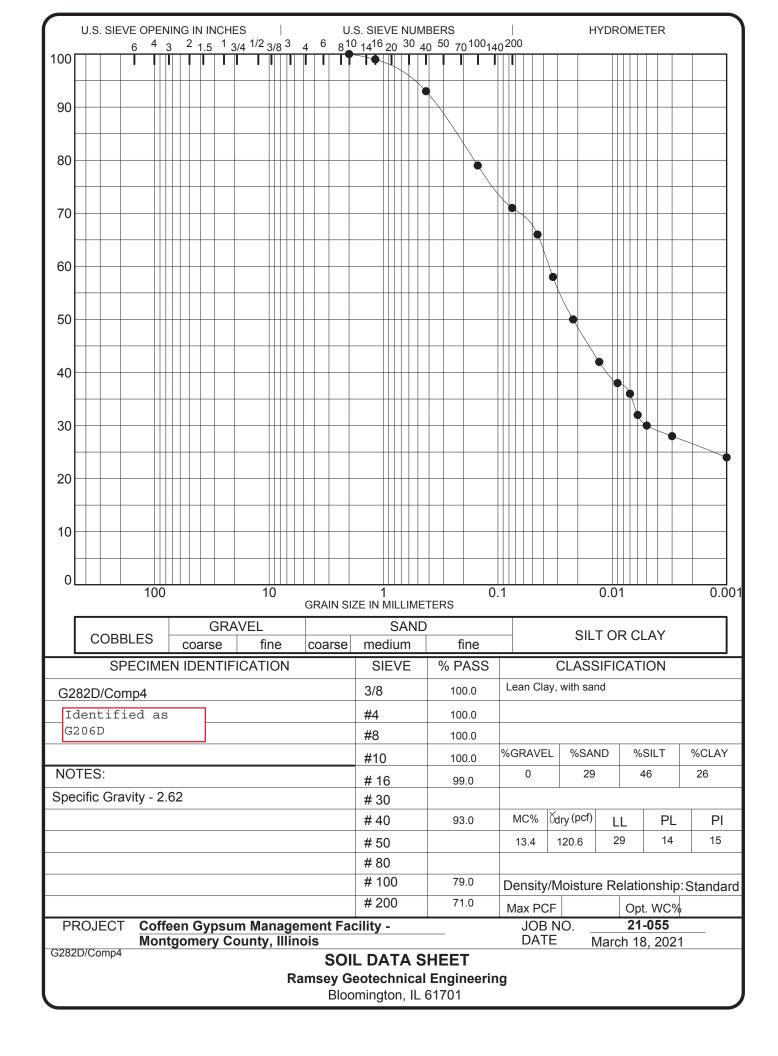
APPENDIX D GEOTECHNICAL LABORATORY REPORTS

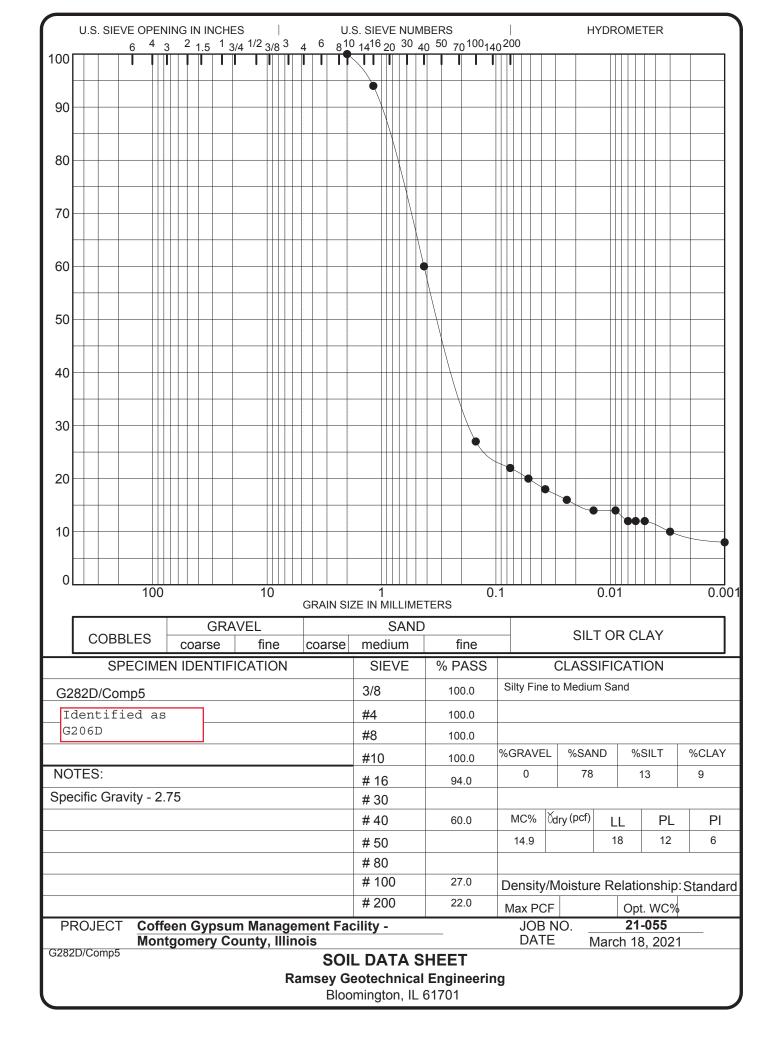
RAMSEY GEOTECHNICAL REPORT

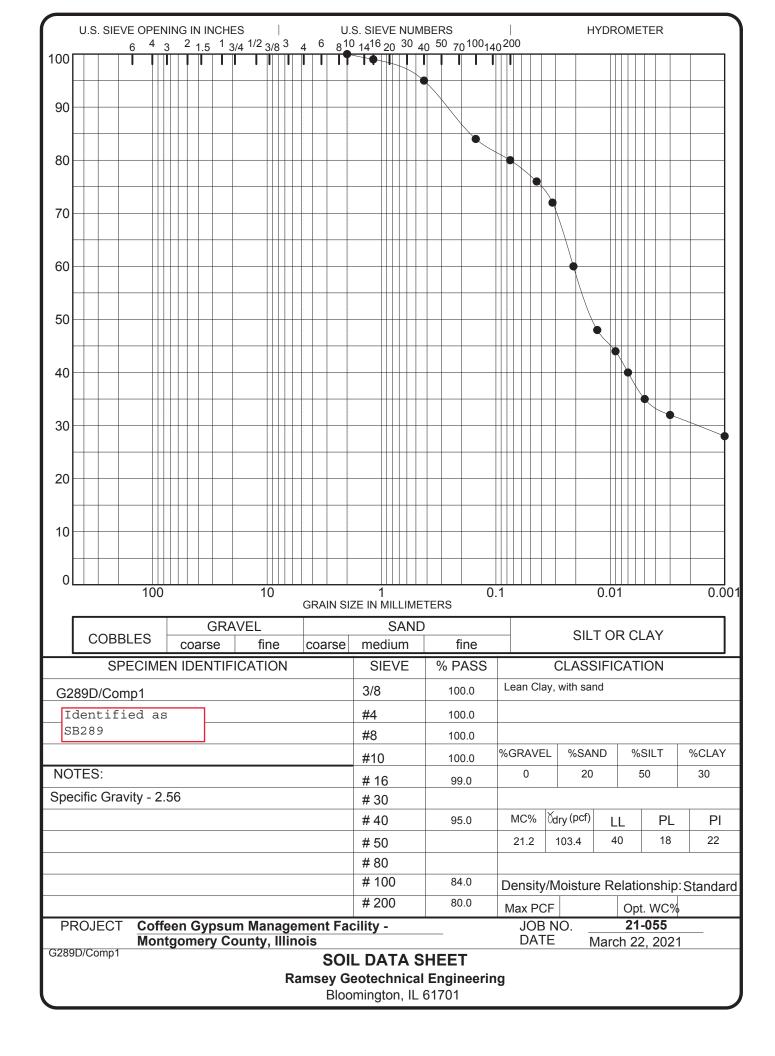


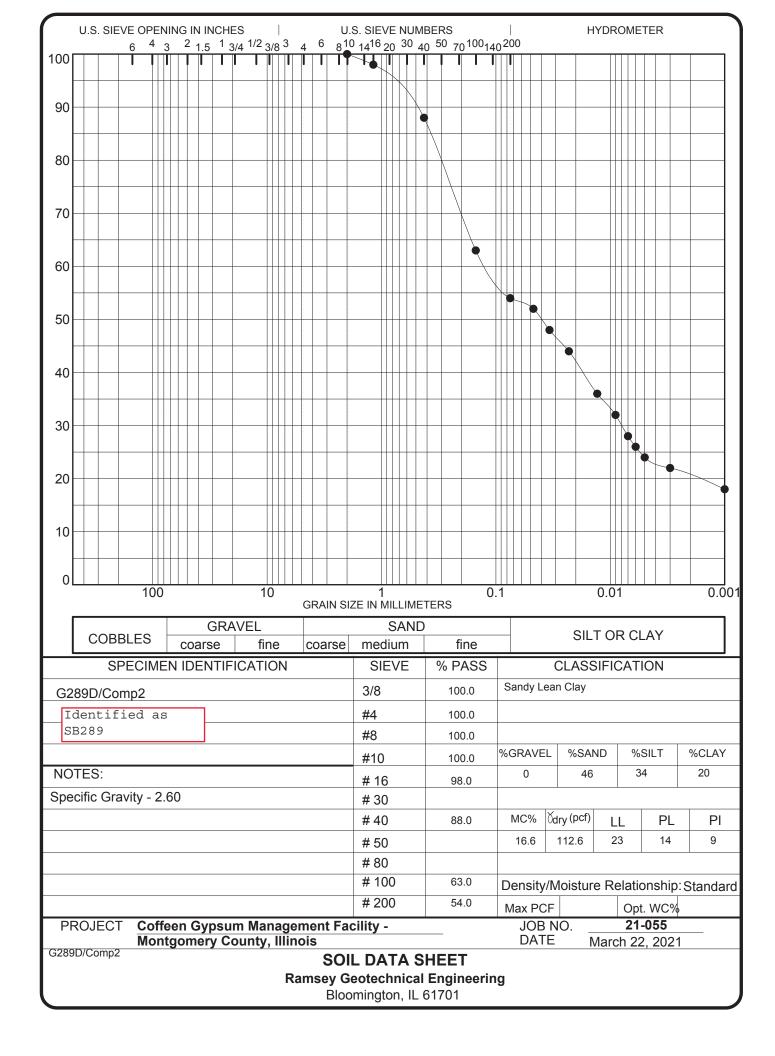


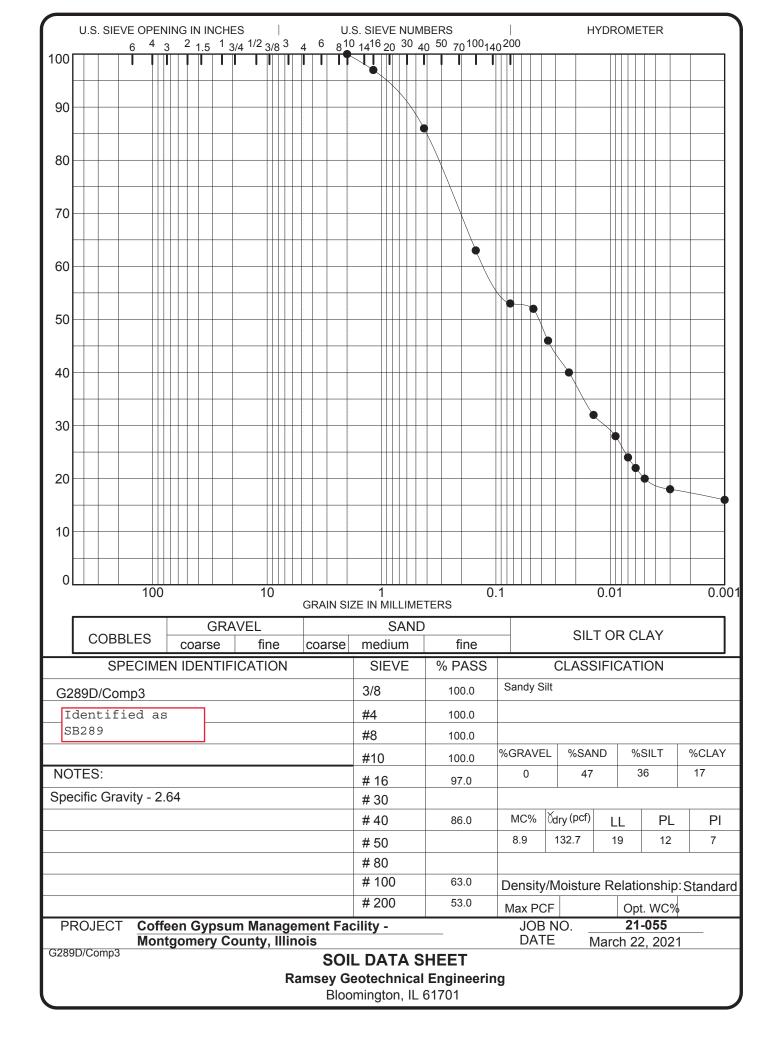


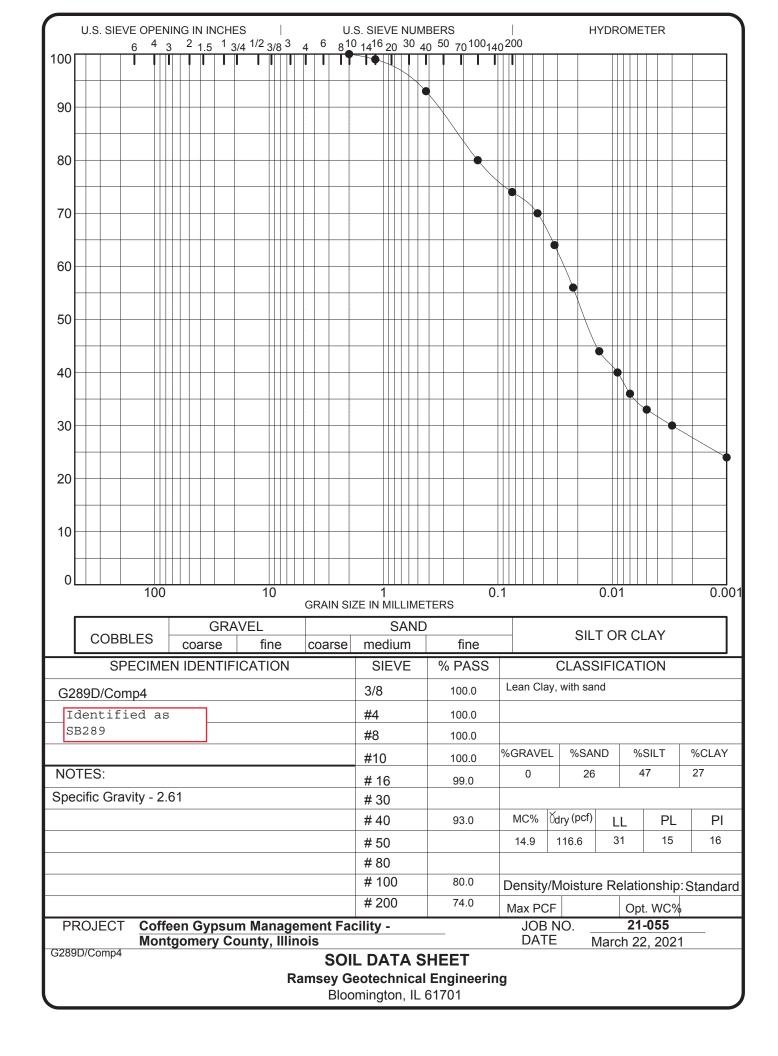


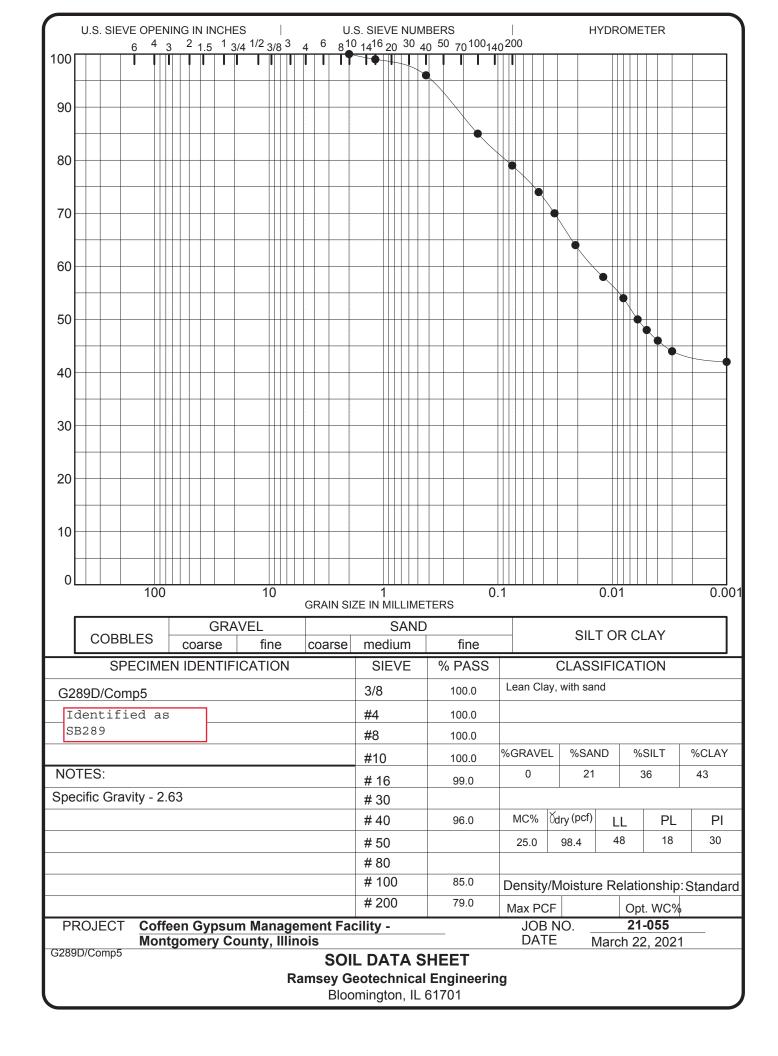












GEOTECHNOLOGY GEOTECHNICAL	REPORT



Via email: dramsey@ramgeoeng.com

April 5, 2021 J037264.01.6002

Mr. Douglas P. Ramsey, P.E. Ramsey Geotechnical Engineering 1701 W. Market Street Bloomington, Illinois 61701

Re: Coffeen Gypsum Management

Montgomery County, Illinois

Dear Mr. Ramsey:

Included in this report are the test results from two Shelby tubes and one bulk sample of gypsum received in our laboratory on March 15, 2021. The samples were tested in general accordance with the test method listed below.

Test to Determine	Method of Test
Water (Moisture) Content of Soils	ASTM D2216
Description and Identification of Soils (Visual-Manual)	ASTM D2488
Hydraulic Conductivity of Soil	ASTM D5084
Using Flexible Wall Permeameter	
Density (Unit Weight) of Soil Specimens	ASTM D7263

We trust this is the information you require. Please contact the undersigned if you have any questions regarding this report.

Respectfully submitted,

GEOTECHNOLOGY, INC.

Janet M. May

Illinois Laboratory Manager

JMM/LPH:jmm

Attachment: Test Result Summary

Hydraulic Conductivity Test Data Sheets Proctor Curve (Provided by Ramsey)

Shelby Tube Logs

Testing Assignment Sheets

11 French Village Industrial Park • Fairview Heights, IL 62208 • 618-345-4811 • Fax 618-345-5108 • geotechnology.com

Ramsey Geotechnical Engineering April 5, 2021 Page 2

TEST RESULT SUMMARY

Coffeen Power Station Gypsum Management Facility Montgomery County, Illinois

			ASTM D2216	ASTM D7263	ASTM I) 5084
Boring Number	Sample Number	Depth, feet	Moisture Content, %	Dry Unit Weight, pcf	Hydraulic Conductivity, cm/sec	Range of Hydraulic Gradient
G275D	ST-7	12.0-14.0	15.8	115.9	1.6 x 10 ⁻⁴	0 - 1.5
G289D	ST-5	8.0-10.0	20.2	105.9	1.1 x 10 ⁻⁸	11.1 - 19.3

Sample Number/ Material	Optimum Moisture Content, %	Maximum Dry Unit Weight, pcf	Percent Compact	Moisture Content, %	Dry Unit Weight, pcf	Hydraulic Conductivity, cm/sec	Range of Hydraulic Gradient
LSN-3783 Gypsum	24.1	85.8	90.9	25.3	78.0	8.9 x 10 ⁻⁴	0.2 - 2.0

		·	ASTM D2488
Boring Number	Sample Number	Depth, feet	Material Description
G275D	ST-7	12.0-14.0	Brown CLAYEY SAND, some gravel – SC
G289D	ST-5	8.0-10.0	Dark to very dark olive-brown, LEAN to FAT CLAY – CL/CH

Notes and abbreviations:

% - Percent cm/sec - Centimeters per second pcf - Pounds per cubic foot

Hanson Professional Services Inc.

Subcontract Agreement: RGE2014 Task Order No. 20E0111A/2000B

					-			UL). :																								
	QC: KLT	QC: KLT			3	CF	IEL	JUL	<u>. C</u>	Or	· L/	ΗD	Ur	(A	10	Γī	- 11	ES	1111	NG.																
	7/21/21	7/21/21	Ro	outi	ne ·	Tes	ting) 1						Со	mp	lex	Τe	estii	ng ¹											An	ıaly	rtica	al To	est	ing	1
	Top Depth	Bottom Depth		cation					/es	Particle Size - Sieves + Hyd	tion	on		Strength				CU Traxail Comp Strength			Soil	Permeability Granular Soil	Hyd Conduct Cohesive Soil			si		ock	Uniaxial Comp Str - Rock						Matter	
	(Info taken from boring logs by	(Info taken from boring logs by	Rimac Comp Strength	nal Cla	Unified Classification	Moisture Content	Liquid / Plastic Limits	Particle Size < #200	Particle Size - Sieves	e - Sie	Standard Compaction	Modified Compaction	ty	Comp	Consolidation Test	avity	Comp	Comp	Shear S	Swell Test for Soil	Collapse Test for Soil	ty Gran	ct Cohe	-actor	vity	IBR and IBV of Soils		Elastic Moduli - Rock	omp Str					Total Organic Content	\sh & 0	
	Ramboll)	Ramboll)	Son	·Man	Cla	re C	/ Pla	e Siz	Siz	e Siz	o p	χ Ö	ensi	fined	lidati	c Gr	axial	axail	ect (Fest	se T	abilli	npuc	afe F	ssisti	d B	est	Mo	a C	ivity		<u>e</u>)rgai	re, ⊿	
Sample ID	,	,	Rimac	Visual-	Unified	Moistu	Liquid	Particle	Particl	Particl	Stande	Modifie	Bulk Density	Uncon	Conso	Specific Gravity	UU Tri	CU Tra	CD Dir	Swell -	Collap	Perme	Hyd C	Shrinkafe Factor	Soil Resistivity	IBR an	CBR Test	Elastic	Uniaxi	Corrosivity	둰	Chloride	Sulfate	Total (Moistu	
G282D											ĺ																									
3A	4	6																														L				
4A	6	8																																		
5A	8	10																																L		
6A	10	12																																L		
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A8	14	16												\vdash																L	L	<u> </u>		L		
Comp1	4	16				Х	Х	4	4	Х			Χ	 		х														<u> </u>		\vdash		<u> </u>		L
G282D																																		-		_
10B	18.8	20						T																								Ī	Г			
Comp2	18.8	20				х	Х			Х		1	Х			х																				
11A	20	22																												_						_
12A	22	24												П																						
13A	24	26												П																						
14A	26	28																																		
15A	28	30																																		
16A	30	31.8																																		
Comp3	20	31.8				Х	х			Х			Х			Х																				
18A	34	36																																		
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24A	46	48																																		
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Comp4	34	52				Х	Х		1	Х			Х			Х														<u> </u>		<u> </u>			-	H
28A	54	54.9						+	1																								H			
29A	56	58																														H	Г			
Comp5	54	58				х	х			х			х			х																				
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3A	4	6												1																		T	П			
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	QC: KLT 7/21/21	QC: KLT 7/21/21	_	outine Testing													т.	_4:												Analytical Testing								
	1/21/21	1/21/21	KC												mp	iex	ıe	Stir	ng I								1	1		Ar	ıaıy	/tic	ai i	ESI				
Sample ID	Top Depth (Info taken from boring logs by Ramboll)	Bottom Depth (Info taken from boring logs by Ramboll)	Rimac Comp Strength	Visual-Manual Classification	Unified Classification	Moisture Content	Liquid / Plastic Limits	Particle Size < #200	Particle Size - Sieves	Particle Size - Sieves + Hyd	Standard Compaction	Modified Compaction	Bulk Density	Unconfined Comp Strength	Consolidation Test	Specific Gravity	UU Triaxial Comp Strength	CU Traxail Comp Strength	CD Direct Shear Strength	Swell Test for Soil	Collapse Test for Soil	Permeability Granular Soil	Hyd Conduct Cohesive Soil	Shrinkafe Factor	Soil Resistivity	IBR and IBV of Soils	CBR Test	Elastic Moduli - Rock	Uniaxial Comp Str - Rock	Corrosivity	Hd	Chloride	Sulfate	Total Organic Content	Moisture, Ash & Organ Matter			
G289D																																						
10A	18	18.6																																				
11A	20	22																																				
Comp2	18	22				Х	Χ			х			х			Х																						
12A	22	24																																				
13A	24	24.7																																				
14A	26	28																																				
15A	28	29.9																																				
16A	30	31.4																																				
17A	32	32.9																																				
Comp3	22	32.9				Х	Χ			Х			Х			Х																		L				
18A	34	36																																_	-			
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30A	58	60																																<u> </u>	T	+		
Comp5	54	60				х	Х			х			х			Х														T				T	T	T		
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Gypsum	grab	grab				х	Х			х	х		х			Х							х*	Re	mo	lde	d pe	erm	eat	ility	,			T	T	T		
Gypsum	grab	grab																																				

Note 1: All testing to be in accordance with Laboratory Testing Specifications.

See Task Order or Attachment for any special instructions regarding scheduled testing.

APPENDIX E GROUNDWATER CONTOUR MAPS, ELEVATIONS, AND VERTICAL GRADIENTS

GROUNDWATER CONTOUR MAPS

DRAWN BY/DATE: SDS 1/23/17 REVIEWED BY/DATE: TBN 1/25/17 APPROVED BY/DATE: JJW 2/7/17 COFFEEN GMF GYPSUM STACK POND (UNIT ID: 103)
AND COFFEEN GMF RECYCLE POND (UNIT ID: 104)
UPPERMOST AQUIFER UNIT
GROUNDWATER ELEVATION CONTOUR MAP
ROUND 1: NOVEMBER 16, 2015

DYNEGY CCR RULE GROUNDWATER MONITORING COFFEEN POWER STATION COFFEEN, ILLINOIS PROJECT NO: 2285



DRAWN BY/DATE: SDS 1/23/17 REVIEWED BY/DATE: TBN 1/25/17 APPROVED BY/DATE: JJW 2/8/17 COFFEEN GMF GYPSUM STACK POND (UNIT ID: 103)
AND COFFEEN GMF RECYCLE POND (UNIT ID: 104)
UPPERMOST AQUIFER UNIT
GROUNDWATER ELEVATION CONTOUR MAP
ROUND 2: FEBRUARY 8, 2016

DYNEGY CCR RULE GROUNDWATER MONITORING COFFEEN POWER STATION COFFEEN, ILLINOIS PROJECT NO: 2285



DRAWN BY/DATE: SDS 1/23/17 REVIEWED BY/DATE: TBN 1/25/17 APPROVED BY/DATE: JJW 2/8/17 COFFEEN GMF GYPSUM STACK POND (UNIT ID: 103)
AND COFFEEN GMF RECYCLE POND (UNIT ID: 104)
UPPERMOST AQUIFER UNIT
GROUNDWATER ELEVATION CONTOUR MAP
ROUND 3: MAY 9, 2016

DYNEGY CCR RULE GROUNDWATER MONITORING COFFEEN POWER STATION COFFEEN, ILLINOIS PROJECT NO: 2285



DRAWN BY/DATE: SDS 1/23/17 REVIEWED BY/DATE: TBN 1/25/17 APPROVED BY/DATE: JJW 2/8/17 COFFEEN GMF GYPSUM STACK POND (UNIT ID: 103)
AND COFFEEN GMF RECYCLE POND (UNIT ID: 104)
UPPERMOST AQUIFER UNIT
GROUNDWATER ELEVATION CONTOUR MAP
ROUND 4: JULY 25, 2016

DYNEGY CCR RULE GROUNDWATER MONITORING COFFEEN POWER STATION COFFEEN, ILLINOIS PROJECT NO: 2285

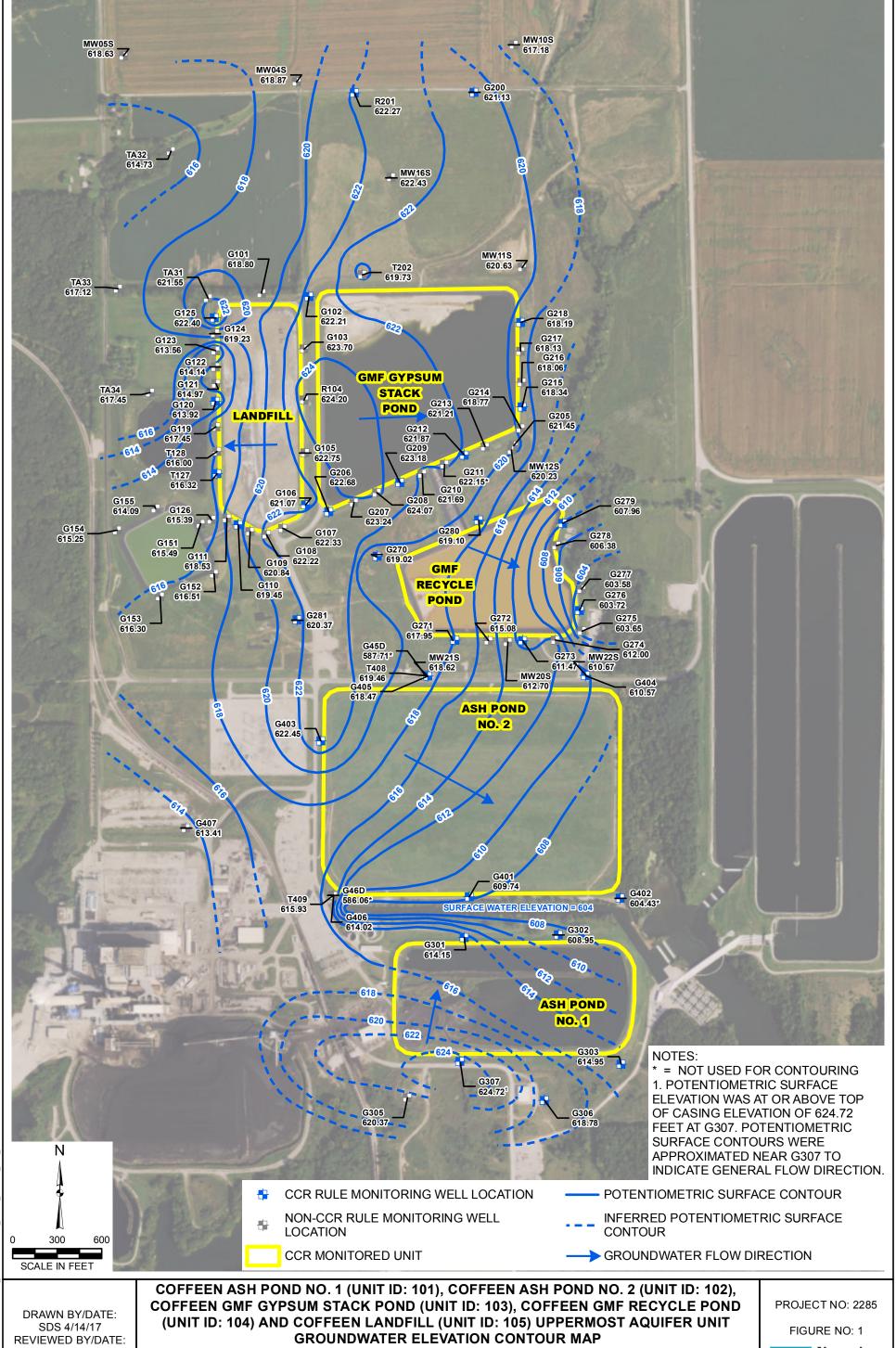


DRAWN BY/DATE: SDS 3/3/17 REVIEWED BY/DATE: TBN 3/3/17 APPROVED BY/DATE: JJW 8/30/17 COFFEEN ASH POND NO. 1 (UNIT ID: 101), COFFEEN ASH POND NO. 2 (UNIT ID: 102), COFFEEN GMF GYPSUM STACK POND (UNIT ID: 103), COFFEEN GMF RECYCLE POND (UNIT ID: 104) AND COFFEEN LANDFILL (UNIT ID: 105) UPPERMOST AQUIFER UNIT GROUNDWATER ELEVATION CONTOUR MAP

ROUND 5: NOVEMBER 12, 2016

DYNEGY CCR RULE GROUNDWATER MONITORING COFFEEN POWER STATION COFFEEN, ILLINOIS PROJECT NO: 2285 FIGURE NO: 1



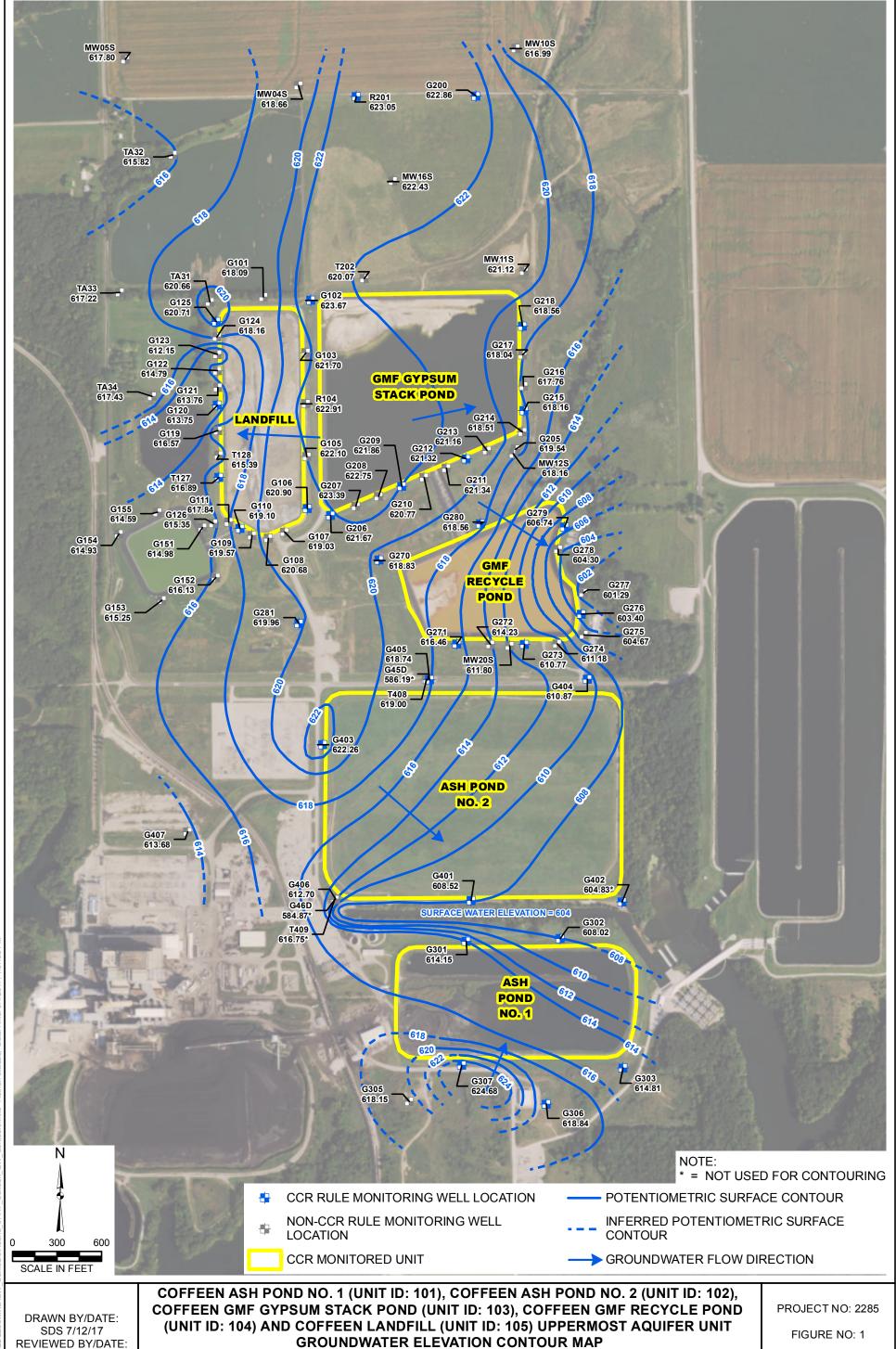


TBN 4/14/17 APPROVED BY/DATE: JJW 8/30/17

ROUND 6: FEBRUARY 4, 2017

DYNEGY CCR RULE GROUNDWATER MONITORING **COFFEEN POWER STATION** COFFEEN, ILLINOIS



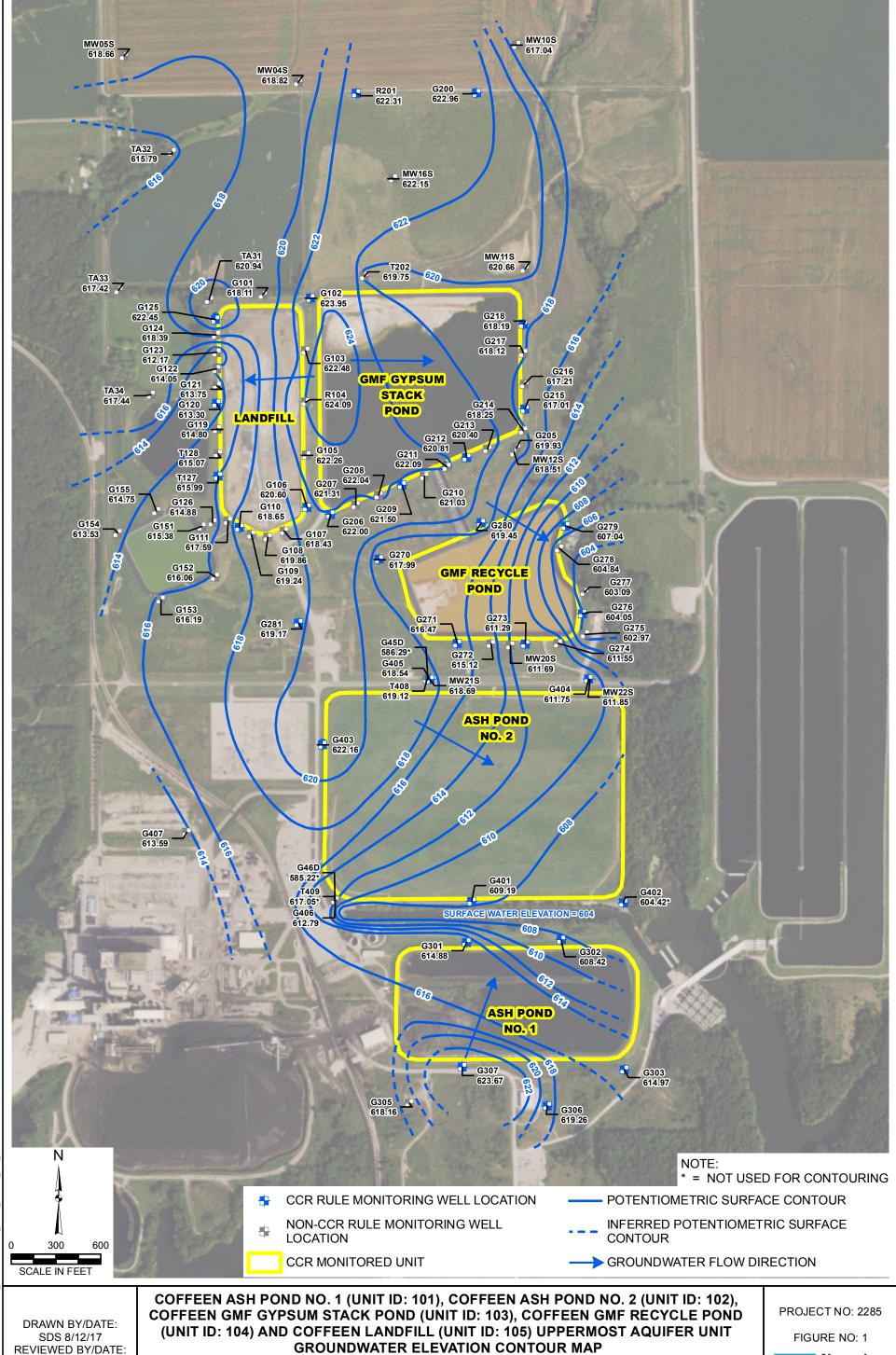


DRAWN BY/DATE: SDS 7/12/17 REVIEWED BY/DATE: TBN 7/12/17 APPROVED BY/DATE: JJW 8/30/17

ROUND 7: MAY 13, 2017

DYNEGY CCR RULE GROUNDWATER MONITORING COFFEEN POWER STATION COFFEEN, ILLINOIS



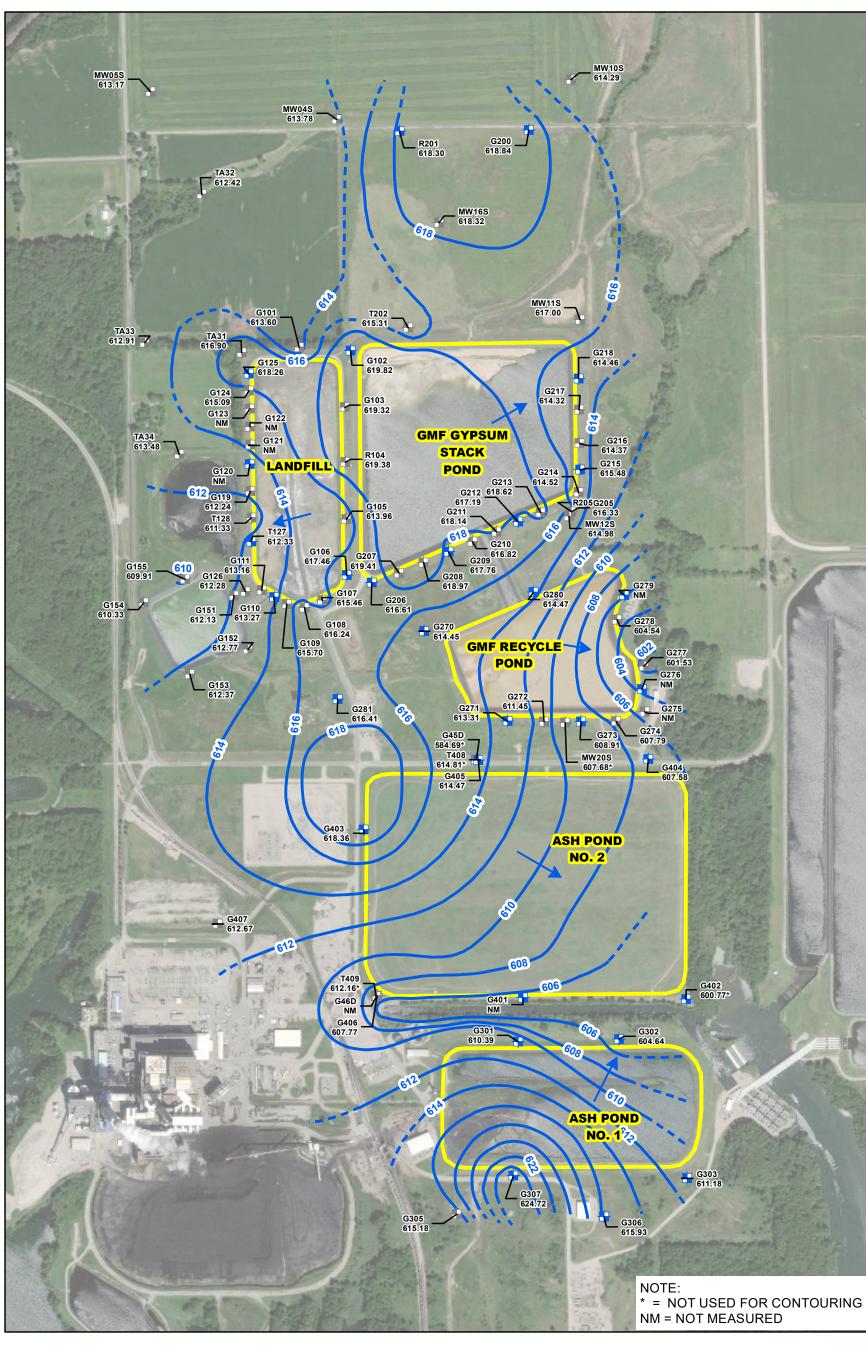


TBN 8/10/17 APPROVED BY/DATE: JJW 8/30/17

ROUND 8: JULY 8, 2017

DYNEGY CCR RULE GROUNDWATER MONITORING **COFFEEN POWER STATION** COFFEEN, ILLINOIS





LEGEND

CCR RULE MONITORING WELL

LOCATION

NON-CCR RULE MONITORING WELL LOCATION
GROUNDWATER ELEVATION

GROUNDWATER ELEVATION CONTOUR (2-FT CONTOUR INTERVAL, NAVD88)

INFERRED GROUNDWATER

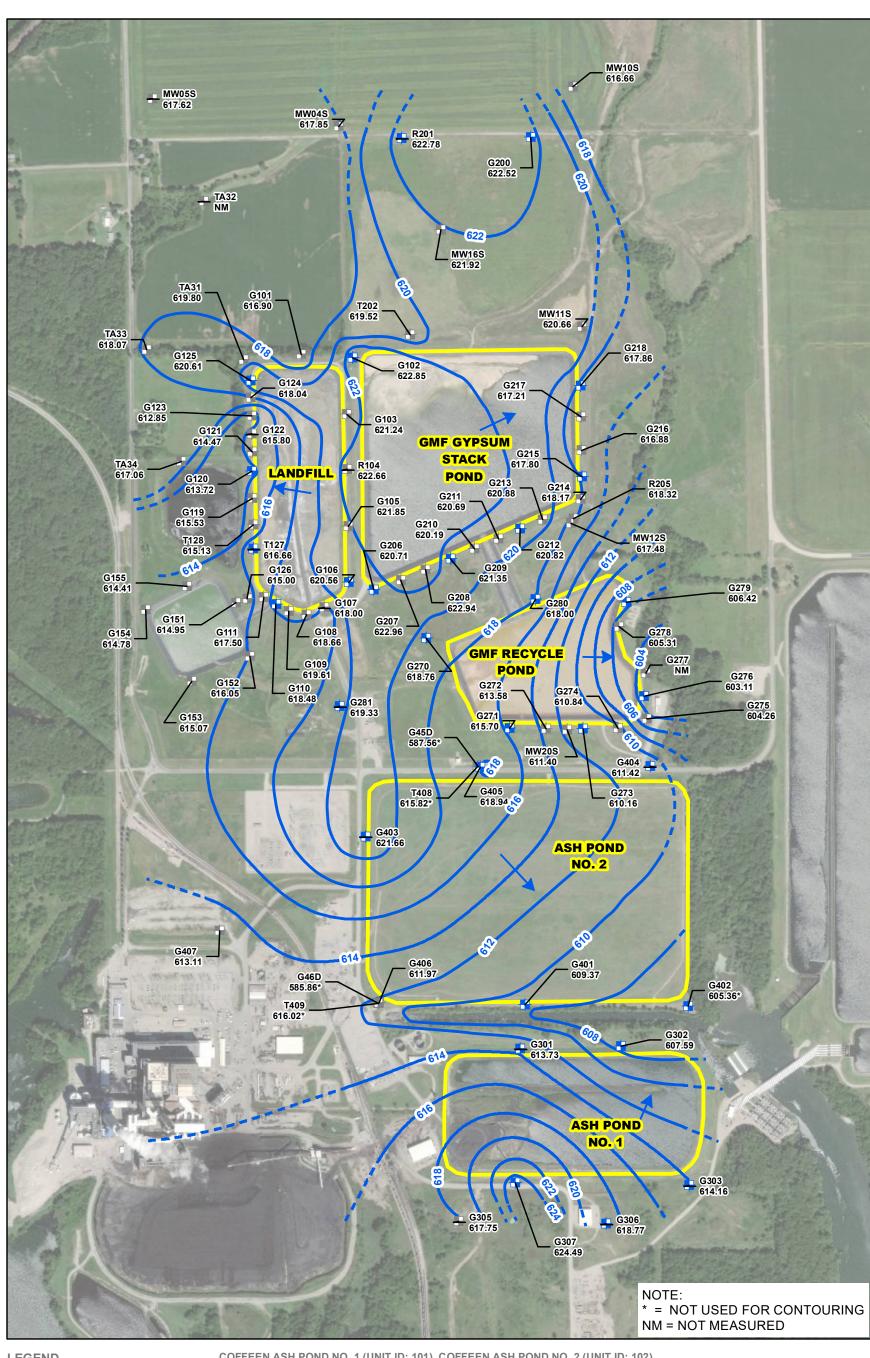
COFFEEN ASH POND NO. 1 (UNIT ID: 101), COFFEEN ASH POND NO. 2 (UNIT ID: 102), COFFEEN GMF GYPSUM STACK POND (UNIT ID: 103), COFFEEN GMF RECYCLE POND (UNIT ID: 104) AND COFFEEN LANDFILL (UNIT ID: 105)
GROUNDWATER ELEVATION CONTOUR MAP

OCTOBER 21, 2017

CCR RULE GROUNDWATER MONITORING COFFEEN POWER STATION COFFEEN, ILLINOIS









- CCR RULE MONITORING WELL LOCATION
- NON-CCR RULE MONITORING WELL LOCATION **GROUNDWATER ELEVATION**
- CONTOUR (2-FT CONTOUR INTERVAL, NAVD88)

CCR MONITORED UNIT

- INFERRED GROUNDWATER ELEVATION CONTOUR GROUNDWATER FLOW DIRECTION
- COFFEEN ASH POND NO. 1 (UNIT ID: 101), COFFEEN ASH POND NO. 2 (UNIT ID: 102), COFFEEN GMF GYPSUM STACK POND (UNIT ID: 103), COFFEEN GMF RECYCLE POND (UNIT ID: 104) AND COFFEEN LANDFILL (UNIT ID: 105)
 GROUNDWATER ELEVATION CONTOUR MAP

MAY 8, 2018 CCR RULE GROUNDWATER MONITORING COFFEEN POWER STATION

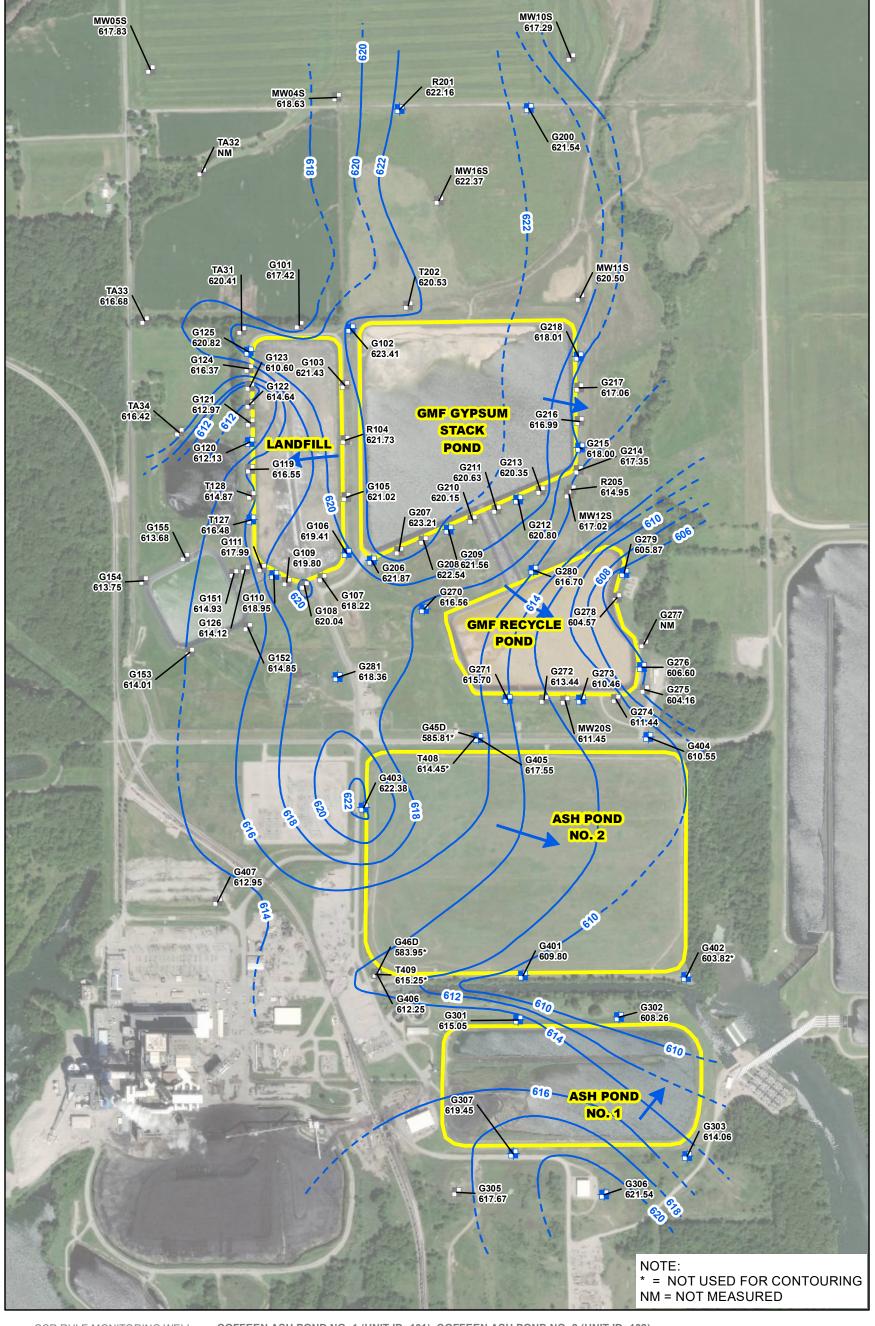
COFFEEN, ILLINOIS

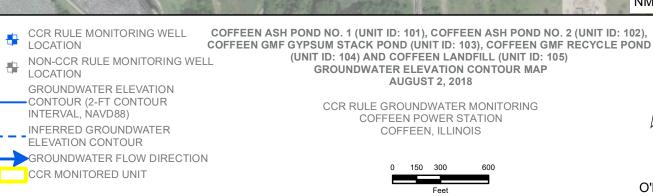




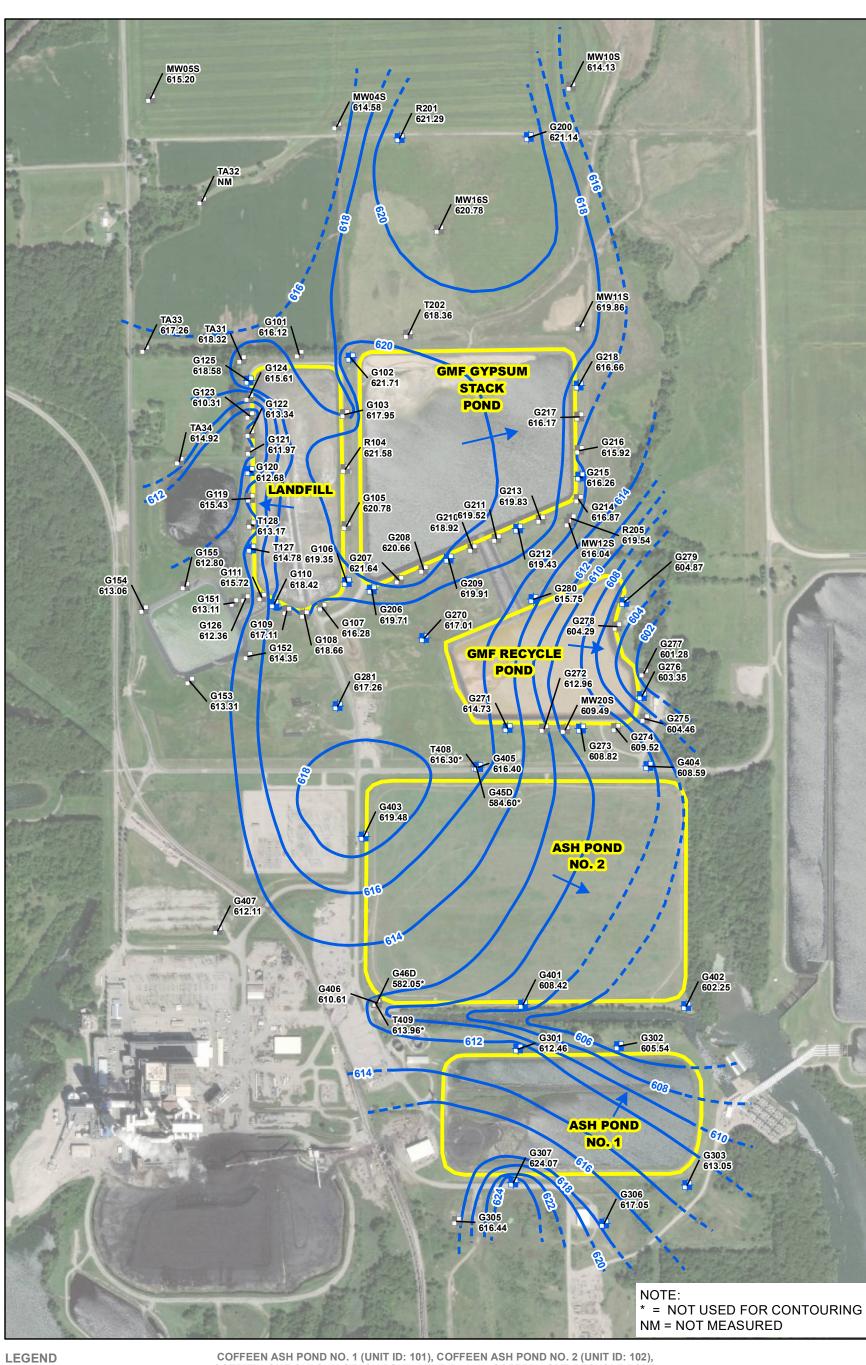
FILE_NO. 70099 DATE 8/1/2018

O'BRIEN & GERE ENGINEERS, INC.





O'BRIEN & GERE ENGINEERS, INC.



CCR RULE MONITORING WELL LOCATION

NON-CCR RULE MONITORING WELL LOCATION

GROUNDWATER ELEVATION CONTOUR (2-FT CONTOUR INTERVAL, NAVD88)

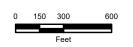
INFERRED GROUNDWATER ELEVATION CONTOUR GROUNDWATER FLOW DIRECTION

CCR MONITORED UNIT

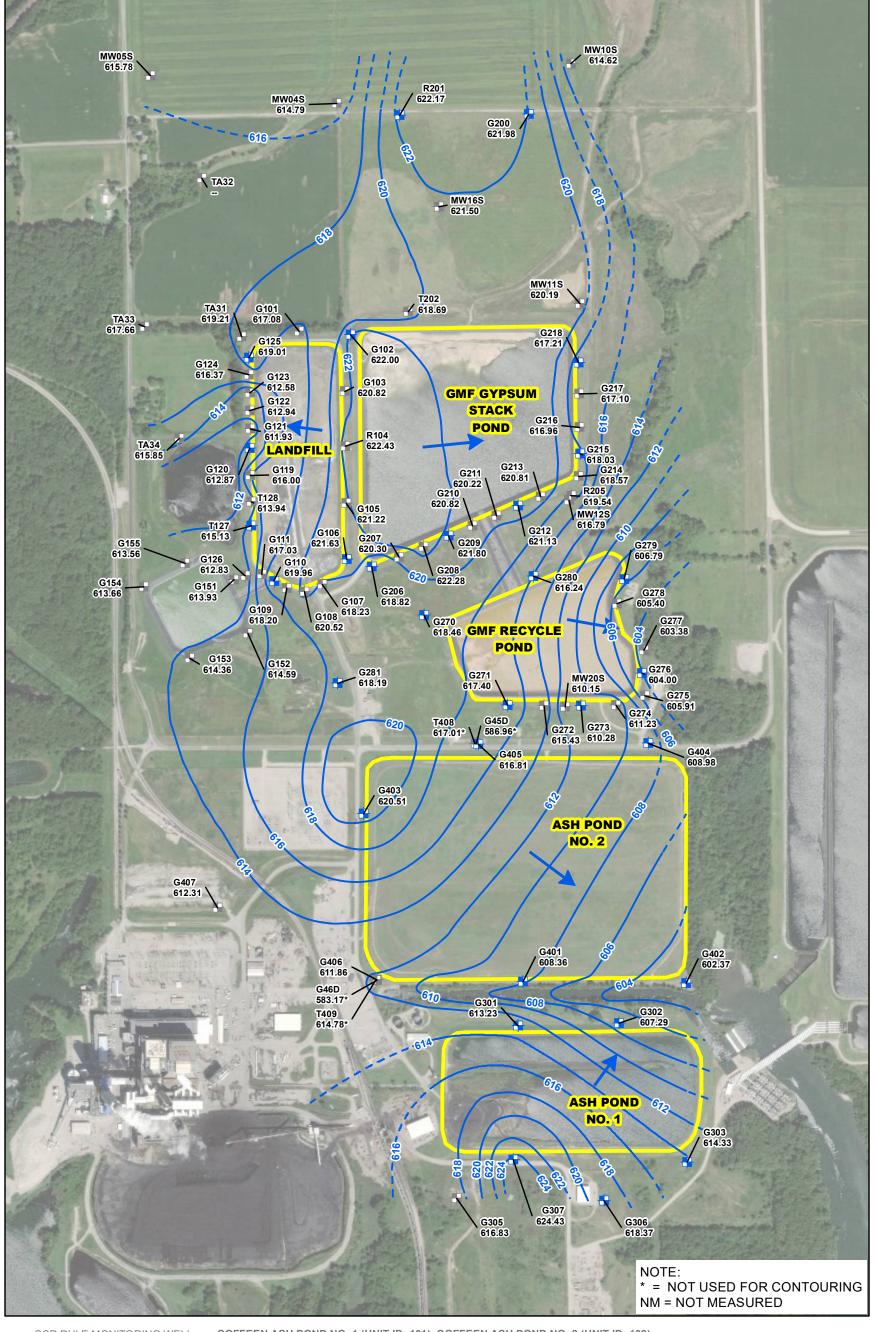
COFFEEN ASH POND NO. 1 (UNIT ID: 101), COFFEEN ASH POND NO. 2 (UNIT ID: 102), COFFEEN GMF GYPSUM STACK POND (UNIT ID: 103), COFFEEN GMF RECYCLE POND (UNIT ID: 104) AND COFFEEN LANDFILL (UNIT ID: 105)
GROUNDWATER ELEVATION CONTOUR MAP

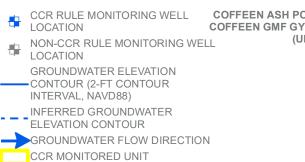
OCTOBER 23, 2018

CCR RULE GROUNDWATER MONITORING COFFEEN POWER STATION COFFEEN, ILLINOIS









COFFEEN ASH POND NO. 1 (UNIT ID: 101), COFFEEN ASH POND NO. 2 (UNIT ID: 102), COFFEEN GMF GYPSUM STACK POND (UNIT ID: 103), COFFEEN GMF RECYCLE POND (UNIT ID: 104) AND COFFEEN LANDFILL (UNIT ID: 105)

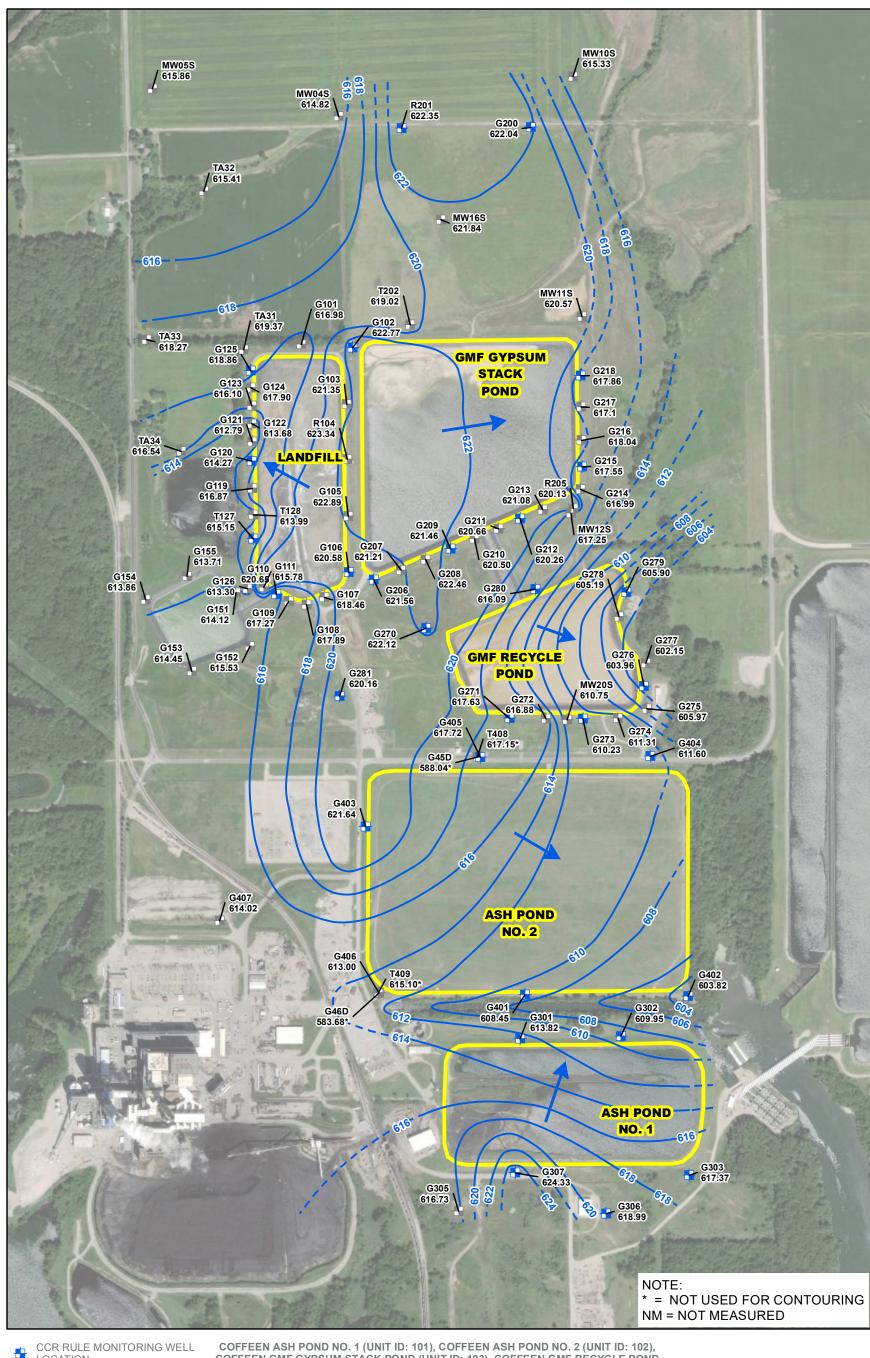
GROUNDWATER ELEVATION CONTOUR MAP

CCR RULE GROUNDWATER MONITORING COFFEEN POWER STATION COFFEEN, ILLINOIS

JANUARY 15, 2019





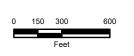




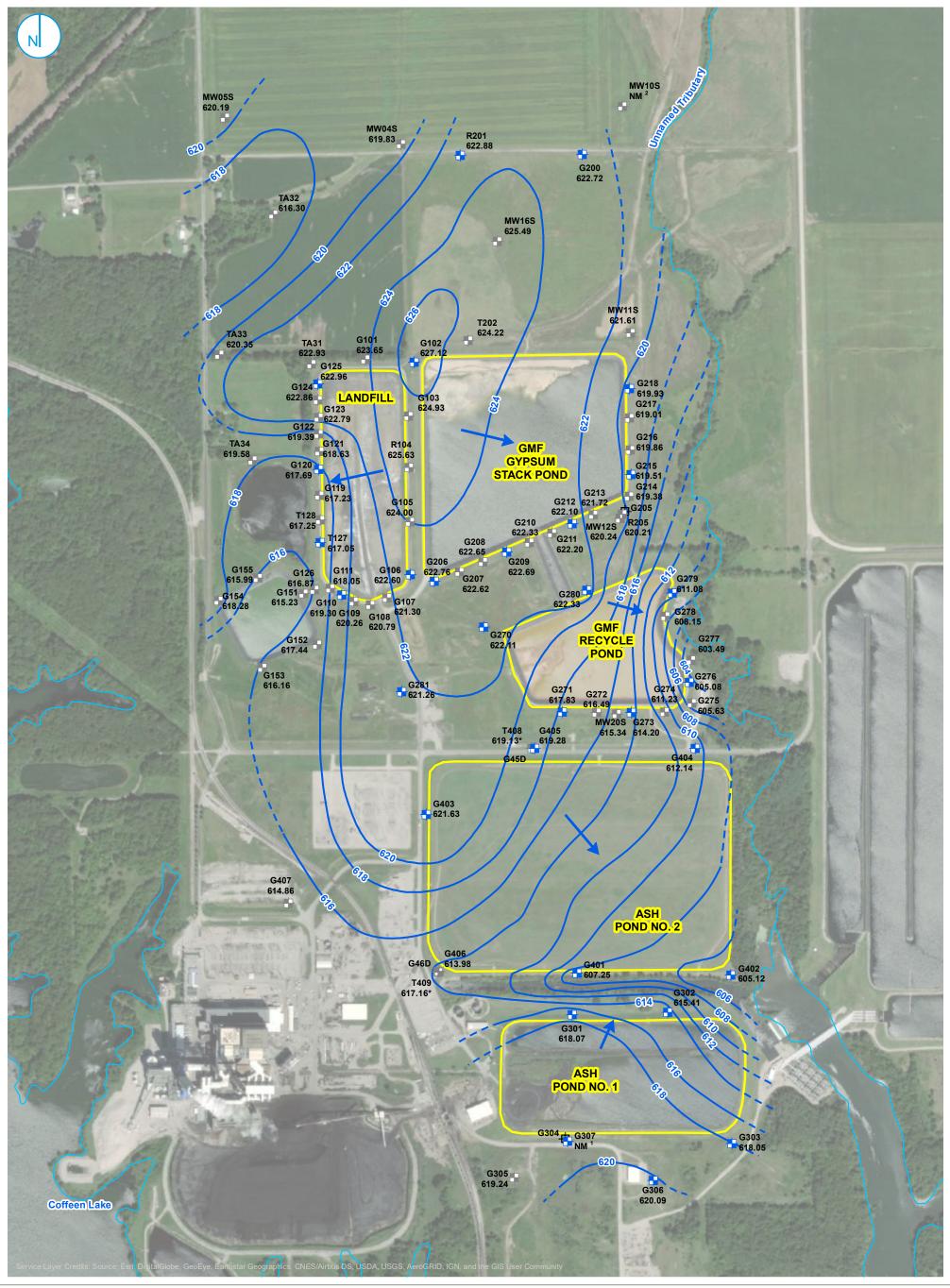
COFFEEN ASH POND NO. 1 (UNIT ID: 101), COFFEEN ASH POND NO. 2 (UNIT ID: 102), COFFEEN GMF GYPSUM STACK POND (UNIT ID: 103), COFFEEN GMF RECYCLE POND (UNIT ID: 104) AND COFFEEN LANDFILL (UNIT ID: 105)

GROUNDWATER ELEVATION CONTOUR MAP AUGUST 5, 2019

CCR RULE GROUNDWATER MONITORING COFFEEN POWER STATION COFFEEN, ILLINOIS







- CCR RULE MONITORING WELL LOCATION
- ♣ NON-CCR RULE MONITORING WELL LOCATION
- ABANDONED MONITORING WELL
- GROUNDWATER ELEVATION CONTOUR (2-FT CONTOUR INTERVAL, NAVD88)
- - INFERRED GROUNDWATER ELEVATION CONTOUR
- GROUNDWATER FLOW DIRECTION

CCR UNIT BOUNDARY

SURFACE WATER FEATURE

* = NOT USED FOR CONTOURING
NM = NOT MEASURED

1 G307 WAS FROZEN DURING THE
JANUARY 20, 2020 SAMPLING EVENT AND
WATER LEVEL COULD NOT BE
COLLECTED.

2 MW10S WAS DAMAGED PRIOR TO THE

² MW10S WAS DAMAGED PRIOR TO THE JANUARY 20, 2020 SAMPLING EVENT AND WATER LEVEL COULD NOT BE COLLECTED. GROUNDWATER ELEVATION CONTOUR MAP JANUARY 20, 2020

CCR RULE GROUNDWATER MONITORING

COFFEEN POWER STATION

COFFEEN, ILLINOIS

RAMBOLL US CORPORATION A RAMBOLL COMPANY



TABLE E-1. GROUNDWATER ELEVATIONS

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G045D	11/12/2016	584.91
G045D	02/04/2017	587.71
G045D	05/13/2017	586.19
G045D	07/08/2017	586.29
G045D	10/21/2017	584.69
G045D	05/08/2018	587.56
G045D	08/02/2018	585.81
G045D	10/23/2018	584.60
G045D	01/15/2019	586.96
G045D	08/05/2019	588.04
G045D	08/10/2020	614.21
G045D	01/20/2021	614.60
G045D	04/20/2021	614.32
G045D	07/26/2021	613.58
G045D	08/16/2021	613.83
G046D	11/12/2016	583.59
G046D	02/04/2017	586.06
G046D	05/13/2017	584.87
G046D	07/08/2017	585.22
G046D	05/08/2018	585.86
G046D	08/02/2018	583.95
G046D	10/23/2018	582.05
G046D	01/15/2019	583.17
G046D	08/05/2019	583.68
G046D	08/10/2020	609.00
G046D	01/20/2021	610.49
G046D	04/20/2021	611.06
G046D	07/26/2021	607.21
G046D	08/16/2021	608.17
G101	01/20/2015	614.48
G101	04/08/2015	618.87
G101	07/23/2015	618.53
G101	10/06/2015	617.15
G101	11/16/2015	612.95
G101	02/08/2016	618.46
G101	05/09/2016	618.89
G101	07/25/2016	618.44
G101	11/12/2016	617.65
G101	02/04/2017	618.80
G101	05/13/2017	618.09
G101	07/08/2017	618.11
G101	10/21/2017	613.60
G101	05/08/2018	616.90
G101	08/02/2018	617.42
G101	10/23/2018	616.12
G101	01/15/2019	617.08



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G101	08/05/2019	616.98
G101	01/20/2020	623.65
G101	08/10/2020	616.70
G101	10/15/2020	613.61
G101	01/20/2021	617.20
G101	01/28/2021	617.80
G101	04/20/2021	622.85
G101	07/26/2021	619.94
G101	08/16/2021	619.95
G102	01/20/2015	619.18
G102	04/08/2015	622.06
G102	10/06/2015	622.02
G102	11/16/2015	618.96
G102	02/08/2016	624.04
G102	05/09/2016	625.34
G102	07/25/2016	623.92
G102	11/12/2016	623.39
G102	02/04/2017	622.21
G102	05/13/2017	623.67
G102	07/08/2017	623.95
G102	10/21/2017	619.82
G102	01/26/2018	621.79
G102	05/08/2018	622.85
G102	08/02/2018	623.41
G102	10/23/2018	621.71
G102	01/15/2019	622.00
G102	08/05/2019	622.77
G102	01/20/2020	627.12
G102	08/10/2020	621.72
G102	10/15/2020	618.94
G102	01/20/2021	619.79
G102	01/26/2021	621.71
G102	04/20/2021	623.86
G102	05/03/2021	624.28
G102	05/17/2021	623.83
G102	06/09/2021	623.09
G102	06/23/2021	621.22
G102	07/12/2021	622.92
G102	07/26/2021	622.97
G102	08/16/2021	622.69
G103	01/20/2015	620.82
G103	04/08/2015	622.58
G103	07/23/2015	621.70
G103	10/06/2015	620.69
G103	02/08/2016	621.68
G103	05/09/2016	623.26
0103	03/03/2010	023.20



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G103	07/25/2016	622.88
G103	11/12/2016	621.21
G103	02/04/2017	623.70
G103	05/13/2017	621.70
G103	07/08/2017	622.48
G103	10/21/2017	619.32
G103	05/08/2018	621.24
G103	08/02/2018	621.43
G103	10/23/2018	617.95
G103	01/15/2019	620.82
G103	08/05/2019	621.35
G103	01/20/2020	624.93
G103	08/10/2020	622.45
G103	10/15/2020	618.91
G103	01/20/2021	621.01
G103	01/28/2021	621.38
G103	04/20/2021	623.84
G103	07/26/2021	624.14
G103	08/16/2021	624.29
G105	01/20/2015	621.95
G105	04/08/2015	623.73
G105	07/23/2015	622.72
G105	10/06/2015	621.65
G105	02/08/2016	623.03
G105	05/09/2016	623.60
G105	07/25/2016	622.08
G105	11/12/2016	622.13
G105	02/04/2017	622.75
G105	05/13/2017	622.10
G105	07/08/2017	622.26
G105	10/21/2017	613.96
G105	05/08/2018	621.85
G105	08/02/2018	621.02
G105	10/23/2018	620.78
G105	01/15/2019	621.22
G105	08/05/2019	622.89
G105	01/20/2020	624.00
G105	08/10/2020	623.11
G105	10/15/2020	620.10
G105	01/20/2021	622.21
G105	01/28/2021	622.33
G105	04/20/2021	623.23
G105	07/26/2021	623.76
G105	08/16/2021	623.70
G106	01/20/2015	620.45
G106	04/08/2015	622.19
3100	0 1,00,2013	V22.17



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G106	07/23/2015	621.43
G106	10/06/2015	620.50
G106	11/16/2015	619.32
G106	02/08/2016	621.55
G106	05/09/2016	622.11
G106	07/25/2016	620.62
G106	11/12/2016	620.65
G106	02/04/2017	621.07
G106	05/13/2017	620.90
G106	07/08/2017	620.60
G106	10/21/2017	617.46
G106	05/08/2018	620.56
G106	08/02/2018	619.41
G106	10/23/2018	619.35
G106	01/15/2019	621.63
G106	08/05/2019	620.58
G106	01/20/2020	622.60
G106	08/10/2020	620.48
G106	10/14/2020	618.19
G106	01/20/2021	620.90
G106	01/26/2021	620.90
G106	04/20/2021	621.69
G106	06/29/2021	621.95
G106	07/26/2021	621.88
G106	08/16/2021	621.90
G107	01/20/2015	619.23
G107	04/08/2015	620.85
G107	07/23/2015	620.15
G107	10/06/2015	619.10
G107	02/08/2016	620.26
G107	05/09/2016	620.78
G107	07/25/2016	618.37
G107	11/12/2016	618.72
G107	02/04/2017	622.33
G107	05/13/2017	619.03
G107	07/08/2017	618.43
G107	10/21/2017	615.46
G107	05/08/2018	618.00
G107	08/02/2018	618.22
G107	10/23/2018	616.28
G107	01/15/2019	618.23
G107	08/05/2019	618.46
G107	01/20/2020	621.30
G107	08/10/2020	618.68
G107	10/14/2020	616.56
G107	01/20/2021	619.58



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G107	01/28/2021	619.74
G107	04/20/2021	620.19
G107	07/26/2021	620.26
G107	08/16/2021	620.39
G108	01/19/2015	618.42
G108	04/08/2015	620.31
G108	07/24/2015	621.22
G108	10/07/2015	618.92
G108	02/08/2016	619.53
G108	05/09/2016	620.15
G108	07/25/2016	619.78
G108	11/12/2016	620.46
G108	02/04/2017	622.22
G108	05/13/2017	620.68
G108	07/08/2017	619.86
G108	10/21/2017	616.24
G108	05/08/2018	618.66
G108	08/02/2018	620.04
G108	10/23/2018	618.66
G108	01/15/2019	620.52
G108	08/05/2019	617.89
G108	01/20/2020	620.79
G108	08/10/2020	617.86
G108	10/14/2020	616.02
G108	01/20/2021	618.72
G108	01/28/2021	618.82
G108	04/20/2021	619.37
G108	07/26/2021	619.40
G108	08/16/2021	619.66
G109	01/19/2015	617.78
G109	04/08/2015	619.71
G109	07/24/2015	620.41
G109	10/06/2015	618.12
G109	02/08/2016	618.94
G109	05/09/2016	619.56
G109	07/25/2016	619.11
G109	11/12/2016	619.35
G109	02/04/2017	620.84
G109	05/13/2017	619.57
G109	07/08/2017	619.24
G109	10/21/2017	615.70
G109	05/08/2018	619.61
G109	08/02/2018	619.80
G109	10/23/2018	617.11
G109	01/15/2019	618.20
G109	08/05/2019	617.27



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G109	01/20/2020	620.26
G109	08/10/2020	617.16
G109	10/14/2020	615.52
G109	01/20/2021	617.91
G109	01/28/2021	618.58
G109	04/20/2021	618.59
G109	07/26/2021	618.68
G109	08/16/2021	618.96
G110	01/19/2015	616.76
G110	04/08/2015	618.60
G110	07/24/2015	619.55
G110	10/07/2015	617.70
G110	11/16/2015	616.55
G110	02/08/2016	617.88
G110	05/09/2016	618.53
G110	07/25/2016	617.64
G110	11/12/2016	618.86
G110	02/04/2017	619.45
G110	05/13/2017	619.10
G110	07/08/2017	618.65
G110	10/21/2017	613.27
G110	01/26/2018	616.74
G110	05/08/2018	618.48
G110	08/02/2018	618.95
G110	10/23/2018	618.42
G110	01/15/2019	619.96
G110	08/05/2019	620.65
G110	01/20/2020	619.30
G110	08/10/2020	616.14
G110	10/14/2020	614.90
G110	01/20/2021	616.81
G110	01/28/2021	616.81
G110	04/20/2021	617.71
G110	07/26/2021	617.76
G110	08/16/2021	617.97
G111	01/19/2015	615.93
G111	04/08/2015	617.48
G111	07/24/2015	618.03
G111	10/07/2015	616.79
G111	02/08/2016	616.92
G111	05/09/2016	617.52
G111	07/25/2016	617.35
G111	11/12/2016	617.61
G111	02/04/2017	618.53
G111	05/13/2017	617.84
	 	



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G111	10/21/2017	613.16
G111	05/08/2018	617.50
G111	08/02/2018	617.99
G111	10/23/2018	615.72
G111	01/15/2019	617.03
G111	08/05/2019	615.78
G111	01/20/2020	618.05
G111	08/10/2020	615.59
G111	10/14/2020	614.40
G111	01/20/2021	615.84
G111	01/28/2021	616.67
G111	04/20/2021	616.73
G111	07/26/2021	616.84
G111	08/16/2021	618.10
G119	01/19/2015	615.64
G119	04/08/2015	615.86
G119	07/23/2015	616.55
G119	10/06/2015	615.31
G119	02/08/2016	615.83
G119	05/09/2016	615.87
G119	07/25/2016	614.73
G119	11/12/2016	616.03
G119	02/04/2017	617.45
G119	05/13/2017	616.57
G119	07/08/2017	614.80
G119	10/21/2017	612.24
G119	05/08/2018	615.53
G119	08/02/2018	616.55
G119	10/23/2018	615.43
G119	01/15/2019	616.00
G119	08/05/2019	616.87
G119	01/20/2020	617.23
G119	08/10/2020	616.02
G119	10/13/2020	615.16
G119	01/20/2021	616.09
G119	01/28/2021	616.14
G119	04/20/2021	616.80
G119	07/26/2021	616.62
G119	08/16/2021	616.75
G120	01/19/2015	612.75
G120	04/08/2015	613.43
G120	07/23/2015	613.47
G120	10/06/2015	612.94
G120	11/16/2015	612.37
G120	02/08/2016	613.06
G120	05/09/2016	613.37
	1	



Sample Date	Groundwater Elevation (ft NAVD88)
	612.87
	613.45
	613.92
	613.75
	613.30
	612.69
	613.72
	612.13
	612.68
	612.87
	618.15
	614.27
	617.69
	618.23
	615.22
	614.39
	615.80
	615.80
	617.55
	616.95
	617.19
	613.63
	614.63
	614.09
10/06/2015	613.31
02/08/2016	614.10
05/09/2016	614.81
07/25/2016	613.62
11/12/2016	613.33
02/04/2017	614.97
05/13/2017	613.76
07/08/2017	613.75
05/08/2018	614.47
08/02/2018	612.97
10/23/2018	611.97
01/15/2019	611.93
08/05/2019	612.79
01/20/2020	618.63
08/10/2020	615.02
10/13/2020	613.69
01/20/2021	615.44
01/27/2021	616.14
04/20/2021	618.73
07/26/2021	616.79
08/16/2021	617.27
01/19/2015	610.79
	05/09/2016 07/25/2016 11/12/2016 02/04/2017 05/13/2017 07/08/2017 05/08/2018 08/02/2018 10/23/2018 10/23/2018 01/15/2019 08/05/2019 01/20/2020 08/10/2020 10/13/2020 01/20/2021 01/27/2021 04/20/2021 08/16/2021



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G122	04/08/2015	615.94
G122	07/23/2015	615.26
G122	10/06/2015	614.39
G122	02/08/2016	615.52
G122	05/09/2016	616.84
G122	07/25/2016	614.06
G122	11/12/2016	614.64
G122	02/04/2017	614.14
G122	05/13/2017	614.79
G122	07/08/2017	614.05
G122	05/08/2018	615.80
G122	08/02/2018	614.64
G122	10/23/2018	613.34
G122	01/15/2019	612.94
G122	08/05/2019	613.68
G122	01/20/2020	619.39
G122	08/10/2020	613.48
G122	10/13/2020	611.41
G122	01/20/2021	613.99
G122	01/27/2021	614.08
G122	04/20/2021	620.41
G122	07/26/2021	616.92
G122	08/16/2021	617.28
G123	01/19/2015	610.84
G123	04/08/2015	612.41
G123	07/23/2015	612.76
G123	10/06/2015	611.89
G123	02/08/2016	611.74
G123	05/09/2016	611.73
G123	07/25/2016	611.91
G123	11/12/2016	612.03
G123	02/04/2017	613.56
G123	05/13/2017	612.15
G123	07/08/2017	612.17
G123	05/08/2018	612.85
G123	08/02/2018	610.60
G123	10/23/2018	610.31
G123	01/15/2019	612.58
G123	08/05/2019	616.10
G123	01/20/2020	622.79
G123	08/10/2020	615.96
G123	10/14/2020	613.01
G123	01/20/2021	615.92
G123	01/27/2021	616.24
G123	04/20/2021	622.41
G123	07/26/2021	619.35



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G123	08/16/2021	617.73
G124	01/19/2015	615.27
G124	04/08/2015	617.85
G124	07/23/2015	618.25
G124	10/06/2015	617.27
G124	02/08/2016	616.47
G124	05/09/2016	616.81
G124	07/25/2016	618.27
G124	11/12/2016	617.85
G124	02/04/2017	619.23
G124	05/13/2017	618.16
G124	07/08/2017	618.39
G124	10/21/2017	615.09
G124	05/08/2018	618.04
G124	08/02/2018	616.37
G124	10/23/2018	615.61
G124	01/15/2019	616.37
G124	08/05/2019	617.90
G124	01/20/2020	622.86
G124	08/10/2020	615.53
G124	10/14/2020	612.59
G124	01/20/2021	615.96
G124	01/27/2021	616.10
G124	04/20/2021	622.44
G124	07/26/2021	619.05
G124	08/16/2021	619.43
G125	01/19/2015	617.83
G125	04/08/2015	620.45
G125	07/23/2015	620.71
G125	10/06/2015	619.66
G125	11/16/2015	614.60
G125	02/08/2016	619.95
G125	05/09/2016	620.22
G125	07/25/2016	621.53
G125	11/12/2016	620.46
G125	02/04/2017	622.40
G125	05/13/2017	620.71
G125	07/08/2017	622.45
G125	10/21/2017	618.26
G125	01/26/2018	613.88
G125	05/08/2018	620.61
G125	08/02/2018	620.82
G125	10/23/2018	618.58
G125	01/15/2019	619.01
G125	08/05/2019	618.86
G125	01/20/2020	622.96



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G125	05/05/2020	623.39
G125	08/10/2020	615.38
G125	10/14/2020	612.46
G125	01/20/2021	615.66
G125	01/27/2021	615.66
G125	04/20/2021	622.46
G125	07/26/2021	619.11
G125	08/16/2021	619.49
G126	01/20/2015	615.22
G126	04/08/2015	616.45
G126	07/23/2015	616.34
G126	10/07/2015	614.13
G126	02/08/2016	616.12
G126	05/09/2016	616.58
G126	07/25/2016	614.82
G126	11/12/2016	615.19
G126	02/04/2017	615.39
G126	05/13/2017	615.35
G126	07/08/2017	614.88
G126	10/21/2017	612.28
G126	05/08/2018	615.00
G126	08/02/2018	614.12
G126	10/23/2018	612.36
G126	01/15/2019	612.83
G126	08/05/2019	613.30
G126	01/20/2020	616.87
G126	08/10/2020	614.91
G126	10/14/2020	613.97
G126	01/20/2021	614.95
G126	01/29/2021	615.98
G126	04/20/2021	615.68
G126	07/26/2021	615.85
G126	08/16/2021	616.05
G151	07/23/2015	615.43
G151	10/06/2015	614.86
G151	11/12/2016	614.85
G151	02/04/2017	615.49
G151	05/13/2017	614.98
G151 G151	07/08/2017	615.38
G151 G151	10/21/2017	612.13
G151 G151	05/08/2018	614.95
G151 G151	08/02/2018	614.93
G151 G151	10/23/2018	613.11
G151 G151	01/15/2019	613.93
G151	08/05/2019	614.12
G151	01/20/2020	615.23



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G151	08/10/2020	614.11
G151	10/13/2020	613.39
G151 G151	01/20/2021	613.86
G151	02/01/2021	615.07
G151	04/20/2021	614.53
G151	07/26/2021	614.83
G151	08/16/2021	615.06
G152	07/23/2015	616.47
G152	10/06/2015	614.06
G152	11/12/2016	615.74
G152	02/04/2017	616.51
G152	05/13/2017	616.13
G152	07/08/2017	616.06
G152	10/21/2017	612.77
G152	05/08/2018	616.05
G152	08/02/2018	614.85
G152	10/23/2018	614.35
G152	01/15/2019	614.59
G152	08/05/2019	615.53
G152	01/20/2020	617.44
G152	08/10/2020	614.46
G152	10/13/2020	613.13
G152	01/20/2021	614.87
G152	02/01/2021	613.13
G152	04/20/2021	615.34
G152	07/26/2021	616.76
G152	08/16/2021	615.30
G153	07/23/2015	615.93
G153	10/06/2015	614.45
G153	11/12/2016	615.15
G153	02/04/2017	616.30
G153	05/13/2017	615.25
G153	07/08/2017	616.19
G153	10/21/2017	612.37
G153	05/08/2018	615.07
G153	08/02/2018	614.01
G153	10/23/2018	613.31
G153	01/15/2019	614.36
G153	08/05/2019	614.45
G153	01/20/2020	616.16
G153	08/10/2020	613.72
G153	10/13/2020	612.16
G153	01/20/2021	612.66
G153	02/01/2021	613.18
G153	04/20/2021	615.52
G153	07/26/2021	613.97
3133	3.,20,2021	013.57



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G153	08/16/2021	614.19
G154	07/23/2015	614.85
G154	10/06/2015	612.24
G154	11/12/2016	614.68
G154	02/04/2017	615.25
G154	05/13/2017	614.93
G154	07/08/2017	613.53
G154	10/21/2017	610.33
G154	05/08/2018	614.78
G154	08/02/2018	613.75
G154	10/23/2018	613.06
G154	01/15/2019	613.66
G154	08/05/2019	613.86
G154	01/20/2020	618.28
G154	08/10/2020	612.57
G154	10/13/2020	610.84
G154	01/20/2021	612.41
G154	02/01/2021	617.01
G154	04/20/2021	614.81
G154	07/26/2021	615.21
G154	08/16/2021	615.45
G155	07/23/2015	614.45
G155	10/06/2015	613.51
G155	11/12/2016	613.93
G155	02/04/2017	614.09
G155	05/13/2017	614.59
G155	07/08/2017	614.75
G155	10/21/2017	609.91
G155	05/08/2018	614.41
G155	08/02/2018	613.68
G155	10/23/2018	612.80
G155	01/15/2019	613.56
G155	08/05/2019	613.71
G155	01/20/2020	615.99
G155	08/10/2020	613.09
G155	10/13/2020	612.10
G155	01/20/2021	612.72
G155	02/01/2021	614.59
G155	04/20/2021	613.94
G155	07/26/2021	613.81
G155	08/16/2021	614.01
G200	10/05/2015	621.05
G200	11/16/2015	621.66
G200	02/08/2016	623.29
G200	05/09/2016	622.52
G200	07/25/2016	622.82



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G200	11/12/2016	622.82
G200	02/04/2017	621.13
G200	05/13/2017	622.86
G200	07/08/2017	622.96
G200	10/21/2017	618.84
G200	01/25/2018	620.39
G200	05/08/2018	622.52
G200	08/02/2018	621.54
G200	10/23/2018	621.14
G200	01/15/2019	621.98
G200	08/05/2019	622.04
G200	01/20/2020	622.72
G200	08/10/2020	618.16
G200	10/13/2020	615.63
G200	01/20/2021	619.63
G200	01/29/2021	619.63
G200	03/29/2021	623.27
G200	04/20/2021	621.86
G200	04/21/2021	622.19
G200	05/03/2021	622.69
G200	05/06/2021	623.36
G200	05/17/2021	622.10
G200	06/09/2021	620.84
G200	06/23/2021	619.38
G200	07/12/2021	620.52
G200	07/26/2021	619.74
G200	07/28/2021	619.56
G200	08/16/2021	619.88
G205	02/08/2016	620.10
G205	05/09/2016	620.48
G205	07/25/2016	619.81
G205	11/12/2016	620.04
G205	02/04/2017	621.45
G205	05/13/2017	619.54
G205	07/08/2017	619.93
G205	10/21/2017	616.33
G206	10/07/2015	620.69
G206	11/16/2015	619.27
G206	02/08/2016	621.92
G206	05/09/2016	622.30
G206	06/27/2016	620.51
G206	07/25/2016	621.71
G206	11/12/2016	621.44
G206	02/04/2017	622.68
G206	05/13/2017	621.67
G206	07/08/2017	622.00



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G206	10/21/2017	616.61
G206	05/08/2018	620.71
G206	08/02/2018	621.87
G206	10/23/2018	619.71
G206	01/15/2019	618.82
G206	08/05/2019	621.56
G206	01/20/2020	622.76
G206	05/05/2020	623.02
G206	08/10/2020	619.92
G206	10/13/2020	617.84
G206	01/20/2021	621.50
G206	01/27/2021	621.50
G206	04/20/2021	622.07
G206	05/03/2021	622.60
G206	05/17/2021	622.31
G206	06/09/2021	621.71
G206	06/23/2021	620.54
G206	07/12/2021	622.39
G206	07/26/2021	622.00
G206	08/16/2021	622.08
G206D	03/29/2021	583.94
G206D	03/30/2021	584.34
G206D	04/20/2021	585.96
G206D	04/22/2021	584.64
G206D	05/03/2021	587.42
G206D	05/05/2021	586.96
G206D	05/17/2021	587.81
G206D	05/18/2021	587.82
G206D	06/09/2021	584.19
G206D	06/23/2021	589.66
G206D	07/12/2021	590.72
G206D	07/26/2021	591.14
G206D	07/27/2021	591.15
G206D	08/16/2021	592.00
G207	10/07/2015	620.72
G207	02/08/2016	622.18
G207	05/09/2016	622.56
G207	07/25/2016	622.06
G207	11/12/2016	622.54
G207	02/04/2017	623.24
G207	05/13/2017	623.39
G207	07/08/2017	621.31
G207	10/21/2017	619.41
G207	05/08/2018	622.96
G207	08/02/2018	623.21
G207	10/23/2018	621.64
	10, 20, 2010	021.07



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G207	01/15/2019	620.30
G207	08/05/2019	621.21
G207	01/20/2020	622.62
G207	08/10/2020	619.71
G207	10/13/2020	617.71
G207	01/20/2021	621.85
G207	01/28/2021	621.86
G207	04/20/2021	622.30
G207	07/26/2021	622.25
G207	08/16/2021	622.36
G208	10/07/2015	620.62
G208	02/08/2016	622.19
G208	05/09/2016	622.63
G208	07/25/2016	622.20
G208	11/12/2016	622.61
G208	02/04/2017	624.07
G208	05/13/2017	622.75
G208	07/08/2017	622.04
G208	10/21/2017	618.97
G208	05/08/2018	622.94
G208	08/02/2018	622.54
G208	10/23/2018	620.66
G208	01/15/2019	622.28
G208	08/05/2019	622.46
G208	01/20/2020	622.65
G208	08/10/2020	619.56
G208	10/13/2020	617.65
G208	01/20/2021	622.09
G208	01/27/2021	622.13
G208	04/20/2021	622.37
G208	07/26/2021	622.37
G208	08/16/2021	622.50
G209	10/07/2015	620.56
G209	11/16/2015	620.06
G209	02/08/2016	622.26
G209	05/09/2016	622.74
G209	07/25/2016	621.52
G209	11/12/2016	621.65
G209	02/04/2017	623.18
G209	05/13/2017	621.86
G209	07/08/2017	621.50
G209	10/21/2017	617.76
G209	01/25/2018	619.95
G209	05/08/2018	621.35
G209	08/02/2018	621.56
G209	10/23/2018	619.91



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G209	01/15/2019	621.80
G209	05/03/2019	623.09
G209	08/05/2019	621.46
G209	01/20/2020	622.69
G209	05/05/2020	622.96
G209	08/10/2020	619.59
G209	10/13/2020	617.69
G209	01/20/2021	621.94
G209	01/27/2021	621.94
G209	04/20/2021	622.40
G209	05/03/2021	622.82
G209	05/17/2021	622.51
G209	06/09/2021	622.03
G209	06/23/2021	620.89
G209	07/12/2021	622.08
G209	07/26/2021	622.41
G209	08/16/2021	622.56
G210	10/07/2015	619.83
G210	02/08/2016	621.72
G210	05/09/2016	622.50
G210	07/25/2016	620.94
G210	11/12/2016	621.05
G210	02/04/2017	621.69
G210	05/13/2017	620.77
G210	07/08/2017	621.03
G210	10/21/2017	616.82
G210	05/08/2018	620.19
G210	08/02/2018	620.15
G210	10/23/2018	618.92
G210	01/15/2019	620.82
G210	08/05/2019	620.50
G210	01/20/2020	622.33
G210	08/10/2020	619.97
G210	10/13/2020	618.00
G210	01/20/2021	620.58
G210	01/27/2021	620.40
G210	04/20/2021	622.18
G210	07/26/2021	622.18
G210	08/16/2021	622.28
G211	10/07/2015	619.00
G211	02/08/2016	622.08
G211	05/09/2016	622.45
G211	07/25/2016	621.81
G211	11/12/2016	621.28
G211	02/04/2017	622.15
G211	05/13/2017	621.34



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G211	07/08/2017	622.09
G211	10/21/2017	618.14
G211	05/08/2018	620.69
G211	08/02/2018	620.63
G211	10/23/2018	619.52
G211	01/15/2019	620.22
G211	08/05/2019	620.66
G211	01/20/2020	622.20
G211	08/10/2020	619.83
G211	10/13/2020	617.77
G211	01/20/2021	620.22
G211	01/27/2021	620.50
G211	04/20/2021	622.04
G211	07/26/2021	621.82
G211	08/16/2021	621.91
G212	10/07/2015	620.76
G212	11/16/2015	618.54
G212	02/08/2016	621.99
G212	05/09/2016	622.04
G212	07/25/2016	620.89
G212	11/12/2016	621.00
G212	02/04/2017	621.87
G212	05/13/2017	621.32
G212	07/08/2017	620.81
G212	10/21/2017	617.19
G212	05/08/2018	620.82
G212	08/02/2018	620.80
G212	10/23/2018	619.43
G212	01/15/2019	621.13
G212	08/05/2019	620.26
G212	01/20/2020	622.10
G212	08/10/2020	619.14
G212	10/13/2020	616.90
G212	01/20/2021	620.08
G212	01/26/2021	620.08
G212	04/20/2021	621.60
G212	05/03/2021	622.12
G212	05/17/2021	621.74
G212	06/09/2021	621.19
G212	06/23/2021	619.96
G212	06/29/2021	620.08
G212	07/12/2021	620.55
G212	07/26/2021	621.13
G212	08/16/2021	621.41
G213	10/07/2015	620.21
G213	02/08/2016	621.20



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G213	05/09/2016	621.69
G213	07/25/2016	620.36
G213	11/12/2016	620.78
G213	02/04/2017	621.21
G213	05/13/2017	621.16
G213	07/08/2017	620.40
G213	10/21/2017	618.62
G213	05/08/2018	620.88
G213	08/02/2018	620.35
G213	10/23/2018	619.83
G213	01/15/2019	620.81
G213	08/05/2019	621.08
G213	01/20/2020	621.72
G213	08/10/2020	618.66
G213	10/13/2020	616.56
G213	01/20/2021	619.61
G213	01/27/2021	619.97
G213	04/20/2021	621.28
G213	07/26/2021	620.96
G213	08/16/2021	621.20
G214	10/07/2015	617.56
G214	02/08/2016	618.11
G214	05/09/2016	619.39
G214	07/25/2016	617.75
G214	11/12/2016	618.16
G214	02/04/2017	618.77
G214	05/13/2017	618.51
G214	07/08/2017	618.25
G214	10/21/2017	614.52
G214	05/08/2018	618.17
G214	08/02/2018	617.35
G214	10/23/2018	616.87
G214	01/15/2019	618.57
G214	08/05/2019	616.99
G214	01/20/2020	619.38
G214	08/10/2020	616.32
G214	10/13/2020	614.47
G214	01/20/2021	616.45
G214	01/27/2021	616.64
G214	04/20/2021	618.60
G214	07/26/2021	618.39
G214	08/16/2021	618.55
G215	10/07/2015	616.56
G215	11/16/2015	616.38
G215	02/08/2016	618.31



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G215	07/25/2016	617.10
G215	11/12/2016	617.91
G215	02/04/2017	618.34
G215	05/13/2017	618.16
G215	07/08/2017	617.01
G215	10/21/2017	615.48
G215	05/08/2018	617.80
G215	08/02/2018	618.00
G215	10/23/2018	616.26
G215	01/15/2019	618.03
G215	08/05/2019	617.55
G215	01/20/2020	619.51
G215	08/10/2020	617.11
G215	10/14/2020	618.58
G215	01/20/2021	617.19
G215	01/26/2021	617.19
G215	04/20/2021	618.83
G215	05/03/2021	619.20
G215	05/17/2021	619.10
G215	06/09/2021	618.65
G215	06/23/2021	617.45
G215	06/29/2021	617.72
G215	07/12/2021	618.24
G215	07/26/2021	618.79
G215	08/16/2021	618.91
G216	10/07/2015	616.66
G216	02/08/2016	618.74
G216	05/09/2016	619.81
G216	07/25/2016	617.68
G216	11/12/2016	617.68
G216	02/04/2017	618.06
G216	05/13/2017	617.76
G216	07/08/2017	617.21
G216	10/21/2017	614.37
G216	05/08/2018	616.88
G216	08/02/2018	616.99
G216	10/23/2018	615.92
G216	01/15/2019	616.96
G216	08/05/2019	618.04
G216	01/20/2020	619.86
G216	08/10/2020	617.54
G216	10/14/2020	615.85
G216	01/20/2021	617.65
G216	01/28/2021	617.48
G216	04/20/2021	619.25
G216	07/26/2021	619.20



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G216	08/16/2021	619.30
G217	10/07/2015	616.71
G217	02/08/2016	618.25
G217	05/09/2016	619.13
G217	07/25/2016	617.81
G217	11/12/2016	617.81
G217	02/04/2017	618.13
G217	05/13/2017	618.04
G217	07/08/2017	618.12
G217	10/21/2017	614.32
G217	05/08/2018	617.21
G217	08/02/2018	617.06
G217	10/23/2018	616.17
G217	01/15/2019	617.10
G217	08/05/2019	617.10
G217	01/20/2020	619.01
G217	08/10/2020	616.20
G217	10/14/2020	614.57
G217	01/20/2021	616.74
G217	01/28/2021	616.84
G217	04/20/2021	618.45
G217	07/26/2021	617.93
G217	08/16/2021	618.04
G218	10/07/2015	616.93
G218	11/16/2015	617.11
G218	02/08/2016	619.05
G218	05/09/2016	620.10
G218	07/25/2016	618.01
G218	11/12/2016	618.39
G218	02/04/2017	618.19
G218	05/13/2017	618.56
G218	07/08/2017	618.19
G218	10/21/2017	614.46
G218	01/26/2018	616.46
G218	05/08/2018	617.87
G218	08/02/2018	618.01
G218	10/23/2018	616.66
G218	01/15/2019	617.21
G218	08/05/2019	617.86
G218	01/20/2020	619.93
G218	08/10/2020	617.42
G218	10/14/2020	615.65
G218	01/20/2021	617.53
G218	01/26/2021	617.53
G218	04/20/2021	619.53
G218	05/03/2021	619.90



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G218	05/17/2021	619.72
G218	06/09/2021	619.32
G218	06/23/2021	617.87
G218	07/12/2021	618.60
G218	07/26/2021	619.11
G218	08/16/2021	619.33
G270	10/05/2015	616.07
G270	11/16/2015	621.06
G270	02/08/2016	622.94
G270	05/09/2016	622.77
G270	07/25/2016	617.73
G270	11/12/2016	618.31
G270	02/04/2017	619.02
G270	05/13/2017	618.83
G270	07/08/2017	617.99
G270	10/21/2017	614.45
G270	05/08/2018	618.76
G270	08/02/2018	616.56
G270	10/23/2018	617.01
G270	01/15/2019	618.46
G270	08/05/2019	622.12
G270	01/20/2020	622.11
G270	08/10/2020	618.11
G270	10/14/2020	616.17
G270	01/20/2021	622.51
G270	01/21/2021	622.57
G270	03/29/2021	623.38
G270	03/30/2021	623.44
G270	04/20/2021	622.74
G270	04/21/2021	622.85
G270	05/03/2021	623.08
G270	05/06/2021	623.27
G270	05/17/2021	622.87
G270	05/19/2021	623.30
G270	06/09/2021	621.75
G270	06/15/2021	620.09
G270	06/23/2021	619.06
G270	06/29/2021	621.69
G270	07/12/2021	622.56
G270	07/26/2021	622.39
G270	07/27/2021	622.30
G270	08/16/2021	622.54
G271	10/08/2015	614.12
G271	11/16/2015	613.77
G271	02/08/2016	615.87
G271	05/09/2016	616.05



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G271	07/25/2016	616.62
G271	11/12/2016	616.49
G271	02/04/2017	617.95
G271	05/13/2017	616.46
G271	07/08/2017	616.47
G271	07/17/2017	616.47
G271	10/21/2017	613.31
G271	05/08/2018	615.70
G271	08/02/2018	615.70
G271	10/23/2018	614.73
G271	01/15/2019	617.40
G271	08/05/2019	617.63
G271	01/20/2020	617.83
G271	08/10/2020	614.18
G271	08/13/2020	614.18
G271	10/14/2020	612.90
G271	01/20/2021	613.91
G271	02/01/2021	613.91
G271	04/20/2021	615.51
G271	05/03/2021	615.96
G271	05/17/2021	615.78
G271	06/09/2021	615.52
G271	06/23/2021	615.02
G271	07/12/2021	615.57
G271	07/26/2021	615.67
G271	08/16/2021	615.78
G272	10/08/2015	612.56
G272	02/08/2016	614.93
G272	05/09/2016	614.96
G272	07/25/2016	614.79
G272	11/12/2016	614.34
G272	02/04/2017	615.08
G272	05/13/2017	614.23
G272	07/08/2017	615.12
G272	10/21/2017	611.45
G272	05/08/2018	613.58
G272	08/02/2018	613.44
G272	10/23/2018	612.96
G272	01/15/2019	615.43
G272	08/05/2019	616.88
G272	01/20/2020	616.49
G272	08/10/2020	613.19
G272	10/14/2020	611.89
G272	01/20/2021	613.01
G272	02/01/2021	616.48
G272	04/20/2021	614.50



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G272	07/26/2021	614.44
G272	08/16/2021	614.47
G273	10/08/2015	610.41
G273	11/16/2015	611.82
G273	02/08/2016	613.26
G273	05/09/2016	612.83
G273	07/25/2016	611.27
G273	11/12/2016	610.83
G273	02/04/2017	611.47
G273	05/13/2017	610.77
G273	07/08/2017	611.29
G273	07/17/2017	611.29
G273	10/21/2017	608.91
G273	05/08/2018	610.16
G273	08/02/2018	610.46
G273	10/23/2018	608.82
G273	01/15/2019	610.28
G273	08/05/2019	610.23
G273	01/20/2020	614.20
G273	08/10/2020	611.52
G273	08/13/2020	611.52
G273	10/14/2020	610.31
G273	01/20/2021	611.52
G273	02/01/2021	611.52
G273	04/20/2021	612.42
G273	05/03/2021	612.90
G273	05/17/2021	612.63
G273	06/09/2021	612.24
G273	06/23/2021	611.79
G273	07/12/2021	612.22
G273	07/26/2021	612.35
G273	08/16/2021	613.52
G274	10/08/2015	610.06
G274	02/08/2016	610.22
G274	05/09/2016	609.97
G274	07/25/2016	611.06
G274	11/12/2016	610.86
G274	02/04/2017	612.00
G274	05/13/2017	611.18
G274	07/08/2017	611.55
G274	10/21/2017	607.79
G274	05/08/2018	610.84
G274	08/02/2018	611.44
G274	10/23/2018	609.52
G274	01/15/2019	611.23
G274	08/05/2019	611.31



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G274	01/20/2020	611.23
G274	08/10/2020	609.29
G274	10/14/2020	608.49
G274	01/20/2021	610.36
G274	02/01/2021	611.18
G274	04/20/2021	609.89
G274	07/26/2021	609.82
G274	08/16/2021	609.88
G275	02/08/2016	604.71
G275	05/09/2016	604.76
G275	07/25/2016	603.17
G275	11/12/2016	604.28
G275	02/04/2017	603.65
G275	05/13/2017	604.67
G275	07/08/2017	602.97
G275	05/08/2018	604.26
G275	08/02/2018	604.16
G275	10/23/2018	604.46
G275	01/15/2019	605.91
G275	08/05/2019	605.97
G275	01/20/2020	605.63
G275	08/10/2020	604.95
G275	01/20/2021	605.02
G275	04/20/2021	605.00
G275	07/13/2021	605.63
G275	07/13/2021	605.05
G275	08/16/2021	605.09
G275D	03/30/2021	570.32
G275D	04/20/2021	570.98
G275D	04/20/2021	568.33
G275D	05/03/2021	569.75
G275D	05/05/2021	570.26
G275D	05/03/2021	568.67
G275D	05/17/2021	569.00
G275D	06/09/2021	570.31
G275D	06/23/2021	569.71
G275D	07/12/2021	570.43
	07/12/2021	570.35
G275D G275D	07/26/2021	570.68
G275D	08/16/2021	571.48
G275D	11/16/2015	603.25
G276 G276	02/08/2016	603.71
G276 G276	· ·	603.71
	05/09/2016	
G276	07/25/2016	604.92
G276	11/12/2016	603.60
G276	02/04/2017	603.72



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G276	05/13/2017	603.40
G276	07/08/2017	604.05
G276	07/18/2017	604.05
G276	05/08/2018	603.11
G276	08/02/2018	606.60
G276	10/23/2018	603.35
G276	01/15/2019	604.00
G276	08/05/2019	603.96
G276	01/20/2020	605.08
G276	08/10/2020	604.63
G276	08/12/2020	604.63
G276	10/14/2020	603.59
G276	01/20/2021	603.71
G276	04/20/2021	604.65
G276	05/03/2021	604.71
G276	05/17/2021	604.88
G276	06/09/2021	604.93
G276	06/23/2021	604.53
G276	06/28/2021	604.58
G276	07/12/2021	604.55
G276	07/26/2021	604.68
G276	08/16/2021	604.73
G277	02/08/2016	602.98
G277	05/09/2016	603.79
G277	07/25/2016	602.08
G277	11/12/2016	601.23
G277	02/04/2017	603.58
G277	05/13/2017	601.29
G277	07/08/2017	603.09
G277	10/21/2017	601.53
G277	10/23/2018	601.28
G277	01/15/2019	603.38
G277	08/05/2019	602.15
G277	01/20/2020	603.49
G277	08/10/2020	603.29
G277	04/20/2021	603.33
G277	07/26/2021	603.33
G278	02/08/2016	606.56
G278	05/09/2016	607.00
G278	07/25/2016	604.57
G278	11/12/2016	604.29
G278	02/04/2017	606.38
G278	05/13/2017	604.30
G278	07/08/2017	604.84
G278	10/21/2017	604.54
G278	05/08/2018	605.31



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G278	08/02/2018	604.57
G278	10/23/2018	604.29
G278	01/15/2019	605.40
G278	08/05/2019	605.19
G278	01/20/2020	608.15
G278	08/10/2020	606.83
G278	10/14/2020	605.55
G278	01/20/2021	605.18
G278	04/20/2021	606.47
G278	07/26/2021	607.49
G278	08/16/2021	607.62
G279	10/08/2015	608.14
G279	11/16/2015	607.80
G279	02/08/2016	609.16
G279	05/09/2016	610.17
G279	07/25/2016	606.94
G279	11/12/2016	606.93
G279	02/04/2017	607.96
G279	05/13/2017	606.74
G279	07/08/2017	607.04
G279	07/18/2017	607.04
G279	05/08/2018	606.42
G279	08/02/2018	605.87
G279	10/23/2018	604.87
G279	01/15/2019	606.79
G279	08/05/2019	605.90
G279	01/20/2020	611.08
G279	08/10/2020	607.17
G279	08/12/2020	607.17
G279	10/14/2020	605.54
G279	01/20/2021	607.07
G279	01/28/2021	607.07
G279	04/20/2021	608.97
G279	05/03/2021	609.38
G279	05/17/2021	609.22
G279	06/09/2021	599.69
G279	06/23/2021	607.74
G279	07/12/2021	608.18
G279	07/26/2021	608.57
G279	08/16/2021	608.95
G280	10/08/2015	614.54
G280	11/16/2015	618.45
G280	02/08/2016	621.37
G280	05/09/2016	621.94
G280	07/25/2016	618.21
G280	11/12/2016	618.46



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G280	02/04/2017	619.10
G280	05/13/2017	618.56
G280	07/08/2017	619.45
G280	07/18/2017	619.45
G280	10/21/2017	614.47
G280	05/08/2018	618.00
G280	08/02/2018	616.70
G280	10/23/2018	615.75
G280	01/15/2019	616.24
G280	08/05/2019	616.09
G280	01/20/2020	622.33
G280	08/10/2020	619.50
G280	08/11/2020	619.50
G280	10/14/2020	617.45
G280	01/20/2021	618.20
G280	01/28/2021	618.70
G280	03/29/2021	620.61
G280	03/30/2021	621.22
G280	04/20/2021	619.76
G280	04/22/2021	620.13
G280	05/03/2021	620.21
G280	05/06/2021	620.89
G280	05/17/2021	619.98
G280	05/19/2021	620.72
G280	06/09/2021	619.75
G280	06/23/2021	618.93
G280	06/28/2021	619.02
G280	07/12/2021	619.26
G280	07/13/2021	619.50
G280	07/26/2021	619.75
G280	07/27/2021	619.66
G280	08/16/2021	620.00
G281	11/16/2015	619.56
G281	02/08/2016	621.21
G281	05/09/2016	620.93
G281	07/25/2016	620.30
G281	11/12/2016	620.01
G281	02/04/2017	620.37
G281	05/13/2017	619.96
G281	07/08/2017	619.17
G281	10/21/2017	616.41
G281	05/08/2018	619.33
G281	08/02/2018	618.36
G281	10/23/2018	617.26
G281	01/15/2019	618.19
G281	08/05/2019	620.16



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G281	01/20/2020	621.26
G281	08/10/2020	619.26
G281	01/20/2021	619.36
G281	01/29/2021	619.36
G281	03/29/2021	621.68
G281	03/31/2021	621.29
G281	04/20/2021	619.62
G281	04/21/2021	619.77
G281	05/03/2021	620.60
G281	05/05/2021	620.85
G281	05/17/2021	620.13
G281	06/09/2021	619.65
G281	06/14/2021	619.46
G281	06/23/2021	618.71
G281	06/28/2021	619.77
G281	07/12/2021	620.23
G281	07/26/2021	620.02
G281	07/27/2021	619.92
G281	08/16/2021	619.81
G283	03/29/2021	607.80
G283	03/31/2021	607.34
G283	04/20/2021	606.34
G283	04/22/2021	606.09
G283	05/03/2021	606.81
G283	05/06/2021	606.79
G283	05/17/2021	606.30
G283	05/18/2021	606.54
G283	06/09/2021	605.13
G283	06/15/2021	604.95
G283	06/23/2021	604.56
G283	06/29/2021	605.29
G283	07/12/2021	605.50
G283	07/13/2021	605.82
G283	07/26/2021	605.18
G283	07/27/2021	605.08
G283	08/16/2021	605.12
G284	03/29/2021	611.14
G284	03/30/2021	610.95
G284	04/20/2021	608.16
G284	04/21/2021	607.65
G284	05/03/2021	609.33
G284	05/06/2021	610.72
G284	05/17/2021	608.16
G284	05/18/2021	609.49
G284	06/09/2021	607.07
G284	06/14/2021	606.95
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Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G284	06/23/2021	606.17
G284	06/28/2021	608.02
G284	07/12/2021	607.68
G284	07/13/2021	607.69
G284	07/26/2021	607.11
G284	07/27/2021	606.95
G284	08/16/2021	606.98
G285	03/29/2021	608.62
G285	03/30/2021	608.81
G285	04/20/2021	608.13
G285	04/22/2021	603.79
G285	05/03/2021	606.99
G285	05/06/2021	607.57
G285	05/17/2021	607.47
G285	05/18/2021	607.51
G285	06/09/2021	607.39
G285	06/15/2021	607.08
G285	06/23/2021	604.33
G285	06/28/2021	604.93
G285	07/12/2021	604.80
G285	07/13/2021	604.92
G285	07/26/2021	605.27
G285	07/27/2021	605.37
G285	08/16/2021	606.28
G286	03/29/2021	609.08
G286	03/31/2021	608.22
G286	04/20/2021	606.63
G286	04/22/2021	606.15
G286	05/03/2021	606.97
G286	05/06/2021	608.56
G286	05/17/2021	606.44
G286	05/18/2021	606.57
G286	06/09/2021	604.68
G286	06/15/2021	602.98
G286	07/12/2021	605.90
G286	07/13/2021	606.00
G287	03/29/2021	610.22
G287	04/20/2021	608.67
G287	04/22/2021	608.03
G287	05/03/2021	609.28
G287	05/06/2021	610.29
G287	05/17/2021	608.41
G287	05/18/2021	609.32
G287	06/09/2021	607.59
G287	06/14/2021	617.45
G287	07/12/2021	610.83



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G287	07/13/2021	607.78
G287	08/16/2021	607.76
G288	03/29/2021	616.32
G288	03/30/2021	615.89
G288	04/20/2021	613.90
G288	04/21/2021	613.56
G288	05/03/2021	614.51
G288	05/06/2021	616.00
G288	05/17/2021	613.87
G288	05/18/2021	616.15
G288	06/09/2021	612.90
G288	06/15/2021	612.47
G288	06/23/2021	611.90
G288	06/28/2021	612.91
G288	07/12/2021	613.59
G288	07/13/2021	615.11
G288	07/26/2021	612.85
G288	07/27/2021	612.75
G288	08/16/2021	612.98
G301	11/16/2015	616.51
G301	02/08/2016	617.21
G301	05/09/2016	616.75
G301	07/25/2016	614.65
G301	11/12/2016	614.08
G301	02/04/2017	614.15
G301	05/13/2017	614.15
G301	07/08/2017	614.88
G301	10/21/2017	610.39
G301	05/08/2018	613.73
G301	08/02/2018	615.05
G301	10/23/2018	612.46
G301	01/15/2019	613.23
G301 G301	08/05/2019	613.82
G301 G301	01/20/2020	618.07
G301 G301	08/10/2020	615.16
G301 G301	01/20/2021	616.03
G301 G301	01/20/2021	616.03
G301	04/20/2021	616.05
G301	05/03/2021	616.12
G301	05/17/2021	615.99
G301	06/09/2021	615.63
G301	06/23/2021	615.02
G301	07/12/2021	615.79
G301	07/26/2021	615.31
G301	08/16/2021	615.45
G302	11/16/2015	610.74



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G302	02/08/2016	613.14
G302	05/09/2016	614.60
G302	07/25/2016	608.16
G302	11/12/2016	607.92
G302	02/04/2017	608.95
G302	05/13/2017	608.02
G302	07/08/2017	608.42
G302	10/21/2017	604.64
G302	05/08/2018	607.59
G302	08/02/2018	608.26
G302	10/23/2018	605.54
G302	01/15/2019	607.29
G302	08/05/2019	609.95
G302	01/20/2020	615.41
G302	08/10/2020	608.05
G302	01/20/2021	609.99
G302	01/27/2021	609.99
G302	04/20/2021	611.85
G302	05/03/2021	612.07
G302	05/17/2021	612.06
G302	06/09/2021	610.29
G302	06/23/2021	608.79
G302	07/12/2021	611.79
G302	07/26/2021	610.98
G302	08/16/2021	611.77
G303	11/16/2015	616.70
G303	02/08/2016	617.87
G303	05/09/2016	617.97
G303	07/25/2016	614.92
G303	11/12/2016	614.38
G303	02/04/2017	614.95
G303	05/13/2017	614.81
G303	07/08/2017	614.97
G303	10/21/2017	611.18
G303	05/08/2018	614.16
G303	08/02/2018	614.06
G303	10/23/2018	613.05
G303	01/15/2019	614.33
G303	08/05/2019	617.37
G303	01/20/2020	618.05
G303	08/10/2020	615.16
G303	01/20/2021	616.17
G303	01/26/2021	616.17
G303	04/20/2021	617.27
G303	05/03/2021	618.02
G303	05/17/2021	617.37



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G303	06/09/2021	616.52
G303	06/23/2021	614.92
G303	07/12/2021	617.15
G303	07/26/2021	616.44
G303	08/16/2021	616.58
G304	11/16/2015	623.78
G304	02/08/2016	624.07
G304	05/09/2016	623.91
G304	07/25/2016	626.72
G305	05/09/2016	618.48
G305	07/01/2016	616.28
G305	07/25/2016	618.24
G305	09/29/2016	617.33
G305	11/12/2016	618.06
G305	02/04/2017	620.49
G305	05/13/2017	618.27
G305	07/08/2017	618.28
G305	10/21/2017	615.30
G305	05/08/2018	617.87
G305	08/02/2018	617.79
G305	10/23/2018	616.56
G305	01/15/2019	616.95
G305	08/05/2019	616.85
G305	01/20/2020	619.36
G305	08/10/2020	617.02
G305	01/20/2021	618.63
G305	04/20/2021	618.77
G305	05/03/2021	619.11
G305	05/17/2021	618.90
G305	06/09/2021	618.04
G305	06/23/2021	616.94
G305	07/12/2021	618.55
G305	07/26/2021	618.18
G305	08/16/2021	618.31
G306	05/09/2016	619.74
G306	07/01/2016	615.11
G306	07/25/2016	619.26
G306	09/29/2016	617.64
G306	11/12/2016	618.77
G306	02/04/2017	618.97
G306	05/13/2017	619.03
G306	07/08/2017	619.45
G306	10/21/2017	616.12
G306	05/08/2018	618.96
G306	08/02/2018	621.73
G306	10/23/2018	617.24



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G306	01/15/2019	618.56
G306	08/05/2019	619.18
G306	01/20/2020	620.28
G306	08/10/2020	617.26
G306	01/20/2021	619.17
G306	01/26/2021	618.98
G306	03/29/2021	620.42
G306	04/20/2021	619.30
G306	04/21/2021	619.53
G306	05/03/2021	619.96
G306	05/05/2021	620.27
G306	05/17/2021	619.44
G306	05/18/2021	619.56
G306	06/09/2021	618.04
G306	06/15/2021	617.29
G306	06/23/2021	616.32
G306	06/28/2021	618.31
G306	07/12/2021	620.59
G306	07/14/2021	620.17
G306	07/26/2021	618.84
G306	07/27/2021	618.70
G306	08/16/2021	618.92
G307	07/25/2016	624.30
G307	09/29/2016	623.85
G307	11/12/2016	624.44
G307	02/04/2017	624.60
G307	05/13/2017	624.56
G307	07/08/2017	623.55
G307	10/21/2017	624.60
G307	05/08/2018	624.37
G307	08/02/2018	619.33
G307	10/23/2018	623.95
G307	01/15/2019	624.31
G307	08/05/2019	624.21
G307	05/06/2020	624.72
G307	08/10/2020	624.36
G307	01/20/2021	624.10
G307	01/27/2021	624.10
G307	04/20/2021	624.50
G307	05/17/2021	624.45
G307	07/12/2021	624.45
G307	08/16/2021	624.46
G307D	03/29/2021	622.43
G307D	04/20/2021	622.48
G307D	04/21/2021	622.46
G307D	05/03/2021	622.47



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G307D	05/04/2021	622.44
G307D	05/17/2021	622.44
G307D	05/18/2021	622.46
G307D	06/09/2021	622.43
G307D	06/15/2021	622.42
G307D	06/23/2021	622.42
G307D	07/12/2021	622.59
G307D	07/26/2021	622.26
G307D	07/27/2021	622.51
G307D	08/16/2021	621.49
G308	03/29/2021	621.03
G308	04/20/2021	619.67
G308	04/21/2021	620.15
G308	05/03/2021	620.04
G308	05/05/2021	621.01
G308	05/17/2021	619.93
G308	06/09/2021	619.17
G308	06/14/2021	619.06
G308	06/23/2021	618.54
G308	06/28/2021	620.44
G308	07/12/2021	620.22
G308	07/14/2021	620.67
G308	07/26/2021	619.68
G308	07/27/2021	619.44
G308	08/16/2021	619.45
G309	03/29/2021	621.09
G309	04/20/2021	618.88
G309	04/21/2021	618.88
G309	04/22/2021	618.88
G309	05/03/2021	619.04
G309	05/05/2021	619.84
G309	05/17/2021	618.83
G309	06/09/2021	618.43
G309	06/14/2021	618.25
G309	06/23/2021	617.89
G309	06/28/2021	618.95
G309	07/12/2021	619.31
G309	07/13/2021	620.17
G309	07/26/2021	618.88
G309	07/27/2021	618.78
G309	08/16/2021	618.91
G310	03/29/2021	617.27
G310	04/20/2021	614.41
G310	04/22/2021	614.40
G310	05/03/2021	614.61



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G310	05/17/2021	614.47
G310	05/19/2021	616.01
G310	06/09/2021	613.83
G310	06/15/2021	613.54
G310	06/23/2021	613.20
G310	06/28/2021	614.15
G310	07/12/2021	614.81
G310	07/13/2021	615.88
G310	07/26/2021	614.13
G310	07/28/2021	614.00
G310	08/16/2021	614.29
G311	03/29/2021	616.54
G311	03/30/2021	616.21
G311	04/20/2021	613.75
G311	04/22/2021	613.68
G311	05/03/2021	614.01
G311	05/04/2021	615.13
G311	05/17/2021	613.86
G311	05/19/2021	615.78
G311	06/09/2021	613.13
G311	06/15/2021	612.78
G311	06/23/2021	612.45
G311	06/29/2021	613.31
G311	07/12/2021	613.75
G311	07/14/2021	615.37
G311	07/26/2021	613.05
G311	07/27/2021	612.94
G311	08/16/2021	613.30
G311D	03/29/2021	575.42
G311D	03/30/2021	575.73
G311D	04/20/2021	575.29
G311D	04/22/2021	575.74
G311D	05/03/2021	573.09
G311D	05/04/2021	573.23
G311D	05/17/2021	572.40
G311D	05/19/2021	572.91
G311D	06/09/2021	573.85
G311D	06/15/2021	575.25
G311D	06/23/2021	571.74
G311D	07/12/2021	571.63
G311D	07/26/2021	569.74
G311D	07/28/2021	569.98
G311D	08/16/2021	570.34
G312	03/29/2021	612.19
G312	03/30/2021	611.97
G312	04/20/2021	609.11



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G312	04/22/2021	608.97
G312	05/03/2021	609.47
G312	05/04/2021	610.07
G312	05/17/2021	609.27
G312	05/19/2021	610.89
G312	06/09/2021	608.31
G312	06/15/2021	607.64
G312	06/23/2021	606.99
G312	06/29/2021	608.07
G312	07/12/2021	608.70
G312	07/13/2021	610.23
G312	07/26/2021	608.56
G312	07/27/2021	608.47
G312	08/16/2021	609.09
G313	03/29/2021	611.78
G313	03/30/2021	611.75
G313	04/20/2021	611.46
G313	04/22/2021	611.41
G313	05/03/2021	611.68
G313	05/04/2021	611.66
G313	05/17/2021	611.62
G313	05/18/2021	611.66
G313	06/09/2021	611.57
G313	06/14/2021	611.55
G313	06/23/2021	611.29
G313	06/28/2021	611.58
G313	07/12/2021	611.70
G313	07/13/2021	611.81
G313	07/26/2021	611.71
G313	07/27/2021	611.73
G313	08/16/2021	611.90
G314	03/29/2021	596.40
G314	03/30/2021	597.11
G314	04/20/2021	603.16
G314	04/21/2021	603.48
G314	05/03/2021	604.66
G314	05/04/2021	604.64
G314	05/17/2021	605.61
G314	06/09/2021	607.54
G314	06/14/2021	608.16
G314	06/23/2021	605.19
G314	06/28/2021	606.45
G314	07/12/2021	605.32
G314	07/13/2021	605.60
G314	07/26/2021	606.66
G314	07/27/2021	606.84



D88)



Sample Location Sample Date Groundwater Elevation (ft NAVI G316 07/26/2021 590.73 G316 07/27/2021 590.68 G316 08/16/2021 590.59 G317 03/29/2021 610.40 G317 03/30/2021 610.89 G317 04/20/2021 610.94 G317 04/22/2021 610.84 G317 05/03/2021 611.75 G317 05/05/2021 611.15 G317 05/17/2021 611.65	
G316 07/27/2021 590.68 G316 08/16/2021 590.59 G317 03/29/2021 610.40 G317 03/30/2021 610.89 G317 04/20/2021 610.94 G317 04/22/2021 610.84 G317 05/03/2021 611.75 G317 05/05/2021 611.15	
G316 08/16/2021 590.59 G317 03/29/2021 610.40 G317 03/30/2021 610.89 G317 04/20/2021 610.94 G317 04/22/2021 610.84 G317 05/03/2021 611.75 G317 05/05/2021 611.15	
G317 03/29/2021 610.40 G317 03/30/2021 610.89 G317 04/20/2021 610.94 G317 04/22/2021 610.84 G317 05/03/2021 611.75 G317 05/05/2021 611.15	
G317 03/30/2021 610.89 G317 04/20/2021 610.94 G317 04/22/2021 610.84 G317 05/03/2021 611.75 G317 05/05/2021 611.15	
G317 04/20/2021 610.94 G317 04/22/2021 610.84 G317 05/03/2021 611.75 G317 05/05/2021 611.15	
G317 04/22/2021 610.84 G317 05/03/2021 611.75 G317 05/05/2021 611.15	
G317 05/03/2021 611.75 G317 05/05/2021 611.15	
G317 05/05/2021 611.15	
G31/ U3/17/2021 611.65	
G317 05/18/2021 611.57	
G317 06/09/2021 610.59	
G317 06/15/2021 609.63	
G317 06/23/2021 606.57	
G317 06/28/2021 608.25	
G317 07/12/2021 607.93	
G317 07/13/2021 607.92	
G317 07/26/2021 608.27	
G317 07/28/2021 608.11	
G317 08/16/2021 608.46	
G401 11/16/2015 607.82	
G401 02/08/2016 608.14	
G401 05/09/2016 608.00	
G401 07/25/2016 608.47	
G401 11/12/2016 607.84	
G401 02/04/2017 609.74	
G401 05/13/2017 608.52	
G401 07/08/2017 609.19	
G401 05/08/2018 609.37	
G401 08/02/2018 609.80	
G401 10/23/2018 608.42	
G401 01/15/2019 608.36	
G401 08/05/2019 608.45	
G401 01/20/2020 607.25	
G401 05/06/2020 607.02	
G401 08/10/2020 606.77	
G401 01/29/2021 604.22	
G401 04/20/2021 604.14	
G401 07/26/2021 603.94	
G401 08/16/2021 604.04	
G402 11/16/2015 604.02	
G402 02/08/2016 604.90	
G402 05/09/2016 605.18	
G402 07/25/2016 604.33	
G402 11/12/2016 604.24	
G402 02/04/2017 604.43	



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G402	05/13/2017	604.83
G402	07/08/2017	604.42
G402	10/21/2017	600.77
G402	05/08/2018	605.36
G402	08/02/2018	603.82
G402	10/23/2018	602.25
G402	01/15/2019	602.37
G402	08/05/2019	603.82
G402	01/20/2020	605.12
G402	08/10/2020	602.09
G402	01/20/2021	603.01
G402	01/28/2021	603.01
G402	04/20/2021	603.78
G402	07/26/2021	602.83
G402	08/16/2021	603.29
G403	11/16/2015	621.81
G403	02/08/2016	621.78
G403	05/09/2016	621.76
G403	07/25/2016	622.16
G403	11/12/2016	621.80
G403	02/04/2017	622.45
G403	05/13/2017	622.26
G403	07/08/2017	622.16
G403	10/21/2017	618.36
G403	05/08/2018	621.66
G403	08/02/2018	622.38
G403	10/23/2018	619.48
G403	01/15/2019	620.51
G403	08/05/2019	621.64
G403	01/20/2020	621.63
G403	08/10/2020	621.14
G403	01/20/2021	619.88
G403	01/21/2021	619.88
G403	04/20/2021	619.41
G403	07/26/2021	619.56
G403	08/16/2021	619.27
G404	11/16/2015	611.67
G404	02/08/2016	611.58
G404	05/09/2016	611.46
G404	07/25/2016	611.67
G404	11/12/2016	610.58
G404	02/04/2017	610.57
G404	05/13/2017	610.87
G404	07/08/2017	611.75
G404	10/21/2017	607.58
G404	05/08/2018	611.42



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G404	08/02/2018	610.55
G404	10/23/2018	608.59
G404	01/15/2019	608.98
G404	08/05/2019	611.60
G404	01/20/2020	612.14
G404	08/10/2020	610.37
G404	01/20/2021	611.63
G404	01/21/2021	611.63
G404	04/20/2021	611.51
G404	07/26/2021	611.29
G404	08/16/2021	610.95
G405	11/16/2015	618.85
G405	02/08/2016	618.90
G405	05/09/2016	618.99
G405	07/25/2016	618.51
G405	11/12/2016	618.48
G405	02/04/2017	618.47
G405	05/13/2017	618.74
G405	07/08/2017	618.54
G405	10/21/2017	614.47
G405	05/08/2018	618.94
G405	08/02/2018	617.55
G405	10/23/2018	616.40
G405	01/15/2019	616.81
G405	08/05/2019	617.72
G405	01/20/2020	619.28
G405	08/10/2020	617.62
G405	01/20/2021	617.12
G405	01/21/2021	617.12
G405	04/20/2021	617.13
G405	07/26/2021	617.37
G405	08/16/2021	617.28
G406	11/12/2016	616.01
G406	02/04/2017	617.52
G406	05/13/2017	616.20
G406	07/08/2017	616.29
G406	10/21/2017	611.27
G406	05/08/2018	615.47
G406	08/02/2018	615.75
G406	10/23/2018	614.11
G406	01/15/2019	615.36
G406	08/05/2019	616.50
G406	01/20/2020	617.48
G406	08/10/2020	615.54
G406	01/20/2021	612.97
G406	04/20/2021	613.78



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G406	07/26/2021	614.20
G406	08/16/2021	613.82
G407	11/12/2016	613.27
G407	02/04/2017	613.41
G407	05/13/2017	613.68
G407	07/08/2017	613.59
G407	10/21/2017	612.67
G407	05/08/2018	613.11
G407	08/02/2018	612.95
G407	10/23/2018	612.11
G407	01/15/2019	612.31
G407	08/05/2019	614.02
G407	01/20/2020	614.86
G407	08/10/2020	613.74
G407	01/20/2021	614.70
G407	04/20/2021	614.49
G407	07/26/2021	614.38
G407	08/16/2021	614.41
G410	10/23/2018	610.41
G410	01/15/2019	610.91
G410	08/05/2019	611.75
G410	01/20/2020	612.70
G410	08/10/2020	610.88
G410	01/20/2021	610.91
G410	04/20/2021	611.38
G410	07/26/2021	611.51
G410	08/16/2021	611.29
G411	10/23/2018	613.20
G411	01/15/2019	613.82
G411	08/05/2019	614.25
G411	01/20/2020	617.53
G411	08/10/2020	615.51
G411	01/20/2021	615.91
G411	04/20/2021	616.12
G411	07/26/2021	616.20
G411	08/16/2021	616.03
MW03D	04/20/2021	597.90
MW03D	05/03/2021	598.18
MW03D	05/17/2021	598.06
MW03D	06/09/2021	598.13
MW03D	06/23/2021	598.09
MW03D	07/12/2021	598.12
MW03D	07/26/2021	598.09
MW03D	08/16/2021	598.10
MW04S	02/08/2016	621.62
MW04S	05/09/2016	620.45



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
MW04S	07/25/2016	618.84
MW04S	11/12/2016	618.66
MW04S	02/04/2017	618.97
MW04S	05/13/2017	618.76
MW04S	07/08/2017	618.92
MW04S	10/21/2017	613.88
MW04S	05/08/2018	617.95
MW04S	08/02/2018	618.73
MW04S	10/23/2018	614.68
MW04S	01/15/2019	614.89
MW04S	08/05/2019	614.92
MW04S	01/20/2020	619.93
MW04S	08/10/2020	617.74
MW04S	01/20/2021	620.63
MW04S	04/20/2021	619.39
MW04S	07/26/2021	618.55
MW04S	08/16/2021	618.50
MW05S	02/08/2016	620.92
MW05S	05/09/2016	620.53
MW05S	07/25/2016	618.20
MW05S	11/12/2016	617.38
MW05S	02/04/2017	618.78
MW05S	05/13/2017	617.95
MW05S	07/08/2017	618.81
MW05S	10/21/2017	613.32
MW05S	05/08/2018	617.77
MW05S	08/02/2018	617.98
MW05S	10/23/2018	615.35
MW05S	01/15/2019	615.93
MW05S	08/05/2019	616.01
MW05S	01/20/2020	620.34
MW05S	08/10/2020	617.09
MW05S	01/20/2021	618.33
MW05S	04/20/2021	619.07
MW05S	07/26/2021	618.14
MW05S	08/16/2021	617.84
MW10S	02/08/2016	620.43
MW10S	05/09/2016	619.47
MW10S	07/25/2016	617.69
MW10S	11/12/2016	616.69
MW10S	02/04/2017	617.41
MW10S	05/13/2017	617.22
MW10S	07/08/2017	617.27
MW10S	10/21/2017	614.52
MW10S	05/08/2018	616.89



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
MW10S	10/23/2018	614.36
MW10S	01/15/2019	614.85
MW10S	08/05/2019	615.56
MW10S	08/10/2020	617.11
MW10S	01/20/2021	619.48
MW10S	04/20/2021	619.03
MW10S	07/26/2021	617.74
MW10S	08/16/2021	617.35
MW11S	02/08/2016	621.30
MW11S	05/09/2016	622.19
MW11S	07/25/2016	620.99
MW11S	11/12/2016	620.92
MW11S	02/04/2017	620.82
MW11S	05/13/2017	621.31
MW11S	07/08/2017	620.85
MW11S	10/21/2017	617.19
MW11S	05/08/2018	620.85
MW11S	08/02/2018	620.69
MW11S	10/23/2018	620.05
MW11S	01/15/2019	620.38
MW11S	08/05/2019	620.76
MW11S	01/20/2020	621.80
MW11S	08/10/2020	618.12
MW11S	01/20/2021	619.64
MW11S	04/20/2021	621.76
MW11S	05/03/2021	622.01
MW11S	05/17/2021	621.94
MW11S	06/09/2021	621.45
MW11S	06/23/2021	618.83
MW11S	07/12/2021	620.54
MW11S	07/26/2021	620.97
MW11S	08/16/2021	621.49
MW11D	04/20/2021	621.13
MW11D	05/03/2021	621.36
MW11D	05/17/2021	621.27
MW11D	06/09/2021	620.96
MW11D	06/23/2021	618.72
MW11D	07/12/2021	619.88
MW11D	07/26/2021	620.57
MW11D	08/16/2021	621.01
MW12S	02/08/2016	620.37
MW12S	05/09/2016	620.48
MW12S	07/25/2016	618.53
MW12S	11/12/2016	617.97
MW12S	02/04/2017	620.33
MW12S	05/13/2017	618.26
	55, 15, 2017	010.20



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
MW12S	07/08/2017	618.61
MW12S	10/21/2017	615.08
MW12S	05/08/2018	617.58
MW12S	08/02/2018	617.12
MW12S	10/23/2018	616.14
MW12S	01/15/2019	616.89
MW12S	08/05/2019	617.35
MW12S	01/20/2020	620.34
MW12S	08/10/2020	615.69
MW12S	01/20/2021	611.42
MW12S	04/20/2021	618.96
MW12S	05/03/2021	619.66
MW12S	05/17/2021	619.23
MW12S	06/09/2021	618.20
MW12S	06/23/2021	616.52
MW12S	07/12/2021	619.35
MW12S	07/26/2021	618.43
MW12S	08/16/2021	618.79
MW12D	04/20/2021	611.97
MW12D	05/03/2021	611.87
MW12D	05/17/2021	611.95
MW12D	06/09/2021	611.87
MW12D	06/23/2021	611.79
MW12D	07/12/2021	611.55
MW12D	07/26/2021	611.50
MW12D	08/16/2021	611.51
MW16S	02/08/2016	625.29
MW16S	05/09/2016	624.54
MW16S	07/25/2016	622.13
MW16S	11/12/2016	622.26
MW16S	02/04/2017	622.53
MW16S	05/13/2017	622.53
MW16S	07/08/2017	622.25
MW16S	10/21/2017	618.42
MW16S	05/08/2018	622.02
MW16S	08/02/2018	622.47
MW16S	10/23/2018	620.88
MW16S	01/15/2019	621.60
MW16S MW16S	08/05/2019	621.94
MW16S MW16S	01/20/2020	625.59
MW16S MW16S	01/20/2020	618.52
MW16S	01/20/2021	618.34
MW16S	04/20/2021	623.78
MW16S	05/03/2021	624.58
MW16S	05/17/2021	623.87
MW16S	06/09/2021	622.57



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
MW16S	06/23/2021	620.48
MW16S	07/12/2021	620.58
MW16S	07/26/2021	620.68
MW16S	08/16/2021	620.65
MW16D	04/20/2021	615.42
MW16D	05/03/2021	615.92
MW16D	05/17/2021	616.36
MW16D	06/09/2021	616.87
MW16D	06/23/2021	616.90
MW16D	07/12/2021	616.76
MW16D	07/26/2021	616.63
MW16D	08/16/2021	616.35
MW20S	02/08/2016	614.36
MW20S	05/09/2016	614.09
MW20S	07/25/2016	611.61
MW20S	11/12/2016	611.51
MW20S	02/04/2017	612.76
MW20S	05/13/2017	611.86
MW20S	07/08/2017	611.75
MW20S	10/21/2017	607.74
MW20S	05/08/2018	611.46
MW20S	08/02/2018	611.51
MW20S	10/23/2018	609.55
MW20S	01/15/2019	610.21
MW20S	08/05/2019	610.81
MW20S	01/20/2020	615.40
MW20S	08/10/2020	612.37
MW20S	01/20/2021	612.27
MW20S	04/20/2021	613.45
MW20S	07/26/2021	613.35
MW20S	08/16/2021	612.31
R104	01/20/2015	623.03
R104	04/08/2015	624.77
R104	10/06/2015	621.69
R104	11/16/2015	621.34
R104	02/08/2016	624.11
R104	05/09/2016	624.89
R104	07/25/2016	623.65
R104	11/12/2016	623.49
R104	02/04/2017	624.20
R104	05/13/2017	622.91
R104	07/08/2017	624.09
R104	10/21/2017	619.38
R104	05/08/2018	622.66
R104	08/02/2018	621.73
R104	10/23/2018	621.58



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
R104	01/15/2019	622.43
R104	08/05/2019	623.34
R104	01/20/2020	625.63
R104	08/10/2020	624.56
R104	10/15/2020	621.10
R104	01/20/2021	623.31
R104	01/28/2021	620.57
R104	04/20/2021	624.95
R104	07/26/2021	625.41
R104	08/16/2021	625.92
R201	10/05/2015	619.94
R201	11/16/2015	622.44
R201	02/08/2016	623.40
R201	05/09/2016	622.81
R201	07/25/2016	622.36
R201	11/12/2016	622.82
R201	02/04/2017	622.27
R201	05/13/2017	623.05
R201	07/08/2017	622.31
R201	10/21/2017	618.30
R201	01/25/2018	622.00
R201	05/08/2018	622.78
R201	08/02/2018	622.16
R201	10/23/2018	621.29
R201	01/15/2019	622.17
R201	08/05/2019	622.35
R201	01/20/2020	622.88
R201	08/10/2020	618.89
R201	10/13/2020	616.57
R201	01/20/2021	620.52
R201	01/29/2021	620.52
R201	03/29/2021	623.52
R201	04/20/2021	622.16
R201	04/21/2021	622.59
R201	05/03/2021	622.91
R201	05/06/2021	623.40
R201	05/17/2021	622.68
R201	06/09/2021	621.12
R201	06/14/2021	620.63
R201	06/23/2021	619.92
R201	06/29/2021	621.16
R201	07/12/2021	621.34
R201	07/13/2021	621.36
R201	07/26/2021	620.37
R201	07/28/2021	620.16
R201	08/16/2021	620.61



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
R205	05/08/2018	618.32
R205	08/02/2018	614.95
R205	10/23/2018	618.85
R205	01/15/2019	619.54
R205	08/05/2019	620.13
R205	01/20/2020	620.21
R205	08/10/2020	615.92
R205	10/14/2020	613.87
R205	01/20/2021	617.80
		618.45
R205	01/28/2021	
R205	04/20/2021	619.12
R205	07/26/2021	618.66
R205	08/16/2021	618.99
T127	01/19/2015	615.65
T127	04/08/2015	616.04
T127	07/23/2015	616.04
T127	10/06/2015	615.66
T127	11/16/2015	615.91
T127	02/08/2016	616.04
T127	05/09/2016	616.15
T127	07/25/2016	615.96
T127	11/12/2016	616.73
T127	02/04/2017	616.32
T127	05/13/2017	616.89
T127	07/08/2017	615.99
T127	10/21/2017	612.33
T127	01/27/2018	611.06
T127	05/08/2018	616.66
T127	08/02/2018	616.48
T127	10/23/2018	614.78
T127	01/15/2019	615.13
T127	05/03/2019	617.26
T127	08/05/2019	615.15
T127	01/20/2020	617.05
T127	05/05/2020	617.02
T127	08/10/2020	615.90
T127	10/14/2020	615.08
T127	01/20/2021	615.89
T127	01/29/2021	615.89
T127	04/20/2021	616.54
T127	06/29/2021	616.72
T127	07/26/2021	616.53
T127	08/16/2021	616.65
T128	01/19/2015	614.73
T128	04/08/2015	614.89
T128	07/23/2015	615.40
	1	



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
T128	10/06/2015	614.67
T128	02/08/2016	614.90
T128	05/09/2016	615.01
T128	07/25/2016	614.75
T128	11/12/2016	614.95
T128	02/04/2017	616.00
T128	05/13/2017	615.39
T128	07/08/2017	615.07
T128	10/21/2017	611.33
T128	05/08/2018	615.13
T128	08/02/2018	614.87
T128	10/23/2018	613.17
T128	01/15/2019	613.94
T128	08/05/2019	613.99
T128	01/20/2020	617.25
T128	08/10/2020	616.15
T128	10/14/2020	615.36
T128	01/20/2021	616.20
T128	01/28/2021	616.33
T128	04/20/2021	616.94
T128	07/26/2021	616.81
T128	08/16/2021	616.93
T202	02/08/2016	622.82
T202	05/09/2016	623.66
T202	07/25/2016	619.49
T202	11/12/2016	619.88
T202	02/04/2017	619.73
T202	05/13/2017	620.07
T202	07/08/2017	619.75
T202	10/21/2017	615.31
T202	05/08/2018	619.52
T202	08/02/2018	620.53
T202	10/23/2018	618.36
T202	01/15/2019	618.69
T202	08/05/2019	619.02
T202	01/20/2020	624.22
T202 T202	08/10/2020 01/20/2021	620.39
T202	01/20/2021	620.08
T202	07/26/2021	622.64
T202	08/16/2021	622.69
T408	11/12/2016	618.58
T408	02/04/2017	619.46
T408	05/13/2017	619.00
T408	07/08/2017	619.12
T408	10/21/2017	614.81
1 100	-0, -1, 2017	011101



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
T408	05/08/2018	615.82
T408	08/02/2018	614.45
T408	10/23/2018	616.30
T408	01/15/2019	617.01
T408	08/05/2019	617.15
T408	01/20/2020	619.13
T408	08/10/2020	617.38
T408	01/20/2021	616.85
T408	04/20/2021	616.65
T408	07/26/2021	617.21
T408	08/16/2021	617.22
T409	11/12/2016	615.98
T409	02/04/2017	615.93
T409	05/13/2017	616.75
T409	07/08/2017	617.05
T409	10/21/2017	612.16
T409	05/08/2018	616.02
T409	08/02/2018	615.25
T409	10/23/2018	613.96
T409	01/15/2019	614.78
T409	08/05/2019	615.10
T409	01/20/2020	617.16
T409	08/10/2020	615.43
T409	01/20/2021	614.41
T409	04/20/2021	615.33
T409	07/26/2021	615.72
T409	08/16/2021	615.42
TA31	02/08/2016	621.56
TA31	05/09/2016	621.32
TA31	07/25/2016	620.63
TA31	11/12/2016	620.50
TA31	02/04/2017	621.55
TA31	05/13/2017	620.66
TA31	07/08/2017	620.94
TA31	10/21/2017	616.90
TA31	05/08/2018	619.80
TA31	08/02/2018	620.41
TA31	10/23/2018	618.32
TA31	01/15/2019	619.21
TA31	08/05/2019	619.37
TA31	01/20/2020	622.93
TA31	08/10/2020	614.89
TA31	01/20/2021	615.79
TA31	04/20/2021	622.14
TA31	07/26/2021	618.76
TA31	08/16/2021	619.17



Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
TA32	02/08/2016	615.46
TA32	05/09/2016	616.02
TA32	07/25/2016	615.61
TA32	11/12/2016	615.53
TA32	02/04/2017	614.73
TA32	05/13/2017	615.82
TA32	07/08/2017	615.79
TA32	10/21/2017	612.42
TA32	08/05/2019	615.41
TA32	01/20/2020	616.30
TA33	02/08/2016	619.67
TA33	05/09/2016	619.75
TA33	07/25/2016	616.91
TA33	11/12/2016	616.81
TA33	02/04/2017	617.12
TA33	05/13/2017	617.22
TA33	07/08/2017	617.42
TA33	10/21/2017	612.91
TA33	05/08/2018	618.07
TA33	08/02/2018	616.68
TA33	10/23/2018	617.26
TA33	01/15/2019	617.66
TA33	08/05/2019	618.27
TA33	01/20/2020	620.35
TA33	08/10/2020	614.10
TA33	01/20/2021	614.34
TA33	04/20/2021	619.07
TA33	07/26/2021	616.82
TA33	08/16/2021	616.86
TA34	02/08/2016	619.29
TA34	05/09/2016	619.35
TA34	07/25/2016	617.37
TA34	11/12/2016	617.40
TA34	02/04/2017	617.45
TA34	05/13/2017	617.43
TA34	07/08/2017	617.44
TA34	10/21/2017	613.48
TA34	05/08/2018	617.06
TA34	08/02/2018	616.42
TA34	10/23/2018	614.92
TA34	01/15/2019	615.85
TA34	08/05/2019	616.54
TA34	01/20/2020	619.58
TA34	08/10/2020	615.68
TA34	01/20/2021	616.16
TA34	04/20/2021	618.74



TABLE E-1. GROUNDWATER ELEVATIONS

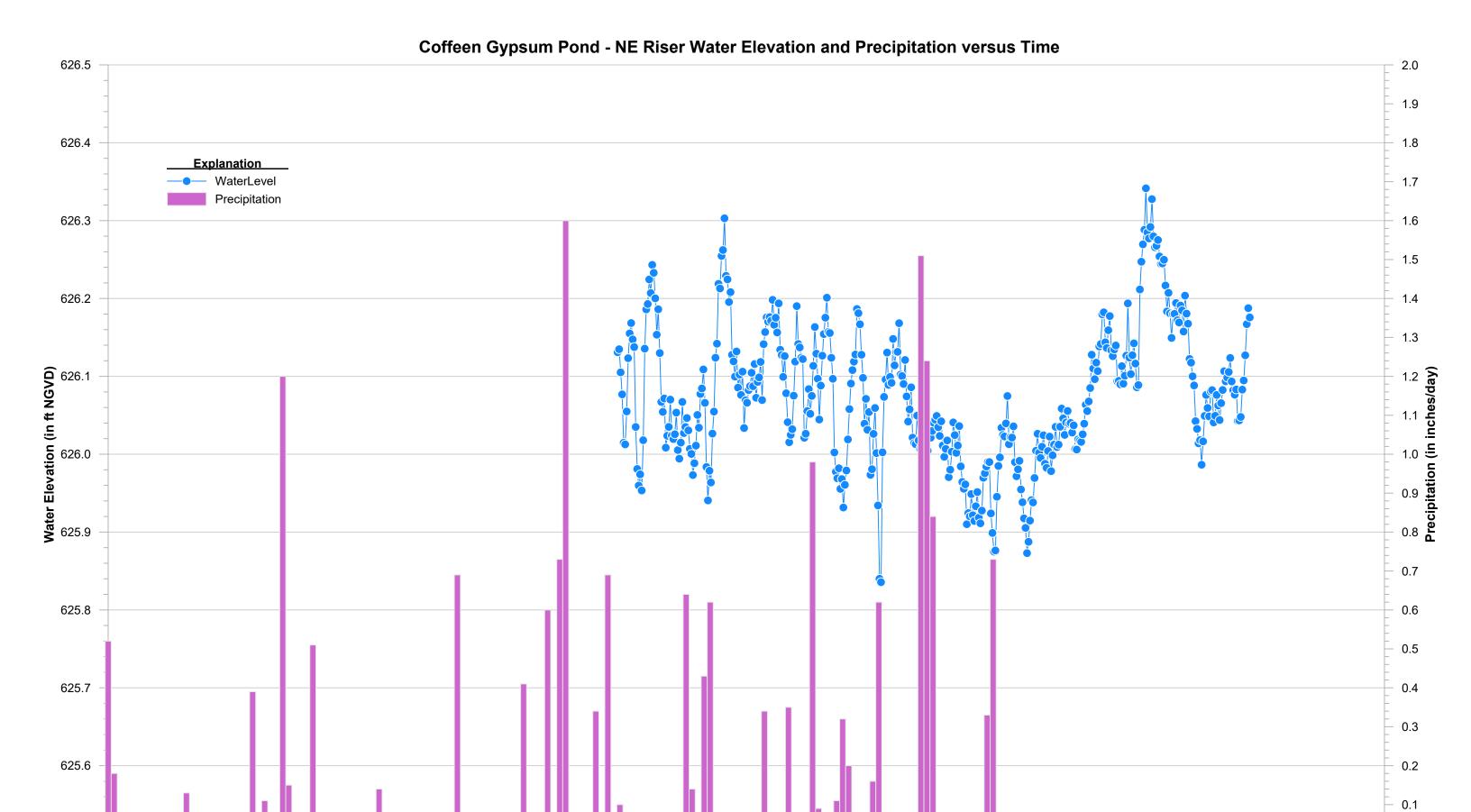
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT GMF GYPSUM STACK POND COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
TA34	07/26/2021	617.18
TA34	08/16/2021	617.42
TR32	08/05/2019	615.67
TR32	01/20/2020	616.56
TR32	08/10/2020	614.92
TR32	01/20/2021	614.50
TR32	04/20/2021	615.59
TR32	07/26/2021	616.09
TR32	08/16/2021	616.18
SG-02	03/29/2021	598.75
SG-02	04/20/2021	598.56
SG-02	05/03/2021	598.74
SG-02	05/17/2021	598.56
SG-02	06/09/2021	598.37
SG-02	06/23/2021	598.34
SG-02	07/12/2021	598.75
SG-02	07/26/2021	598.44
SG-02	08/16/2021	598.39
SG-03	04/20/2021	589.81
SG-03	05/03/2021	589.84
SG-03	05/17/2021	589.84
SG-03	06/09/2021	589.65
SG-03	06/23/2021	589.51
SG-03	07/12/2021	589.97
SG-03	07/26/2021	589.77
SG-03	08/16/2021	589.70
SG-04	04/20/2021	592.99
SG-04	05/03/2021	592.93
SG-04	05/17/2021	593.00
SG-04	06/09/2021	592.82
SG-04	06/23/2021	592.72
SG-04	07/12/2021	591.94
SG-04	07/26/2021	592.83
SG-04	08/16/2021	593.01

Notes

ft NAVD88 = feet relative to the North American Vertical Datum 1988, GEOID 12A $_{\tt generated\ 10/12/2021,\ 9:27:36\ AM\ CDT}$





4/1/21

5/1/21

Time (in months)

6/1/21

7/1/21

0.0

8/1/21

625.5

1/1/21

2/1/21

3/1/21

ADDITIONAL VERTICAL HYDRAULIC GRADIENTS

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT ASH POND NO. 1 COFFEEN, ILLINOIS

Date	G405 Groundwater Elevation (ft NAVD88)	T408 Groundwater Elevation (ft NAVD88)	Head Change (ft)	Distance Change ¹ (ft)	Vertical I Gradi (dh,	ient ²
2/4/2017	UA	LCU (upper)	0.00	40.00	0.00	1
2/4/2017	618.47	619.46	-0.99	12.00	-0.08	up
5/13/2017	618.74	619.00	-0.26	12.00	-0.02	up
7/8/2017	618.54	619.12	-0.58	12.00	-0.05	up
10/21/2017	614.47	614.81	-0.34	12.00	-0.03	up
5/8/2018	618.94	615.82	3.12	12.00	0.26	down
8/2/2018	617.55	614.45	3.10	12.00	0.26	down
10/23/2018	616.40	616.30	0.10	12.00	0.01	down
1/15/2019	616.81	617.01	-0.20	12.00	-0.02	up
8/5/2019	617.72	617.15	0.57	12.00	0.05	down
1/20/2020	619.28	619.13	0.15	12.00	0.01	down
8/10/2020	617.62	617.38	0.24	12.00	0.02	down
1/20/2021	617.12	616.85	0.27	12.00	0.02	down
4/20/2021	617.13	616.65	0.48	12.00	0.04	down
7/26/2021	617.37	617.21	0.16	12.00	0.01	down
						610.0
			Middle of screen elevation T408 59			598.0

Date	G406 Groundwater Elevation (ft NAVD88) UA	T409 Groundwater Elevation (ft NAVD88) LCU (upper)	Head Change (ft)	Distance Change ¹ (ft)	Vertical F Gradi (dh,	ent ²
2/4/2017	617.52	615.93	1.59	8.23	0.19	down
5/13/2017	616.20	616.75	-0.55	8.23	-0.07	up
7/8/2017	616.29	617.05	-0.76	8.23	-0.09	up
10/21/2017	611.27	612.16	-0.89	8.23	-0.11	up
5/8/2018	615.47	616.02	-0.55	8.23	-0.07	up
8/2/2018	615.75	615.25	0.50	8.23	0.06	down
10/23/2018	614.11	613.96	0.15	8.23	0.02	down
1/15/2019	615.36	614.78	0.58	8.23	0.07	down
8/5/2019	616.50	615.10	1.40	8.23	0.17	down
1/20/2020	617.48	617.16	0.32	8.23	0.04	down
8/10/2020	615.54	615.43	0.11	8.23	0.01	down
1/20/2021	612.97	614.41	-1.44	8.23	-0.17	up
4/20/2021	613.78	615.33	-1.55	8.23	-0.19	up
7/26/2021	614.20	615.72	-1.52	8.23	-0.18	up
			Middle	of screen elevatio	n G406	605.9
			Middle	of screen elevatio	n T409	597.7



HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT ASH POND NO. 1 COFFEEN, ILLINOIS

Date	T408 Groundwater Elevation (ft NAVD88) LCU (upper)	G45D Groundwater Elevation (ft NAVD88) LCU (lower)	Head Change (ft)	Distance Change ¹ (ft)	Vertical H Gradio (dh/	ent ²
2/4/2017	619.46	587.71	31.75	13.78	2.30	down
5/13/2017	619.00	586.19	32.81	13.78	2.38	down
7/8/2017	619.12	586.29	32.83	13.78	2.38	down
10/21/2017	614.81	584.69	30.12	13.78	2.19	down
5/8/2018	615.82	587.56	28.26	13.78	2.05	down
8/2/2018	614.45	585.81	28.64	13.78	2.08	down
10/23/2018	616.30	584.60	31.70	13.78	2.30	down
1/15/2019	617.01	586.96	30.05	13.78	2.18	down
8/5/2019	617.15	588.04	29.11	13.78	2.11	down
8/10/2020	617.38	614.21	3.17	13.78	0.23	down
1/20/2021	616.85	614.60	2.25	13.78	0.16	down
4/20/2021	616.65	614.32	2.33	13.78	0.17	down
7/26/2021	617.21	613.58	3.63	13.78	0.26	down
		_	Middle of screen elevation T408			598.0
			Middle of screen elevation G45D 5			584.2

Date	T409 Groundwater Elevation (ft NAVD88) LCU (upper)	G46D Groundwater Elevation (ft NAVD88) LCU (lower)	Head Change (ft)	Distance Change ¹ (ft)	Vertical I Gradi (dh,	ient ²
2/4/2017	615.93	586.06	29.87	22.19	1.35	down
5/13/2017	616.75	584.87	31.88	22.19	1.44	down
7/8/2017	617.05	585.22	31.83	22.19	1.43	down
5/8/2018	616.02	585.86	30.16	22.19	1.36	down
8/2/2018	615.25	583.95	31.30	22.19	1.41	down
10/23/2018	613.96	582.05	31.91	22.19	1.44	down
1/15/2019	614.78	583.17	31.61	22.19	1.42	down
8/5/2019	615.10	583.68	31.42	22.19	1.42	down
8/10/2020	615.43	609.00	6.43	22.19	0.29	down
1/20/2021	614.41	610.49	3.92	22.19	0.18	down
4/20/2021	615.33	611.06	4.27	22.19	0.19	down
7/26/2021	615.72	607.21	8.51	22.19	0.38	down
			Middle	of screen elevation	n T409	597.7
			Middle of screen elevation G46D 575.			



HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT ASH POND NO. 1 COFFEEN, ILLINOIS

Date	G307 Groundwater Elevation (ft NAVD88) UA	G307D Groundwater Elevation (ft NAVD88) LCU (lower)	Head Change (ft)	Distance Change ¹ (ft)	Vertical H Gradie (dh/	ent ²
4/20/2021	624.50	622.48	2.02	38.06	0.05	down
5/17/2021	624.45	622.44	2.01	38.06	0.05	down
7/12/2021	624.45	622.59	1.86	38.06	0.05	down
			Middle	of screen elevation	n G307	606.7
			Middle o	of screen elevation	G307D	568.6

Date	G311 Groundwater Elevation (ft NAVD88) UA	G311D Groundwater Elevation (ft NAVD88) LCU (lower)	Head Change (ft)	Distance Change ¹ (ft)	Vertical H Gradi (dh/	ent ²	
3/29/2021	616.54	575.42	41.12	43.41	0.95	down	
4/22/2021	613.68	575.74	37.94	43.41	0.87	down	
5/3/2021	614.01	573.09	40.92	43.41	0.94	down	
5/17/2021	613.86	572.40	41.46	43.41	0.96	down	
6/9/2021	613.13	573.85	39.28	43.41	0.90	down	
6/15/2021	612.78	575.25	37.53	43.41	0.86	down	
6/23/2021	612.45	571.74	40.71	43.41	0.94	down	
7/12/2021	613.75	571.63	42.12	43.41	0.97	down	
7/26/2021	613.05	569.74	43.31	43.41	1.00	down	
			Middle	of screen elevatio	n G311	606.7	
			Middle	Middle of screen elevation G311D			

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT ASH POND NO. 1 COFFEEN, ILLINOIS

Date	G314 Groundwater Elevation (ft NAVD88) LCU (upper)	G314D Groundwater Elevation (ft NAVD88) DA (PMP)	Head Change (ft)	Distance Change ¹ (ft)	Vertical H Gradi (dh/	ent ²	
3/29/2021	596.40	572.75	23.65	29.76	0.79	down	
4/20/2021	603.16	571.76	31.40	27.40	1.15	down	
5/3/2021	604.66	568.77	35.89	27.40	1.31	down	
5/17/2021	605.61	566.84	38.77	27.40	1.42	down	
6/9/2021	607.54	567.45	40.09	27.40	1.46	down	
6/14/2021	608.16	568.60	39.56	27.40	1.44	down	
6/23/2021	605.19	566.77	38.42	27.40	1.40	down	
7/12/2021	605.32	566.88	38.44	27.40	1.40	down	
7/26/2021	606.66	566.65	40.01	27.40	1.46	down	
_			Middle	of screen elevatio	n G314	594.0	
			Middle o	Middle of screen elevation G314D			

[O: KLT 6/4/21, C:YMD 6/7/21; U:KLT 8/25/21, C:EDP 8/31/21]

Notes:

- - = no data collected on date / no vertical gradient calculated

DA = deep aquifer

dh = head change

dl = distance change

ft = foot/feet

LCU (lower) = lower confining unit (Smithboro)

LCU (upper) = lower confining unit (Vandalia)

NAVD88 = North American Vertical Datum of 1988

PMP = potential migration pathway

UA = uppermost aquifer

¹ Distance change was calculated using the midpoint of the piezometer screen and water table surface. If the water table surface was above the top of the monitoring well screen, then distance change was calculated using the midpoint of both screens.

 $^{^{2}}$ Vertical gradients between ± 0.0015 are considered flat, and typically have less than 0.02 foot difference in groundwater elevation between wells.

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT GYPSUM MANAGEMENT FACILITY RECYCLE POND COFFEEN, ILLINOIS

Date	G405 Groundwater Elevation (ft NAVD88) UA	T408 Groundwater Elevation (ft NAVD88) LCU (upper)	Head Change (ft)	Distance Change ¹ (ft)	-	tical Gradient ² /dl)
2/4/2017	618.47	619.46	-0.99	12.00	-0.08	ир
5/13/2017	618.74	619.00	-0.26	12.00	-0.02	up
7/8/2017	618.54	619.12	-0.58	12.00	-0.05	up
10/21/2017	614.47	614.81	-0.34	12.00	-0.03	up
5/8/2018	618.94	615.82	3.12	12.00	0.26	down
8/2/2018	617.55	614.45	3.10	12.00	0.26	down
10/23/2018	616.40	616.30	0.10	12.00	0.01	down
1/15/2019	616.81	617.01	-0.20	12.00	-0.02	up
8/5/2019	617.72	617.15	0.57	12.00	0.05	down
1/20/2020	619.28	619.13	0.15	12.00	0.01	down
8/10/2020	617.62	617.38	0.24	12.00	0.02	down
1/20/2021	617.12	616.85	0.27	12.00	0.02	down
4/20/2021	617.13	616.65	0.48	12.00	0.04	down
7/26/2021	617.37	617.21	0.16	12.00	0.01	down
			Middle of screen elevation G405D 610.0			610.0
			Middle of	Middle of screen elevation T408 598.0		

Date	G275 Groundwater Elevation (ft NAVD88) UA	G275D Groundwater Elevation (ft NAVD88) DA (PMP)	Head Change (ft)	Distance Change ¹ (ft)	Vertical Hydraulic Gradient ² (dh/dl)	
4/20/21-4/21/20	605.00	568.33	36.67	42.14	0.87	down
7/12/21-7/13/21	605.63	570.43	35.20	42.77	0.82	down
7/26/2021	605.05	570.35	34.70	42.18	0.82	down
	_	_	Middle of	screen elevation	n G275	605.7
			Middle of	screen elevation	n G275D	562.9



HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT GYPSUM MANAGEMENT FACILITY RECYCLE POND COFFEEN, ILLINOIS

Date	T408 Groundwater Elevation (ft NAVD88) LCU (upper)	G45D Groundwater Elevation (ft NAVD88) LCU (lower)	Head Change (ft)	Distance Change ¹ (ft)	Vertical Hydraulic Gradient ² (dh/dl)	
2/4/2017	619.46	587.71	31.75	13.78	2.30	down
5/13/2017	619.00	586.19	32.81	13.78	2.38	down
7/8/2017	619.12	586.29	32.83	13.78	2.38	down
10/21/2017	614.81	584.69	30.12	13.78	2.19	down
5/8/2018	615.82	587.56	28.26	13.78	2.05	down
8/2/2018	614.45	585.81	28.64	13.78	2.08	down
10/23/2018	616.30	584.60	31.70	13.78	2.30	down
1/15/2019	617.01	586.96	30.05	13.78	2.18	down
8/5/2019	617.15	588.04	29.11	13.78	2.11	down
8/10/2020	617.38	614.21	3.17	13.78	0.23	down
1/20/2021	616.85	614.60	2.25	13.78	0.16	down
4/20/2021	616.65	614.32	2.33	13.78	0.17	down
7/26/2021	617.21	613.58	3.63	13.78	0.26	down
			Middle of screen elevation T408			598.0
			Middle of screen elevation G45D			584.2

[O: KLT 6/4/21, C:YMD 6/7/21][U:KLT 8/25/21, C:EDP 8/31/21]

Notes:

DA = deep aguifer

dh = head change

dl = distance change

ft = foot/feet

LCU (lower) = lower confining unit (Smithboro)

LCU (upper) = lower confining unit (Vandalia)

NAVD88 = North American Vertical Datum of 1988

PMP = potential migration pathway

UA = uppermost aquifer

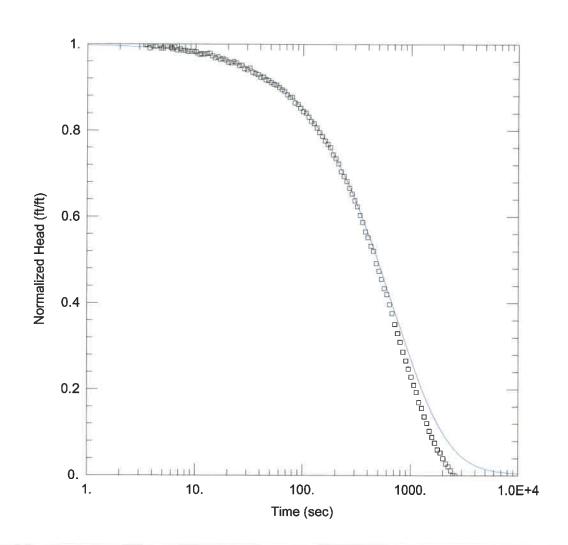


¹ Distance change was calculated using the midpoint of the piezometer screen and water table surface. If the water table surface was above the top of the monitoring well screen, then distance change was calculated using the midpoint of both screens.

 $^{^{2}}$ Vertical gradients between ± 0.0015 are considered flat, and typically have less than 0.02 foot difference in groundwater elevation between wells.

^{-- =} no data collected on date / no vertical gradient calculated

APPENDIX F HYDRAULIC CONDUCTIVITY TEST DATA



G206 FALLING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G206-fh.aqt

Date: 06/02/21

Time: 15:40:13

PROJECT INFORMATION

Client: Coffeen Power Station

Project: 20E0111A Location: Coffeen, IL Test Well: G206

Test Date: 17 March 2021

AQUIFER DATA

Saturated Thickness: 2.3 ft

WELL DATA (G206)

Initial Displacement: 1.5 ft

Total Well Penetration Depth: 13.97 ft

Casing Radius: 0.08333 ft

Static Water Column Height: 14.47 ft

Screen Length: 4.41 ft Well Radius: 0.08333 ft

SOLUTION

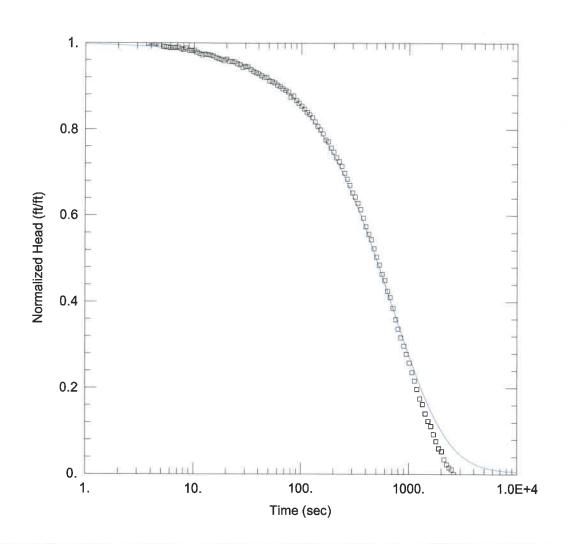
Aquifer Model: Confined

Kr = 0.0005 cm/sec

Kz/Kr = 1.

Solution Method: KGS Model

Ss = 1.0E-6 ft⁻¹



G206 RISING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G206-rh.aqt

Date: 06/02/21

Time: 15:40:14

PROJECT INFORMATION

Client: Coffeen Power Station

Project: 20E0111A Location: Coffeen, IL Test Well: G206

Test Date: 17 March 2021

AQUIFER DATA

Saturated Thickness: 2.3 ft

WELL DATA (G206)

Initial Displacement: 1.385 ft

Total Well Penetration Depth: 13.97 ft

Casing Radius: 0.08333 ft

Static Water Column Height: 14.47 ft

Screen Length: 4.41 ft Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Confined

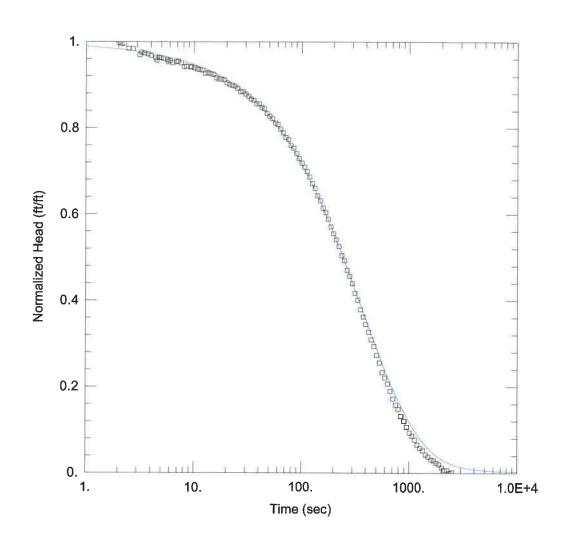
= 0.00049 cm/sec

Kz/Kr = 1.

Kr

Solution Method: KGS Model

Ss = $1.0E-6 \text{ ft}^{-1}$



G209 RISING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G209-rh.aqt

Date: 06/02/21

Time: 15:40:16

PROJECT INFORMATION

Client: Coffeen Power Station

Project: 20E0111A Location: Coffeen, IL Test Well: G209

Test Date: 17 March 2021

AQUIFER DATA

Saturated Thickness: 8. ft

WELL DATA (G209)

Initial Displacement: 1.575 ft

Total Well Penetration Depth: 11.89 ft

Casing Radius: 0.08333 ft

Static Water Column Height: 14.76 ft

Screen Length: 4.54 ft Well Radius: 0.08333 ft

SOLUTION

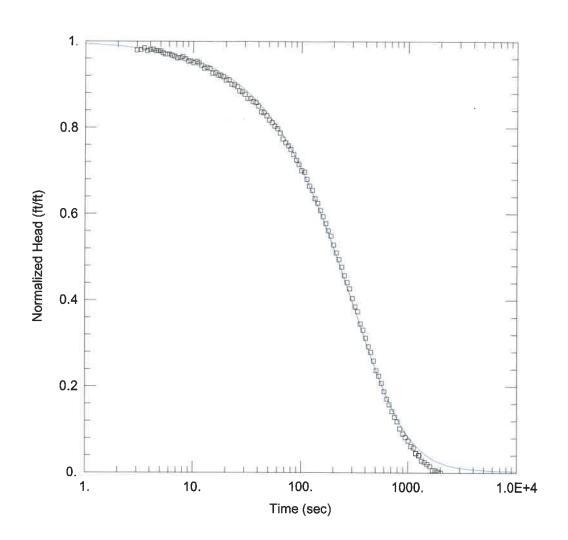
Aquifer Model: Confined

= 0.00025 cm/sec

Kz/Kr = 1.

Solution Method: KGS Model

Ss = 0.0007 ft^{-1}



G212 FALLING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G212-fh.aqt

Date: 06/02/21

Time: 15:40:17

PROJECT INFORMATION

Client: Coffeen Power Station

Project: 20E0111A Location: Coffeen, IL Test Well: G212

Test Date: 16 March 2021

AQUIFER DATA

Saturated Thickness: 1. ft

WELL DATA (G212)

Initial Displacement: 1.45 ft

Total Well Penetration Depth: 12.62 ft

Casing Radius: 0.08333 ft

Static Water Column Height: 13.14 ft

Screen Length: 4.55 ft Well Radius: 0.08333 ft

SOLUTION

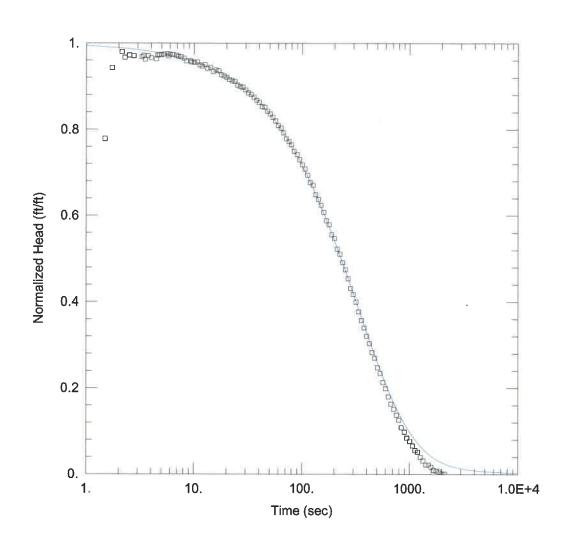
Aquifer Model: Confined

r = 0.0021 cm/sec

Kz/Kr = 1.

Solution Method: KGS Model

Ss = $1.5E-5 \text{ ft}^{-1}$



G212 RISING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G212-rh.aqt

Date: 06/02/21

Time: 15:40:19

PROJECT INFORMATION

Client: Coffeen Power Station

Project: 20E0111A Location: Ciffeen, IL Test Well: G212

Test Date: 16 March 2021

AQUIFER DATA

Saturated Thickness: 1. ft

WELL DATA (G212)

Initial Displacement: 1.525 ft

Total Well Penetration Depth: 12.62 ft

Casing Radius: 0.08333 ft

Static Water Column Height: 13.14 ft

Screen Length: 4.55 ft Well Radius: 0.08333 ft

SOLUTION

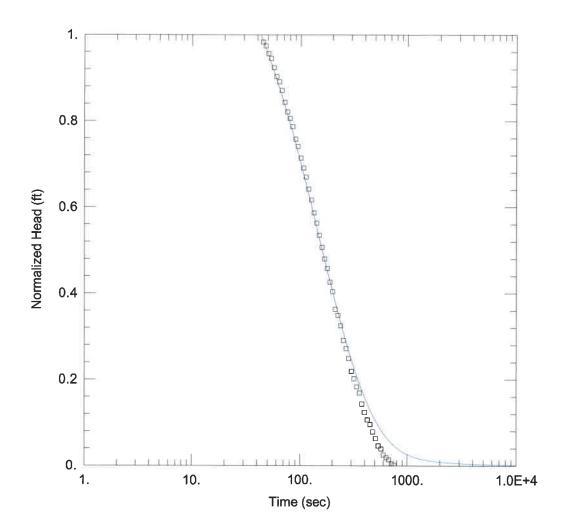
Aquifer Model: Confined

Kr = 0.0018 cm/sec

Kz/Kr = 1.

Solution Method: KGS Model

Ss = $4.0E-5 \text{ ft}^{-1}$



G215 FALLING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G215-fh.aqt

Date: 06/02/21

Time: 15:40:20

PROJECT INFORMATION

Client: Coffeen Power Station

Project: 20E0111A Location: Coffeen, IL Test Well: G215

Test Date: 16 March 2021

AQUIFER DATA

Saturated Thickness: 1.2 ft

WELL DATA (G215)

Initial Displacement: 1.35 ft

Total Well Penetration Depth: 12.4 ft

Casing Radius: 0.08333 ft

Static Water Column Height: 12.4 ft

Screen Length: 4.9 ft Well Radius: 0.08333 ft

SOLUTION

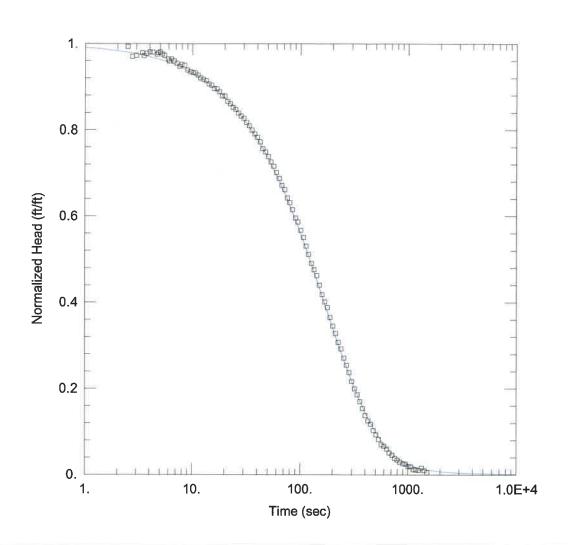
Aquifer Model: Confined

Kr = 0.004 cm/sec

Kz/Kr = 1.

Solution Method: KGS Model

Ss = $1.0E-5 \text{ ft}^{-1}$



G215 RISING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G215-rh.agt

Date: 06/02/21

Time: 15:40:21

PROJECT INFORMATION

Client: Coffeen Power Station

Project: 20E0111A Location: Coffeen, IL Test Well: G215

Test Date: 16 March 2021

AQUIFER DATA

Saturated Thickness: 1.2 ft

WELL DATA (G215)

Initial Displacement: 1.4 ft

Total Well Penetration Depth: 12.4 ft

Casing Radius: 0.08333 ft

Static Water Column Height: 12.4 ft

Screen Length: 4.9 ft Well Radius: 0.08333 ft

SOLUTION

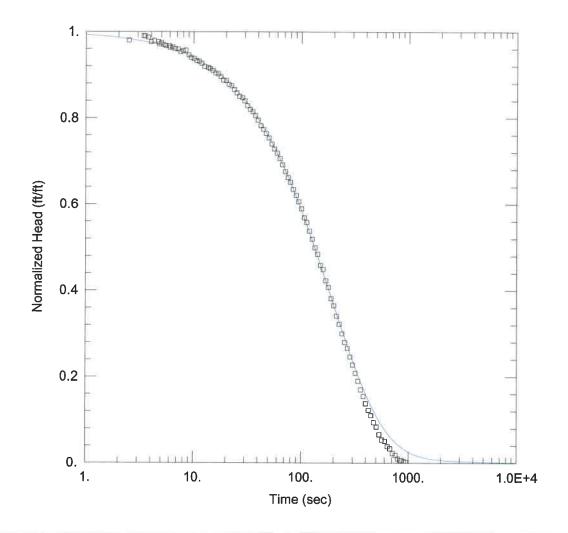
Aquifer Model: Confined

Kr = 0.0035 cm/sec

Kz/Kr = 1.

Solution Method: KGS Model

Ss = $1.0E-5 \text{ ft}^{-1}$



G218 FALLING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G218-fh.aqt

Date: 06/02/21

Time: 15:40:23

PROJECT INFORMATION

Client: Coffeen Power Station

Project: 20E0111A Location: Coffeen, IL Test Well: G218

Test Date: 16 March 2021

AQUIFER DATA

Saturated Thickness: 1.9 ft

WELL DATA (G218)

Initial Displacement: 1.45 ft

Total Well Penetration Depth: 12.86 ft

Casing Radius: 0.08333 ft

Static Water Column Height: 20.86 ft

Screen Length: 4.44 ft Well Radius: 0.08333 ft

SOLUTION

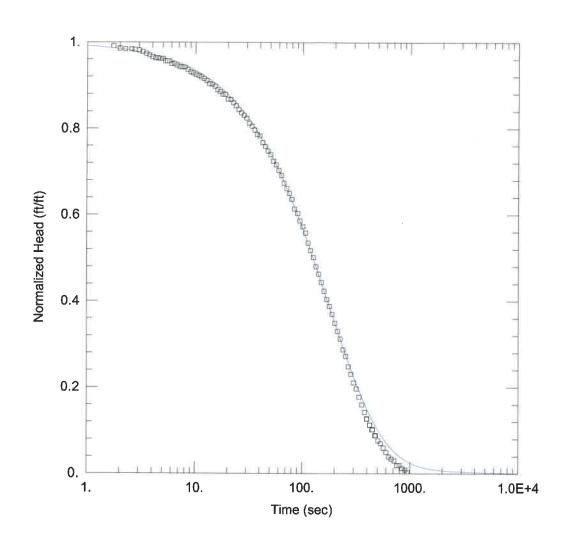
Aquifer Model: Confined

Kr = 0.0026 cm/sec

Kz/Kr = 1.

Solution Method: KGS Model

Ss = 1.0E-6 ft⁻¹



G218 RISING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G218-rh.aqt

Date: 06/02/21

Time: 15:40:24

PROJECT INFORMATION

Client: Coffeen Power Station

Project: 20E0111A Location: Coffeen, IL Test Well: G218

Test Date: 16 March 2021

AQUIFER DATA

Saturated Thickness: 1.9 ft

WELL DATA (G218)

Initial Displacement: 1.5 ft

Total Well Penetration Depth: 12.86 ft

Casing Radius: 0.08333 ft

Static Water Column Height: 20.86 ft

Screen Length: 4.44 ft Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Confined

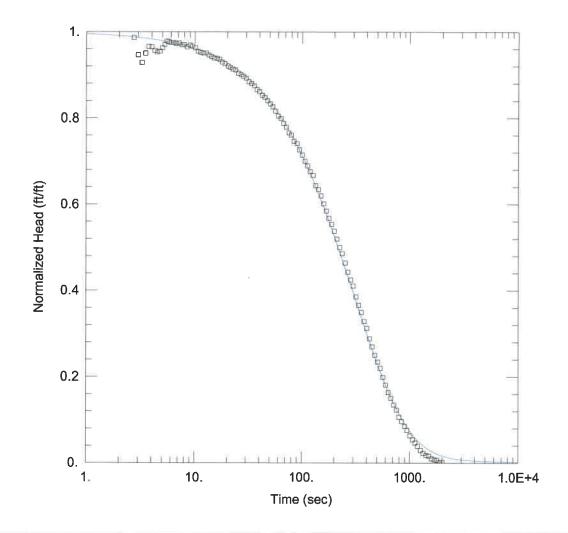
= 0.0024 cm/sec

 $Kz/Kr = \overline{1}$.

Kr

Solution Method: KGS Model

Ss $= 1.0E-5 \text{ ft}^{-1}$



G272 FALLING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G272-fh.aqt

Date: 06/02/21

Time: 15:40:26

PROJECT INFORMATION

Client: Coffeen Power Station

Project: 20E0111A Location: Coffeen, IL Test Well: G272

Test Date: 16 March 2021

AQUIFER DATA

Saturated Thickness: 1.7 ft

WELL DATA (G272)

Initial Displacement: 1.425 ft
Total Well Penetration Depth: 7.7 ft

Casing Radius: 0.08333 ft

Static Water Column Height: 7.22 ft

Screen Length: 4.87 ft Well Radius: 0.08333 ft

SOLUTION

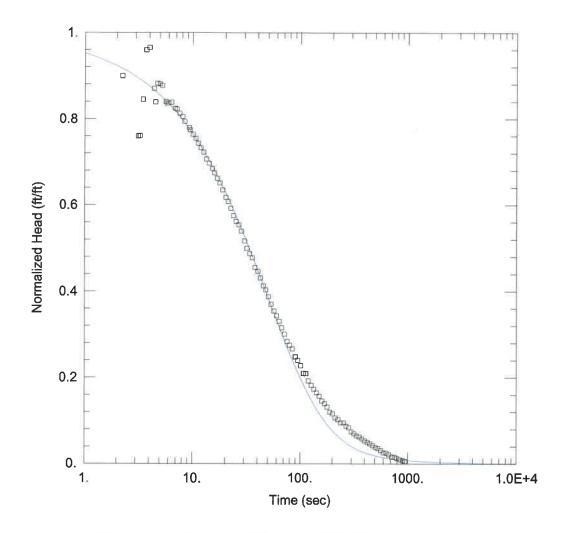
Aquifer Model: Confined

Kr = 0.0017 cm/sec

Kz/Kr = 1.

Solution Method: KGS Model

Ss = 1.0E-6 ft⁻¹



G283 FALLING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G283-fh.agt

Date: 06/02/21

Time: 15:41:16

PROJECT INFORMATION

Client: Coffeen Power Station

Project: 20E0111A Location: Coffeen, IL Test Well: G283

Test Date: 16 March 2021

AQUIFER DATA

Saturated Thickness: 1.8 ft

WELL DATA (G283)

Initial Displacement: 1.55 ft

Total Well Penetration Depth: 16.58 ft

Casing Radius: 0.08333 ft

Static Water Column Height: 16.77 ft

Screen Length: 9.78 ft Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Confined

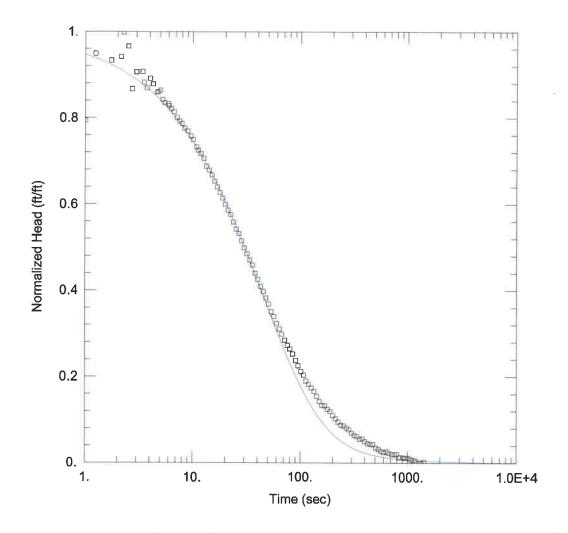
Kr

= 0.0042 cm/sec

Kz/Kr = 1.

Solution Method: KGS Model

 $= 0.005 \text{ ft}^{-1}$ Ss



G283 RISING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G283-rh.aqt

Date: 06/02/21

Time: 15:41:17

PROJECT INFORMATION

Client: Coffeen Power Station

Project: 20E0111A Location: Coffeen, IL Test Well: G283

Test Date: 16 March 2021

AQUIFER DATA

Saturated Thickness: 1.8 ft

WELL DATA (G283)

Initial Displacement: 1.6 ft

Total Well Penetration Depth: 16.58 ft

Casing Radius: 0.08333 ft

Static Water Column Height: 16.77 ft

Screen Length: 9.78 ft Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Confined

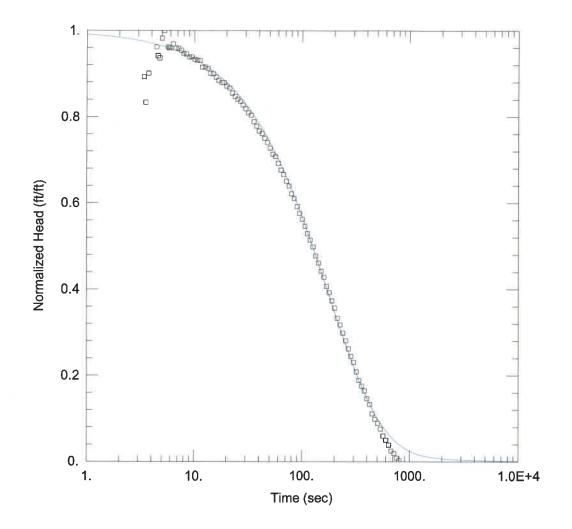
= 0.0045 cm/sec

 $Kz/Kr = \overline{1.}$

Kr

Solution Method: KGS Model

Ss = 0.006 ft^{-1}



G284 FALLING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G284-fh.aqt

Date: 06/02/21

Time: 15:41:19

PROJECT INFORMATION

Client: Coffeen Power Station

Project: 20E0111A Location: Coffeen, IL Test Well: G284

Test Date: 12 March 2021

AQUIFER DATA

Saturated Thickness: 3.8 ft

WELL DATA (G284)

Initial Displacement: 1.425 ft

Total Well Penetration Depth: 6.67 ft

Casing Radius: 0.08333 ft

Static Water Column Height: 7.05 ft

Screen Length: 4.77 ft Well Radius: 0.08333 ft

SOLUTION

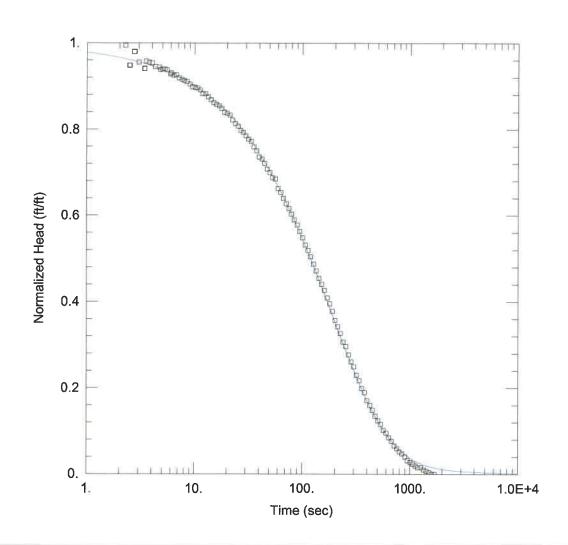
Aquifer Model: Confined

= 0.0012 cm/sec

Kz/Kr = 1.

Solution Method: KGS Model

Ss = $2.5E-5 \text{ ft}^{-1}$



G284 RISING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G284-rh.aqt

Date: 06/02/21

Time: 15:41:20

PROJECT INFORMATION

Client: Coffeen Power Station

Project: 20E0111A Location: Coffeen, IL Test Well: G284

Test Date: 12 March 2021

AQUIFER DATA

Saturated Thickness: 3.8 ft

WELL DATA (G284)

Initial Displacement: 1.55 ft

Total Well Penetration Depth: 6.67 ft

Casing Radius: 0.08333 ft

Static Water Column Height: 7.05 ft

Screen Length: 4.77 ft Well Radius: 0.08333 ft

SOLUTION

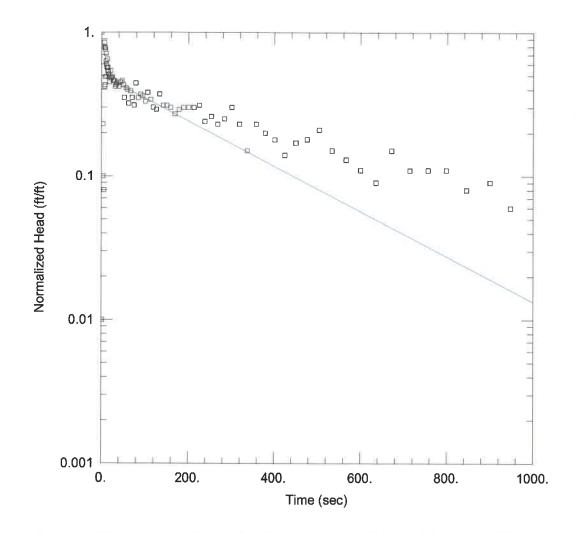
Aquifer Model: Confined

Kr = 0.00078 cm/sec

 $Kz/Kr = \frac{5.65}{1.}$

Solution Method: KGS Model

Ss = 0.0037 ft^{-1}



G285 FALLING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G285-fh.aqt

Date: 06/02/21

Time: 15:41:22

PROJECT INFORMATION

Client: Coffeen Power Station

Project: 20E0111A Location: Coffeen, IL Test Well: G285

Test Date: 12 March 2021

AQUIFER DATA

Saturated Thickness: 5.6 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (G285)

Initial Displacement: 0.1 ft

Total Well Penetration Depth: 17.73 ft

Casing Radius: 0.08333 ft

Static Water Column Height: 18.11 ft

Screen Length: 9.77 ft Well Radius: 0.08333 ft

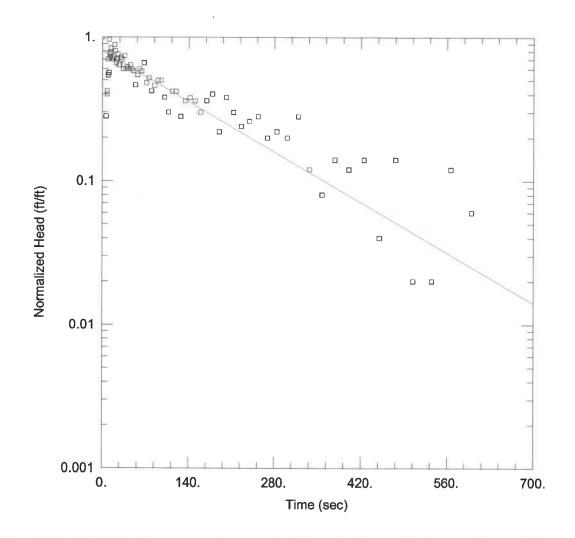
SOLUTION

Aquifer Model: Confined

Solution Method: Bouwer-Rice

K = 0.00027 cm/sec

y0 = 0.05 ft





Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G285-rh.aqt

Date: 06/02/21

Time: 15:41:23

PROJECT INFORMATION

Client: Coffeen Power Station

Project: 20E0111A Location: Coffeen, IL Test Well: G285

Test Date: 12 March 2021

AQUIFER DATA

Saturated Thickness: 5.6 ft

Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (G285)

Initial Displacement: 0.05 ft

Total Well Penetration Depth: 17.73 ft

Casing Radius: 0.08333 ft

Static Water Column Height: 18.11 ft

Screen Length: 9.77 ft Well Radius: 0.08333 ft

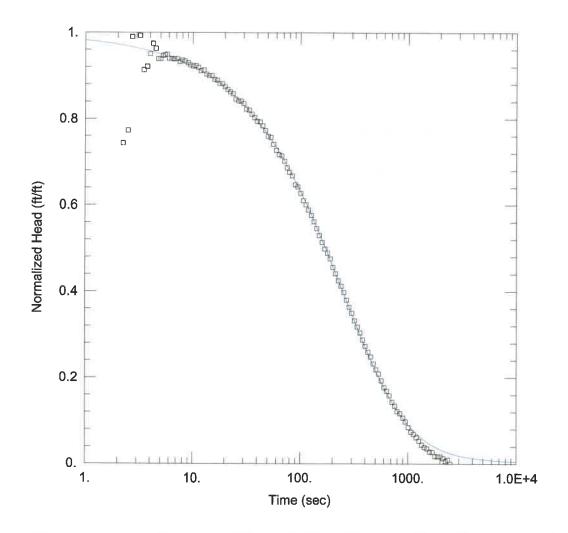
SOLUTION

Aquifer Model: Confined

K = 0.00043 cm/sec

Solution Method: Bouwer-Rice

y0 = 0.04 ft



G286 FALLING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G286-fh.aqt

Date: 06/02/21

Time: 15:41:25

PROJECT INFORMATION

Client: Coffeen Power Station

Project: 20E0111A Location: Coffeen, IL Test Well: G286

Test Date: 12 March 2021

AQUIFER DATA

Saturated Thickness: 1. ft

WELL DATA (G286)

Initial Displacement: 1.25 ft

Total Well Penetration Depth: 5.92 ft

Casing Radius: 0.08333 ft

Static Water Column Height: 6.26 ft

Screen Length: 4.79 ft Well Radius: 0.08333 ft

SOLUTION

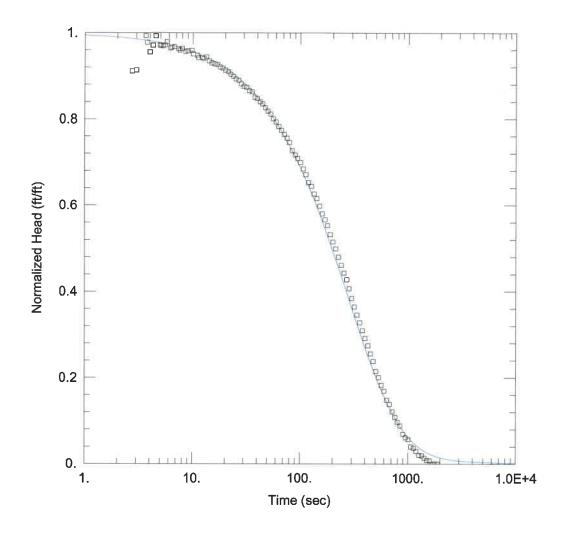
Aquifer Model: Confined

Kr = 0.0012 cm/sec

Kz/Kr = 1.

Solution Method: KGS Model

Ss = $0.01 \, \text{ft}^{-1}$



G287 FALLING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G287-fh.aqt

Date: 06/02/21 Time: 15:41:26

PROJECT INFORMATION

Client: Coffeen Power Station

Project: 20E0111A Location: Coffeen, IL Test Well: G287

Test Date: 17 March 2021

AQUIFER DATA

Saturated Thickness: 2.7 ft

WELL DATA (G287)

Initial Displacement: 1.425 ft

Total Well Penetration Depth: 5.71 ft

Casing Radius: 0.08333 ft

Static Water Column Height: 6.05 ft

Screen Length: 4.82 ft Well Radius: 0.08333 ft

SOLUTION

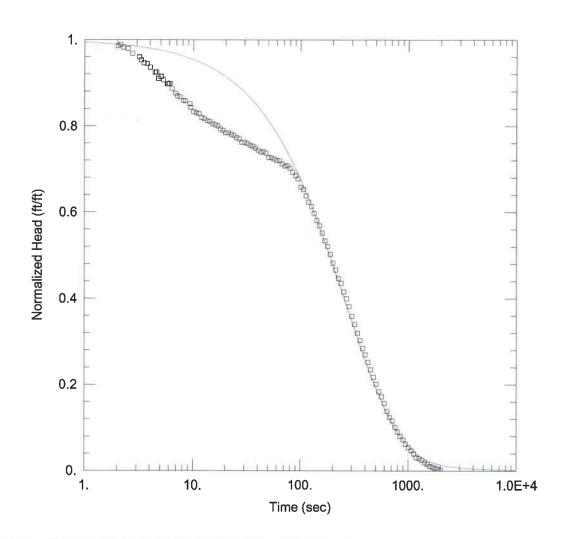
Aquifer Model: Confined

Solution Method: KGS Model

Kr = 0.0011 cm/sec

Ss = $3.5E-6 \text{ ft}^{-1}$

Kz/Kr = 1.



G287 RISING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G287-rh.aqt

Date: 06/02/21 Time: 15:41:28

PROJECT INFORMATION

Client: Coffeen Power Station

Project: 20E0111A Location: Coffeen, IL Test Well: G287

Test Date: 17 March 2021

AQUIFER DATA

Saturated Thickness: 2.7 ft

WELL DATA (G287)

Initial Displacement: 1.5 ft

Total Well Penetration Depth: 5.71 ft

Casing Radius: 0.08333 ft

Static Water Column Height: 6.05 ft

Screen Length: 4.82 ft Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Confined

= 0.0011 cm/sec

 $Kz/Kr = \overline{1}$.

Solution Method: KGS Model

Ss = $7.0E-6 \text{ ft}^{-1}$

ADDITIONAL FIELD HYDRAULIC CONDUCTIVITIES

FIELD HYDRAULIC CONDUCTIVITIES

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT **COFFEEN POWER PLANT** ASH POND NO. 1 COFFEEN, ILLINOIS

Well ID	Gradient Position	Bottom of Screen Elevation (ft NAVD88)	Screen Length ¹ (ft)	Field Identified Screened Material	Slug Type	Analysis Method	Falling Head (Slug In) Hydraulic Conductivity (cm/s)	Rising Head (Slug Out) Hydraulic Conductivity (cm/s)	Minimum Hydraulic Conductivity (cm/s)	Maximum Hydraulic Conductivity (cm/s)	Hydraulic Conductivity Geometric Mean (cm/s)
Uppermos	t Aquifer										
G301	D	604.31	4.65	(ML)s	solid	Kansas Geological Survey	1.1E-03	1.2E-03			
G303	D	599.07	10	CL	solid	Kansas Geological Survey	2.8E-04	2.6E-04			
G308	D	606.70	4.79	s(ML), s(CL), (CL)s	solid	Kansas Geological Survey	5.5E-03	1.6E-03			
G309	D	605.02	4.78	SP, s(CL), (ML)s	solid	Kansas Geological Survey	9.1E-03	8.8E-04		9.1E-03	
G310	D	604.86	4.79	SM, s(ML)	solid	Kansas Geological Survey	7.5E-03	5.9E-03	2.6E-04		2.0E-03
G311	D	604.28	4.77	s(ML), s(CL)	solid	Bouwer-Rice	1.5E-03				
G312	D	602.34	4.79	s(ML), s(CL)	solid	Kansas Geological Survey	1.1E-03	1.1E-03			
G313	D	600.40	4.81	SP, s(ML), (CL)s	solid	Kansas Geological Survey	2.7E-03	3.5E-03			
G315	D	606.46	4.79	s(CL)	solid	Kansas Geological Survey	6.6E-03	5.8E-03			
Lower Con	fining Unit		•								
G307D	D	563.76	9.77	(CL)s	solid	Kansas Geological Survey	3.2E-04	1.2E-04			
G311D	D	558.29	9.94	CL	solid	Kansas Geological Survey	3.8E-04	2.1E-04	1.2E-04	2.3E-03	5.0E-04
G316	D	584.82	4.80	SP, s(ML), (CL)s	solid	Kansas Geological Survey	2.3E-03	2.3E-03			
Deep Aqui	fer (PMP)										
G314D	D	561.76	9.77	SP, s(CL)	solid	Bouwer-Rice	3.3E-04	2.3E-05	2.3E-05	3.3E-04	8.7E-05
										[O: KLT 07/0	9/21; C:EDP 8/31/21]

1. All wells are constructed from 2 inch PVC with 0.01 inch slotted screens.

- - = Test not analyzed/performed

cm/s = centimeters per second

D = downgradient

ft = foot/feet

NAVD88 = North American Vertical Datum of 1988

PMP= potential migration pathway

PVC = polyvinyl chloride

USCS = Unified Soil Classification System CL = Lean Clay s(CL) = Sandy Lean Clay (CL)s = Lean Clay with Sand s(ML) = Sandy Silt (ML)s = Silt with Sand SP = Poorly-Graded Sand



FIELD HYDRAULIC CONDUCTIVITIES

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT GMF RECYCLE POND COFFEEN, ILLINOIS

Well ID	Gradient Position	Bottom of Screen Elevation (ft NAVD88)	Screen Length ¹ (ft)	Field Identified Screened Material	Slug Type	Analysis Method	Falling Head (Slug In) Hydraulic Conductivity (cm/s)	Rising Head (Slug Out) Hydraulic Conductivity (cm/s)	Minimum Hydraulic Conductivity (cm/s)	Maximum Hydraulic Conductivity (cm/s)	Hydraulic Conductivity Geometric Mean (cm/s)
Uppermos	t Aquifer										
G272	D	606.74	4.87	SP to ML, (CL)s	solid	Kansas Geological Survey	1.7E-03			1.7E-03	1.1E-03
G284	D	602.48	4.77	ML	solid	Kansas Geological Survey	1.2E-03	7.8E-04	7.8E-04		
G286	D	601.81	4.79	SP, ML, CL	solid	Kansas Geological Survey	1.2E-03		7.0E-U4		
G287	D	604.09	4.82	SP, ML, CL	solid	Kansas Geological Survey	1.1E-03	1.1E-03			
Lower Con	nfining Unit	(PMP)									
G283	D	590.13	9.78	SP, ML	solid	Kansas Geological Survey	4.2E-03	4.5E-03	2.75.04	4 FE 02	1 25 02
G285	D	587.09	9.77	CL	solid	Bouwer-Rice	2.7E-04	4.3E-04	2.7E-04	4.5E-03	1.2E-03
-			•			-	•	•	•	[(D: KLT, C:EDP 8/31/21]

Notes:

1. All wells are constructed from 2 inch PVC with 0.01 inch slotted screens.

- - = Test not analyzed/performed

cm/s = centimeters per second

D = downgradient

ft = foot/feet

NAVD88 = North American Vertical Datum of 1988

PVC = polyvinyl chloride

PMP = potential migration pathway

USCS = Unified Soil Classification System

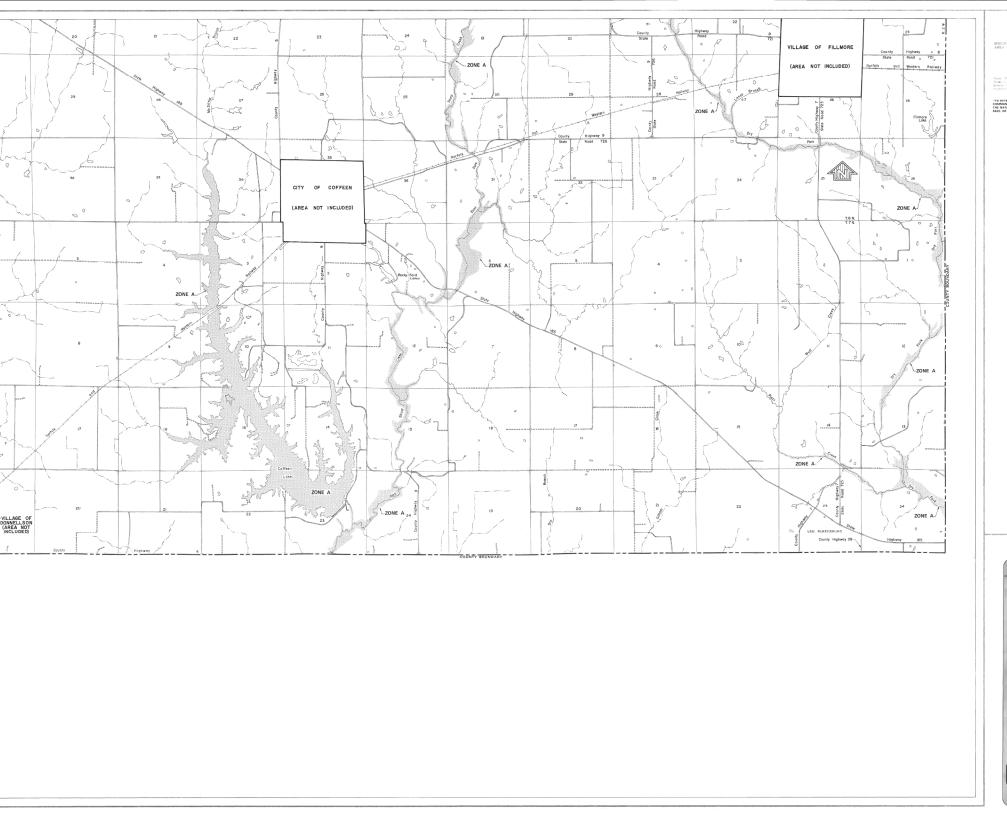
CL = Lean Clay

(CL)s = Lean Clay with Sand ML = Silt

SP = Poorly-Graded Sand



APPENDIX G FEMA FLOOD HAZARD MAP



SPECIAL FLOOD HAZARD AREA

Note: These might may not inc. Areas in the commitment, After Boeter Front Heard Areas who mouthers and other commanders.

"TO DETERMINE IF FLOOD INSUIN COMMUNITY, CONTACT YOUR IN THE NATIONAL FLOOD INSURANC 6620, OR (800) 424-8872."

FHBN FLOOD HAZA

MONT COUN ILLIN UNINCOR

PANEL 9 01

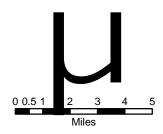
COMMU

federal emerge



This map illustrates stream reaches at risk for potential flooding that were not designated as Special Flood Hazard Areas (SFHAs) and were not shown on Federal Emergency Management Agency (FEMA) Flood Insurance Rate Maps as of July 1, 2007. Also shown are areas of Risk Class A (high), B (medium), and C (low) adopted by FEMA Region 5. Streams lacking a SFHA designation and draining more than one square mile in a Risk Class A area or draining more than 10 square miles in a Risk Class B or C area are shown with red lines.

A table associated with this map lists streams with unmapped potential flood risk, their names, and the number of miles not within a Special Flood Hazard Area.



Legend

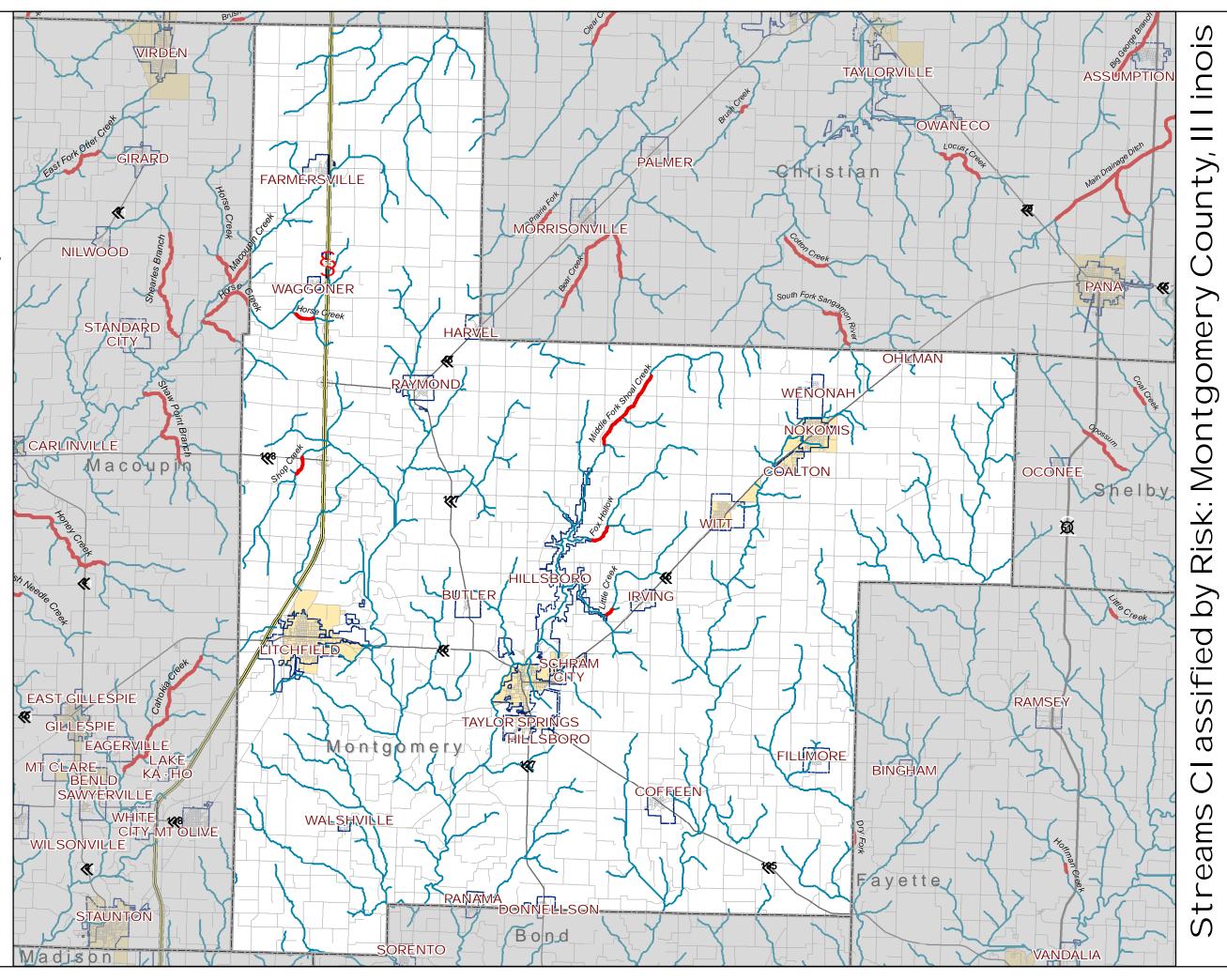
Streams without SFHAs
Streams
Municipal Boundaries
County Boundaries
Interstate Hwy
US Highways
State Routes
Roads

Risk Classes

A - High Risk

B - Medium Risk

C - Low Risk



ATTACHMENT I

Intended for

Illinois Power Generating Company

Date

October 25, 2021

Project No.

1940100806-002

GROUNDWATER MONITORING PLAN

GMF GYPSUM STACK POND COFFEEN POWER PLANT COFFEEN, ILLINOIS



GROUNDWATER MONITORING PLAN COFFEEN POWER PLANT GMF GYPSUM STACK POND

Project Name Coffeen Power Plant GMF Gypsum Stack Pond

Project No. **1940100806-002**

Recipient Illinois Power Generating Company
Document Type Groundwater Monitoring Plan

Revision FINAL

Date **October 25, 2021**

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Eric J. Tlachac, PE Senior Managing Engineer

Nathaniel R. Keller Senior Hydrogeologist Chase J. Christenson, PG Hydrogeologist

LICENSED PROFESSIONAL CERTIFICATIONS

35 I.A.C. § 845.630 Groundwater Monitoring Systems (PE)

I, Eric J. Tlachac, a qualified professional engineer in good standing in the State of Illinois, certify that the groundwater monitoring system described in this document (Groundwater Monitoring Plan, Coffeen Power Plant GMF Gypsum Stack Pond), has been designed and constructed to meet the requirements of 35 I.A.C. § 845.630. The monitoring system was developed based on information included in the Hydrogeologic Site Characterization Report (Ramboll 2021; included in the Operating Permit to which this Groundwater Monitoring Plan is attached).

Eric J. Tlachac

Qualified Professional Engineer

062-063091 Illinois

Date: October 25, 2021



35 I.A.C. § 845.630 Groundwater Monitoring Systems (PG)

I, Brian G. Hennings, a qualified professional geologist in good standing in the State of Illinois, certify that the groundwater monitoring system described in this document (Groundwater Monitoring Plan, Coffeen Power Plant GMF Gypsum Stack Pond), has been designed and constructed to meet the requirements of 35 I.A.C. § 845.630. The monitoring system was developed based on information included in the Hydrogeologic Site Characterization Report (Ramboll 2021; included in the Operating Permit to which this Groundwater Monitoring Plan is attached).

Brian G. Hennings

Professional Geologist

196.001482 Illinois

Date: October 25, 2021

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COF GMF GSP GMP FINAL 10.21.2021 3/21

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Table E	Part 845 Groundwater Monitoring Program Parameters
Table F	Part 845 Sampling Schedule

TABLES (ATTACHED)

Table 1-1	Part 845 Requirements Checklist
Table 2-1	Monitoring Well Locations and Construction Details
Table 3-1	Background Groundwater Quality and Standards
Table 4-1	Sampling and Analysis Summary
Table 4-2	Detection and Reporting Limits for Part 845 Parameter

FIGURES (ATTACHED)

Figure 1-1	Site Location Map
Figure 1-2	Site Map
Figure 1-3	Uppermost Aquifer Groundwater Elevation Contours, April 20, 2021
Figure 2-1	Proposed Part 845 Groundwater Monitoring Well Network

APPENDICES

Appendix A Statistical Analysis Plan

ACRONYMS AND ABBREVIATIONS

35 I.A.C. Title 35 of the Illinois Administrative Code 40 C.F.R. Title 40 of the Code of Federal Regulations

ASD Alternate Source Demonstration

bgs below ground surface
CCR coal combustion residuals
cm/s centimeters per second
CPP Coffeen Power Plant

DA deep aquifer

DCU deep confining unit

GMF Gypsum Management Facility
GMP Groundwater Monitoring Plan

GSP Gypsum Stack Pond

GWPS Groundwater Protection Standard

HCR Hydrogeologic Site Characterization Report

HDPE high-density polyethylene

ID identification

IEPA Illinois Environmental Protection Agency
IPGC Illinois Power Generating Company

LCU lower confining unit mg/L milligrams per liter NA not applicable

NID National Inventory of Dams

No. number

NRT/OBG Natural Resources Technology, Inc., an OBG Company

Part 845 Standards for the Disposal of Coal Combustion Residuals in Surface Impoundments:

Title 35 of the Illinois Administrative Code § 845

PMP Potential Migration Pathway
QA/QC quality assurance/quality control

Ramboll Ramboll Americas Engineering Solutions, Inc.

RL reporting limit
RP Recycle Pond

SI surface impoundment
TDS total dissolved solids
UA uppermost aquifer
UCU upper confining unit

USEPA United States Environmental Protection Agency

WLO water level only

WPCP Water Pollution Control Permit

1. INTRODUCTION

1.1 Overview

In accordance with requirements of the Standards for the Disposal of Coal Combustion Residuals (CCR) in Surface Impoundments (SIs): Title 35 of the Illinois Administrative Code (35 I.A.C.) § 845 (Part 845) (Illinois Environmental Protection Agency [IEPA], April 15, 2021), Ramboll Americas Engineering Solutions, Inc. (Ramboll) has prepared this Groundwater Monitoring Plan (GMP) on behalf of Coffeen Power Plant (CPP), operated by Illinois Power Generating Company (IPGC). This report will apply specifically to the CCR Units referred to as the Gypsum Management Facility (GMF) Gypsum Stack Pond (GSP), Vistra identification (ID) number (No.) 103, IEPA ID No. W1350150004-03, and National Inventory of Dams (NID) No. IL50579. This GMP includes Part 845 content requirements specific to 35 I.A.C. § 845.630 (Groundwater Monitoring System), 35 I.A.C. § 845.640 (Groundwater Sampling and Analysis), and 35 I.A.C. § 845.650 (Groundwater Monitoring Program) for the GMF GSP at the CPP.

A checklist which identifies the specific requirements of 35 I.A.C. § 845.630, 35 I.A.C. § 845.640, and 35 I.A.C. § 845.650 is included in **Table 1-1**. The table provides references to sections, tables, and figures included in this document to locate the information that meets specific requirements of 35 I.A.C. § 845.630, 35 I.A.C. § 845.640, and 35 I.A.C. § 845.650.

1.2 Site Location and Background

The CPP is approximately two miles south of the city of Coffeen, Illinois and approximately eight miles southeast of the city of Hillsboro, Illinois (**Figure 1-1**). The GMF GSP is located in Montgomery County, in central Illinois, within Section 11, Township 7 North, and Range 7 East. The CPP is located between the two lobes of Coffeen Lake to the west, east, and south, and is bordered by agricultural land to the north. The CPP operated as a coal-fired power plant from 1964 to November 2019 and has five CCR management units. The approximately 1,100-acre Coffeen Lake was built by damming the McDavid Branch of the East Fork of Shoal Creek in 1963 for use as an artificial cooling lake for the CPP.

The two GMFs, consisting of the 77-acre GMF GSP and the 17-acre GMF Recycle Pond (RP) (**Figure 1-2**), receive blowdown from the air emission scrubbers and have been in operation since 2010. Construction of the GMFs were in accordance with the IEPA Water Pollution Control Permit (WPCP) No. 2008-EA-4661 and feature a composite high-density polyethylene (HDPE) liner with three feet of compacted soil with a hydraulic conductivity of 1×10^{-7} centimeters per second (cm/s). Both GMF ponds have a groundwater underdrain system. The GMF GSP system was actively pumped during construction but is currently not used. The GMF RP underdrain is a passive, gravity drained system. IPGC ceased receipt of waste to the GMF GSP prior to April 11, 2021.

1.3 Conceptual Model

Significant site investigation has been completed at the CPP to characterize the geology, hydrogeology, and groundwater quality. Based on extensive investigation and monitoring, the CPP has been well characterized and detailed in the Hydrogeologic Site Characterization Report (HCR; Ramboll, 2021), included in the Operating Permit to which this Plan is attached. A site conceptual model has been developed and is discussed below.

In addition to the CCR present at the GMF GSP, there are five principal layers of unlithified material present above the bedrock, which are categorized into hydrostratigraphic units below based on stratigraphic relationships and common hydrogeologic characteristics.

- **Upper Confining Unit (UCU):** Composed of the Roxana and Peoria Silts (Loess Unit) and the upper clayey portion of the Hagarstown member which are classified as silts to clayey silts and gravelly clay below the surficial soil. Construction of the GMF GSP required the excavation and removal of this layer within the unit footprint and the UCU has been eroded east of the GMF GSP, near the Unnamed Tributary.
- **Uppermost Aquifer:** The uppermost aquifer is the Hagarstown Member which is classified as primarily sandy to gravelly silts and clays with thin beds of sands. Similar to the Loess Unit, the Hagarstown Member was excavated to facilitate construction of the GMF GSP and the Hagarstown is also absent in some locations near the Unnamed Tributary.
- Lower Confining Unit (LCU): Comprised of the Vandalia Member, Mulberry Grove Member, and Smithboro Member. These units include a sandy to silty till with thin, discontinuous sand lenses, a discontinuous and limited extent sandy silt which has infilled prior erosional features, and silty to clayey diamicton, respectively.
- **Deep Aquifer (DA):** Sand and sandy silt/clay units of the Yarmouth Soil, which include accretionary deposits of fine sediment and organic materials, typically less than five feet thick and discontinuous across the site. Where present, the DA has been identified as a potential migration pathway (PMP) due to presence of downward gradients and the relatively greater hydraulic conductivities measured in the DA.
- **Deep Confining Unit (DCU):** Comprised of the Banner Formation, generally consists of clays, silts, and sands. The Lierle Clay Member is the upper layer of the Banner Formation which was encountered at the Site.

Flow of groundwater from central portions of the CPP to Coffeen Lake or the Unnamed Tributary through the uppermost aquifer are the primary pathways for contaminant migration. Groundwater elevations are primarily controlled by surface topography, geologic unit topography, and water levels within Coffeen Lake and the Unnamed Tributary. A groundwater divide trending north-south is observed running through the approximate center of the CPP (**Figure 1-3**). Water levels within the SIs are generally consistent and have not been observed to fluctuate with groundwater elevations indicating limited hydraulic connection with the SIs.

Part 845 parameters were monitored in the uppermost aquifer monitoring wells at the GMF GSP as part of the IEPA WPCP No. 2020-EO-65043 monitoring program and Title 40 of the Code of Federal Regulations (40 C.F.R.) § 257 monitoring program beginning in 2015. These data were supplemented by sampling of additional locations in 2021. The results indicate that the following parameters were detected at concentrations/measurements greater than (or less than for pH) the applicable 35 I.A.C. § 845.600 groundwater protection standards (GWPSs) and are considered potential exceedances:

- Arsenic in compliance uppermost aquifer well G215.
- Beryllium in compliance uppermost aquifer well G209.
- Cobalt in compliance uppermost aquifer wells G209, G213, and G217.
- Lead in compliance uppermost aquifer wells G209 and G213.

- pH (lower limit) in compliance uppermost aquifer well G206.
- Sulfate in compliance uppermost aquifer well G215 and in compliance DA well G206D.
- Thallium in compliance uppermost aquifer well G209.
- Total dissolved solids (TDS) in compliance DA well G206D.

Concentration results for the above parameters were compared directly to 35 I.A.C. § 845.600 GWPS, without an evaluation of background concentrations. Evaluation of background groundwater quality has been completed as part of this GMP, and compliance with Part 845 will be determined following the first round of groundwater sampling. The first round of groundwater sampling for compliance will be completed the quarter following issuance of the Operating Permit and in accordance with this GMP.

2. GROUNDWATER MONITORING SYSTEMS

2.1 Existing Monitoring Well Network and Analysis

This GMP is being provided to propose a groundwater monitoring network and monitoring program specific to the GMF GSP that will comply with Part 845. Monitoring networks and programs that apply to other units are not discussed in this GMP. Those programs will continue to be performed as specified in IEPA approvals. Any future modifications will be proposed and submitted to IEPA for approval in a separate document. The remaining discussion in this document will include only these networks and monitoring programs that are applicable and specific to the GMF GSP, specifically the IEPA WPCP monitoring network, the 40 C.F.R. § 257 network, and the proposed Part 845 monitoring network.

2.1.1 IEPA Groundwater Monitoring Program

Routine quarterly groundwater monitoring is completed for a monitoring well network that includes wells for both the GMF GSP and GMF RP. The monitoring well network consists of thirty-one monitoring wells screened in the uppermost aquifer (G102, G103, R104, G105, G106, G200, G205, G206, G207, G208, G209, G210, G211, G212, G213, G214, G215, G216, G217, G218, G270, G271, G272, G273, G274, G275, G276, G277, G279, G280, and R201) in accordance with IEPA WPCP No. 2020-EO-65043, issued on March 11, 2020. The boring logs and well construction forms for the GMF well network are included in Appendix C of the HCR (included in the Operating Permit to which this Plan is attached). Quarterly and annual samples are analyzed for the following field and laboratory parameters listed in **Table A** below.

Table A. IEPA Groundwater Monitoring Program Parameters

Field Parameters ¹				
рН		Elevation of Measuring Point	Specific Conductance	
Depth to Water (below point, below ground s	-	Elevation of Groundwater Surface	Temperature	
Metals (Dissolved)				
Antimony	Cadmium	Manganese	Thallium	
Arsenic	Chromium	n Mercury	Vanadium	
Aluminum	Cobalt	Molybdenum	Zinc	
Barium	Copper	Nickel		
Beryllium	Iron	Selenium		
Boron	Lead	Silver		
Inorganics (Dissolv	ved)			
Chloride	Fluoride	TDS		
Cyanide	Sulfate			
Other (Total)				
Phenols		·		

Note: Parameters are monitored as dissolved quarterly, and as dissolved and total annually.

¹Dissolved oxygen, oxidation/reduction potential, and turbidity were recorded during sample collection.

2.1.2 40 C.F.R. § 257 Monitoring Program

The 40 C.F.R. § 257 well network for the GMF GSP consists of seven monitoring wells installed nearby or adjacent to the GMF GSP within the uppermost aquifer. The GMF GSP 40 C.F.R. § 257 well network consists of two background monitoring wells (G200 and R201) and five compliance monitoring wells (G206, G209, G212, G215, and G218). The boring logs, well construction forms, and other related monitoring well forms are available in the Operating Records as required by 40 C.F.R. § 257.91 for each monitored CCR Unit or CCR Multi-Unit, and are included in Appendix C of the HCR (included in the Operating Permit to which this Plan is attached).

Groundwater is being monitored at the GMF GSP in accordance with the Detection Monitoring Program requirements specified in 40 C.F.R. § 257.94. Details on the procedures and techniques used to fulfill the groundwater sampling and analysis program requirements are found in the Sampling and Analysis Plan for GMF GSP (Natural Resource Technology, an OBG Company [NRT/OBG], 2017).

Groundwater samples are collected semiannually and analyzed for the laboratory and field parameters from Appendix III of 40 C.F.R. § 257, summarized in **Table B** below.

Table B. 40 C.F.R. § 257 Groundwater Monitoring Program Parameters

Field Parameters ¹		
Groundwater Elevati	on pH	
Appendix III Para	meters (Total, except	TDS)
Boron	Chloride	Sulfate
Calcium	Fluoride	TDS

¹Dissolved oxygen, temperature, specific conductance, oxidation/reduction potential, and turbidity are recorded during sample collection.

Results and analysis of groundwater sampling are reported annually by January 31 of the following year and made available on the CCR public website as required by 40 C.F.R. § 257.

2.1.3 Part 845 Well Installation and Monitoring

In 2021, one additional monitoring well (G206D), one source sample collection point (NE Riser), and one soil boring (G289) were installed along the perimeter of the GMF GSP to assess the vertical and horizontal lithology, stratigraphy, chemical properties, and physical properties of geologic layers to a minimum of 100 feet below ground surface (bgs) as specified in 35 I.A.C. § 845.620(b).

Prospective Part 845 monitoring wells were sampled for eight rounds from March to August 2021 and the results were assessed for selection of the GMF GSP Part 845 monitoring well network. Groundwater samples were collected and analyzed for 35 I.A.C. § 845.600 parameters as summarized in **Table C** below.

Table C. Part 845 Groundwater Monitoring Program Parameters

Field Parameters ¹					
Groundwater Elevation	рН	Turbidity			
Metals (Total)					
Antimony	Boron	Cobalt	Molybdenum		
Arsenic	Cadmium	Lead	Selenium		
Barium	Calcium	Lithium	Thallium		
Beryllium	Chromium	Mercury			
Inorganics (Total, exc	Inorganics (Total, except TDS)				
Fluoride	Sulfate	Chloride	TDS		
Other (Total)					
Radium 226 and 228 combined					

 $^{^{1}}$ Dissolved oxygen, temperature, specific conductance, and oxidation/reduction potential were recorded during sample collection.

Data and results from the Part 845 background monitoring were included in the water quality discussion included in the HCR (included in the Operating Permit to which this Plan is attached). The data collected from background locations during the Part 845 monitoring were used to evaluate and calculate background concentrations for the GMF GSP. The evaluation and discussion are included in **Section 3.2** of this report.

Data collected from the 40 C.F.R. § 257 monitoring network from 2015 to 2021, and from the Part 845 background monitoring were used for selection of the Part 845 monitoring well network proposed in **Section 2.2**.

2.2 Proposed Part 845 Monitoring Well Network

The groundwater monitoring network proposed in this plan will include nine monitoring wells screened in the uppermost aquifer (G200, G206, G209, G212, G213, G215, G217, G218, and R201), one monitoring well screened in the DA (G206D), one temporary water level only well (NE Riser), and one temporary water level only surface water staff gage (SG-04). The proposed network is summarized in **Table D** below and displayed on **Figure 2-1**. Ten wells (two background and eight compliance) will be used to monitor groundwater concentrations within the hydrostratigraphic units.

The groundwater samples collected from the ten wells will be used to monitor and evaluate groundwater quality and demonstrate compliance with the groundwater quality standards listed in 35 I.A.C. § 845.600(a). The proposed monitoring wells will yield groundwater samples that represent the quality of downgradient groundwater at the CCR boundary (as required in 35 I.A.C. § 845.630(a)(2)). Monitoring well depths and construction details are listed in **Table 2-1** and summarized in **Table D** below.

Table D. Proposed Part 845 Monitoring Well Network

Well ID	Monitored Unit	Well Screen Interval (feet bgs)	Well Type ¹
G200	UA	12.2 - 17.0	Background
G206	UA	17.5 - 21.9	Compliance
G206D	DA	49.2 - 59.0	Compliance
G209	UA	17.7 - 22.3	Compliance
G212	UA	16.7 - 21.3	Compliance
G213	UA	16.8 - 21.3	Compliance
G215	UA	19.4 - 23.8	Compliance
G217	UA	20.5 - 24.9	Compliance
G218	UA	20.3 - 24.8	Compliance
R201	UA	14.6 - 19.3	Background
NE Riser ^{2. 3}	CCR	NA	WLO
SG-04 ^{3, 4}	Surface Water	NA	WLO

¹ Well Type refers to the role of the well in the monitoring network.

NA = Not Applicable

UA = uppermost aquifer

WLO = water level only

2.3 Well Abandonment

No wells are currently proposed for abandonment.

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² Well is to be for water level data collection only.

³ Location is temporary pending implementation of impoundment closure per an approved Construction Permit Application.

⁴ Surface water level measuring point.

3. APPLICABLE GROUNDWATER QUALITY STANDARDS

3.1 Groundwater Classification

Groundwater within the uppermost aquifer at the GMF GSP meets the definition of Class I - Potable Resource Groundwater (35 I.A.C. § 620.210), based on the following criteria:

- Groundwater in the uppermost aguifer is located 10 feet or more below the land surface.
- Field hydraulic conductivity testing performed in the uppermost aquifer resulted in an overall (geometric mean) horizontal hydraulic conductivity of 1.4 x 10⁻³ cm/s, exceeding the 1 x 10⁻⁴ cm/s criterion.

3.2 Statistical Evaluation of Background Groundwater Data

A Statistical Analysis Plan (**Appendix A**) has been developed to describe procedures that will be used to establish background conditions and implement compliance monitoring as necessary and required by 35 I.A.C. § 845.640 and 35 I.A.C. § 845.650. The Statistical Analysis Plan was prepared in accordance with the requirements of 35 I.A.C. § 845.640(f), with reference to the acceptable statistical procedures provided in United States Environmental Protection Agency's (USEPA) *Statistical Analysis of Groundwater Monitoring Data at RCRA Facilities, Unified Guidance* (*Unified Guidance*, March 2009), and is intended to provide a logical process and framework for conducting the statistical analysis of the data obtained during groundwater monitoring.

In accordance with 35 I.A.C. § 845.640(f)(1), the statistical method chosen for analysis of background groundwater quality was either the tolerance interval or the prediction interval procedure for each constituent listed in 35 I.A.C. § 845.600(a)(1) at this CCR unit per 35 I.A.C. § 845.640(f)(1)(C). A comparison of the statistical background concentrations and groundwater quality standards listed in 35 I.A.C. § 845.600(a)(1) and the resulting GWPSs are summarized in **Table 3-1**.

3.3 Applicable Groundwater Protection Standards

The applicable GWPS will be established in accordance with 35 I.A.C. § 845.600(a) (greater of the background concentration or numerical limit specified in 35 I.A.C. § 845.600(a)(1)). The results of the statistical analysis of background groundwater data (**Table 3-1**) indicate that most background concentrations in the uppermost aquifer and DA are less than the groundwater quality standards listed in 35 I.A.C. § 845.600(a)(1). Therefore, for these parameters, the groundwater quality standards listed in 35 I.A.C. § 845.600(a)(1) will be applied to the results from the proposed groundwater monitoring network. The only exception being arsenic, where the background concentration is greater than the 35 I.A.C. § 845.600(a)(1) standard (0.011 milligrams per liter [mg/L] versus 0.010 mg/L). In this instance, the GWPS will be the background concentration.

Under most circumstances, the GWPS will be compared to the lower confidence limit for the observed concentrations for each constituent in each compliance well. Exceptions are when there are high percentages (greater than 50 percent) of non-detects in compliance well data, for which a future mean (for 50 to 70 percent non-detects) or median (for greater than 70 percent non-detects) will be compared to the GWPS. Consistent with the *Unified Guidance*, the same general statistical method of confidence interval testing against a fixed GWPS is recommended in compliance and corrective action programs. Confidence intervals provide a flexible and

statistically accurate method to test how a parameter estimated from a single sample compares to a fixed numerical limit. Confidence intervals explicitly account for variation and uncertainty in the sample data used to construct them.

Evaluation of the applicable standards will occur in conjunction with the analysis of groundwater quality results. Background calculations and the resulting concentrations may be updated as appropriate, in accordance with the Statistical Analysis Plan included in **Appendix A.**

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4. GROUNDWATER MONITORING PLAN

The groundwater monitoring plan will monitor and evaluate groundwater quality to demonstrate compliance with the groundwater quality standards included in 40 C.F.R. § 257.94(e), 40 C.F.R. § 257.95(h), and 35 I.A.C. § 845.600(a). The groundwater monitoring program will include sampling and analysis procedures that are consistent and that provide an accurate representation of groundwater quality at the background and compliance wells as required by 35 I.A.C. § 845.630. As discussed in **Section 2**, three monitoring programs specific to GMF GSP exist: the IEPA WPCP monitoring program, the 40 C.F.R. § 257 monitoring program, and the proposed Part 845 monitoring program. These networks will continue to be monitored until USEPA approves Part 845. Upon approval of the Operating Permits (and by extension the GMPs) for the GMF GSP and RP, the IEPA WPCP monitoring will be replaced by the Part 845 monitoring program. It is expected that upon USEPA approval of Part 845, the 40 C.F.R. § 257 monitoring program and reporting will be eliminated, and the proposed Part 845 monitoring and reporting included in this Plan will continue until requirements of Part 845 have been achieved.

4.1 Monitoring Networks and Parameters

4.1.1 IEPA Monitoring Program

The existing IEPA monitoring program was discussed in detail in **Section 2.1.1**. Thirty-one monitoring wells are sampled quarterly for dissolved analyses, and annually for totals. No changes are proposed to this monitoring network. Well locations and parameters will continue to be monitored and reported as required by the WPCP until IEPA approves this GMP and the GMP prepared for the GMF RP.

4.1.2 40 C.F.R. § 257 Groundwater Monitoring

The existing 40 C.F.R. § 257 monitoring program was discussed in detail in **Section 2.1.2**. Seven wells (two background and five compliance) are sampled for Appendix III parameters on a semi-annual frequency. No changes are proposed to this monitoring network. Well locations and parameters will continue to be monitored and reported as required by 40 C.F.R. § 257 until USEPA approves Part 845.

4.1.3 Part 845 Groundwater Monitoring

The proposed Part 845 Monitoring Network will consist of two background monitoring wells (G200 and R201), eight compliance monitoring wells (G206, G206D, G209, G212, G213, G215, G217, and G218), one temporary water level only well (NE Riser) and one temporary water level only surface staff gage (SG-04) to monitor potential impacts from the GMF GSP (**Figure 2-1**). These monitoring wells are screened within the uppermost aquifer and DA along the perimeter of the GMF GSP. Groundwater samples will be collected and analyzed for the laboratory and field parameters in **Table E** below.

Table E. Part 845 Groundwater Monitoring Program Parameters

Field Parameters ¹				
Groundwater Elevation	рН	Turbidity		
Metals (Total)				
Antimony	Boron	Cobalt	Molybdenum	
Arsenic	Cadmium	Lead	Selenium	
Barium	Calcium	Lithium	Thallium	
Beryllium	Chromium	Mercury		
Inorganics (Total, except TDS)				
Fluoride	Sulfate	Chloride	TDS	
Other (Total)				
Radium 226 and 228 combined				

¹ Dissolved oxygen, temperature, specific conductance, and oxidation/reduction potential will be recorded during sample collection.

All parameters listed above were sampled a minimum of eight times by October 18, 2021 to establish background groundwater quality in accordance with 35 I.A.C. § 845.650 (b)(1)(A). Discussion of background groundwater quality is included in **Section 3.2**.

4.2 Sampling Schedule

Groundwater sampling for the Part 845 monitoring well network will initially be performed quarterly according to the following schedule:

Table F. Part 845 Sampling Schedule

Frequency	Duration
Monthly	Begins: the quarter following approval of this plan and issuance of the Operating Permit.
(groundwater elevations only)	Ends: Following the 30-year post closure care period and following IEPA approval of documentation that groundwater concentrations are below standards in 35 I.A.C. § 845.600 and concentrations exceeding background are not increasing and meet requirements in 35 I.A.C. § 845.780 (c)(2)(B)(i) and (ii).
Quarterly	Begins: the quarter following approval of this plan and issuance of the Operating Permit.
(groundwater quality)	Ends: Following the 30-year post closure care period and following IEPA approval of documentation that groundwater concentrations are below standards in 35 I.A.C. § 845.600 and concentrations exceeding background are not increasing and meet requirements in 35 I.A.C. § 845.780 (c)(2)(B)(i) and (ii), or upon IEPA approval of an alternate schedule as allowed by 35 I.A.C. § 845.650(b)(4).
Semi-annual (groundwater quality)	Begins: Following 5 years of quarterly groundwater monitoring and IEPA approval of a demonstration that groundwater concentrations are below standards in 35 I.A.C. § 845.600 and not exhibiting statistically-significant increasing trends, monitoring effectiveness is not compromised by a semi-annual schedule, and sufficient data has been collected to characterize groundwater.
	Ends: Following detection of a statistically-significant increasing trend in groundwater concentrations or an exceedance of the standards in 35 I.A.C. § 845.600 (quarterly monitoring shall be resumed in these circumstances), or following the 30-year post closure care period and following IEPA approval of documentation that groundwater concentrations

are below standards in 35 I.A.C. § 845.600 and concentrations exceeding background are not increasing and meet requirements in 35 I.A.C. § 845.780 (c)(2)(B)(i) and (ii).

4.3 Groundwater Sample Collection

Groundwater sampling procedures have been developed and the collection of groundwater samples is being implemented to meet the requirements of 35 I.A.C. § 845.640. In addition to groundwater well samples, quality assurance samples will be collected as described in **Section 4.5** (**Table 4-1**).

4.4 Laboratory Analysis

Laboratory analysis will be performed consistent with the requirements of 35 I.A.C. § 845.640(j) by a state-certified laboratory using methods approved by IEPA and USEPA. Laboratory methods may be modified based on laboratory equipment availability or procedures, but the Reporting Limit (RL) for all parameters analyzed, regardless of method, will be lower than the applicable groundwater quality standard. RLs for the applicable parameters are summarized in **Table 4-2**. Concentrations lower than the RL will be reported as less than the RL.

4.5 Quality Assurance Program

Consistent with the requirements of 35 I.A.C. § 845.640(a)(5), the sampling and analysis program includes procedures and techniques for quality assurance/quality control (QA/QC). Additional quality assurance samples to be collected will include the following:

- Field duplicates will be collected at a frequency of one per group of ten or fewer investigative water samples.
- One equipment blank sample will be collected and analyzed for each day of sampling. If dedicated sampling equipment is used, then equipment blank samples will not be collected.
- The duplicate and equipment blank quality assurance samples will be supplemented by the laboratory QA/QC program, which typically includes:
 - Regular generation of instrument calibration curves to assure instrument reliability
 - Laboratory control samples and/or quality control check standards that have been spiked,
 and analyses to monitor the performance of the analytical method
 - Matrix spike/matrix spike duplicate analyses to determine percent recoveries and relative percent differences for each of the parameters detected
 - Analysis of replicate samples to check the precision of the instrumentation and/or methodology employed for all analytical methods
 - Analysis of method blanks to assure that the system is free of contamination

Water quality meters used to measure pH and turbidity will be calibrated according to manufacturer's specifications. At a minimum, it is recommended that calibration of pH occur daily prior to sampling and checked for accuracy at the end of each day. Unusual or suspect pH measurements during sampling events will be flagged, evaluated, and additional calibration may be performed throughout the sampling events. Turbidity meters will be checked daily, prior to and following sampling. Unusual measurements or erratic meter performance will be flagged and evaluated for overall effects on the data prior to reporting.

4.6 Groundwater Monitoring System Maintenance Plan

Consistent with the requirements of 35 I.A.C. § 845.630(e)(2), maintenance will be performed as needed to assure that the monitoring wells provide representative groundwater samples. Monitoring wells will be inspected during each groundwater sampling event; inspections will consist of the following:

- Visual inspection, clearing of vegetation, replacement of markers, and painting of protective casings as needed to assure that monitoring wells are clearly marked and accessible
- Visual inspection and repair or replacement of well aprons as needed to assure that they are intact, drain water away from the well, and have not heaved
- Visual inspection and repair or replacement of protective casings as needed to assure that they are undamaged, and that locks are present and functional
- Checks to assure that well caps are intact and vented, unless in flood-prone areas in which case caps will not be vented
- Annual measurement of monitoring well depths to determine the degree of siltation within the wells. Wells will be redeveloped as needed to remove siltation from the screened interval if it impedes flow of water into the well
- Checks to assure that wells are clear of internal obstructions, and flow freely

If maintenance of a monitoring well cannot address an identified deficiency, a replacement well will be installed.

4.7 Statistical Analysis

Statistical analysis will be consistent with procedures listed in 35 I.A.C. § 845.640(f). A Statistical Analysis Plan, provided in **Appendix A**, has been developed to summarize the statistical procedures that will be used to evaluate the groundwater results.

4.8 Data Reporting

Data reporting for the 40 C.F.R. § 257 monitoring program will be consistent with recordkeeping, notification, and internet posting requirements described in 40 C.F.R. § 257.105 through 257.107.

Groundwater monitoring and analysis completed in accordance with the Part 845 monitoring under an approved monitoring program will be reported to IEPA within 60 days after completion of sampling and the data placed in the facility's operating record as required by 35 I.A.C. § 845.610(b)(3)(D). Within 14 days of posting to the operating record, information will be posted to the publicly accessible internet site "Illinois CCR Rule Compliance Data and Information" as required by 35 I.A.C. § 845.810(d). Information will also be submitted to IEPA annually by January 31 as required by 35 I.A.C. § 845.550, for data collected the preceding year. The annual report will include the status of the groundwater monitoring and any required corrective action plan for the GMP GSP in addition to other requirements detailed in 35 I.A.C. § 845.610(e).

4.9 Compliance with Applicable On-site Groundwater Protection Standards

In accordance with 35 I.A.C. § 845.600(a)(1), the groundwater protection standard at the waste boundary will be the higher of either the 35 I.A.C. § 845.600 standard or the concentration determined by background groundwater monitoring.

As provided in 35 I.A.C. § 845.780(c)(2), at the end of the 30-year post-closure care period, groundwater monitoring will continue to be conducted in post-closure care until the groundwater results show the concentrations are:

- Below the GWPS in 35 I.A.C. § 845.600; and
- Not increasing for those constituents over background, using the statistical procedures and performance standards in 35 I.A.C. § 845.640(f) and (q), provided that:
 - Concentrations have been reduced to the maximum extent feasible; and
 - Concentrations are protective of human health and the environment.

Following detection of an exceedance of the GWPS, an Alternate Source Demonstration (ASD) will be evaluated as described in **Section 4.10**.

4.10 Alternate Source Demonstrations

As allowed in 35 I.A.C. § 845.650(e), following detection of an exceedance of the GWPS, an ASD will be evaluated and, if completed, submitted to IEPA within 60 days. The ASD will provide lines of evidence that a source other than the GMF GSP caused the contamination and the GMF GSP did not contribute to the contamination, or that the exceedance of the GWPS resulted from error in sampling, analysis, statistical evaluation, natural variation in groundwater quality, or a change in the potentiometric surface and groundwater flow direction.

The ASD will include information and analysis that supports the conclusions and a certification of accuracy by a qualified professional engineer. Once the ASD is approved by IEPA, the Part 845 groundwater monitoring will continue as defined in **Section 4.1.3**.

If an ASD is not completed and submitted, or IEPA does not approve the ASD, a notification of the exceedance will be provided to IEPA and placed in the operating record. Additional actions will also be completed as required by 35 I.A.C § 845.650(d)(1) through (3), including initiation of an assessment of corrective measures under 35 I.A.C § 845.660. As allowed in 35 I.A.C § 845.650(e)(7) a petition for review of IEPA's non-concurrence under 35 I.A.C. § 105 may also be filed.

4.11 Assessment of Corrective Measures and Corrective Action

As described in 35 I.A.C. § 845.660, if the ASD summarized in **Section 4.10** has not been approved by IEPA, an assessment of corrective measures will be initiated within 90 days of the detection of a result exceeding 35 I.A.C. § 845.600 standards (*i.e.*, receipt of laboratory data). The assessment of corrective measures will include at least the following (35 I.A.C. § 845.660 (c)):

- The performance, reliability, ease of implementation, and potential impacts of appropriate
 potential remedies, including safety impacts, cross-media impacts, and control of exposure to
 any residual contamination;
- The time required to begin and complete the corrective action plan; and
- The institutional requirements, such as State or local permit requirements or other environmental or public health requirements that may substantially affect implementation of the corrective action plan.

Within one year of completing the assessment of corrective measures, a corrective action plan will be developed to identify the selected remedy in accordance with 35 I.A.C. § 845.670. If closure of the CCR Unit is required, a closure alternatives analysis will be completed as specified in 35 I.A.C. § 845.710. The analysis and selected alternative will be submitted to IEPA in a Closure Plan as specified by 35 I.A.C. § 845.720. Groundwater monitoring proposed in this Addendum will continue as specified until the post closure care period has expired and IEPA has approved termination of post-closure care.

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5. REFERENCES

Illinois Environmental Protection Agency, 2021. Standards for the Disposal of Coal Combustion Residuals in Surface Impoundments: Title 35 of the Illinois Administrative Code § 845, April 15, 2021.

Natural Resource Technology, an OBG Company (NRT/OBG), 2017. Sampling and Analysis Plan, Coffeen GMF Gypsum Stack Pond, Coffeen Power Station, Coffeen, Illinois, Project No. 2285, Revision 0, October 17, 2017.

Ramboll Americas Engineering Solutions, Inc. (Ramboll), 2021. *Hydrogeologic Site*Characterization Report. Coffeen GMF Gypsum Stack Pond, Coffeen Power Plant. Coffeen, Illinois.

United States Environmental Protection Agency (USEPA), March 2009. *Statistical Analysis of Groundwater Monitoring Data at RCRA Facilities – Unified Guidance*. Office of Resource Conservation and Recovery, Program Implementation and Information Division, United States Environmental Protection Agency, Washington D.C. EPA/530/R-09/007.

TABLES

TABLE 1-1. PART 845 REQUIREMENTS CHECKLIST

GROUNDWATER MONITORING PLAN
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Part 845 Reference	Part 845 Components	Location of Information in GMP			
845.630	Groundwater Monitoring Systems				
845.630(a)(2)	Potential contaminant pathways must be monitored.	Sections 2.2 & 4.1.3			
845.630(a) 845.630(b) 845.630(c)	At least two upgradient wells and four downgradient wells (min. 1 and 3, but requires additional documentation)	Sections 2.2 & 4.1.3 Table 2-1 Figure 2-1			
845.630(a) 845.630(b) 845.630(c)	Downgradient Well Density	Figure 2-1			
845.630(a)(2)	Downgradient wells at waste boundary	Figure 2-1			
845.640	Groundwater Sampling and Analysis Requirements				
845.640(a)	Consistent sampling and analysis procedures	Section 4 Tables 4-1 & 4-2			
845.640(b)	Methods are appropriate	Section 4 Tables 4-1 & 4-2			
845.640(c)	Groundwater elevations must be measured in each well prior to purging, each time groundwater is sampled.	Section 4.3			
845.640 (d)(e)(f)(g)(h)	Establishment of background and application of statistical methods	Sections 3.2 & 4.7 Appendix A			
845.640(i)	Analyze total recoverable metals	Section 4.1.3			
845.640(j)	Analyze groundwater samples using a certified laboratory	Section 4.4			

TABLE 1-1. PART 845 REQUIREMENTS CHECKLIST

GROUNDWATER MONITORING PLAN COFFEEN POWER PLANT GMF GYPSUM STACK POND COFFEEN, ILLINOIS

Part 845 Reference	Part 845 Components	Location of Information in GMP		
845.650	Groundwater Monitoring Program			
845.650(a)	Must include monitoring for all constituents with a groundwater protection standard in Section 845.600(a), calcium, and turbidity	Section 4.1.3		
845.650(b)(c)	Groundwater Monitoring Frequency	Sections 4.1.3 & 4.2		
845.650(d)(e)	Exceedances of the groundwater protection standard	Sections 4.9, 4.10 & 4.11		
845.650(b)(2) and (3)	Staff gauge/ piezometer to monitor head in impoundment	Sections 2.2 & 4.1.3 Figure 2-1 (NE Riser)		
NA	Staff gauge/ piezometer to monitor head of neighboring surface water body	Sections 2.2 & 4.1.3 Figure 2-1 (SG-04)		

[O: CJC 09/02/21; C: SSW 09/16/21]

Notes:

GMP = Groundwater Monitoring Plan NA = Not Applicable



TABLE 2-1. MONITORING WELL LOCATIONS AND CONSTRUCTION DETAILS

GROUNDWATER MONITORING PLAN COFFEEN POWER PLANT GMF GYPSUM STACK POND COFFEEN, ILLINOIS

Well Number	Туре	HSU	Date Constructed	Top of PVC Elevation (ft)	Measuring Point Elevation (ft)	Measuring Point Description	Ground Elevation (ft)	Screen Top Depth (ft BGS)	Screen Bottom Depth (ft BGS)	Screen Top Elevation (ft)	Screen Bottom Elevation (ft)	Well Depth (ft BGS)	Bottom of Boring Elevation (ft)	Screen Length (ft)	Screen Diameter (inches)	Latitude (Decimal Degrees)	Longitude (Decimal Degrees)
G200	В	UA	02/25/2008		625.94	Top of Disk	623.27	12.19	16.98	611.08	606.29	17.36	605.30	4.8	2	39.075139	-89.395009
G206	С	UA	10/14/2010		632.82	Top of Disk	630.53	17.51	21.92	613.02	608.61	22.42	606.50	4.4	2	39.067399	-89.398548
G206D	С	DA	01/25/2021	634.14	634.14	Top of PVC	631.41	49.20	59.00	582.21	572.41	59.39	571.41	9.8	2	39.067428	-89.398493
G209	С	UA	10/07/2010		632.91	Top of Disk	630.57	17.74	22.28	612.83	608.29	22.81	606.60	4.5	2	39.067923	-89.39685
G212	С	UA	10/11/2010		632.89	Top of Disk	630.59	16.74	21.29	613.85	609.30	21.81	606.60	4.6	2	39.06843	-89.395318
G213	С	UA	10/12/2010		632.81	Top of Disk	630.34	16.75	21.29	613.59	609.05	21.82	606.30	4.5	2	39.068585	-89.394822
G215	С	UA	10/13/2010		633.06	Top of Disk	630.48	19.41	23.80	611.07	606.68	24.31	606.20	4.4	2	39.069309	-89.39394
G217	С	UA	10/12/2010		633.10	Top of Disk	630.67	20.49	24.88	610.18	605.79	25.38	604.70	4.4	2	39.07034	-89.393959
G218	С	UA	10/12/2010		633.11	Top of Disk	630.64	20.33	24.77	610.31	605.87	25.27	604.60	4.4	2	39.070876	-89.393956
R201	В	UA	10/08/2010		626.34	Top of Disk	624.02	14.59	19.32	609.43	604.70	19.85	604.20	4.7	2	39.075142	-89.397855
NE Riser	WLO	S		-1-	626.13											39.071111	-89.393889
SG-04	WLO	SW			599.52	Top of Prot Casing	599.52									39.064146	-89.390504

Notes:

All elevation data are presented relative to the North American Vertical Datum 1988 (NAVD88), GEOID 12A

Type refers to the role of the well in the monitoring network: background (B), compliance (C), or water level measurements only (WLO)

WLO wells are temporary pending implementation of impoundment closure per an approved Construction Permit application

-- = data not available

-- = data not available
BGS = below ground surface
DA = deep aquifer
ft = foot or feet
HSU = Hydrostratigraphic Unit
PVC = polyvinyl chloride
S = source water

SW = surface water UA = uppermost aquifer

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TABLE 3-1. BACKGROUND GROUNDWATER QUALITY AND STANDARDS

GROUNDWATER MONITORING PLAN COFFEEN POWER PLANT GMF GYPSUM STACK POND COFFEEN, ILLINOIS

Parameter	Background Concentration	845 Limit	Groundwater Protection Standard	Unit
Antimony, total	0.003	0.006	0.006	mg/L
Arsenic, total	0.011	0.010	0.011	mg/L
Barium, total	0.13	2.0	2.0	mg/L
Beryllium, total	0.001	0.004	0.004	mg/L
Boron, total	0.11	2	2	mg/L
Cadmium, total	0.001	0.005	0.005	mg/L
Chloride, total	94.9	200	200	mg/L
Chromium, total	0.0096	0.1	0.1	mg/L
Cobalt, total	0.0037	0.006	0.006	mg/L
Fluoride, total	0.552	4.0	4.0	mg/L
Lead, total	0.0059	0.0075	0.0075	mg/L
Lithium, total	0.02	0.04	0.04	mg/L
Mercury, total	0.0011	0.002	0.002	mg/L
Molybdenum, total	0.044	0.1	0.1	mg/L
pH (field)	7.4 / 6.8	9.0 / 6.5	9.0 / 6.5	SU
Radium 226 and 228 combined	1.48	5	5	pCi/L
Selenium, total	0.0035	0.05	0.05	mg/L
Sulfate, total	387	400	400	mg/L
Thallium, total	0.001	0.002	0.002	mg/L
Total Dissolved Solids	975	1200	1200	mg/L

Notes:

For pH, the values presented are the upper / lower limits

Groundwater protection standards for calcium and turbidity do not apply per 35 I.A.C. § 845.600(b)

mg/L = milligrams per liter

SU = standard units

pCi/L = picocuries per liter

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TABLE 4-1. SAMPLING AND ANALYSIS SUMMARY

GROUNDWATER MONITORING PLAN
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Parameter	Analytical Method ¹	Number of Samples	Field Duplicates ²	Field Blanks ³	Equipment Blanks ³	MS/MSD 4	Total	Container Type	Minimum Volume ⁵	Preservation (Cool to 4 °C for all samples)	Sample Hold Time from Collection Date
Metals											
Metals ⁶	6020, Li - EPA 200.7	10	1	0	0	1	12	plastic	600 mL	HNO ₃ to pH<2	6 months
Mercury	7470A or 6020	10	1	0	0	1	12	plastic	400 mL	HNO ₃ to pH<2	28 days
Inorganic Parameters		-			_						
Fluoride	9214 or EPA 300	10	1	0	0	1	12	plastic	300 mL	Cool to 4 °C	28 days
Chloride	9251 or EPA 300	10	1	0	0	1	12	plastic	100 mL	Cool to 4 °C	28 days
Sulfate	9036 or EPA 300	10	1	0	0	1	12	plastic	50 mL	Cool to 4 °C	28 days
Total Dissolved Solids	SM 2540 C	10	1	0	0	1	12	plastic	200 mL	Cool to 4 °C	7 days
Radium		_									
Radium 226	9315 or EPA 903	10	0	0	0	0	10	plastic	1000 mL	HNO ₃ to pH<2	6 months
Radium 228	9320 or EPA 904	10	0	0	0	0	10	plastic	1000 mL	HNO ₃ to pH<2	6 months
Field Parameters		-			_						
pH	SM 4500-H+ B	10	NA	NA	NA	NA	10	flow-through cell	NA	none	immediately
Dissolved Oxygen ⁸	SM 4500-O/405.1	10	NA	NA	NA	NA	10	flow-through cell	NA	none	immediately
Temperature ⁸	SM 2550	10	NA	NA	NA	NA	10	flow-through cell	NA	none	immediately
Oxidation/Reduction Potential ⁸	SM 2580 B	10	NA	NA	NA	NA	10	flow-through cell	NA	none	immediately
Specific Conductance ⁸	SM 2510 B	10	NA	NA	NA	NA	10	flow-through cell	NA	none	immediately
Turbidity ⁷	SM 2130 B	10	NA	NA	NA	NA	10	flow-through cell or hand-held turbidity meter	NA	none	immediately

[O: CJC 09/02/21; C: SSW 09/16/21]

Notes:

- ¹ Analytical method numbers are from SW-846 unless otherwise indicated. Analytical methods may be updated with more recent versions as appropriate.
- ² Field duplicates will be collected at a frequency of one per group of 10 or fewer investigative water samples. Field duplicates will not be collected for radium analysis.
- ³ Field blanks will be collected at the discretion of the project manager; Equipment blanks will be collected at a rate of 1 per sampling event if non-dedicated equipment is used.
- ⁴ Matrix Spike/Matrix Spike Duplicate (MS/MSD) samples will be collected at a frequency of one per group of 20 or fewer investigative water samples per CCR unit/multi-unit. Additional volume to be determined by laboratory.
- ⁵ Sample volume is estimated and will be determined by the laboratory.
- Metals = antimony, arsenic, barium, beryllium, boron, cadmium, calcium, chromium, cobalt, lead, lithium, molybdenum, selenium, thallium. Metals may be analyzed via ICP/ ICP-MS USEPA methods 6010 or 6020 depending on laboratory instrument availability
- ⁷ If turbidity exceeds 10 NTUs, a duplicate sample filtered through a .45 micron filter may be collected for metals analysis in addition to the unfiltered sample. Both samples would be submitted for analysis.
- ⁸ Parameter collected for quality assurance and quality control for field sampling purposes only; not required to be collected or reported under Part 845; collection of parameter may be discontinued without notification.
- < = less than

°C = degrees Celsius

 $HNO_3 = nitric acid$ mL = milliliter

NA = not applicable

NTU = nephelometric turbidity unit



TABLE 4-2. DETECTION AND REPORTING LIMITS FOR PART 845 PARAMETERS

GROUNDWATER MONITORING PLAN
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Constituent	CAS	Unit	Analytical Methods ¹	USEPA MCL ²	35 I.A.C. § 845.600	RL ^{4, 5}	MDL ⁵
Metals			<u> </u>				
Antimony	7440-36-0	mg/L	6020	0.006	0.006	0.003	0.00036
Arsenic	7440-38-2	mg/L	6020	0.01	0.01	0.001	0.00013
Barium	7440-39-3	mg/L	6020	2	2	0.001	0.00028
Beryllium	7440-41-7	mg/L	6020	0.004	0.004	0.001	0.000017
Boron	7440-42-8	mg/L	6020	NS	2	0.01	0.0023
Cadmium	7440-43-9	mg/L	6020	0.005	0.005	0.001	0.000042
Calcium	7440-70-2	mg/L	6020	NS	NS	0.15	0.15
Chromium	7440-47-3	mg/L	6020	0.1	0.1	0.004	0.00027
Cobalt	7440-48-4	mg/L	6020	0.006	0.006	0.002	0.000017
Lead	7439-92-1	mg/L	6020	0.015	0.0075	0.001	0.000025
Lithium	7439-93-2	mg/L	6020 or EPA 200.7	0.04	0.04	0.02	0.0001
Mercury	7439-97-6	mg/L	6020 or 7470A	0.002	0.002	0.0002	0.000078
Molybdenum	7439-98-7	mg/L	6020	0.1	0.1	0.001	0.000063
Selenium	7782-49-2	mg/L	6020	0.05	0.05	0.001	0.00032
Thallium	7440-28-0	mg/L	6020	0.002	0.002	0.001	0.000062
Inorganics							
Fluoride	7681	mg/L	9214 or EPA 300	4	4	0.25	0.065
Chloride	16887-00-6	mg/L	9251 or EPA 300	250 ³	200	1	0.15
Sulfate	18785-72-3	mg/L	9036 or EPA 300	250 ³	400	1	0.24
Total Dissolved Solids	10052	mg/L	SM 2540C	500 ³	1200	17	
Other							
Radium 226 and 228 combined	7440-14-4	pCi/L	9315/9320 or EPA 903/904	5	5	⁶	7

TABLE 4-2. DETECTION AND REPORTING LIMITS FOR PART 845 PARAMETERS

GROUNDWATER MONITORING PLAN COFFEEN POWER PLANT GMF GYPSUM STACK POND COFFEEN, ILLINOIS

Constituent	CAS	Unit	Analytical Methods ¹	USEPA MCL ²	35 I.A.C. § 845.600	RL ^{4, 5}	MDL ⁵
Field							
pH	NA	SU	SM 4500-H+ B	NS	6.5-9.0	NA	NA
Oxidation/Reduction Potential	NA	mV	SM 2580 B	NS	NS	NA	NA
Dissolved Oxygen	NA	mg/L	SM 4500-O/405.1	NS	NS	NA	NA
Temperature	NA	°C	SM 2550	NS	NS	NA	NA
Specific Conductivity	NA	μS/cm	SM 2510 B	NS	NS	NA	NA
Turbidity	NA	NTU	SM 2130 B	NS	NS	NA	NA

[O: CJC 09/02/21; C: SSW 09/16/21]

Notes:

 μ S/cm = microSiemens per centimeter

CAS = Chemical Abstract Number

MDL = Method detection limit as established by the laboratory

mg/L = milligrams per liter

mV = millivolts

NS = No standard

NTU = nephelometric turbidity unit

pCi/L = picoCuries per liter

RL = Reporting limit as established by the laboratory

SM = Standard Methods for the Examination of Water and Wastewater

SU = standard units



¹ Analytical method numbers are from SW-846 unless otherwise indicated. Metals will be analyzed via Method 6020 or 6010 depending on laboratory equipment availability. Selected method will ensure reporting limits (RL) are below Title 35 of the Illinois Administrative Code (35 I.A.C.) § 845.600 groundwater protection standards.

² USEPA MCL = United States Environmental Protection Agency Maximum Contaminant Level.

³ USEPA SMCL = United States Environmental Protection Agency Secondary Maximum Contaminant Level.

 $^{^4}$ RLs will be less than the 35 I.A.C. § 845.600 groundwater protection standards.

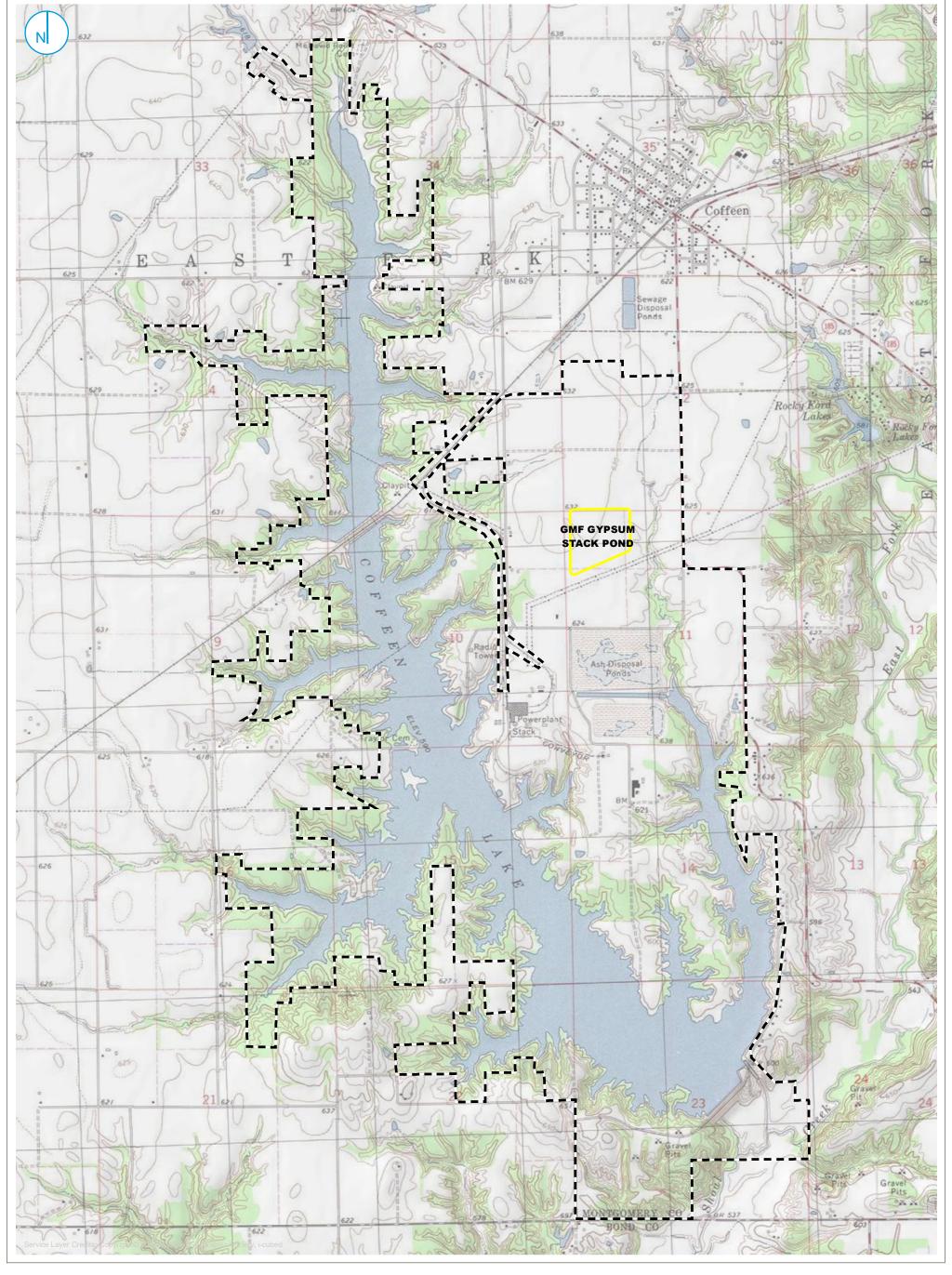
⁵ RLs and method detection limits (MDL) will vary depending on the laboratory performing the work.

⁶ All radium results will be reported (values may be positive or negative) and will include uncertainty and the calculated MDC.

⁷ Laboratories calculate a minimum detectable concentration (MDC) based on the sample.

[°]C = degrees Celsius

FIGURES



PART 845 REGULATED UNIT (SUBJECT UNIT)
PROPERTY BOUNDARY

SITE LOCATION MAP

FIGURE 1-1

RAMBOLL AMERICAS ENGINEERING SOLUTIONS, INC.





■ COAL MINE SHAFT

PART 845 REGULATED UNIT (SUBJECT UNIT)

SITE FEATURE

LIMITS OF FINAL COVER

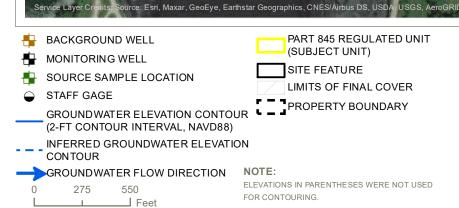
PROPERTY BOUNDARY

SITE MAP

FIGURE 1-2

GROUNDWATER MONITORING PLAN GMF GYPSUM STACK POND COFFEEN POWER PLANT COFFEEN, ILLINOIS RAMBOLL AMERICAS ENGINEERING SOLUTIONS, INC.





UPPERMOST AQUIFER GROUNDWATER ELEVATION CONTOURS APRIL 20, 2021

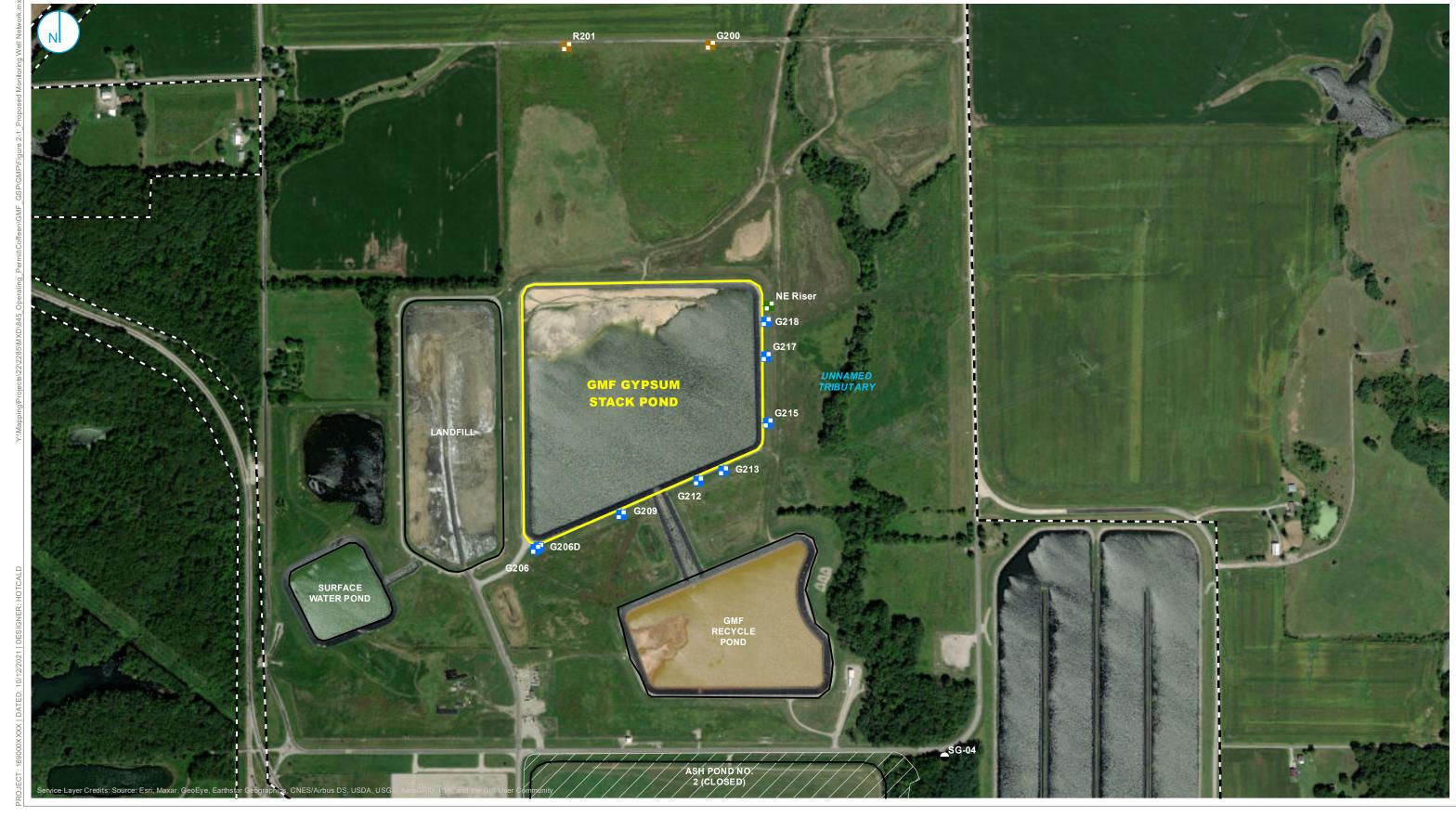
> GROUNDWATER MONITORING PLAN GMF GYPSUM STACK POND COFFEEN POWER PLANT COFFEEN, ILLINOIS

FIGURE 1-3

RAMBOLL AMERICAS ENGINEERING SOLUTIONS, INC.

G317 610.94





BACKGROUND WELL
PART 845 REGULATED UNIT (SUBJECT UNIT)
COMPLIANCE WELL
SITE FEATURE
SOURCE SAMPLE LOCATION
STAFF GAGE
PART 845 REGULATED UNIT (SUBJECT UNIT)
LIMITS OF FINAL COVER
PROPERTY BOUNDARY

PROPOSED PART 845 GROUNDWATER MONITORING WELL NETWORK

FIGURE 2-1

RAMBOLL AMERICAS ENGINEERING SOLUTIONS, INC.

RAMBOLL

APPENDIX A STATISTICAL ANALYSIS PLAN

Prepared for

Illinois Power Generating Company

Date

October 25, 2021

Project No.

1940100806-002

STATISTICAL ANALYSIS PLAN

GMF GYPSUM STACK POND COFFEEN POWER PLANT COFFEEN, ILLINOIS

STATISTICAL ANALYSIS PLAN COFFEEN POWER PLANT GMF GYPSUM STACK POND

Project Name Coffeen Power Plant GMF Gypsum Stack Pond

Project No. **1940100806-002**

Recipient Illinois Power Generating Company

Document Type Statistical Analysis Plan

Version FINAL

Date October 25, 2021

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Rachel A. Banoff, EIT Project Statistician

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LICENSED PROFESSIONAL CERTIFICATIONS

This certification is based on the description of the statistical methods selected to evaluate groundwater as presented in the following Statistical Analysis Plan; Coffeen Power Plant GMF Gypsum Stack Pond. The procedures described in the plan will be used to establish background conditions and implement compliance monitoring as necessary and required by 35 I.A.C. § 845.640 and 35 I.A.C. § 845.650. The Statistical Analysis Plan was prepared in accordance with the requirements of 35 I.A.C. § 845.640(f), with reference to the acceptable statistical procedures provided in the United States Environmental Protection Agency (USEPA)'s Statistical Analysis of Groundwater Monitoring Data at RCRA Facilities, Unified Guidance (Unified Guidance, March 2009), and is intended to provide a logical process and framework for conducting the statistical analysis of the data obtained during groundwater monitoring. In accordance with 35 I.A.C. § 845.640(f)(1), the statistical method chosen for analysis of background groundwater quality will be either the tolerance interval or the prediction interval procedure for each constituent listed in 35 I.A.C. § 845.600(a)(1) at this CCR unit per 35 I.A.C. § 845.640(f)(1)(C). Groundwater Protection Standards (GWPS) will be established in accordance with 35 I.A.C. § 845.600(a) (greater of the background concentration or numerical limit specified in 35 I.A.C. § 845.600(a)(1)). The GWPS will be compared to the lower confidence limit for the observed concentrations for each constituent in each compliance well. Consistent with the Unified Guidance, the same general statistical method of confidence interval testing against a fixed GWPS is recommended in compliance and corrective action programs. Confidence intervals provide a flexible and statistically accurate method to test how a parameter estimated from a single sample compares to a fixed numerical limit. Confidence intervals explicitly account for variation and uncertainty in the sample data used to construct them.

Description of the statistical methods chosen for analysis of groundwater monitoring data and application of these methods for determining exceedances of the GWPS identified in 35 I.A.C. § 845.600(a) is provided in this Statistical Analysis Plan.

35 I.A.C. § 845.640 Statistical Analysis (PE)

I, Eric J. Tlachac, a qualified professional engineer in good standing in the State of Illinois, certify that the statistical methods summarized above and described in this document (Statistical Analysis Plan; Coffeen Power Plant GMF Gypsum Stack Pond) are appropriate for evaluating the groundwater monitoring data collected as described in the attached document and are in substantial compliance with 35 I.A.C. § 845.640.

Eric J. Tlachac

Qualified Professional Engineer

062-063091

Illinois

Date: October 25, 2021



35 I.A.C. § 845.640 Statistical Analysis (PG)

I, Brian G. Hennings, a qualified professional geologist in good standing in the State of Illinois, certify that the statistical methods described in this document (Statistical Analysis Plan; Coffeen Power Plant GMF Gypsum Stack Pond) are appropriate for evaluating the groundwater monitoring data collected as described in the attached document and are in substantial compliance with 35 I.A.C. § 845.640.

Brian G. Hennings $_{\mathcal{L}}$

Professional Geologist

196.001482 Illinois

Date: October 25, 2021



35 I.A.C. § 845.640 Statistical Analysis

I, Rachel A. Banoff, a qualified professional, certify that the statistical methods described in this document (Statistical Analysis Plan; Coffeen Power Plant GMF Gypsum Stack Pond), are appropriate for evaluating the groundwater monitoring data collected as described in the attached document and are in substantial compliance with 35 I.A.C. § 845.640.

Rachel A. Banoff, EIT

Project Statistician
Date: October 25, 2021

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ACRONYMS AND ABBREVIATIONS

§ Section

35 I.A.C. Title 35 of the Illinois Administrative Code

ANOVA analysis of variance

CCR coal combustion residuals
COC constituents of concern

GWPS groundwater protection standard

IEPA Illinois Environmental Protection Agency

LCL lower confidence limit
LTL lower tolerance limit
MSE mean squared error

P probability

Part 845 Residuals in Surface Impoundments: Title 35 of the Illinois Administrative Code

§ 845

RCRA Resource Conservation and Recovery Act

RL reporting limit

ROS regression on order statistics

SI surface impoundment

SSI statistically significant increase
SWFPR site-wide false positive rate

Unified Guidance Statistical Analysis of Groundwater Monitoring Data at RCRA Facilities,

Unified Guidance (USEPA, 2009)

UPL upper prediction limit

USEPA United States Environmental Protection Agency

UTL upper tolerance limit

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1. INTRODUCTION

In April 2021, the Illinois Environmental Protection Agency (IEPA) issued a final rule for the regulation and management of Coal Combustion Residuals (CCR) in surface impoundments (SIs) under the Standards for the Disposal of CCR in Surface Impoundments: Title 35 of the Illinois Administrative Code (35 I.A.C.) § 845 (Part 845). Facilities regulated under Part 845 are required to develop and sample a groundwater monitoring well network to evaluate whether impounded CCR materials are impacting downgradient groundwater quality. The groundwater quality evaluation must include selection and certification by a qualified professional engineer of the statistical procedures to be used. The procedures described in the evaluation will be used to establish background conditions and implement compliance and corrective action monitoring as necessary and required by 35 I.A.C. § 845.640 and 35 I.A.C. § 845.650. This Statistical Analysis Plan was prepared in accordance with the requirements of 35 I.A.C. § 845.640(f), with reference to the acceptable statistical procedures provided in United States Environmental Protection Agency's (USEPA's) *Statistical Analysis of Groundwater Monitoring Data at RCRA Facilities, Unified Guidance* (*Unified Guidance*) (March 2009).

This Statistical Analysis Plan does not include procedures for groundwater sample collection and analysis, as these activities are conducted in accordance with the Sampling and Analysis Plan prepared for each CCR unit in accordance with 35 I.A.C. § 845.640. This Statistical Analysis Plan will be used as the primary reference for evaluating groundwater quality during operation and post-closure care.

1.1 Statistical Analysis Objectives

This Statistical Analysis Plan is intended to provide a logical process and framework for conducting the statistical analyses of data obtained during groundwater monitoring conducted in accordance with the Sampling and Analysis Plan for each CCR unit. The Statistical Analysis Plan will enable a qualified professional engineer to certify that the selected statistical methods are appropriate for evaluating the groundwater monitoring data for the applicable CCR unit(s).

1.2 Statistical Analysis Plan Approach

The main sections of this Statistical Analysis Plan should be viewed as a "generic" outline of statistical methods utilized for each CCR unit and constituent required to be monitored. The statistical analysis of the groundwater monitoring data, however, will be conducted on an individual-constituent or well basis, and may involve the use of appropriate statistical procedures depending on multiple factors such as detection frequency and normality distributions.

The CCR Rule outlines two phases of groundwater monitoring:

- Background Monitoring in accordance with 35 I.A.C. § 845.650(b)(1)
- Compliance Monitoring in accordance with 35 I.A.C. § 845.650

Each phase of the groundwater monitoring program requires specific statistical procedures to accomplish the intended purpose. During the background monitoring phase, background groundwater quality will be established utilizing upgradient and background wells and downgradient groundwater quality data will be collected to facilitate statistics in subsequent phases. Compliance Monitoring is then initiated through the evaluation of the downgradient

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groundwater monitoring data for exceedances of the groundwater protection standard (GWPS) established by Part 845 (concentration specified in 35 I.A.C. § 845.600 or an IEPA-approved background concentration). The developed statistical analysis plan will be implemented for each monitoring phase and in accordance with the statistical procedures.

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2. BACKGROUND MONITORING AND DATA PREPARATION

The background and compliance monitoring wells were sampled and analyzed for constituents, as listed in Part 845 (antimony, arsenic, barium, beryllium, boron, cadmium, calcium, chloride, chromium, cobalt, fluoride, lead, lithium, mercury, molybdenum, pH, radium 226 and 228 combined, selenium, sulfate, thallium, total dissolved solids, and turbidity), during the baseline phase of the groundwater monitoring program.

The background monitoring well(s) were placed upgradient of the CCR unit, or at an alternative background location, where they are not affected by potential leakage from the CCR unit. Compliance monitoring wells were placed at the waste boundary of the CCR unit, along the same groundwater flow path. As 35 I.A.C. § 845.630(a) specifies, the location of these wells ensures that background accurately represents the quality of unaffected groundwater, while compliance wells accurately represent groundwater quality at the waste boundary and monitor all potential contaminant pathways.

As required by 35 I.A.C. § 845.650(a)(1), eight sampling events were completed within 180 days of April 21, 2021. As outlined, groundwater sampling procedures included sampling of the background and compliance wells using low-flow sampling methods, collection of one field quality control sample per event, and groundwater samples were not field filtered before laboratory analysis of total recoverable metals.

Following completion of the eight sampling events, background groundwater quality was established for Part 845 constituents. Groundwater monitoring will be conducted quarterly for at least the first five years. In accordance with 35 I.A.C. § 845.650(b)(4), after the first five years, a request to reduce the monitoring frequency to semiannual may be submitted to IEPA if all of the following can be demonstrated:

- Groundwater monitoring effectiveness will not be compromised by the reduced frequency
- Sufficient data has been collected to characterize groundwater
- Monitoring to date does not show any statistically significant increasing trends
- The concentrations of monitored constituents at the compliance monitoring wells are below the applicable GWPSs established in 35 I.A.C. § 845.600

The following subsections outline the statistical tests and procedures (methods) that will be utilized to evaluate data collected for each constituent in both background and compliance wells for Background and Compliance Monitoring. When necessary and contingent upon equivalent statistical power, an alternative test not included in this Statistical Analysis Plan may be chosen due to site-specific data requirements.

2.1 Sample Independence

Independence of sample results is a major assumption for most statistical analyses. To ensure physical independence of groundwater sampling results, the minimum time between sampling events must be longer than the time required for groundwater to move through the monitoring well. The sampling schedules for both the baseline and compliance monitoring periods are specified in 35 I.A.C. § 845.650(b) and may conflict with the statistical assumption of independence of sample results.

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2.2 Non-Detect Data Processing

The reporting limit (RL) will be used as the lower level for the reporting of non-detected groundwater quality data. For all summary statistics (box plots, timeseries, etc.), the RL will be substituted for concentrations reported below the RL, including non-detects. With professional judgement, analytical results between the RL and the method detection limit, *i.e.*, estimated values, typically identified with a "J" flag, may be utilized if provided by the laboratory.

For all statistical test procedures:

- If the frequency of non-detect data are less than or equal to 15 percent, half of the RL will be substituted for these data
- If the non-detect frequency is between 15 percent and 50 percent, either the Kaplan-Meier or robust regression on order statistics (ROS) will be used to estimate the mean and standard deviation adjusted for the presence of left-censored values
- If the non-detect frequency is greater than 50 percent, a non-parametric test will be used
- If only one background result is detected that value will be used as the non-parametric upper prediction limit (UPL)

2.3 Testing for Normality

Many statistical analyses assume that sample data are normally distributed (parametric). However, environmental data are frequently not normally distributed (nonparametric). 35 I.A.C. § 845.640(g) requires the knowledge of the background data distribution for comparison to compliance results. The *Unified Guidance* document recommends the Shapiro-Wilk normality test for sample sizes of 50 or less, and the Shapiro-Francia normality test for sample sizes greater than 50.

When possible, transformation of datasets to achieve normal distributions is preferred.

2.4 Testing for Outliers

Part 845 constituents will be screened for the existence of outliers using a method described by the *Unified Guidance*. Outliers are extreme data points that may represent an anomaly or erroneous data point. To test for outliers, one or more of the following outlier tests will be utilized:

- Dixon's test, for well-constituent pairs with less than 25 samples, assumes normally distributed data.
- Rosner's test, for well-constituent pairs with more than 20 samples, assumes normally distributed data.
- Grubb's test for well-constituent pairs with seven or more samples, assumes normally distributed data.
- Time series, box-whisker plots, and probability plots provide visual tools to identify potential outliers, and evaluation of seasonal, spatial, or temporal variability for both normally and non-normally distributed data.

Data quality control, groundwater geochemistry, and sampling procedures will be evaluated as potential sources of error leading to an outlier result. The outlier tests cannot be used alone to determine whether a value is a true outlier that should be excluded from future statistical

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analysis. Corroborating evidence needed to exclude values includes a discrete data reporting or analytical error, or potential laboratory bias. Absent corroborating evidence, the flagged values are considered true, but extreme, values in the data set. Professional judgement will be used to exclude extreme outliers from further statistical analyses. Outliers will be retained in the database.

With professional judgement, a confirmatory sample may be collected to allow for the distinction between an outlier and a true representation of groundwater quality at the monitoring point. If re-sampling is conducted, this sample will be collected within 90 days following outlier identification. If the confirmatory sample indicates the original result as an outlier, it will be reported as such.

2.5 Trend Analysis

Statistical analyses supporting the lack of trend are a fundamental step to confirm the assumption that groundwater quality values are stationary or constant over time at a CCR unit. These analyses allow for evaluation of variation in the background and compliance data for each constituent over time. A statistically significant increasing trend in background data could indicate an existing release from the CCR unit or alternate source, requiring further investigation. In addition, statistically significant trending background data can result in increased standard deviation and, therefore, greater prediction or control limits. Consequently, the increased prediction or control limit will have less power or ability to identify a release from the CCR unit.

A linear regression, coupled with a t-test for slope significance at a 95 percent confidence level (0.05 significance level), may be used on datasets for each constituent with few non-detects and a normally distributed variance of the mean to evaluate time trends. The Theil-Sen trend line, coupled with the Mann-Kendall test for slope significance at a 95 percent confidence level (0.05 significance level), will be used for datasets with frequent non-detects or non-normal variance. Similarly, trend analyses could also be used on compliance data to evaluate a possible release from the CCR unit.

2.6 Spatial Variation

Spatial trends and/or variation between background wells could indicate an existing release from a CCR unit. If the spatial variability is not due to an existing release, intrawell comparisons in compliance wells may be used to account for spatial variability and monitor for a future release. However, the CCR unit being monitored was placed into service prior to the start of groundwater monitoring and it is unknown whether a previous release has occurred. Accordingly, intrawell comparisons in compliance wells cannot be used to determine the occurrence of a future release. Interwell comparisons between compliance wells and background wells will be used.

2.7 Temporal Variation

Time series plots can be used to identify temporal dependence. Potentially significant temporal components of variability can be identified by graphing single constituent data from multiple wells together on a time series plot. With temporal dependence, the time series plot as a pattern of parallel traces, in which the individual wells will tend to rise and fall together across the sequence of sampling dates. Time series plots can be helpful by plotting multiple constituents over time for the same well, or averaging values for each constituent across wells on each sampling event and then plotting the averages over time. In either case, the plots can signify whether the general concentration pattern over time is simultaneously observed for different

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constituents. If so, it may indicate that a group of constituents is highly correlated in groundwater or that the same artifacts of sampling and/or lab analysis impacted the results of several monitoring parameters.

Hydrologic factors such as drought, recharge patterns or regular (e.g., seasonal) water table fluctuations may be responsible for the temporal variation. In these cases, it may be useful to test for the presence of a significant temporal effect by first constructing a parallel time series plot and then running a formal one-way analysis of variance (ANOVA) ($\alpha=0.05$) for temporal effects. A one-way ANOVA for temporal effects considers multiple well data sets for individual sampling events or seasons as the relevant statistical factor. If event-specific analytical differences or seasonality appear to be an important temporal factor, the one-way ANOVA for temporal effects can be used to formally identify seasonality, parallel trends, or changes in lab performance that affect other temporal effects. The one-way ANOVA for temporal effects assumes that the data groups are normally distributed with constant variance. It is also assumed that for each of a series of background wells, measurements are collected at each well on sampling events or dates common to all the wells. Results of the ANOVA can also be used to create temporally stationary residuals, where the temporal effect has been 'subtracted from' the original measurements. These stationary residuals may be used to replace the original data in subsequent statistical testing.

If the data cannot be normalized, a similar test for a temporal or seasonal effect can be performed using the Kruskal-Wallis test ($\alpha=0.05$). Each sampling event should be treated as a separate 'well,' while each well is treated as a separate 'sampling event.' In this case, no residuals can be computed since the Kruskal-Wallis test employs ranks of the data rather than the measurements themselves.

Where both spatial and temporal variation occur, two-way ANOVA can be considered where both well location and sampling event/season are treated as statistical factors. This procedure is described in Davis (1994).

2.8 Updating Background

Updating the background dataset periodically by adding recent results to an existing background dataset can improve the statistical power and accuracy of the statistical analysis, especially for non-parametric prediction intervals. The Unified Guidance recommends updating statistical limits (background) when at least four to eight new measurements (every 1 to 2 years under a quarterly monitoring program), are available for comparison to historical data. Professional judgement will be used to evaluate whether any background data appear to be affected by a release and need to be excluded from a background update. A t-test for equal means (if normal data distribution) or appropriate non-parametric test (if non-normal data distribution) such as a Mann-Whitney (or Wilcoxon) rank-sum or box-whisker plots, will be conducted to evaluate whether the two groups of background sample populations are statistically different prior to updating any background datasets. A 0.05 significance level will be utilized when evaluating the two populations, with the null hypothesis that they are equivalent. In addition, time series graphs or other trend evaluation statistics will be conducted on the new background dataset to verify the absence of a release or changing groundwater quality. If the tests indicate that there are no statistical differences between the two background populations, the new data will be combined with the existing dataset. If the two populations are found to be different, the data will be reviewed to evaluate the cause of the difference. If the differences appear to be caused by a

release (if the new data are significantly higher, or lower for pH), then the previous background dataset may continue to be used. Furthermore, verified outliers will not be added to an existing background dataset. In accordance with the *Unified Guidance*, continual background updates will not be conducted due to the lack of sufficient samples for a statistical comparison.

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3. COMPLIANCE MONITORING

Compliance monitoring is designed to monitor groundwater for evidence of a release by comparing Part 845 constituents in compliance wells to both background concentrations and the GWPS. Compliance Monitoring will begin the quarter following approval of this Groundwater Monitoring Plan and issuance of the Operating Permit. The selected Compliance Monitoring statistical method used to compare compliance groundwater quality data for each constituent to the GWPS will provide for adequate statistical power, error levels and individual test false positive rates, and be appropriate for the distribution and detection frequency of the background dataset. Statistical power is the ability of a statistical test to detect a true exceedance.

In accordance with 35 I.A.C. § 845.610(b)(3)(D), compliance monitoring statistical analyses will be completed and submitted to IEPA within 60 days after completion of sampling.

3.1 GWPS Establishment and Exceedance Determination

In accordance with 35 I.A.C. § 845.600(a), the GWPS will be the constituent concentrations specified in 35 I.A.C. § 845.600(a)(1) except for when the background concentration is greater, or no concentration is specified (*i.e.*, for calcium and turbidity), in which case the GWPS will be the background concentration. The GWPS based on background concentration will be calculated using a parametric upper tolerance limit (UTL), a parametric UPL for a future mean, or a non-parametric UPL for a future median.

Statistical calculations that will be utilized in Compliance Monitoring procedures are summarized in **Table A** below and listed in **Sections 3.1.1** through **3.1.7**. Depending on the distribution of the data and the percentage of non-detects, it may be more appropriate to use a parametric model over a non-parametric model. As necessary, other techniques as mentioned in the *Unified Guidance* and/or new methods will be implemented.

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Table A. Statistical Calculations Used in Compliance Monitoring Procedures

			Compliance M	lonitoring					
C: :C: .		Background	Data	Compliance Data					
Significant Trend?	Percent Non- Distribution Detects		GWPS Determination	Percent Non-Detects	Distribution	Method to Determine Exceedance			
				≤75	Normal	Parametric Lower Confidence Limit around a Normal Mean			
	0 ≤ 50	Normal	35 I.A.C § 845.600(a)(1) constituent concentration or	≤75	Log-Normal	Parametric Lower Confidence Limit around a Lognormal Geometric Mean			
			The Upper Tolerance Limit	NA	Non-Normal	Non-Parametric Lower			
No				>75	Unknown/ Cannot be determined	Confidence Limit around a Median			
	50 ≤ 70	Normal	The Upper Prediction Limit for a Future Mean	NA	NA	Future mean			
	>70	Non-Normal	Upper Prediction Limit for a Future Median	NA	NA	Future median			
	100	Non-Normal	Double Quantification Rule	NA	NA	Individual Retesting Values			
Yes	0 ≤ 50	Normal	UCL of Confidence Band around Linear Regression	≤75	Residuals after subtracting trend are normal, equal variance	Lower Limit from Confidence Band around Linear Regression			
	50 ≤ 100	Non-Normal	UCL of Confidence Band around Thiel-Sen trend line	≤75	Residuals not normal	Lower Limit from Confidence Band around Thiel-Sen			

3.1.1 The Upper Tolerance Limit

The UTL will be used to calculate the GWPS when pooled background data are normally distributed, with a non-detect frequency of 50 percent or less. When non-detect frequency is 15 percent or less, half the RL will be substituted for non-detects. The *Unified Guidance* recommends 95 percent confidence level and 95 percent coverage (95/95 tolerance interval).

• When non-detect frequency is 15 percent or less, half the RL will be substituted for non-detects (simple substitution), and the normal mean and standard deviation will be calculated.

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- The Kaplan-Meier or the ROS method will be used when the detection frequency is between 15 percent and 50 percent. The Kaplan-Meier method assesses the linearity of a censored probability plot to determine whether the background sample can be approximately normalized. If so, then the Kaplan-Meier method will be used to compute estimates of the mean and standard deviation adjusted for the presence of left-censored values. The Kaplan-Meier or ROS estimate of the mean and standard deviation will be substituted for the sample mean and standard deviation.
- If background normality cannot be achieved, non-parametric UTLs will not be calculated until a minimum of 60 background samples have been collected (to achieve 95 percent coverage).

The parametric UTL on a future mean will be calculated from the background dataset as follows:

$$UTL = \overline{x} + \kappa (n, \gamma, \alpha - 1) \cdot s$$

 \overline{x} = background sample mean

s = background sample standard deviation

 κ $(n,\gamma,\alpha-1)$ = one-sided normal tolerance factor based on the chosen coverage (γ) and confidence level $(\alpha$ -1) and the size of the background dataset (n). Values are tabulated in Table 17-3 in Appendix D of the *Unified Guidance*. If exact values are not provided, then κ values can be estimated by linear interpolation.

If the UTL is constructed on the logarithms of original observations to achieve normality, where \overline{y} and s_y are the log-mean and log-standard deviation, the limit will be exponentiated for backtransformation to the concentration scale as follows:

$$UTL = \exp\left[\overline{y} + \kappa (n, \gamma, \alpha - 1) \cdot s_{\gamma}\right]$$

 \overline{y} = background sample log-mean

 s_{ν} = background sample log-standard deviation

When the GWPS is based on the 35 I.A.C. § 845.600(a)(1) constituent concentrations or a UTL derived from the background dataset, an exceedance in compliance wells relative to the GWPS will be evaluated using confidence intervals. A confidence interval defines the upper and lower bound of the true mean of a constituent concentration in groundwater within a specified confidence range.

- Non-detects in compliance data will be handled similarly to upgradient analyses, with half the RL substituted for non-detects when the frequency is 15 percent or less.
- The Kaplan-Meier, or the ROS method, will be used when the detection frequency is between 15 percent and 50 percent to compute estimates of the mean and standard deviation adjusted for the presence of left-censored values. These estimates will then be substituted for the sample mean and standard deviation.

Once the GWPS is established for background data using the UTL, either parametric or non-parametric confidence intervals will be computed for each constituent in compliance wells to identify GWPS exceedances.

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3.1.2 Parametric Confidence Intervals around a Mean

If compliance data are approximately normal, one-sided parametric confidence intervals around a sample mean will be constructed for each constituent and well pair. The lower confidence limit (LCL) will be calculated as:

$$LCL_{1-\alpha} = \overline{x} - t_{1-\alpha,n-1} \cdot \frac{s}{\sqrt{n}}$$

 \overline{x} = compliance sample mean

s = compliance sample standard deviation

n =compliance sample size

 $t_{1-\alpha,n-1}$ = obtained from a Student's t-table with (n-1) degrees of freedom (Table 16-1 in Appendix D of the *Unified Guidance*)

The chosen t value will aim to achieve both a low false-positive rate, and high statistical power. Minimum a values are tabulated in Table 22-2 of Appendix D of the *Unified Guidance*. The selected minimum a value, from which the t value will be derived, will have at least 80 percent power $(1-\beta=0.8)$ when the underlying mean concentration is twice the GWPS.

If compliance data are distributed lognormally, the LCL will be computed around the lognormal geometric mean as:

$$LCL_{1-\alpha} = \exp\left(\overline{y} - t_{1-\alpha,n-1} \cdot \frac{s_y}{\sqrt{n}}\right)$$

 \overline{y} = compliance sample log-mean

 $s_v = compliance sample log-standard deviation$

3.1.3 Non-Parametric Confidence Intervals around a Median

Non-parametric confidence intervals around the median will be computed if the compliance data contain greater than 50 percent non-detects or are not normally distributed. The mathematical algorithm used to construct non-parametric confidence intervals is based on the probability (P) that any randomly selected measurement in a sample of n concentration measurements will be less than an unknown $P \times 100$ th percentile of interest (where P is between 0 and 1). Then the probability that the measurement will exceed the $P \times 100$ th percentile is (1-P). The number of sample values falling below the $P \times 100$ th percentile out of a set of n should follow a binomial distribution with parameters n and success probability P, where 'success' is defined as the event that a sample measurement is below the $P \times 100$ th percentile. The probability that the interval formed by a given pair of order statistics will contain the percentile of interest will then be determined by a cumulative binomial distribution Bin(x;n,p), representing the probability of x or fewer successes occurring in n trials with success probability p. P will be set to 0.50 for an interval around the median.

The sample size n will be ordered from least to greatest. Given P = 0.50, candidate interval endpoints will be chosen by ordered data values with ranks close to the product of $(n+1) \times 0.50$. If the result of $(n+1) \times 0.50$ is a fraction (for even-numbered sample sizes), the rank values immediately above and below will be selected as possible candidate endpoints. If the result of $(n+1) \times 0.50$ is an integer (for odd-numbered sample sizes), one will be added to and subtracted

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from the result to get the upper and lower candidate endpoints. The ranks of the endpoints will be denoted L* and U*. For a one-sided LCL, the confidence level associated with endpoint L* will be computed as:

$$1 - \alpha = Bin(L^* - 1; n, 0.50) = \sum_{x=L^*}^{n} {n \choose x} \left(\frac{1}{2}\right)^n$$

If the candidate endpoint(s) do not achieve the desired confidence level, new candidate endpoints (L^*-1) and (U^*+1) and achieved confidence levels will be calculated. If one candidate endpoint equals the data minimum or maximum, only the rank of the other endpoint will be changed. Achievable confidence levels are tabulated using these equations in Table 21-11 in Appendix D of the *Unified Guidance*.

Both parametric and non-parametric confidence limits will then be compared to the GWPS. The CCR unit is considered to be in compliance if the LCL is equal to or lower than the GWPS for all detected constituents at all compliance monitoring wells. A GWPS exceedance is determined if the LCL exceeds the GWPS.

3.1.4 The Upper Prediction Limit for a Future Mean

The parametric UPL for a future mean will be used to calculate the GWPS if the pooled background data contain 50 to 70 percent non-detects and normality can be achieved. The Kaplan-Meier or ROS methods will be used to estimate the mean and standard deviation. The non-parametric UPL for a future median will be calculated as the GWPS if background samples cannot be normalized or contain greater than 70 percent non-detects. The parametric UPL for a future mean will be calculated from the background dataset at follows:

$$UPL_{1-\alpha} = \overline{x} + \kappa s$$

 \overline{x} = background sample mean

s =background standard deviation

 κ = multiplier based on the order (p) of the future mean to be predicted, the number of compliance wells to be tested (w), the background sample size (n) the number (c) of constituents of concern (COCs), the "1-of-m" retesting scheme, and the evaluation schedule (annual, semi-annual, quarterly). Values are tabulated in 19-5 to 19-9 in Appendix D of the *Unified Guidance*.

The mean of order p will be computed for each well and compared against the UPL. For any compliance point mean that exceeds the limit, p additional resamples may be collected at that well for a 1-of-2 retesting scheme. Resample means will then be compared to the UPL. A GWPS exceedance has been deemed to occur at a compliance well when the initial mean and all resample means exceed the UPL.

3.1.5 The Non-Parametric Upper Prediction Limit for a Future Median

The non-parametric UPL for a future median will be used to calculate the GWPS if the pooled background data contain greater than 70 percent non-detects and normality cannot be achieved. Non-parametric methods assume that the data does not have an underlying distribution. To calculate the non-parametric UPL on a future value, the target per-constituent false positive rate (a_{const}) will be determined as follows:

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$$\alpha_{const} = 1 - (1 - \alpha)^{1/c}$$

 α = the site-wide false positive rate (SWFPR) of 0.10 recommended by the *Unified Guidance*

c = the number of monitoring constituents

The number of yearly statistical evaluation (nE) will be multiplied by the number of compliance wells (w) to determine the look-up table entry, w*. The background sample size (n) and w* will be used to select an achievable per-constituent false positive rate value in Table 19-24 of Appendix D in the *Unified Guidance*. The chosen achievable per-constituent false positive rate value will determine the type of non-parametric prediction limit (maximum or 2nd highest value in background) and a retesting scheme for a future median. The background data will be sorted in ascending order, and the upper prediction limit will be set to the appropriate order statistic previously determined by the achievable per-constituent false positive rate value in Table 19-24. If all constituent measurements in a background sample are non-detect, the Double Quantification rule will be used. The use of the Double Quantification rule in Compliance Monitoring will only be applicable if the RL is above the 35 I.A.C. § 845.600(a)(1) constituent concentration or a constituent concentration is not specified in § 845.600(a)(1). This scenario is highly unlikely. The constituent will also be removed from calculations identifying the target false positive rate.

Two initial measurements per compliance well will be collected. If both do not exceed the upper prediction limit, a third initial measurement will not be collected since the median of order 3 will also not exceed the limit. If both exceed the prediction limit, a third initial measurement will not be collected since the median will also exceed the limit. If one initial measurement is above and one below the limit, a third initial observation may be collected to determine the position of the median relative to the UPL. Up to three resamples will be collected in order to assess the resample median. In all cases, if two or more of the compliance point observations are non-detect, the median will be set equal to the RL. The median value for each compliance well will be compared to the UPL. For the 1-of-2 retesting scheme, if any compliance point median exceeds the limit, up to three additional resamples will may be collected from that well. The resample median will be computed and compared to the UPL. A GWPS exceedance has been deemed to occur at a compliance well when either the initial median, or both the initial median and resample median exceed the UPL.

If the concentrations of detected constituents are below the established GWPS, Compliance Monitoring will continue.

3.1.6 Parametric Linear Regression and Confidence Band

If the t-test detects a significant trend in the parametric linear regression line using either background or compliance data for a particular constituent, confidence bands accounting for trends will be constructed to account for the trend-induced variation. If this is not accounted for, a wider confidence interval will inevitably be calculated for a given confidence level and sample size (n). A wider confidence interval will result in less statistical power, or ability to demonstrate an exceedance or return to compliance. When a linear trend line has been estimated, a series of confidence intervals is estimated at each point along the trend. This creates a simultaneous confidence band that follows the trend line. As the underlying population mean increases or decreases, the confidence band does also to reflect this change at that point in time.

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Linear regression will be used when background or compliance data are approximately normally distributed, with a constant sample variance around the mean, and the frequency of non-detects is low. The linear regression of concentration against sampling date (time) will be computed as follows:

$$\hat{b} = \sum_{i=1}^{n} (t_i - \overline{t}) \cdot x_i / (n-1) \cdot s_t^2$$

 $x_i = i^{th}$ concentration value and

 $t_i = i^{th}$ sampling date

 \overline{t} = sampling mean date

 s_t^2 = variance of the sampling dates

This estimate leads to the following regression equation:

$$\hat{x} = \overline{x} + \hat{b} \cdot (t - \overline{t})$$

 \overline{x} = mean concentration level

 \hat{x} = estimated mean concentration at time t

The regression residuals will also be computed at each sampling event to ensure uniformity and lack of significant skewness. Regression residuals will be computed at each sampling event as follows:

$$r_i = x_i - \hat{x}_i$$

The estimated variance around the regression line, or mean squared error (MSE) will be computed as follows:

$$s_e^2 = \frac{1}{n-2} \sum_{i=1}^n r_i^2$$

The confidence intervals around a linear regression trend line given confidence level (1- α) and a point in time (t₀), will be computed as follows:

$$LCL_{1-\alpha} = \hat{x}_0 - \sqrt{2s_e^2 \cdot F_{1-2\alpha,2,n-1} \cdot \left[\frac{1}{n} + \frac{(t_0 - \overline{t})^2}{(n-1) \cdot s_t^2} \right]}$$

$$UCL_{1-\alpha} = \hat{x}_0 - \sqrt{2s_e^2 \cdot F_{1-2\alpha,2,n-2} \cdot \left[\frac{1}{n} + \frac{\left(t_0 - \overline{t}\right)^2}{(n-1) \cdot s_t^2} \right]}$$

 \hat{x}_0 = estimated mean concentration from the regression equation at time t_0

 $F_{1-2\alpha,2,n-2}$ = upper (1-2 α)th percentage point from an F-distribution with 2 and (n-2) degrees of freedom

For background data, the UCL around the linear regression line will be used as the GWPS for the trending constituent. For compliance data, confidence bands around the linear regression line will be compared to the GWPS. The CCR unit is considered to be in compliance if the LCL is equal to or lower than the GWPS for all detected constituents at all compliance wells. A GWPS exceedance is determined when the LCL based on the trend line first exceeds the GWPS.

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3.1.7 Non-Parametric Thiel-Sen Trend Line and Confidence Band

If the Mann-Kendall test detects a significant trend in the non-parametric Thiel-Sen line using either background or compliance data for a particular constituent, confidence bands accounting for trends will be constructed to account for the trend-induced variation. The Thiel-Sen trend line will be used as a non-parametric alternative to linear regression when trend residuals cannot be normalized or if there are a higher percentage of non-detects in either background or compliance data. The Thiel-Sen trend line estimates the median concentration over time by combining the median pairwise slope with the median concentration value and the median sample date. To compute the Thiel-Sen line, the data will first be ordered by sampling event x1, x2, xn. All possible distinct pairs of measurements (x_i, x_j) for j > i will be considered and the simple pairwise slope estimate will be computed for each pair as follows:

$$m_{ij} = (x_i - x_i)/(j - i)$$

With a sample size of n, there will be a total of N = n(n-1)/2 pairwise estimates (m_{ij}) . If a given observation is a non-detect, half the RL will be substituted. The N pairwise slope estimates (m_{ij}) will be ordered from least to greatest (renamed m(1), m(2),...m(N)). The Thiel-Sen estimate of slope (Q) will be calculated as the median value of the list depending on whether N is even or odd as follows:

$$Q = \begin{cases} m_{([N+1]/2)} \text{ if N is odd} \\ (m_{(N/2)} + m_{([N+2]/2)})/2 \text{ if N is even} \end{cases}$$

The sample concentration magnitude will be ordered from least to greatest, x(1), x(2), to x(n) and the median concentration will be calculated as follows:

$$\tilde{x} = \begin{cases} x_{([n+1]/2)} & \text{if n is odd} \\ (x_{(n/2)} + x_{([n+2]/2)})/2 & \text{if n is even} \end{cases}$$

The median sampling date (\tilde{t}) with ordered times (t(1), t(2), to t(n)) will also be determined in this way. The Thiel-Sen trend line will then be computed for an estimate at any time (t) of the expected median concentration (x) as follows:

$$x = \tilde{x} + O \cdot (t - \tilde{t}) = (\tilde{x} - O \cdot \tilde{t}) + O \cdot t$$

To construct a confidence band around the Thiel-Sen line, sample pairs (ti, xi) will be formed with a sample date (ti) and the concentration measurement from that date (xi). Bootstrap samples (B) will be formed by repeatedly sampling n pairs at random with replacement from the original sample pairs. This will be repeated 500 times. For each bootstrap sample, a Thiel-Sen trend line will be constructed using the equation above. A series of equally spaced time points (tj) will be identified along the range of sampling dates represented in the original sample, j =1 to m. The Thiel-Sen trend line associated with each bootstrap replicate will be used to compute an estimated concentration (\hat{x}_j^B). An LCL will be constructed for the lower α^{th} percentile $\hat{x}_j^{[\alpha]}$ from the distribution of estimated concentrations at each time point (tj). For a UCL, compute the upper (1- α)th percentile, $\hat{x}_j^{[1-\alpha]}$ at each time point (tj).

For background data, the UCL around the Thiel-Sen trend line will be used as the GWPS for the trending constituent. For compliance data, confidence bands around the Thiel-Sen trend line will be compared to the GWPS. The CCR unit is considered to be in compliance if the LCL is equal to or lower than the GWPS for all detected constituents at all compliance wells. A GWPS exceedance is confirmed when the LCL based on the trend line first exceeds the GWPS.

3.2 Determination of Statistically Significant Increases over Background

In accordance with 35 I.A.C. §§ 845.610(b)(3)(B) and 845.640(h), individual monitoring event concentrations for each constituent detected in the compliance monitoring wells during compliance monitoring sampling events will be compared to the background concentration as determined by the methods described above. An exceedance of the background concentration for any constituent measured at any compliance monitoring well, or constituent detection if not detected in the background samples, constitutes a Statistically Significant Increase (SSI). An exception to this method is pH, where two-sided (upper and lower) tolerance limits are established from the distribution of the background groundwater quality data. An exceedance of either the UTL or lower tolerance limit (LTL) would constitute an SSI for pH.

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4. REFERENCES

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ATTACHMENT J

Memorandum



Date: 25 October 2021

Subject: 35 I.A.C. Section 845.430 - Slope Maintenance Documentation for GMF Pond at

Coffeen Power Plant

Illinois Power Generating Company operates the coal fired Coffeen Power Plant (Plant) located in Montgomery County, Illinois. The Coffeen GMF Pond is an inactive surface impoundment storing coal combustion residuals (CCR). The requirements for the Coffeen GMF Pond are found in 35 Ill. Admin. Code 845, Standards for the Disposal of Coal Combustion Residuals in Surface Impoundments (Part 845).

Pursuant to Part 845, Section 845.230(d)(2)(F), the initial operating permit application for existing or inactive CCR surface impoundments that have not completed an Agency approved closure before prior to July 30, 2021, must contain documentation that the CCR surface impoundment, if not incised, will be operated, and maintained with one of the forms of slope protection specified in Section 845.430. This statement addresses the requirements of Part 845, Section 845.430 Slope Maintenance, which states:

<u>Section 845.430:</u> The slopes and pertinent surrounding areas of the CCR surface impoundment must be designed, constructed, operated, and maintained with one of the forms of slope protection specified in subsection (a) that meets all the performance standards of subsection (b).

<u>Section 845.430(a):</u> Slope protection must consist of one of the following: 1) A vegetative cover consisting of grassy vegetation; 2) An engineered cover consisting of a single form or combination of forms of engineered slope protection measures; or 3) A combination of the forms of cover specified in subsections (a)(1) or (a)(2).

Section 845.430(b): Any form of cover for slope protection must meet the following performance standards: 1) The cover must be installed and maintained on the slopes and pertinent surrounding areas of the CCR surface impoundment; 2) The cover must provide protection against surface erosion, wave action, and adverse effects of rapid drawdown; 3) The cover must be maintained to allow for the observation of, and access to, the slopes and pertinent surrounding areas during routine and emergency events; 4) Woody vegetation must be removed from the slopes or pertinent surrounding areas. Any removal of woody vegetation with a diameter greater than 1/2 inch must be directed by a person familiar with the design and operation of the CCR surface impoundment and in consideration of the complexities of removal of a tree or a shrubbery, who must ensure the removal does not create a risk of destabilizing the CCR surface impoundment or otherwise adversely affect the stability and safety of the CCR surface impoundment or

Memorandum (cont'd)



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personnel undertaking the removal; and 5) The height of vegetation must not exceed 12 inches.

Slope protection, consisting of vegetative cover, was installed on the slopes and pertinent surrounding areas of the Coffeen GMF Pond, and is inspected, maintained and repaired as needed. Based on observations from weekly inspections conducted in accordance with Section 845.540(a), and the 2020 annual inspections conducted by Hanson Professional Services Inc., the vegetative cover is described to be in good working condition with a maximum vegetation height of 12 inches. The owner's Operations and Maintenance Plan (O&M Plan) provides details for maintaining grass and removing woody vegetation and addressing erosion features on the slopes. Based on a review of the documentation described above, the owner is implementing the O&M Plan, including the completion of repairs and maintenance as needed and when issues are identified during weekly and/or annual inspections. The slope maintenance portion of the O&M Plan and the Annual Inspection performed by Hanson in 2020 are included in Attachment J. The surface impoundment slope protection (vegetative cover) installed and maintained on the slopes and pertinent areas around the slopes is depicted in the aerial photograph provided below.



Excerpt from the Coffeen GMF Pond Operations and Maintenance Manual

Operation and Maintenance Inspection

Occasional "walk-around" inspections of the dams and appurtenant works are to be made by the dam operator. During these inspections, a checklist of items to be maintained and items to be observed should be recorded. Appendix A provides an example of the Operation and Maintenance Inspection Checklist to be utilized for these inspections. If any of the following items are found to be unusual or are cause for concern, the Shift Supervisor should be notified, and the Emergency Action Plan should be immediately consulted for guidance on an appropriate course of action.

Frequency: Operation and maintenance inspections will be performed by the dam operator monthly and during and after unusual events such as heavy rainfall or an earthquake.

Inspection Items: During each inspection the following items should be noted.

- 1. Water Level Maximum reservoir levels because of heavy rainfall should be recorded.
- 2. Earth Embankment Walk the crest, side slopes and downstream toe of the dam concentrating on surface erosion, seepage, cracks, settlement, slumps, slides, and animal burrows. These are described as follows:
 - Surface Erosion Removal of vegetative cover by water action or pedestrian or vehicle usage forming deep ruts or gullies.
 - Seepage The passage of water through and/or underneath the earth embankment abutment and natural groundline or at the contact between the embankment and outlet works. It can be indicated by cattails or other wet environmental vegetation, erosion, channelization, or slumping on the embankment face.
 - Cracks Deep cracks usually indicate the movement of the dam and/or the
 foundation and can be in either the longitudinal (along the length of the dam) or
 transverse (across the dam) directions. Cracking can be an indicator of the
 beginning of slumps. Shallow cracks may develop during the summer when the
 surface soils of the embankment become severely dried and are usually of no
 concern regarding the safety of the dam.
 - Settlement Settlement is indicated by depressions or low spots and can be signs of consolidation of the dam or foundation or the loss of material beneath the settlement area.
 - Slumps/Slides A slow or sudden movement of the earth embankment slope on either face toward the toe of the dam.

- If seepage indicates the presence of soil particles, or if deep cracks, settlement, slumps, or slides are noticed, a qualified engineer should be contacted immediately for consultation.
- Animal Burrows Animal burrows result in a loss of earth embankment material and can provide seepage paths for water through the embankment.
- 3. Gypsum Embankment Walk the crest, side slopes and downstream toe of the dam concentrating on surface erosion, seepage, cracks, settlement, slumps, slides, and animal burrows. The descriptions for these are the same as for earth embankment.
- 4. Vegetation Grass should be a thick vigorous growth to stabilize the earth embankment soils and prevent erosion from occurring. Note the height of the grass, if greater than 1-foot a mowing of the area should be scheduled before the next inspection. There should be NO trees on the earth embankment and NONE within a minimum of 20 feet of the embankment toes or other structures. The gypsum embankment will not be seeded and is not expected to have any vegetation.
- 5. Gypsum Stack piezometers should be inspected for any damage or loss of function. Damaged piezometers must be promptly repaired or replaced since their function is critical to ensuring stability of the gypsum stack.
- 6. The water level in each Gypsum Stack piezometer must be measured and recorded during each monthly inspection. If the water level in any piezometer is above the "critical elevation" as discussed in Section 4.5.2 of this plan, the Ameren Technical Services Superintendent should be notified, and the Emergency Action Plan should be immediately consulted for guidance on an appropriate course of action.
- 7. Gypsum Pond LD/LCRS Drains The change in location or amount of flow discharging from the Leak Detection/Leachate Collection Recovery System (LD/LCRS) should be recorded. If a significant change has occurred, a qualified engineer should be contacted for consultation.
- 8. Gypsum Stack Ring Drains The change in location or amount of flow discharging from the Ring Drains should be recorded. If a significant change has occurred, a qualified engineer should be contacted for consultation.
- 9. Gypsum Stack Fixed Decant Check the alignment and supports for the pipe. Record the amount of flow discharging from the pipe and any erosion or scour around the discharge point.
- 10. Gypsum Stack Perimeter Ditch The perimeter ditch should have a consistent prismatic shape for the entire length. Inspect the perimeter ditch for evidence of erosion, sediment deposition and irregularity in channel geometry, especially in the vicinity of siphon, decant or ring drain outfall structures. If irregularities are noted, repairs should be scheduled and completed.

- 11. Drawdown Facilities Check to make sure that the drawdown stop logs in the transfer ditch are undamaged, operating well and allowing for the free flow of water over them. Confirm during inspections the valves are opened and closed at least quarterly.
- 12. Transfer Channel Check for any debris or other obstructions which may block or restrict the free flow of water. Check for any pools or undulation of the floor of the channel.
- 13. Recycle Pond Decant Check for any debris or other obstructions around the Recycle Pond decant which may block or restrict the free flow of water. The emergency dewatering valve should be lubricated. If there is no return water in the pipe, the emergency dewatering valve should be exercised. Record the physical and operating conditions of the system.
- 14. Recycle Pond Drop Inlet Spillways Check for any debris or other obstructions around the inlet crest and at the bottom of the drop inlet which may block or restrict the free flow of water. Check for the development of any rusty areas on the concrete, and seepage, cracking, breaking, or spalling of the concrete. Check for settlement or cracking of the crest. Check for any debris in the pipes which may restrict the flow of water. Check for any tears or leaks in the HDPE liner covering the concrete.
- 15. Recycle Pond Rip Rap Basin Check for any debris or other obstructions in the riprap basin which may block or restrict the free flow of water. Check to make sure that the rip rap is remaining in a uniform position. Freeze/thaw action or flow over the rip rap may tend to lift or fracture, thus requiring replacement or leveling to maintain the necessary level of protection. NO trees or woody vegetation should be growing through the rip rap.
- 16. Fences Check for damage, accumulated debris, operation of gates and locks, and adequacy of locations (this may change with time as people access the area or development occurs in the area).
- 17. Perimeter Check the perimeter of the dams for a distance of at least 100 feet beyond the toe for signs of seepage or boils.
- 18. HDPE Liner Wherever exposed, the HDPE Liner should be inspected for tears, gouges, protrusions under the liner and abrasion.

Dam Inspection Report

Name of Dam	Coffeen GMF	- Gypsum S	Dam ID No.	IL50579					
Permit Number	DS2008082	_	Class of Dam	I					
Location NW 1/4	Section11	_Township	7N	Range	3W				
Owner Dyne	gy Midwest Genera	tion, LLC							
	Name			Telephone	Number (Day)				
13	34 CIPS Lane								
	Street			Telephone	Number (Night)				
Coffeen, IL City		_	M	ontgomery					
Type of Dam		E	arth Embankm	ent					
Type of Spillway			Open Channe	el					
Date(s) Inspected			16-Nov-20						
Weather When Insp	ected		Clear						
Temperature When	Inspected			50° F					
Pool Elevation Whe	n Inspected		620.8						
Tailwater Elevation	When Inspected		N.A.						
Tailwater Elevation Tailwater Elevation POFESSIONAL INTERPREDICTION OF THE PROPERTY OF THE P	à	Inspection	Personnel:						
		James Knu	utelski, PE	Geotechi	nical Engineer				
JAMES P. KNUTELSKI	EER		lame		Title				
002-004200	mun _{th}	Jason Can	npbell, PE	Dynegy Dan	n Safety Manager				
A LEINOIS	ren e e e e e e e e e e e e e e e e e e		lame	<u> </u>	Title				
1/// 1/ /2/	20/2020	Gina Kram		IDN	IR-OWR				
4 (m) 14.	30/2020	N	lame		Title				
EXP 11/301	21	John Roma	ang	Coffe	een Plant				
Professional Engine	er's Seal		lame		Title				

The Department of Nautural Resources is requesting information that is necessary to accomplish the statutory purpose as outlined under the River, Lakes and Streams Act, 615 ILCS 5. Submittal of this information is REQUIRED. Failure to provide the required information could result in the initiation of non-compliance procedures as outlined in Section 3702.160 of the "Rules for Construction and Maintenance of Dams".

CONDITION CODES

- NE No evidence of a problem
- GC Good condition
- MM Item needing minor maintenance and/or repairs within the year, the safety or integrity of the item is not yet imperiled
- IM Item needing immediate maintenance to restore or ensure its safety or integrity
- EC Emergency condition which if not immediately repaired or other appropriate measures taken could lead to failure of the dam
- OB Condition requires regular observation to ensure that the condition does not become worse
- NA Not applicable to this dam
- NI Not inspected list the reason for non-inspection under deficiencies

GYPSUM STACK - EARTH EMBANKMENT

ITEM	CONDITION CODE	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES AND IMPLEMENTATION SCHEDULE
Surface Cracks	NE	DEFICIENCIES	AND IMPLEMENTATION SCHEDOLE
Vertical and Horizontal Alignment of Crest	GC		
Unusual Movement or Cracking At or Beyond Toe	NE		
Sloughing or Erosion of Embankment and Abutment Slopes	NE		
Upstream Face Slope Protection (HDPE Liner)	ММ	Woody vegetation on west side liner at north end.	Remove woody vegetation.
Seepage	NE		
Filter and Filter Drains	NA		

EARTH EMBANKMENT

(Continued)

	LCONDITION		DECOMMENDED DEMEDIAL MEACURES
ITEM	CONDITION CODE	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES
I I EIVI	CODE	DEFICIENCIES	AND IMPLEMENTATION SCHEDULE
Animal Damage	NE		
7 minar Damage	11		
Embankment Drainage Ditches	NA		
Vegetative Cover	GC		
3			
Others (Names)			
Other (Name)	NA		
Other	NA		
Other	NA		
Other	INC		
Other	NA		

PRINCIPAL SPILLWAY APPROACH CHANNEL

ITEM	CONDITION CODE	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES AND IMPLEMENTATION SCHEDULE
Debris	NE	BEITOILITOILE	7110 IVII ELIVERTATION CONEDCE
Side Slope Stability	NE		
Slope Protection (HDPE Liner)	GC		
Other (Name)	NA		
Other	NA		
Other	NA		
Other	NA		

PRINCIPAL SPILLWAY

Drop Inlet Spillway		X Overflow Spillway Structure Gated							
ITEM	CONDITION CODE	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES AND IMPLEMENTATION SCHEDULE						
Erosion, Spalling, Cavitation (HDPE Lined)	ММ	Punctures in liner at outlet channel to recycle pond.	Repair punctures in liner.						
Structure to Embankment Junction	NA								
Drains	NA								
Seepage Around or Into Structure	NA								
Surface Cracks	NE								
Structural Cracks	NA								

IF THE SPILLWAY IS GATED FILL OUT THE GATES SECTION

PRINCIPAL SPILLWAY (Continued)

Drop Inlet Spillway		X Overflow Spillway Structure Gated						
ITEM	CONDITION CODE	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES AND IMPLEMENTATION SCHEDULE					
Alignment of Abutment Walls	NA							
Construction Joints (HDPE Liner)	GC							
Filter and Filter Drains	NA							
Trash Racks	NA							
Bridge and Piers	NA							
Differential Settlement	NE							
Other (Name)	NA							

PRINCIPAL SPILLWAY

(Continued)

Х	Chute

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES
ITEM	CODE	DEFICIENCIES	AND IMPLEMENTATION SCHEDULE
Erosion, Spalling, Cavitation	NE		
Structure to Embankment Junction	GC		
Junction			
Construction Joints	GC		
(HDPE Liner)			
Expansion and Contraction	NA		
Joints			
Differential Settlement	NE		
Surface Cracks	NE		
Structural Cracks	NE		
VAZ-III A P	N/A		
Wall Alignment	NA		
Other (Name)	NA		
TE THE ODILLWAY IS SATED FIL			

IF THE SPILLWAY IS GATED FILL OUT THE GATES SECTION

EMERGENCY SPILLWAY

Earth			Other: Name	None
ITEM	CONDITION CODE	DEFICIENCIES	RECOMMENDED REMEDIA AND IMPLEMENTATION SC	
Erosion	NA			
Weeds, Logs, Other Obstructions	NA			
Side Slope Sloughing	NA			
Vegetation	NA			
Sedimentation	NA			
Riprap	NA			
Settlement of Crest	NA			
Downstream Channel	NA			
Other (Name)	NA			

SUMMARY OF MAINTENANCE DONE AND/OR REPAIRS MADE SINCE THE LAST INSPECTION

DA	TE OF PRESENT INSPECTION	16-Nov-20	
DA	TE OF LAST INSPECTION	22-Oct-19	
1.	EARTH EMBANKMENT DAMS Periodic mowing of downstream embankment fac	e.	
2.	CONCRETE MASONRY DAMS		
	NA		
3.	PRINCIPAL SPILLWAY		
	None noted.		
4.	OUTLET WORKS		
	NA		
5.	EMERGENCY SPILLWAY		
	NA		

DOWNSTREAM DEVELOPMENT APPROXIMATE WIDTH OF AFFECTED FLOODPLAIN

0.25 MILES

MILES DOWNSTREAM FROM DAM		DOWNSTREAM DEVELOPMENT				Life Lo			Economic Loss SKETCH IN DEVELOPN Potential DOWNSTREAM OF THI										
	OCCUPIED HOMES	UNOCCUPIED HOMES	AGRICULTURAL BUILDINGS	INDUSTRIAL BUILDINGS	COMMERCIAL BUILDINGS	SCHOOLS	HOSPITALS	ROADS & BRIDGES	DAMS	OVERHEAD UTILITIES	OTHER DEVELOPMENT (Name	OTHER DEVELOPMENT (Name	NONE	1 TO 10	OVER 10	MINIMAL EXPECTED	APPRECIABLE EXPECTED	EXCESSIVE EXPECTED	RESERVOIR
0 to 1/4	2	0	2	1	0	0	0	3	0	0			X			X			
1/4 to 1/2																			
1/2 to 3/4																			/ (/
3/4 to 1																			
1 to 1-1/4																			
1-1/4 to 1-1/2	_									_									
1-1/2 to 1-3/4																			/ \ /
1-3/4 to 2																			/ (/
OVER 2																			Downstream Floodplain

The number of homes, buildings, or other items in the floodplain downstream of the dam should be placed in the appropriate row and column to designate their location.

Owner's Maintenance Statement

I,			- '	owner of		Gypsum Management Gypsum Stack Dam,
Dam Identification Number	IL50579	,	in	Montgo	omery	County,
am maintaining the dam in acco	ordance with the	ассер	ted	maintenanc	e plan wh	ich is part of
Permit Number DS20	08082 .					
				Signatu	ire	
				Da	te	
Owner's Opera	ation and Ma	inten	an	ce Plan S	tatemen	t
l,			- '	owner of		Gypsum Management Gypsum Stack Dam,
Dam Identification Number	IL50579	,	in	Montgo	omery	County,
have reviewed the operation an	d maintenance _l	olan in	cluc	ding the Eme	ergency A	ction Plan (EAP),
which is part of, Permit Number						
1	nave enclosed th	пе арр	rop	riate revisior	ns or	
	nave determined	that n	o re	evisions to th	ne plan ar	e necessary.
				Signatu	ire	
				Da	te	

The Department of Nautural Resources is requesting information that is necessary to accomplish the statutory purpose as outlined under the River, Lakes and Streams Act, 615 ILCS 5. Submittal of this information is REQUIRED. Failure to provide the required information could result in the initiation of non-compliance procedures as outlined in Section 3702.160 of the "Rules for Construction and Maintenance of Dams".



Outlet channel inlet



Punctures in liner at inlet – repair



Outlet channel



West side



West side



West side – remove small woody vegetation growing on liner at north end



South side



South side



North side



North side



East side



East side

ATTACHMENT K

POST-CLOSURE PLAN FOR EXISTING CCR SURFACE IMPOUNDMENT 40 C.F.R. § 257.104 and 35 I.A.C. 845.780 REV 0 – 10/30/2021

SITE INFORMATION			
Site Name / Address	Coffeen Power Plant / 134 Cips Lane, Coffeen, IL 61207		
Owner Name / Address	Illinois Power Generating Company / 6555 Sierra Drive Irving, Texas 75039		
CCR Unit	GMF Pond	Closure Method and Final Cover Type	Close In-Place Clayey Soil Cover with Vegetation

POST-CLOSURE PLAN DESCRIPTION

40 C.F.R. § 257.104(c)(1) and 35 I.A.C. 845.780(c)(1) — Length of post-closure care period.

40 C.F.R. § 257.104(c)(2) and 35 I.A.C. 845.780(c)(2) — Circumstances extending the post closure care period.

Post-closure care will be conducted for a period of 30 years as required by 40 C.F.R. § 257.104(c)(1) and 35 I.A.C. 845.780(c)(1), except as provided by 40 C.F.R. § 257.104(c)(2) and 35 I.A.C. 845.780(c)(2).

If at the end of the post-closure care period the CCR unit is operating under assessment monitoring in accordance with §257.95, the post-closure care as described in this plan will continue until returning to detection monitoring in accordance with §257.95.

Under 35 I.A.C. 845.780(c)(2), the post-closure care period will be extended until groundwater monitoring data demonstrate that concentrations are below the groundwater protection standards in Section 845.600 and are not increasing for those constituents over background, using the statistical procedures and performance standards in Section 845.640(f) and (g), provided that concentrations have been reduced to the maximum extent feasible and concentrations are protective of human health and the environment.

40 C.F.R. § 257.104(d)(1)(i) and 35 I.A.C. 845.780(d)(1)(A) – A description of the monitoring and maintenance activities required in40 C.F.R. § 257.104(b) and 35 I.A.C. 845.780(b), and the frequency at which these activities will be performed, to maintain the integrity and effectiveness of the final cover system, maintain the groundwater monitoring system and monitor the groundwater.

Pursuant to § 257.104(b)(1) and 35 I.A.C. 845.780(b)(1), throughout the post-closure care period, periodic visual observations of the final cover system and stormwater management system will be performed at least annually for evidence of settlement, subsidence, erosion, or other damage that may adversely affect the integrity and effectiveness of the final cover system. When practical, visual observations of the final cover will be made concurrent with groundwater monitoring activities.

Noted evidence of damage, such as rills, surface cracks and settlement, will be repaired to maintain the integrity and effectiveness of the final cover system. Vegetation will be established and maintained on the final cover system, including storm drainage areas, where appropriate, to provide long-term erosion control. Established vegetation and the slope design of the final cover system will prevent potential erosion and damage that may be caused by run-on and run-off.

Repair activities may include, but are not limited to, replacing and compacting soil cover, repairing drainage channels that have been eroded, filling in depressions with soil, regrading, and reseeding areas of

failed vegetation, as necessary.

Pursuant to § 257.104(b)(3) and 35 I.A.C. 845.780(b)(3), the groundwater monitoring system will be maintained, and groundwater will be monitored as required by 40 C.F.R. § 257.90 through 40 C.F.R. § 257.98 and 35 I.A.C. 845.600 through 35 I.A.C. 845.680. Monitoring wells will be inspected during each groundwater sampling event. Monitoring wells and associated instrumentation will be maintained so that they perform to the design specifications throughout the life of the monitoring program. Groundwater monitoring frequency will be at least quarterly, except as provided in 40 C.F.R. § 257.94(d), 257.94(c), and 35 I.A.C. 845.650(b)(4).

40 C.F.R. § 257.104(d)(1)(ii) and 35 I.A.C. 845.780(d)(1)(B) – The name, address, telephone number and email address of the person or

telephone number and email address of the person or office to contact about the facility during the post-closure care period.

Illinois Power Generating Company 6555 Sierra Drive Irving, Texas 75039 800.633.4704 ccr@dynegy.com

40 C.F.R. § 257.104(d)(1)(iii) and 35 I.A.C. 845.780(d)(1)(C) – A description of the planned uses of the property during the post-closure period. The CCR unit is located at a retired electric generation facility. Planned uses of the property during the post-closure period are currently unknown, except for post-closure care of the CCR unit.

Post-closure use of the property will not disturb the integrity of the final cover system or other components of the containment system, or the function of the monitoring systems unless necessary to comply with the requirements of 40 C.F.R. Part § 257, Subpart D and 35 I.A.C. Part 845. Any other disturbance will be conducted following a demonstration that it will not increase the potential threat to human health or the environment, as required by 40 C.F.R. § 257.104(d)(1)(iii) and 35 I.A.C. 845.780 (d)(1)(C). The demonstration will be certified by a qualified professional engineer and submitted to the Illinois Environmental Protection Agency (IEPA). Per 40 C.F.R. § 257.104(d)(1)(iii) notification shall be provided to the State Director that the demonstration has been placed in the operating record and on the owners or operator's publicly accessible internet site.

Following closure of the CCR unit, a notation on the deed to the property, or some other instrument that is normally examined during title search, will be recorded in accordance with 40 C.F.R. § 257.102(i) and 35 I.A.C. 845.760(h). The notation will notify potential purchasers of the property that the land has been used as a CCR unit and its use is restricted under the post- closure care requirements in 40 C.F.R. § 257.104(d)(1)(iii) and 35 I.A.C. 845.780(d)(1)(C) or groundwater monitoring requirements per 35 I.A.C. 845.740(b). Within 30 days of recording the deed notation, a notification stating that the notation has been recorded will be submitted to the IEPA and placed in the facility's operating record per 35 I.A.C. 845.760(h)(3). The notification will be placed on the owner or operator's publicly accessible CCR Web site in accordance with 40 C.F.R. § 257.107(i)(9) and 35 I.A.C. 845.810(e) and placed in the facility's operating record as required by 35 I.A.C. 845.800(d)(26) and §257.105(i)(9).

40 C.F.R. § 257.104(d)(3) and 35 I.A.C. 845.780(d)(3) -Pursuant to 40 C.F.R. § 257.104(d), the initial post closure care plan for Amendments to the initial or subsequent written postthe Coffeen GMF Pond was prepared on October 17, 2016. That plan is closure plan. being amended pursuant to 40 C.F.R. § 257.104(d)(3)(i). This plan also serves as the initial post-closure care plan, prepared in accordance with 35 I.A.C. 845.780(d). Pursuant to § 257.104(d)(3) and 35 I.A.C. 845.780(d)(3), an operating permit modification application to amend the initial or any subsequent written post-closure care plan developed under 35 I.A.C. 845.780 (d)(1) and § 257.104(d)(1) will be submitted to IEPA. The written post-closure care plan will be amended whenever there is a change in the operation of the CCR surface impoundment that would substantially affect the written post-closure care plan in effect; or unanticipated events necessitate a revision of the written post-closure care plan, after postclosure activities have started. The written post-closure care plan will be amended at least 60 days before a planned change in the operation of the facility or CCR surface impoundment, or within 60 days after an unanticipated event requires the need to revise the existing plan. If the plan is revised after postclosure activities have started, a request to modify the operating permit, including an amended written post-closure care plan, will be submitted to the IEPA within 30 days following the triggering event. Certification by a qualified professional engineer will be appended to 40 C.F.R. § 257.104(d)(4) and 35 I.A.C. 845.780(d)(4) -Qualified professional engineering certification. this plan and any amendment of this plan. Upon completion of the post-closure period, a request to terminate 35 I.A.C. 845.780(e) – Termination of post-closure care post-closure care will be submitted to the IEPA. The request will include a certification by a qualified professional engineer verifying that postclosure care has been completed in accordance with the post-closure care plan specified in 35 I.A.C. 845.780(d) and the requirements of 35 I.A.C. 845.780. 40 C.F.R. § 257.104(e) and 35 I.A.C. 845.780(f) -A notification of completion of post-closure care will be prepared and Notification of completion of the post-closure care period placed in the facility's operating record within 30 days after IEPA approval of the request to terminate post-closure care. The notification will be placed in the facility's operating record in accordance with 35 I.A.C. 845.800(d)(31) and § 257.105(i)(13). The notification will be placed on the owner or operator's publicly accessible CCR Internet site in accordance with the requirements of § 257.107(i)(13) and 35 I.A.C. 845.810(e). The IEPA will be notified when the notification has been placed in the operating record and on the owner or operator's publicly accessible Internet site in accordance with the requirements of § 257.106(i)(13).

Certification Statement 40 C.F.R. § 257.104 (d)(4) and 35 I.A.C. 845.780(d)(4) – Amended/Initial Written Post Closure Plan for a CCR Surface Impoundment

CCR Unit: Illinois Power Generating Company; Coffeen Power Plant; GMF Pond

I, John R. Hesemann, being a Registered Professional Engineer in good standing in the State of Illinois, do hereby certify, to the best of my knowledge, information, and belief, that the information contained in this certification has been prepared in accordance with the accepted practice of engineering. I certify, for the above referenced CCR Unit, that the information contained in the amended/initial written post closure plan, dated October 30, 2021, meets the requirements of 40 C.F.R. § 257.104 and 35 I.A.C. 845.780.

John R. Hesemann

Printed Name

9/27/2021

Date

ATTACHMENT L

Date: 25 October 2021

Subject: 35 Ill. Admin. Code Part 845 - CCR Liner Design Criteria Demonstration for GMF Pond at

Coffeen Power Plant

Illinois Power Generating Company operates the coal-fired Duck Creek Power Station Plant (Plant) located in Fulton County, Illinois. The GMF Pond is an inactive surface impoundment storing coal combustion residuals (CCR). The requirements for the GMF Pond are found in 35 Ill. Admin. Code (I.A.C.) Part 845 (Part 845).

This liner design criteria demonstration addresses the requirements of Section 845.400 Liner Design Criteria for Existing CCR Surface Impoundments, which states:

Section 845.400 Liner Design Criteria for Existing CCR Surface Impoundments

a) An existing CCR surface impoundment is considered to be an existing lined surface impoundment if it has been constructed with either a composite liner that meets the requirements of subsection (b) or an alternative composite liner that meets the requirements of subsection (c).

b) Composite Liner

1) A composite liner must consist of two components: the upper component consisting of, at a minimum, a 30-mil geomembrane liner, and the lower component consisting of at least a two-foot layer of compacted soil with a hydraulic conductivity of no more than 1 x 10-7 centimeters per second (cm/sec). The geomembrane liner components consisting of high-density polyethylene (HDPE) must be at least 60 mil. The geomembrane liner or upper liner component must be installed in direct and uniform contact with the compacted soil or lower liner component.

2) The composite liner must be:

- A) Constructed of materials that have appropriate chemical properties and sufficient strength and thickness to prevent failure due to pressure gradients (including static head and external hydrogeologic forces), physical contact with the CCR or leachate to which they are exposed, climatic conditions, the stress of installation, and the stress of daily operation;
- B) Constructed of materials that provide appropriate shear resistance of the upper and lower component interface to prevent sliding of the upper component, including on slopes;
- C) Placed upon a foundation or base capable of providing support to the liner and resistance to pressure gradients above and below the liner to prevent failure of the liner due to settlement, compression, or uplift; and

D) Installed to cover all surrounding earth likely to be in contact with the CCR or leachate.

c) Alternative Composite Liner

- 1) An alternative composite liner must consist of two components: the upper component consisting of, at a minimum, a 30-mil geomembrane liner, and a lower component, that is not a geomembrane, with a liquid flow rate no greater than the liquid flow rate of two feet of compacted soil with a hydraulic conductivity of no more than 1 x 10-7 cm/sec. The geomembrane liner components consisting of high-density polyethylene (HDPE) must be at least 60 mil. If the lower component of the alternative liner is compacted soil, the geomembrane liner must be installed in direct and uniform contact with the compacted soil.
- 2) The liquid flow rate through the lower component of the alternative composite liner must be no greater than the liquid flow rate through two feet of compacted soil with a hydraulic conductivity of 1 x 10–7 cm/sec. The hydraulic conductivity for the two feet of compacted soil used in the comparison must be no greater than 1 x 10–7 cm/sec. The hydraulic conductivity of any alternative to the two feet of compacted soil must be determined using recognized and generally accepted methods.
- 3) The liquid flow rate comparison must be made using the following equation, which is derived from Darcy's Law for gravity flow through porous media.

$$Q/A = q = k ((h/t)+1)$$

Where:

 $Q = flow \ rate \ (cubic \ centimeters/second)$

A = Surface area of the liner (squared centimeters)

q = flow rate per unit area (cubic centimeters/second/squared centimeter)

k = hydraulic conductivity of the liner (centimeters /second)

h = hydraulic head above the liner (centimeters); and

t = thickness of the liner (centimeters)

- 4) The alternative composite liner must meet the requirements specified in subsection (b).
- d) The hydraulic conductivity of the compacted soil must be determined using recognized and generally accepted methods.
- e) The owner or operator of an existing CCR surface impoundment that has not completed an Agency approved closure before July 30, 2021 must submit an initial operating permit application under Section 845.230 that demonstrates whether the CCR surface impoundment was constructed with either of the following:

- 1) A composite liner that meets the requirements of subsection (b); or
- 2) An alternative composite liner that meets the requirements of subsection (c).
- h) The owner or operator of the CCR surface impoundment must obtain a certification from a qualified professional engineer attesting that the CCR surface impoundment meets the requirements of subsection (a) and submit the certification to the Agency in the facility's initial operating permit application.

A liner evaluation was conducted for the Coffeen Power Plant GMF Pond in 2016 to evaluate conformance with liner design criteria set forth in 40 C.F.R. § 257.71 and 257.70 (as referenced in 257.71). The evaluation included a review of design drawings, laboratory test data, field test data, construction records and other available information. The evaluation results were documented in the *Liner Evaluation Report — Coffeen GMF Pond*, prepared by Hanson Professional Services Inc. in October 2016. The report demonstrates that the Coffeen Power Plant GMF Pond was constructed with a composite liner, as defined in 40 C.F.R. § 257.70(b), that meets the design criteria set forth in 40 C.F.R. § 257.71 and 257.70 (as referenced in 257.71). The report includes a certification from a qualified professional engineer licensed in the State of Illinois stating that the Coffeen Power Plant GMF Pond meets the applicable requirements set forth in 40 C.F.R. § 257.71.

Because the liner design criteria set forth in Section 845.400 are nearly identical to those presented in 40 C.F.R. § 257.71 and 257.70, the previous liner evaluation report and associated certification demonstrate that the Coffeen Power Plant GMF Pond meets the design criteria for a composite liner set forth in Section 845.400(b). The previous liner evaluation report and associated certification are provided in Attachment L.

I, John R. Hesemann, being a Registered Professional Engineer in good standing in the State of Illinois, do hereby certify, to the best of my knowledge, information, and belief that, based on the information provided in the Coffeen GMF Pond Liner Evaluation Report prepared by Hanson Professional Services Inc., the Coffeen GMF Pond Liner meets the design criteria set forth in 35 I.A.C.845.400(a).

John R. Hesema	1461
Printed Name	
0/18/2021	
Date	

DOS-058523 PILLENSED PROFESSIONAL ENGINEER OF A. Mediministration of the professional and the

Liner Evaluation Report Coffeen GMF Pond

Illinois Power Generating Company Coffeen Power Station Montgomery County, Illinois

October 2016



COFFEEN GMF POND ILLINOIS POWER GENERATING COMPANY LICENSED PROFESSIONAL ENGINEER CERTIFICATION

As a Qualified Professional Engineer as defined by 40 CFR 257.53 and being a Professional Engineer licensed in the State of Illinois, I certify that I have personally examined and am familiar with the information in the Coffeen GMF Pond Liner Evaluation Report and the Coffeen GMF Pond Liner Documentation Report in the operating record and that, based on my inquiry of those individuals immediately responsible for obtaining the information, I believe that the information is true, accurate and complete.

As required by 40 CFR 257.71(b), the Coffeen GMF Pond Liner Evaluation Report and the Coffeen GMF Pond Liner Documentation Report in the operating record are accurate and the <u>Coffeen GMF Pond</u> meets the requirements set forth in 40 CFR 257.71(a)(1)(ii) as published on April 17, 2015.

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1. Summary

The Coffeen GMF Pond (GMF Pond) was constructed to retain wet-sluiced gypsum produced in the flue-gas scrubber at the Coffeen Power Station. Construction of the GMF Pond began in July 2008 and was completed in October 2010. It encompasses about 43.3 acres within the northwest quarter of Section 11, Township 7 North, Range 3 West of the Third Principal Meridian, just north of the Coffeen Power Station.

The composite liner system in the GMF Pond was designed to comply with Illinois Environmental Protection Agency rules for solid waste landfills. Liner system performance criteria is contained in 35 Illinois Administrative Code (IAC) Part 811 as follows:

35 IAC 811.306 Liner Systems

- d) Compacted Earth Liner Standards
 - 1) The minimum allowable thickness shall be 1.52 meters (5 feet).
 - *The liner shall be compacted to achieve a maximum hydraulic conductivity of 1X10*⁻⁷ *centimeters per second.*
 - 3) The construction and compaction of the liner shall be carried out in accordance with the construction quality assurance procedures of Subpart E so as to reduce void spaces and allow the liner to support the loadings imposed by the waste disposal operation without settling that causes or contributes to the failure of the leachate collection system.
 - 4) The liner shall be constructed from materials whose properties are not affected by contact with the constituents of the leachate expected to be produced.
 - 5) Alternative specifications, using standard construction techniques, for hydraulic conductivity and liner thickness may be utilized under the following conditions:
 - A) The liner thickness shall be no less than 1.52 meter (5 feet) unless a composite liner consisting of a geomembrane immediately overlying a compacted earth liner is installed. The following minimum standards shall apply for a composite liner:
 - i) the geomembrane shall be no less than 60 mils in thickness and meet the requirements of subsection (e); and
 - *ii)* the compacted earth liner shall be no less than 0.91 meter in thickness (3 feet) and meet the requirements of subsection (d)(2) through (d)(4).
 - B) The modified liner shall operate in conjunction with a leachate drainage and collection system to achieve equivalent or superior performance to the requirements of this subsection. Equivalent performance shall be evaluated at maximum annual leachate flow conditions.

In accordance with 35 IAC 811.306(d)(5)(A)(i) and (ii), the GMF Pond composite liner was designed and constructed with 3-feet of compacted clay with a maximum permeability of 1 x 10^{-7} cm/sec, overlain by a 60-mil textured HDPE geomembrane.



In April 2015, the United States Environmental Protection Agency published rules at 40 CFR Part 257, Subpart D, regulating the disposal of coal combustion residuals (CCR) in landfills and surface impoundments located in association with electrical utilities utilizing coal as the primary fuel source. Liner system performance criteria for existing CCR surface impoundments is specified in 40 CFR Part 257, Subpart D, as follows:

257.71 Liner design criteria for existing CCR surface impoundments.

- (a)(1) No later than October 17, 2016, the owner or operator of an existing CCR surface impoundment must document whether or not such unit was constructed with any one of the following:
 - (i) A liner consisting of a minimum of two feet of compacted soil with a hydraulic conductivity of no more than 1×10^{-7} cm/sec;
 - (ii) A composite liner that meets the requirements of §257.70(b); or
 - (iii) An alternative composite liner that meets the requirements of $\S 257.70(c)$.
 - (2) The hydraulic conductivity of the compacted soil must be determined using recognized and generally accepted methods.
 - (3) An existing CCR surface impoundment is considered to be an existing unlined CCR surface impoundment if either:
 - (i) The owner or operator of the CCR unit determines that the CCR unit is not constructed with a liner that meets the requirements of paragraphs (a)(1)(i), (ii), or (iii) of this section; or
 - (ii) The owner or operator of the CCR unit fails to document whether the CCR unit was constructed with a liner that meets the requirements of paragraphs (a)(1)(i), (ii), or (iii) of this section.
 - (4) All existing unlined CCR surface impoundments are subject to the requirements of \$257.101(a).
- (b) The owner or operator of the CCR unit must obtain a certification from a qualified professional engineer attesting that the documentation as to whether a CCR unit meets the requirements of paragraph (a) of this section is accurate.
- (c) The owner or operator of the CCR unit must comply with the recordkeeping requirements specified in §257.105(f), the notification requirements specified in §257.106(f), and the Internet requirements specified in §257.107(f).

As referenced in 40 CFR 257.71(a)(1)(ii), 40 CFR 257.70(b), Design criteria for new CCR landfills and any lateral expansion of a CCR landfill, provides:

- (b) A composite liner must consist of two components; the upper component consisting of, at a minimum, a 30-mil geomembrane liner (GM), and the lower component consisting of at least a two-foot layer of compacted soil with a hydraulic conductivity of no more than 1×10^{-7} centimeters per second (cm/sec). GM components consisting of high density polyethylene (HDPE) must be at least 60-mil thick. The GM or upper liner component must be installed in direct and uniform contact with the compacted soil or lower liner component. The composite liner must be:
 - (1) Constructed of materials that have appropriate chemical properties and sufficient strength and thickness to prevent failure due to pressure gradients (including static head and external hydrogeologic forces), physical contact with the CCR or leachate to which they are exposed, climatic conditions, the stress of installation, and the stress of daily operation;



- (2) Constructed of materials that provide appropriate shear resistance of the upper and lower component interface to prevent sliding of the upper component including on slopes;
- (3) Placed upon a foundation or base capable of providing support to the liner and resistance to pressure gradients above and below the liner to prevent failure of the liner due to settlement, compression, or uplift; and
- (4) Installed to cover all surrounding earth likely to be in contact with the CCR or leachate.

In accordance with 40 CFR 257.70(b)(1) above, the composite liner in the Coffeen GMF Pond was constructed of materials that have appropriate chemical properties and sufficient strength and thickness to prevent failure due to pressure gradients (including static head and external hydrogeologic forces), physical contact with the CCR or leachate to which they are exposed, climatic conditions, the stress of installation, and the stress of daily operation. This information is located in Section 4 and Section 5 of the Coffeen GMF Pond Liner Documentation Report located in the operating record.

In accordance with 40 CFR 257.70(b)(2) above, the composite liner in the Coffeen GMF Pond was constructed of materials that provide appropriate shear resistance of the upper and lower component interface to prevent sliding of the upper component, including on slopes. This information is located in Section 4 of the Coffeen GMF Pond Liner Documentation Report located in the operating record.

In accordance with 40 CFR 257.70(b)(3) above, the composite liner in the Coffeen GMF Pond was placed upon a foundation or base capable of providing support to the liner and resistance to pressure gradients above and below the liner to prevent failure of the liner due to settlement, compression, or uplift. This information is located in Section 4 of the Coffeen GMF Pond Liner Documentation Report located in the operating record.

In accordance with 40 CFR 257.70(b)(4) above, the composite liner in the Coffeen GMF Pond was installed to cover all surrounding earth likely to be in contact with the CCR or leachate. This information is located in the "Geosynthetics Quality Assurance Report, Gypsum Stack, AERG (Ameren) Coffeen Power Station, Coffeen, Montgomery County, Illinois. Feezor Engineering, Inc." in the Coffeen GMF Pond Liner Documentation Report located in the operating record.

The purpose of this report is to document that the existing composite liner in the Coffeen GMF Pond meets the minimum requirements of 40 CFR 257.71(a)(1)(ii), i.e., a composite liner that meets the requirements of §257.70(b).

Briefly, in comparison:

35 IAC 811.306(d)(5)(A) requires a compacted clay liner of at least three (3) feet in thickness with a hydraulic conductivity of less than $1x10^{-7}$ cm/sec, overlain by a geomembrane of no less than 60-mils in thickness.

40 CFR 257.71(a)(1)(ii) requires a compacted soil liner of at least two (2) feet in thickness with a hydraulic conductivity of no more than $1x10^{-7}$ cm/sec, overlain by a geomembrane of no less than 30-mils in thickness. If the geomembrane is composed of high-density polyethylene (HDPE), the geomembrane must be a minimum of 60-mils in thickness.

Thus, the composite liner in the Coffeen GMF Pond was designed and constructed with components exceeding the minimum required in 40 CFR 257.71(a)(1)(ii).



The major components of the construction and Construction Quality Assurance (CQA) of the GMF Pond liner are discussed in the sections below.

2. Foundation Preparation

Foundation preparation consisted of removing the soil to foundation grade in the bottom of the GMF Pond, construction of the perimeter berm containing the GMF Pond, and surveying and certifying the foundation surface.

The construction contractor initially stripped topsoil below the root zone within the GMF Pond and its perimeter berm location.

The bottom of the GMF Pond was excavated to and into the Vandalia Till. The excavated till was used to raise portions of the GMF Pond bottom to foundation grade sloping from the northwest corner to the south and east. The foundation surface was proof rolled and visually observed. Soils in undercut areas were removed and then backfilled with excavated till from the GMF Pond.

Nuclear moisture/density gauge testing was performed during foundation backfill placement at a minimum rate of one test per 10,000 yd³ (and a minimum one test per compacted lift). The material was placed in approximately 8-inch lifts and compacted with a Cat 815 sheepsfoot compactor and/or a smooth drum roller to at least 95% Standard Proctor maximum dry density with moisture contents of -2% to +2% of optimum. Areas that showed deficiencies in compaction or moisture content were reworked or removed, and then compacted with a Cat 815 sheepsfoot compactor and/or a smooth drum roller and tested for moisture/density. Nuclear moisture/density and Standard Proctor test results are included in Appendix D in the Coffeen GMF Pond Liner Documentation Report located in the operating record.

Finished foundation grades were verified by survey and certified (Appendices I and J in the Coffeen GMF Pond Liner Documentation Report located in the operating record), and CQA certifications of completion were provided by the CQA Officer.

3. Berm Construction

The perimeter berm was constructed with structural fill excavated and hauled directly from the GMF Pond and other borrow areas within the facility. Nuclear moisture/density gauge testing was performed during berm construction at a minimum rate of one test per 10,000 yd³ (and a minimum one test per compacted lift). Fill was placed in approximately 8-inch lifts and compacted with a Cat 815 or 825 sheepsfoot compactor to at least 95% Standard Proctor maximum dry density with moisture contents of -2% to +2% of optimum. Areas that showed deficiencies in compaction or moisture content were reworked or removed, and then compacted with a Cat 815 or 825 sheepsfoot compactor and tested for moisture/density. Nuclear moisture/density and Standard Proctor test results are included in Appendix D in the Coffeen GMF Pond Liner Documentation Report located in the operating record.

Four (4) Shelby tube samples (ST-040 through ST-043) were collected from the perimeter berm, and were delivered to Hanson's Geotechnical Laboratory in Springfield, Illinois. A soil sample from each tube was tested to determine in-place hydraulic conductivity (permeability). Table 3.1 summarizes the hydraulic conductivity test results.



Table 3.1 Laboratory Permeability Test Results of Perimeter Berm

Sample No.	Location	Hydraulic Conductivity, (ky)
ST-040	East Berm	1.5 x 10 ⁻⁹ cm/sec
ST-041	North Berm	1.9 x 10 ⁻⁹ cm/sec
ST-042	West Berm	5.4 x 10 ⁻⁹ cm/sec
ST-043	South Berm	4.7 x 10 ⁻⁹ cm/sec

4. Test Soil Liner

Prior to construction of the full scale soil liner, one test soil liner was constructed to the west of Landfill Cell L1 using one borrow source. Landfill Cell L1 is located directly west of the GMF Pond. The purpose of the test soil liner was to verify that the material and methods of construction proposed for the full scale soil liner would provide the appropriate permeability performance criteria required by 35 IAC 811.306(d).

The borrow material for the test soil liner was obtained from clay material excavated from the footprint of Landfill Cell L1 that was stockpiled to the west of Cell L1 ("West Landfill Liner Stockpile"). The clay material from the West Landfill Liner Stockpile was determined to be the same material that was to be used to construct the full scale soil liner in the GMF Pond. Therefore, the single test liner was utilized for both Landfill Cell L1 and the GMF Pond. In accordance with 35 IAC 811.306(d)(2), the material selected had sufficient fines to achieve an in-place hydraulic conductivity of 1.0 x 10⁻⁷ cm/sec or less.

4.1 Prequalification of Borrow Material

While no prequalification testing is required by the regulations, a program of laboratory testing was carried out on typical samples of the material used for the construction of the test soil liner and the GMF Pond's full scale soil liner. Tables 4.1 and 4.2 summarize the borrow material properties determined from testing (Appendix E.1 in the Coffeen GMF Pond Liner Documentation Report located in the operating record contains laboratory test reports).

Table 4.1 Properties of Borrow Material

Sample No.	Percent of Fines	Liquid Limit	Plasticity Index	Maximum Dry Density (MDD)	Optimum Moisture Content (OMC)
West Landfill Line	er Stockpile				
1 L1 Liner	71.0%	36%	22%	110.5 pcf	16.1%
GMF Pond Liner	Stockpile				
SP-002-001	76.7%	40%	25%	110.8 pcf	15.9%
GMF Pond Test P	its (combined sa	mples)			
DP-13 2'-6'	96.9%	54%	35%	98.3 pcf	22.1%
DP-13 4'-8'	85.5%	41%	25%	106.5 pcf	17.4%
DP-21 2'-6'	93.6%	48%	30%	100.0 pcf	19.7%
DP-21 4'-8'	86.2%	41%	25%	105.4 pcf	17.9%
West Borrow Pit	90.1%	41%	23%	103.4 pcf	16.5%



Table 4.2 Constant Head Permeability Test Results of Borrow Material

Sample No.	Dry Density	Moisture Content	Permeability Result
West Landfill Liner Stockpile			
1 L1 Liner	95% of MDD	1.5% West of OMC	5.0 x 10 ⁻⁹ cm/sec
GMF Pond Test Pits (combine	ed samples)		
DP-13 2'-6'	95% of MDD	2.0% Wet of OMC	1.1 x 10 ⁻⁸ cm/sec
DP-13 4'-8'	95% of MDD	0.5% Wet of OMC	4.1 x 10 ⁻⁸ cm/sec
DP-21 2'-6'	95% of MDD	1.5% Wet of OMC	9.1 x 10 ⁻⁹ cm/sec
DP-21 4'-8'	95% of MDD	1.5% Wet of OMC	8.8 x 10 ⁻⁹ cm/sec
West Borrow Pit	95% of MDD	1.0% Wet of OMC	7.9 x 10 ⁻⁸ cm/sec

Based on the test results summarized in Tables 4.1 and 4.2, the field compaction criteria for the soil liner were set as follows to assure that a field permeability value of 1.0×10^{-7} cm/sec or less could be achieved:

Dry Density ≥ 95% MDD Moisture Content = 0% to 5% Wet of OMC

4.2 Test Soil Liner Construction

The construction of the test soil liner was initiated on May 20, 2009. The foundation was proof rolled and visually observed for soft areas or unsuitable soils. No material removal or backfilling was required.

The test soil liner was constructed west of Landfill Cell L1 with material from the West Landfill Liner Stockpile. The material was keyed into the foundation material and compacted with a Cat 815 sheepsfoot compactor in approximately 8-inch lifts. Nuclear moisture/density gauge testing was performed at a minimum rate of one test per acre per lift or one test per 1,000 yd³ to 95% Standard Proctor MDD with moisture contents of optimum to +5% (as determined in Section 4.1 above). Areas that showed deficiencies in compaction or moisture content were reworked or removed, and then compacted with a Cat 815 sheepsfoot compactor and tested for moisture/density. Nuclear moisture/density test results and a drawing showing the testing locations are included in Appendix E.3 in the Coffeen GMF Pond Liner Documentation Report located in the operating record. The Standard Proctor test report is included in Appendix E.4 in the Coffeen GMF Pond Liner Documentation Report located in the operating record.

On May 22, 2009, the test soil liner was completed after it was smooth drum rolled and then covered with a clear plastic covering to prevent moisture loss.

4.2.1 Sampling and Testing Programs for Test Soil Liner

Pursuant to the CQA Plan, the test soil liner was sampled and tested for physical properties utilizing both laboratory and field testing. Required were at least five (5) two-stage field tests to determine the hydraulic conductivity (both vertical and horizontal hydraulic conductivity were calculated), and at least two (2) undisturbed Shelby tube samples tested in the laboratory for vertical hydraulic conductivity to determine a statistical correlation with the field tests.



4.2.1.1 Laboratory Testing

Two (2) Shelby tube samples were collected from the test soil liner on May 22, 2009. ST-01 and ST-02 were delivered to Hanson's Geotechnical Laboratory for laboratory permeability testing. A sample from each tube was tested for particle size analysis, Atterberg limits, and hydraulic conductivity. Table 4.3 summarizes the test results (Appendix E.5 in the Coffeen GMF Pond Liner Documentation Report located in the operating record contains laboratory test reports and Shelby tube sample locations).

Table 4.3 Results of Laboratory Testing Program for Test Soil Liner

Sample No.	Percent of Fines	Liquid Limit	Plasticity Index	Hydraulic Conductivity, ky	Pass / Fail	Lift
ST-01	75.1%	39.2%	25.2%	2.0 x 10 ⁻⁹ cm/sec	Pass	5
ST-02	76.0%	40.8%	26.9%	1.8 x 10 ⁻⁹ cm/sec	Pass	5

4.2.1.2 Field Testing

Six (6) two-stage (Boutwell) field permeability tests were conducted on the test soil liner. The tests were carried out to measure the limiting values of field saturated hydraulic conductivity. The first stage of the test measured the vertical component of hydraulic conductivity (k_V) while the second stage measured the horizontal component (k_h). Table 4.4 summarizes the Boutwell test results (Appendix E.6 in the Coffeen GMF Pond Liner Documentation Report located in the operating record contains Boutwell testing locations and the report of the field testing results).

Table 4.4 Results of Field Testing Program for Test Soil Liner

Boutwell Test No.	Stage 1 Hydraulic Conductivity, kv	Stage 2 Hydraulic Conductivity, kh
1	1.31 x 10 ⁻⁸ cm/sec	6.33 x 10 ⁻⁹ cm/sec
2	5.55 x 10 ⁻⁸ cm/sec	6.18 x 10 ⁻⁸ cm/sec
3	1.09 x 10 ⁻⁸ cm/sec	5.74 x 10 ⁻⁹ cm/sec
4	9.13 x 10 ⁻⁸ cm/sec	1.94 x 10 ⁻⁸ cm/sec
5	$1.02 \times 10^{-8} \text{ cm/sec}$	$3.34 \times 10^{-9} \text{ cm/sec}$
6	1.19 x 10 ⁻⁸ cm/sec	$7.37 \times 10^{-9} \text{ cm/sec}$

4.2.1.3 CQA Certification of Test Soil Liner

Based on the laboratory and field test results, the CQA Officer certified the construction of the test soil liner and that construction of the full scale soil liner could begin using the same soils and procedures used to construct the test liner.



5. Full Scale Soil Liner Construction

Construction of the full scale soil liner began on April 14, 2010 in the northwestern section of the GMF Pond and continued to the south and east. The soil material from the GMF Pond Liner Stockpile and the West Borrow Pit were used to construct the 3-ft thick soil liner within the GMF Pond. Like the test soil liner, the soil was placed in approximately 8-inch thick lifts and compacted with a Cat 815 sheepsfoot compactor. Nuclear moisture/density gauge testing was performed at a minimum rate of one test per acre per lift or one test per 1,000 yd³ to 95% Standard Proctor MDD with moisture contents of optimum to +5%. Areas that showed deficiencies in compaction or moisture content were reworked or removed, and then compacted with a Cat 815 sheepsfoot compactor and tested for moisture/density. Nuclear moisture/density and Standard Proctor test results are included in Appendix F in the Coffeen GMF Pond Liner Documentation Report located in the operating record.

Twenty-one (21) Shelby tube samples were collected at various stages of the liner construction. The hydraulic conductivity of the compacted soil liner was sampled and laboratory tested in accordance with ASTM D5084. The samples were delivered to Hanson's Geotechnical Laboratory for permeability testing. A sample from each tube was tested to determine hydraulic conductivity. The sample intervals included at least one permeability test from each of the five liner construction lifts. Table 5.1 summarizes the hydraulic conductivity test results, which are all less than the 1.0 x 10⁻⁷ cm/sec performance criterion. Permeability test results and a drawing showing Shelby tube sample locations are included in Appendix F.3 in the Coffeen GMF Pond Liner Documentation Report located in the operating record.

Table 5.1 Laboratory Permeability Test Results of Soil Liner

Sample		Pass /	
No.	Hydraulic Conductivity, (kv)	Fail	Lift
ST-019	2.1 x 10 ⁻⁹ cm/sec	Pass	2
ST-020	4.3 x 10 ⁻⁹ cm/sec	Pass	1
ST-021	$3.8 \times 10^{-9} \text{ cm/sec}$	Pass	4
ST-022	1.8 x 10 ⁻⁹ cm/sec	Pass	4
ST-023	$3.8 \times 10^{-9} \text{ cm/sec}$	Pass	5
ST-024	4.0 x 10 ⁻⁹ cm/sec	Pass	1
ST-025	$2.7 \times 10^{-9} \text{ cm/sec}$	Pass	3
ST-026	9.6 x 10 ⁻⁹ cm/sec	Pass	3
ST-027	7.4 x 10 ⁻¹⁰ cm/sec	Pass	4
ST-028	2.7 x 10 ⁻⁹ cm/sec	Pass	2
ST-029	2.7 x 10 ⁻⁹ cm/sec	Pass	5
ST-030	$6.0 \times 10^{-9} \text{ cm/sec}$	Pass	3
ST-031	$6.7 \times 10^{-9} \text{ cm/sec}$	Pass	2
ST-032	4.2 x 10 ⁻⁹ cm/sec	Pass	5
ST-033	2.4 x 10 ⁻⁸ cm/sec	Pass	4
ST-034	$4.5 \times 10^{-9} \text{ cm/sec}$	Pass	4
ST-035	5.3 x 10 ⁻⁹ cm/sec	Pass	5
ST-036	1.1 x 10 ⁻⁸ cm/sec	Pass	3
ST-037	$3.0 \times 10^{-9} \text{ cm/sec}$	Pass	1
ST-038	3.8 x 10 ⁻⁹ cm/sec	Pass	1
ST-039	2.7 x 10 ⁻⁹ cm/sec	Pass	2



Base grades and completed liner elevations were surveyed to ensure construction to the design grades and to verify minimum soil liner thickness. Record drawings and certified survey data are included in Appendices I and J, respectively, in the Coffeen GMF Pond Liner Documentation Report located in the operating record. After the soil liner was smooth drum rolled, CQA certifications of its construction and grades were provided by the CQA Officer prior to installation of the 60 mil HDPE geomembrane liner.

6. HDPE Geomembrane Liner Installation

Prior to installation of the 60-mil textured HDPE geomembrane liner on the floor and side slopes of the GMF Pond, the surface of the full scale soil liner was accepted by the geomembrane installer. Installation of the geomembrane liner began on July 24, 2010, and placement continued through September 17, 2010, with liner installation testing and repairs completed by September 22, 2010. Textured geomembrane liner was used on the GMF Pond floor and side slopes.

On the side slopes, the HDPE geomembrane was laid parallel to the slope and the tie-in weld with the bottom geomembrane was welded a minimum of 5 feet past the toe of the slope. Adjacent panels were overlapped approximately 4-6 inches and were shingled in the direction of the drainage.

Production seaming of the geomembrane panels was made using a dual hot wedge fusion welder. This device creates an air channel between two fused seams that can later be tested with pressurized air to assure there is no leakage. Destruct sample sites and repairs were welded with extrusion welds which were checked for leaks with a vacuum box. All seams were sampled at a rate of one destruct sample for every 500 feet of seam and tested for strength parameters in the laboratory.

A third party engineering firm monitored the installation of all geosynthetic materials and assembled the manufacturing quality control (MQC) data, manufacturing quality assurance (MQA) testing data, installer subgrade acceptance, panel placement information, laboratory CQA test data from destruct samples, and field CQA test data for seam welding integrity. All of this data is included in the "Geosynthetics Quality Assurance Report, Gypsum Stack, AERG (Ameren) Coffeen Power Station, Coffeen, Montgomery County, Illinois. Feezor Engineering, Inc." in the Coffeen GMF Pond Liner Documentation Report located in the operating record.

ATTACHMENT M

HISTORY OF POTENTIAL EXCEEDANCES

This presentation of the History of Potential Exceedances, and any corrective action taken to remediate groundwater, is provided to meet the requirements of Title 35 of the Illinois Administrative Code (35 I.A.C.) § 845.230(d)(2)(M) for the Coffeen Power Plant GMF Gypsum Stack Pond, Illinois Environmental Protection Agency (IEPA) ID No. W1350150004-03.

Note

Groundwater concentrations from 2015 to 2021 presented in the Hydrogeologic Site Characterization Report (HCR) Table 4-1, and evaluated and summarized in the following tables, are considered potential exceedances because the methodology used to determine them is proposed in the Statistical Analysis Plan (Appendix A to Groundwater Monitoring Plan [GMP]), which has not been reviewed or approved by IEPA at the time of submittal of the 35 I.A.C. § 845 Operating Permit application.

Alternate sources for potential exceedances as allowed by 35 I.A.C. § 845.650(e) have not yet been evaluated. These will be evaluated and presented in future submittals to IEPA as appropriate.

Table 1 summarizes how the potential exceedances were determined.

Background Concentrations

Background monitoring wells identified in the GMP include G200 and R201.

For monitoring wells that have been historically monitored in accordance with Title 40, Code of Federal Regulations, Part 257, Subpart D (Standards for the Disposal of Coal Combustion Residuals in Landfills and Surface Impoundments), background concentrations calculated from sampling events in 2015-2017 were compared to the standards identified in 35 I.A.C. § 845.600(a)(1). For constituents with calculated background concentrations in 2015-2017 greater than the standards in 35 I.A.C. § 845.600(a)(1), those calculated background concentrations were used as Groundwater Protection Standards (GWPSs) for comparing to statistical calculation results for each compliance well to determine potential exceedances. Compliance well statistical calculations consider concentrations from all sampling events in 2015-2021.

For all other monitoring wells, either newly constructed in 2021 or existing wells not monitored under Title 40, Code of Federal Regulations, Part 257, Subpart D, background concentrations calculated from the eight sampling events required by 35 I.A.C. § 845.650(b)(1)(A), to be collected within 180 days from April 21, 2021, were compared to the standards identified in 35 I.A.C. § 845.600(a)(1). For constituents with calculated background concentrations greater than the standards in 35 I.A.C. § 845.600(a)(1), those calculated background concentrations were used as GWPSs. Compliance well statistical calculations from that same time period were compared to the GWPSs to determine potential exceedances.

Corrective Action

No corrective actions have been taken to remediate the groundwater.

COF GMF GSP HPE FINAL 10.17.2021 1/1

Sample Location	HSU	Program	Constituent	Result Unit	Sample Date Range	Statistical Calculation	Statistical Result	GWPS	Background	Part 845 Standard	GWPS Source
G102	UA	845	Antimony, total	mg/L	04/08/2015 - 07/09/2017	All ND - Last	0.003	0.006	0.003	0.006	Standard
G102	UA	845	Arsenic, total	mg/L	04/08/2015 - 07/09/2017	CI around median	0.001	0.011	0.011	0.01	Background
G102	UA	845	Barium, total	mg/L	04/08/2015 - 07/09/2017	CI around mean	0.037	2.0	0.13	2	Standard
G102	UA	845	Beryllium, total	mg/L	04/08/2015 - 07/09/2017	All ND - Last	0.001	0.004	0.001	0.004	Standard
G102	UA	845	Boron, total	mg/L	04/08/2015 - 01/26/2021	CI around median	0.010	2.0	0.11	2	Standard
G102	UA	845	Cadmium,total	mg/L	04/08/2015 - 07/09/2017	All ND - Last	0.001	0.005	0.001	0.005	Standard
G102	UA	845	Chloride, total	mg/L	04/08/2015 - 01/26/2021	CI around mean	19	200	95	200	Standard
G102	UA	845	Chromium, total	mg/L	04/08/2015 - 07/09/2017	CI around geomean	0.00437	0.10	0.0096	0.1	Standard
G102	UA	845	Cobalt, total	mg/L	04/08/2015 - 07/09/2017	CI around median	0.002	0.006	0.0037	0.006	Standard
G102	UA	845	Fluoride, total	mg/L	04/08/2015 - 01/26/2021	CI around mean	0.30	4.0	0.55	4	Standard
G102	UA	845	Lead, total	mg/L	04/08/2015 - 07/09/2017	CI around median	0.001	0.0075	0.0059	0.0075	Standard
G102	UA	845	Lithium, total	mg/L	11/16/2015 - 07/09/2017	CI around median	0.010	0.040	0.020	0.04	Standard
G102	UA	845	Mercury, total	mg/L	04/08/2015 - 07/09/2017	CI around median	0.0002	0.002	0.0011	0.002	Standard
G102	UA	845	Molybdenum, total	mg/L	07/23/2015 - 07/09/2017	CI around mean	0.000974	0.10	0.044	0.1	Standard
G102	UA	845	pH (field)	SU	01/20/2015 - 01/26/2021	CI around median	7.1	6.5/9.0	6.8/7.4	6.5/9	Standard/Standard
G102	UA	845	Radium-226 + Radium 228, tot	pCi/L	11/16/2015 - 07/09/2017	CI around mean	0.42	5.0	1.5	5	Standard
G102	UA	845	Selenium, total	mg/L	04/08/2015 - 07/09/2017	CI around mean	0.0014	0.050	0.0035	0.05	Standard
G102	UA	845	Sulfate, total	mg/L	04/08/2015 - 01/26/2021	CI around mean	73	400	387	400	Standard
G102	UA	845	Thallium, total	mg/L	04/08/2015 - 07/09/2017	All ND - Last	0.001	0.002	0.001	0.002	Standard
G102	UA	845	Total Dissolved Solids	mg/L	01/20/2015 - 01/26/2021	CI around mean	389	1200	975	1200	Standard
G103	UA	845	Antimony, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.003	0.006	0.003	0.006	Standard
G103	UA	845	Arsenic, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.0012	0.011	0.011	0.01	Background
G103	UA	845	Barium, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.10	2.0	0.13	2	Standard
G103	UA	845	Beryllium, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.001	0.004	0.001	0.004	Standard
G103	UA	845	Boron, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.010	2.0	0.11	2	Standard
G103	UA	845	Cadmium,total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.001	0.005	0.001	0.005	Standard



Sample Location	HSU	Program	Constituent	Result Unit	Sample Date Range	Statistical Calculation	Statistical Result	GWPS	Background	Part 845 Standard	GWPS Source
G103	UA	845	Chloride, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	66	200	95	200	Standard
G103	UA	845	Chromium, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.019	0.10	0.0096	0.1	Standard
G103	UA	845	Cobalt, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.002	0.006	0.0037	0.006	Standard
G103	UA	845	Fluoride, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.25	4.0	0.55	4	Standard
G103	UA	845	Lead, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.001	0.0075	0.0059	0.0075	Standard
G103	UA	845	Mercury, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.0002	0.002	0.0011	0.002	Standard
G103	UA	845	Molybdenum, total	mg/L	07/23/2015 - 10/06/2015	Most recent sample	0.0034	0.10	0.044	0.1	Standard
G103	UA	845	pH (field)	SU	01/20/2015 - 10/06/2015	CI around mean	6.9	6.5/9.0	6.8/7.4	6.5/9	Standard/Standard
G103	UA	845	Selenium, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.001	0.050	0.0035	0.05	Standard
G103	UA	845	Sulfate, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	69	400	387	400	Standard
G103	UA	845	Thallium, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.001	0.002	0.001	0.002	Standard
G103	UA	845	Total Dissolved Solids	mg/L	01/20/2015 - 10/06/2015	CI around mean	353	1200	975	1200	Standard
G105	UA	845	Antimony, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.003	0.006	0.003	0.006	Standard
G105	UA	845	Arsenic, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.0013	0.011	0.011	0.01	Background
G105	UA	845	Barium, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.078	2.0	0.13	2	Standard
G105	UA	845	Beryllium, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.001	0.004	0.001	0.004	Standard
G105	UA	845	Boron, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.13	2.0	0.11	2	Standard
G105	UA	845	Cadmium,total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.001	0.005	0.001	0.005	Standard
G105	UA	845	Chloride, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	37	200	95	200	Standard
G105	UA	845	Chromium, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.0046	0.10	0.0096	0.1	Standard
G105	UA	845	Cobalt, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.002	0.006	0.0037	0.006	Standard
G105	UA	845	Fluoride, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.30	4.0	0.55	4	Standard
G105	UA	845	Lead, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.0011	0.0075	0.0059	0.0075	Standard
G105	UA	845	Mercury, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.0002	0.002	0.0011	0.002	Standard
G105	UA	845	Molybdenum, total	mg/L	07/23/2015 - 10/06/2015	Most recent sample	0.0028	0.10	0.044	0.1	Standard
G105	UA	845	pH (field)	SU	01/20/2015 - 10/06/2015	CI around mean	6.7	6.5/9.0	6.8/7.4	6.5/9	Standard/Standard



Sample Location	HSU	Program	Constituent	Result Unit	Sample Date Range	Statistical Calculation	Statistical Result	GWPS	Background	Part 845 Standard	GWPS Source
G105	UA	845	Selenium, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.0011	0.050	0.0035	0.05	Standard
G105	UA	845	Sulfate, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	110	400	387	400	Standard
G105	UA	845	Thallium, total	mg/L	04/08/2015 - 10/06/2015	Most recent sample	0.001	0.002	0.001	0.002	Standard
G105	UA	845	Total Dissolved Solids	mg/L	01/20/2015 - 10/06/2015	CI around mean	486	1200	975	1200	Standard
G106	UA	845	Antimony, total	mg/L	04/08/2015 - 07/09/2017	All ND - Last	0.003	0.006	0.003	0.006	Standard
G106	UA	845	Arsenic, total	mg/L	04/08/2015 - 07/09/2017	CB around linear reg	0.00106	0.011	0.011	0.01	Background
G106	UA	845	Barium, total	mg/L	04/08/2015 - 07/09/2017	CI around mean	0.053	2.0	0.13	2	Standard
G106	UA	845	Beryllium, total	mg/L	04/08/2015 - 07/09/2017	All ND - Last	0.001	0.004	0.001	0.004	Standard
G106	UA	845	Boron, total	mg/L	04/08/2015 - 01/26/2021	CB around linear reg	0.020	2.0	0.11	2	Standard
G106	UA	845	Cadmium,total	mg/L	04/08/2015 - 07/09/2017	All ND - Last	0.001	0.005	0.001	0.005	Standard
G106	UA	845	Chloride, total	mg/L	04/08/2015 - 01/26/2021	CB around linear reg	42	200	95	200	Standard
G106	UA	845	Chromium, total	mg/L	04/08/2015 - 07/09/2017	CI around median	0.004	0.10	0.0096	0.1	Standard
G106	UA	845	Cobalt, total	mg/L	04/08/2015 - 07/09/2017	CI around median	0.002	0.006	0.0037	0.006	Standard
G106	UA	845	Fluoride, total	mg/L	04/08/2015 - 01/26/2021	CI around mean	0.40	4.0	0.55	4	Standard
G106	UA	845	Lead, total	mg/L	04/08/2015 - 07/09/2017	All ND - Last	0.001	0.0075	0.0059	0.0075	Standard
G106	UA	845	Lithium, total	mg/L	11/17/2015 - 07/09/2017	All ND - Last	0.010	0.040	0.020	0.04	Standard
G106	UA	845	Mercury, total	mg/L	04/08/2015 - 07/09/2017	All ND - Last	0.0002	0.002	0.0011	0.002	Standard
G106	UA	845	Molybdenum, total	mg/L	07/23/2015 - 07/09/2017	CI around mean	0.00145	0.10	0.044	0.1	Standard
G106	UA	845	pH (field)	SU	01/20/2015 - 06/29/2021	CI around mean	7.0	6.5/9.0	6.8/7.4	6.5/9	Standard/Standard
G106	UA	845	Radium-226 + Radium 228, tot	pCi/L	11/17/2015 - 07/09/2017	CI around mean	0.28	5.0	1.5	5	Standard
G106	UA	845	Selenium, total	mg/L	04/08/2015 - 07/09/2017	CI around mean	0.00108	0.050	0.0035	0.05	Standard
G106	UA	845	Sulfate, total	mg/L	04/08/2015 - 01/26/2021	CB around linear reg	66	400	387	400	Standard
G106	UA	845	Thallium, total	mg/L	04/08/2015 - 07/09/2017	All ND - Last	0.001	0.002	0.001	0.002	Standard
G106	UA	845	Total Dissolved Solids	mg/L	01/20/2015 - 01/26/2021	CI around mean	416	1200	975	1200	Standard
G206	UA	257	Antimony, total	mg/L	04/09/2015 - 07/15/2017	All ND - Last	0.003	0.006	0.003	0.006	Standard
G206	UA	257	Arsenic, total	mg/L	01/21/2015 - 07/15/2017	CI around median	0.001	0.010	0.010	0.01	Standard



Sample Location		Program	Constituent	Result Unit	Sample Date Range	Statistical Calculation	Statistical Result	GWPS	Background	Part 845 Standard	GWPS Source
G206	UA	257	Barium, total	mg/L	04/09/2015 - 07/15/2017	CI around mean	0.048	2.0	0.27	2	Standard
G206	UA	257	Beryllium, total	mg/L	04/09/2015 - 07/15/2017	All ND - Last	0.001	0.0067	0.0067	0.004	Background
G206	UA	257	Boron, total	mg/L	01/21/2015 - 01/27/2021	CI around median	0.010	2.0	0.39	2	Standard
G206	UA	257	Cadmium,total	mg/L	01/21/2015 - 07/15/2017	All ND - Last	0.001	0.005	0.0012	0.005	Standard
G206	UA	257	Chloride, total	mg/L	01/21/2015 - 01/27/2021	CI around mean	25	200	96	200	Standard
G206	UA	257	Chromium, total	mg/L	04/09/2015 - 07/15/2017	CI around median	0.004	0.10	0.013	0.1	Standard
G206	UA	257	Cobalt, total	mg/L	04/09/2015 - 07/15/2017	All ND - Last	0.002	0.0074	0.0074	0.006	Background
G206	UA	257	Fluoride, total	mg/L	04/09/2015 - 01/27/2021	CI around median	0.39	4.0	0.50	4	Standard
G206	UA	257	Lead, total	mg/L	01/21/2015 - 07/15/2017	Future median	0.001	0.018	0.018	0.0075	Background
G206	UA	257	Lithium, total	mg/L	11/18/2015 - 07/15/2017	All ND - Last	0.010	0.040	0.021	0.04	Standard
G206	UA	257	Mercury, total	mg/L	04/09/2015 - 07/15/2017	All ND - Last	0.0002	0.002	0.0002	0.002	Standard
G206	UA	257	Molybdenum, total	mg/L	07/22/2015 - 07/15/2017	CI around mean	0.00108	0.10	0.0069	0.1	Standard
G206	UA	257	pH (field)	SU	01/21/2015 - 01/27/2021	CI around median	7.0	6.5/9.0	6.9/7.3	6.5/9	Standard/Standard
G206	UA	257	Radium-226 + Radium 228, tot	pCi/L	11/18/2015 - 07/15/2017	CI around mean	0.34	9.8	9.8	5	Background
G206	UA	257	Selenium, total	mg/L	04/09/2015 - 07/15/2017	CI around median	0.001	0.050	0.0097	0.05	Standard
G206	UA	257	Sulfate, total	mg/L	01/21/2015 - 01/27/2021	CI around median	110	400	300	400	Standard
G206	UA	257	Thallium, total	mg/L	04/09/2015 - 07/15/2017	All ND - Last	0.001	0.002	0.001	0.002	Standard
G206	UA	257	Total Dissolved Solids	mg/L	01/21/2015 - 01/27/2021	CI around median	450	1200	949	1200	Standard
G206D	DA	845	Antimony, total	mg/L	03/30/2021 - 07/27/2021	All ND - Last	0.003	0.006	0.003	0.006	Standard
G206D	DA	845	Arsenic, total	mg/L	03/30/2021 - 07/27/2021	CI around mean	0.00199	0.011	0.011	0.01	Background
G206D	DA	845	Barium, total	mg/L	03/30/2021 - 07/27/2021	CI around mean	0.068	2.0	0.13	2	Standard
G206D	DA	845	Beryllium, total	mg/L	03/30/2021 - 07/27/2021	All ND - Last	0.001	0.004	0.001	0.004	Standard
G206D	DA	845	Boron, total	mg/L	03/30/2021 - 07/27/2021	CI around mean	0.11	2.0	0.11	2	Standard
G206D	DA	845	Cadmium,total	mg/L	03/30/2021 - 07/27/2021	All ND - Last	0.001	0.005	0.001	0.005	Standard
G206D	DA	845	Chloride, total	mg/L	03/30/2021 - 07/27/2021	CI around mean	38	200	95	200	Standard
G206D	DA	845	Chromium, total	mg/L	03/30/2021 - 07/27/2021	CI around median	0	0.10	0.0096	0.1	Standard



Sample Location	HSU	Program	Constituent	Result Unit	Sample Date Range	Statistical Calculation	Statistical Result	GWPS	Background	Part 845 Standard	GWPS Source
G206D	DA	845	Cobalt, total	mg/L	03/30/2021 - 07/27/2021	CI around median	0	0.006	0.0037	0.006	Standard
G206D	DA	845	Fluoride, total	mg/L	03/30/2021 - 07/27/2021	CI around mean	0.44	4.0	0.55	4	Standard
G206D	DA	845	Lead, total	mg/L	03/30/2021 - 07/27/2021	CI around mean	0.0000301	0.0075	0.0059	0.0075	Standard
G206D	DA	845	Lithium, total	mg/L	03/30/2021 - 07/27/2021	All ND - Last	0.020	0.040	0.020	0.04	Standard
G206D	DA	845	Mercury, total	mg/L	03/30/2021 - 07/27/2021	All ND - Last	0.0002	0.002	0.0011	0.002	Standard
G206D	DA	845	Molybdenum, total	mg/L	03/30/2021 - 07/27/2021	CI around mean	0.026	0.10	0.044	0.1	Standard
G206D	DA	845	pH (field)	SU	03/30/2021 - 07/27/2021	CI around mean	7.0	6.5/9.0	6.8/7.4	6.5/9	Standard/Standard
G206D	DA	845	Radium-226 + Radium 228, tot	pCi/L	03/30/2021 - 07/27/2021	CI around mean	-0.093	5.0	1.5	5	Standard
G206D	DA	845	Selenium, total	mg/L	03/30/2021 - 07/27/2021	All ND - Last	0.001	0.050	0.0035	0.05	Standard
G206D	DA	845	Sulfate, total	mg/L	03/30/2021 - 07/27/2021	CI around mean	224	400	387	400	Standard
G206D	DA	845	Thallium, total	mg/L	03/30/2021 - 07/27/2021	All ND - Last	0.001	0.002	0.001	0.002	Standard
G206D	DA	845	Total Dissolved Solids	mg/L	03/30/2021 - 07/27/2021	CI around mean	1080	1200	975	1200	Standard
G207	UA	845	Antimony, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.003	0.006	0.003	0.006	Standard
G207	UA	845	Arsenic, total	mg/L	01/21/2015 - 10/07/2015	CI around mean	-0.000614	0.011	0.011	0.01	Background
G207	UA	845	Barium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.12	2.0	0.13	2	Standard
G207	UA	845	Beryllium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.001	0.004	0.001	0.004	Standard
G207	UA	845	Boron, total	mg/L	01/21/2015 - 10/07/2015	CI around mean	-0.00421	2.0	0.11	2	Standard
G207	UA	845	Cadmium,total	mg/L	01/21/2015 - 10/07/2015	All ND - Last	0.001	0.005	0.001	0.005	Standard
G207	UA	845	Chloride, total	mg/L	01/21/2015 - 10/07/2015	CI around mean	51	200	95	200	Standard
G207	UA	845	Chromium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.004	0.10	0.0096	0.1	Standard
G207	UA	845	Cobalt, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.002	0.006	0.0037	0.006	Standard
G207	UA	845	Fluoride, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.44	4.0	0.55	4	Standard
G207	UA	845	Lead, total	mg/L	01/21/2015 - 10/07/2015	CI around mean	-0.000508	0.0075	0.0059	0.0075	Standard
G207	UA	845	Mercury, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.0002	0.002	0.0011	0.002	Standard
G207	UA	845	Molybdenum, total	mg/L	07/22/2015 - 10/07/2015	Most recent sample	0.0016	0.10	0.044	0.1	Standard
G207	UA	845	pH (field)	SU	01/21/2015 - 10/07/2015	CI around mean	6.7	6.5/9.0	6.8/7.4	6.5/9	Standard/Standard



Sample Location	HSU	Program	Constituent	Result Unit	Sample Date Range	Statistical Calculation	Statistical Result	GWPS	Background	Part 845 Standard	GWPS Source
G207	UA	845	Selenium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.001	0.050	0.0035	0.05	Standard
G207	UA	845	Sulfate, total	mg/L	01/21/2015 - 10/07/2015	CI around mean	-23.9	400	387	400	Standard
G207	UA	845	Thallium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.001	0.002	0.001	0.002	Standard
G207	UA	845	Total Dissolved Solids	mg/L	01/21/2015 - 07/22/2015	Most recent sample	440	1200	975	1200	Standard
G208	UA	845	Antimony, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.003	0.006	0.003	0.006	Standard
G208	UA	845	Arsenic, total	mg/L	01/21/2015 - 10/07/2015	All ND - Last	0.001	0.011	0.011	0.01	Background
G208	UA	845	Barium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.091	2.0	0.13	2	Standard
G208	UA	845	Beryllium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.001	0.004	0.001	0.004	Standard
G208	UA	845	Boron, total	mg/L	01/21/2015 - 10/07/2015	CI around median	0	2.0	0.11	2	Standard
G208	UA	845	Cadmium,total	mg/L	01/21/2015 - 10/07/2015	All ND - Last	0.001	0.005	0.001	0.005	Standard
G208	UA	845	Chloride, total	mg/L	01/21/2015 - 10/07/2015	CI around mean	21	200	95	200	Standard
G208	UA	845	Chromium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.004	0.10	0.0096	0.1	Standard
G208	UA	845	Cobalt, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.002	0.006	0.0037	0.006	Standard
G208	UA	845	Fluoride, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.39	4.0	0.55	4	Standard
G208	UA	845	Lead, total	mg/L	01/21/2015 - 10/07/2015	All ND - Last	0.001	0.0075	0.0059	0.0075	Standard
G208	UA	845	Mercury, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.0002	0.002	0.0011	0.002	Standard
G208	UA	845	Molybdenum, total	mg/L	07/22/2015 - 10/07/2015	Most recent sample	0.0017	0.10	0.044	0.1	Standard
G208	UA	845	pH (field)	SU	01/21/2015 - 10/07/2015	CI around mean	6.8	6.5/9.0	6.8/7.4	6.5/9	Standard/Standard
G208	UA	845	Selenium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.0033	0.050	0.0035	0.05	Standard
G208	UA	845	Sulfate, total	mg/L	01/21/2015 - 10/07/2015	CI around median	0	400	387	400	Standard
G208	UA	845	Thallium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.001	0.002	0.001	0.002	Standard
G208	UA	845	Total Dissolved Solids	mg/L	01/21/2015 - 10/07/2015	CI around mean	156	1200	975	1200	Standard
G209	UA	257	Antimony, total	mg/L	04/09/2015 - 07/15/2017	CI around median	0.003	0.006	0.003	0.006	Standard
G209	UA	257	Arsenic, total	mg/L	01/21/2015 - 07/15/2017	CI around geomean	0.0012	0.010	0.010	0.01	Standard
G209	UA	257	Barium, total	mg/L	04/09/2015 - 07/15/2017	CI around mean	0.057	2.0	0.27	2	Standard
G209	UA	257	Beryllium, total	mg/L	04/09/2015 - 07/15/2017	Future median	0.001	0.0067	0.0067	0.004	Background



Sample Location	HSU	Program	Constituent	Result Unit	Sample Date Range	Statistical Calculation	Statistical Result	GWPS	Background	Part 845 Standard	GWPS Source
G209	UA	257	Boron, total	mg/L	01/21/2015 - 01/27/2021	CI around median	0.010	2.0	0.39	2	Standard
G209	UA	257	Cadmium,total	mg/L	01/21/2015 - 07/15/2017	CI around median	0.001	0.005	0.0012	0.005	Standard
G209	UA	257	Chloride, total	mg/L	01/21/2015 - 01/27/2021	CI around mean	63	200	96	200	Standard
G209	UA	257	Chromium, total	mg/L	04/09/2015 - 07/15/2017	CI around median	0.004	0.10	0.013	0.1	Standard
G209	UA	257	Cobalt, total	mg/L	04/09/2015 - 07/15/2017	Future median	0.002	0.0074	0.0074	0.006	Background
G209	UA	257	Fluoride, total	mg/L	04/09/2015 - 01/27/2021	CI around mean	0.39	4.0	0.50	4	Standard
G209	UA	257	Lead, total	mg/L	01/21/2015 - 07/15/2017	Future median	0.001	0.018	0.018	0.0075	Background
G209	UA	257	Lithium, total	mg/L	11/18/2015 - 07/15/2017	All ND - Last	0.010	0.040	0.021	0.04	Standard
G209	UA	257	Mercury, total	mg/L	04/09/2015 - 07/15/2017	CI around median	0.0002	0.002	0.0002	0.002	Standard
G209	UA	257	Molybdenum, total	mg/L	07/22/2015 - 07/15/2017	CI around geomean	0.00118	0.10	0.0069	0.1	Standard
G209	UA	257	pH (field)	SU	01/21/2015 - 01/27/2021	CI around median	7.0	6.5/9.0	6.9/7.3	6.5/9	Standard/Standard
G209	UA	257	Radium-226 + Radium 228, tot	pCi/L	11/18/2015 - 07/15/2017	CI around mean	0.32	9.8	9.8	5	Background
G209	UA	257	Selenium, total	mg/L	04/09/2015 - 07/15/2017	CI around median	0.001	0.050	0.0097	0.05	Standard
G209	UA	257	Sulfate, total	mg/L	01/21/2015 - 01/27/2021	CB around T-S line	159	400	300	400	Standard
G209	UA	257	Thallium, total	mg/L	04/09/2015 - 07/15/2017	CI around median	0.001	0.002	0.001	0.002	Standard
G209	UA	257	Total Dissolved Solids	mg/L	01/21/2015 - 01/27/2021	CI around mean	778	1200	949	1200	Standard
G210	UA	845	Antimony, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.003	0.006	0.003	0.006	Standard
G210	UA	845	Arsenic, total	mg/L	01/21/2015 - 10/07/2015	All ND - Last	0.001	0.011	0.011	0.01	Background
G210	UA	845	Barium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.033	2.0	0.13	2	Standard
G210	UA	845	Beryllium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.001	0.004	0.001	0.004	Standard
G210	UA	845	Boron, total	mg/L	01/21/2015 - 10/07/2015	CI around geomean	0.00255	2.0	0.11	2	Standard
G210	UA	845	Cadmium,total	mg/L	01/21/2015 - 10/07/2015	All ND - Last	0.001	0.005	0.001	0.005	Standard
G210	UA	845	Chloride, total	mg/L	01/21/2015 - 10/07/2015	CI around mean	46	200	95	200	Standard
G210	UA	845	Chromium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.011	0.10	0.0096	0.1	Standard
G210	UA	845	Cobalt, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.002	0.006	0.0037	0.006	Standard
G210	UA	845	Fluoride, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.37	4.0	0.55	4	Standard



Sample Location	HSU	Program	Constituent	Result Unit	Sample Date Range	Statistical Calculation	Statistical Result	GWPS	Background	Part 845 Standard	GWPS Source
G210	UA	845	Lead, total	mg/L	01/21/2015 - 10/07/2015	All ND - Last	0.001	0.0075	0.0059	0.0075	Standard
G210	UA	845	Mercury, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.0002	0.002	0.0011	0.002	Standard
G210	UA	845	Molybdenum, total	mg/L	07/22/2015 - 10/07/2015	Most recent sample	0.0018	0.10	0.044	0.1	Standard
G210	UA	845	pH (field)	SU	01/21/2015 - 10/07/2015	CI around mean	6.7	6.5/9.0	6.8/7.4	6.5/9	Standard/Standard
G210	UA	845	Selenium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.001	0.050	0.0035	0.05	Standard
G210	UA	845	Sulfate, total	mg/L	01/21/2015 - 10/07/2015	CI around mean	76	400	387	400	Standard
G210	UA	845	Thallium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.001	0.002	0.001	0.002	Standard
G210	UA	845	Total Dissolved Solids	mg/L	01/21/2015 - 10/07/2015	CI around mean	457	1200	975	1200	Standard
G211	UA	845	Antimony, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.003	0.006	0.003	0.006	Standard
G211	UA	845	Arsenic, total	mg/L	01/21/2015 - 10/07/2015	All ND - Last	0.001	0.011	0.011	0.01	Background
G211	UA	845	Barium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.092	2.0	0.13	2	Standard
G211	UA	845	Beryllium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.001	0.004	0.001	0.004	Standard
G211	UA	845	Boron, total	mg/L	01/21/2015 - 10/07/2015	CI around median	0	2.0	0.11	2	Standard
G211	UA	845	Cadmium,total	mg/L	01/21/2015 - 10/07/2015	All ND - Last	0.001	0.005	0.001	0.005	Standard
G211	UA	845	Chloride, total	mg/L	01/21/2015 - 10/07/2015	CI around mean	34	200	95	200	Standard
G211	UA	845	Chromium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.004	0.10	0.0096	0.1	Standard
G211	UA	845	Cobalt, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.002	0.006	0.0037	0.006	Standard
G211	UA	845	Fluoride, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.31	4.0	0.55	4	Standard
G211	UA	845	Lead, total	mg/L	01/21/2015 - 10/07/2015	All ND - Last	0.001	0.0075	0.0059	0.0075	Standard
G211	UA	845	Mercury, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.0002	0.002	0.0011	0.002	Standard
G211	UA	845	Molybdenum, total	mg/L	07/22/2015 - 10/07/2015	Most recent sample	0.001	0.10	0.044	0.1	Standard
G211	UA	845	pH (field)	SU	01/21/2015 - 10/07/2015	CI around mean	6.8	6.5/9.0	6.8/7.4	6.5/9	Standard/Standard
G211	UA	845	Selenium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.0013	0.050	0.0035	0.05	Standard
G211	UA	845	Sulfate, total	mg/L	01/21/2015 - 10/07/2015	CI around mean	67	400	387	400	Standard
G211	UA	845	Thallium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.001	0.002	0.001	0.002	Standard
G211	UA	845	Total Dissolved Solids	mg/L	01/21/2015 - 10/07/2015	CI around mean	443	1200	975	1200	Standard



Sample Location	HSU	Program	Constituent	Result Unit	Sample Date Range	Statistical Calculation	Statistical Result	GWPS	Background	Part 845 Standard	GWPS Source
G212	UA	257	Antimony, total	mg/L	04/09/2015 - 07/15/2017	All ND - Last	0.003	0.006	0.003	0.006	Standard
G212	UA	257	Arsenic, total	mg/L	01/21/2015 - 07/15/2017	CI around median	0.001	0.010	0.010	0.01	Standard
G212	UA	257	Barium, total	mg/L	04/09/2015 - 07/15/2017	CI around mean	0.051	2.0	0.27	2	Standard
G212	UA	257	Beryllium, total	mg/L	04/09/2015 - 07/15/2017	All ND - Last	0.001	0.0067	0.0067	0.004	Background
G212	UA	257	Boron, total	mg/L	01/21/2015 - 01/26/2021	CI around median	0.010	2.0	0.39	2	Standard
G212	UA	257	Cadmium,total	mg/L	01/21/2015 - 07/15/2017	All ND - Last	0.001	0.005	0.0012	0.005	Standard
G212	UA	257	Chloride, total	mg/L	01/21/2015 - 01/26/2021	CI around mean	39	200	96	200	Standard
G212	UA	257	Chromium, total	mg/L	04/09/2015 - 07/15/2017	CI around median	0.004	0.10	0.013	0.1	Standard
G212	UA	257	Cobalt, total	mg/L	04/09/2015 - 07/15/2017	All ND - Last	0.002	0.0074	0.0074	0.006	Background
G212	UA	257	Fluoride, total	mg/L	04/09/2015 - 01/26/2021	CI around median	0.32	4.0	0.50	4	Standard
G212	UA	257	Lead, total	mg/L	01/21/2015 - 07/15/2017	Future median	0.001	0.018	0.018	0.0075	Background
G212	UA	257	Lithium, total	mg/L	11/18/2015 - 07/15/2017	All ND - Last	0.010	0.040	0.021	0.04	Standard
G212	UA	257	Mercury, total	mg/L	04/09/2015 - 07/15/2017	All ND - Last	0.0002	0.002	0.0002	0.002	Standard
G212	UA	257	Molybdenum, total	mg/L	07/22/2015 - 07/15/2017	CI around median	0.001	0.10	0.0069	0.1	Standard
G212	UA	257	pH (field)	SU	01/21/2015 - 06/29/2021	CI around mean	7.1	6.5/9.0	6.9/7.3	6.5/9	Standard/Standard
G212	UA	257	Radium-226 + Radium 228, tot	pCi/L	11/18/2015 - 07/15/2017	CI around mean	0.28	9.8	9.8	5	Background
G212	UA	257	Selenium, total	mg/L	04/09/2015 - 07/15/2017	CI around mean	0.00397	0.050	0.0097	0.05	Standard
G212	UA	257	Sulfate, total	mg/L	01/21/2015 - 01/26/2021	CB around linear reg	46	400	300	400	Standard
G212	UA	257	Thallium, total	mg/L	04/09/2015 - 07/15/2017	All ND - Last	0.001	0.002	0.001	0.002	Standard
G212	UA	257	Total Dissolved Solids	mg/L	01/21/2015 - 01/26/2021	CI around geomean	367	1200	949	1200	Standard
G213	UA	845	Antimony, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.003	0.006	0.003	0.006	Standard
G213	UA	845	Arsenic, total	mg/L	01/21/2015 - 10/07/2015	CI around mean	-0.00341	0.011	0.011	0.01	Background
G213	UA	845	Barium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.10	2.0	0.13	2	Standard
G213	UA	845	Beryllium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.002	0.004	0.001	0.004	Standard
G213	UA	845	Boron, total	mg/L	01/21/2015 - 10/07/2015	CI around median	0	2.0	0.11	2	Standard
G213	UA	845	Cadmium,total	mg/L	01/21/2015 - 10/07/2015	CI around median	0	0.005	0.001	0.005	Standard



Sample Location	HSU	Program	Constituent	Result Unit	Sample Date Range	Statistical Calculation	Statistical Result	GWPS	Background	Part 845 Standard	GWPS Source
G213	UA	845	Chloride, total	mg/L	01/21/2015 - 10/07/2015	CI around mean	36	200	95	200	Standard
G213	UA	845	Chromium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.026	0.10	0.0096	0.1	Standard
G213	UA	845	Cobalt, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.0057	0.006	0.0037	0.006	Standard
G213	UA	845	Fluoride, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.31	4.0	0.55	4	Standard
G213	UA	845	Lead, total	mg/L	01/21/2015 - 10/07/2015	CI around mean	-0.00915	0.0075	0.0059	0.0075	Standard
G213	UA	845	Mercury, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.0002	0.002	0.0011	0.002	Standard
G213	UA	845	Molybdenum, total	mg/L	07/22/2015 - 10/07/2015	Most recent sample	0.004	0.10	0.044	0.1	Standard
G213	UA	845	pH (field)	SU	01/21/2015 - 10/07/2015	CI around mean	6.6	6.5/9.0	6.8/7.4	6.5/9	Standard/Standard
G213	UA	845	Selenium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.0038	0.050	0.0035	0.05	Standard
G213	UA	845	Sulfate, total	mg/L	01/21/2015 - 10/07/2015	CI around mean	46	400	387	400	Standard
G213	UA	845	Thallium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.0014	0.002	0.001	0.002	Standard
G213	UA	845	Total Dissolved Solids	mg/L	01/21/2015 - 10/07/2015	CI around mean	362	1200	975	1200	Standard
G214	UA	845	Antimony, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.003	0.006	0.003	0.006	Standard
G214	UA	845	Arsenic, total	mg/L	01/21/2015 - 10/07/2015	All ND - Last	0.001	0.011	0.011	0.01	Background
G214	UA	845	Barium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.089	2.0	0.13	2	Standard
G214	UA	845	Beryllium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.001	0.004	0.001	0.004	Standard
G214	UA	845	Boron, total	mg/L	01/21/2015 - 10/07/2015	CI around median	0	2.0	0.11	2	Standard
G214	UA	845	Cadmium,total	mg/L	01/21/2015 - 10/07/2015	All ND - Last	0.001	0.005	0.001	0.005	Standard
G214	UA	845	Chloride, total	mg/L	01/21/2015 - 10/07/2015	CI around mean	46	200	95	200	Standard
G214	UA	845	Chromium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.0067	0.10	0.0096	0.1	Standard
G214	UA	845	Cobalt, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.002	0.006	0.0037	0.006	Standard
G214	UA	845	Fluoride, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.32	4.0	0.55	4	Standard
G214	UA	845	Lead, total	mg/L	01/21/2015 - 10/07/2015	All ND - Last	0.001	0.0075	0.0059	0.0075	Standard
G214	UA	845	Mercury, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.0002	0.002	0.0011	0.002	Standard
G214	UA	845	Molybdenum, total	mg/L	07/22/2015 - 10/07/2015	Most recent sample	0.0019	0.10	0.044	0.1	Standard
G214	UA	845	pH (field)	SU	01/21/2015 - 10/07/2015	CI around mean	6.7	6.5/9.0	6.8/7.4	6.5/9	Standard/Standard



Sample Location	HSU	Program	Constituent	Result Unit	Sample Date Range	Statistical Calculation	Statistical Result	GWPS	Background	Part 845 Standard	GWPS Source
G214	UA	845	Selenium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.001	0.050	0.0035	0.05	Standard
G214	UA	845	Sulfate, total	mg/L	01/21/2015 - 10/07/2015	CI around mean	62	400	387	400	Standard
G214	UA	845	Thallium, total	mg/L	04/09/2015 - 10/07/2015	Most recent sample	0.001	0.002	0.001	0.002	Standard
G214	UA	845	Total Dissolved Solids	mg/L	01/21/2015 - 10/07/2015	CI around mean	450	1200	975	1200	Standard
G215	UA	257	Antimony, total	mg/L	04/09/2015 - 07/15/2017	CI around median	0.003	0.006	0.003	0.006	Standard
G215	UA	257	Arsenic, total	mg/L	01/21/2015 - 07/15/2017	CI around mean	0.00752	0.010	0.010	0.01	Standard
G215	UA	257	Barium, total	mg/L	04/09/2015 - 07/15/2017	CI around mean	0.094	2.0	0.27	2	Standard
G215	UA	257	Beryllium, total	mg/L	04/09/2015 - 07/15/2017	All ND - Last	0.001	0.0067	0.0067	0.004	Background
G215	UA	257	Boron, total	mg/L	01/21/2015 - 01/26/2021	CI around geomean	0.027	2.0	0.39	2	Standard
G215	UA	257	Cadmium,total	mg/L	01/21/2015 - 07/15/2017	All ND - Last	0.001	0.005	0.0012	0.005	Standard
G215	UA	257	Chloride, total	mg/L	01/21/2015 - 06/29/2021	CB around linear reg	63	200	96	200	Standard
G215	UA	257	Chromium, total	mg/L	04/09/2015 - 07/15/2017	All ND - Last	0.004	0.10	0.013	0.1	Standard
G215	UA	257	Cobalt, total	mg/L	04/09/2015 - 07/15/2017	Future median	0.002	0.0074	0.0074	0.006	Background
G215	UA	257	Fluoride, total	mg/L	04/09/2015 - 01/26/2021	CI around median	0.33	4.0	0.50	4	Standard
G215	UA	257	Lead, total	mg/L	01/21/2015 - 07/15/2017	Future median	0.001	0.018	0.018	0.0075	Background
G215	UA	257	Lithium, total	mg/L	11/24/2015 - 07/15/2017	All ND - Last	0.010	0.040	0.021	0.04	Standard
G215	UA	257	Mercury, total	mg/L	04/09/2015 - 07/15/2017	All ND - Last	0.0002	0.002	0.0002	0.002	Standard
G215	UA	257	Molybdenum, total	mg/L	07/22/2015 - 07/15/2017	CI around median	0.001	0.10	0.0069	0.1	Standard
G215	UA	257	pH (field)	SU	01/21/2015 - 06/29/2021	CI around mean	6.9	6.5/9.0	6.9/7.3	6.5/9	Standard/Standard
G215	UA	257	Radium-226 + Radium 228, tot	pCi/L	11/24/2015 - 07/15/2017	CI around mean	0.18	9.8	9.8	5	Background
G215	UA	257	Selenium, total	mg/L	04/09/2015 - 07/15/2017	CI around median	0.001	0.050	0.0097	0.05	Standard
G215	UA	257	Sulfate, total	mg/L	01/21/2015 - 06/29/2021	CI around median	110	400	300	400	Standard
G215	UA	257	Thallium, total	mg/L	04/09/2015 - 07/15/2017	All ND - Last	0.001	0.002	0.001	0.002	Standard
G215	UA	257	Total Dissolved Solids	mg/L	01/21/2015 - 06/29/2021	CI around median	480	1200	949	1200	Standard
G216	UA	845	Antimony, total	mg/L	04/10/2015 - 10/07/2015	Most recent sample	0.003	0.006	0.003	0.006	Standard
G216	UA	845	Arsenic, total	mg/L	01/21/2015 - 10/07/2015	CI around mean	-0.00056	0.011	0.011	0.01	Background



Sample Location	HSU	Program	Constituent	Result Unit	Sample Date Range	Statistical Calculation	Statistical Result	GWPS	Background	Part 845 Standard	GWPS Source
G216	UA	845	Barium, total	mg/L	04/10/2015 - 10/07/2015	Most recent sample	0.16	2.0	0.13	2	Standard
G216	UA	845	Beryllium, total	mg/L	04/10/2015 - 10/07/2015	Most recent sample	0.001	0.004	0.001	0.004	Standard
G216	UA	845	Boron, total	mg/L	01/21/2015 - 10/07/2015	CI around mean	-0.00463	2.0	0.11	2	Standard
G216	UA	845	Cadmium,total	mg/L	01/21/2015 - 10/07/2015	All ND - Last	0.001	0.005	0.001	0.005	Standard
G216	UA	845	Chloride, total	mg/L	01/21/2015 - 10/07/2015	CI around mean	53	200	95	200	Standard
G216	UA	845	Chromium, total	mg/L	04/10/2015 - 10/07/2015	Most recent sample	0.004	0.10	0.0096	0.1	Standard
G216	UA	845	Cobalt, total	mg/L	04/10/2015 - 10/07/2015	Most recent sample	0.002	0.006	0.0037	0.006	Standard
G216	UA	845	Fluoride, total	mg/L	04/10/2015 - 10/07/2015	Most recent sample	0.29	4.0	0.55	4	Standard
G216	UA	845	Lead, total	mg/L	01/21/2015 - 10/07/2015	CI around median	0	0.0075	0.0059	0.0075	Standard
G216	UA	845	Mercury, total	mg/L	04/10/2015 - 10/07/2015	Most recent sample	0.0002	0.002	0.0011	0.002	Standard
G216	UA	845	Molybdenum, total	mg/L	07/22/2015 - 10/07/2015	Most recent sample	0.013	0.10	0.044	0.1	Standard
G216	UA	845	pH (field)	SU	01/21/2015 - 10/07/2015	CI around mean	6.7	6.5/9.0	6.8/7.4	6.5/9	Standard/Standard
G216	UA	845	Selenium, total	mg/L	04/10/2015 - 10/07/2015	Most recent sample	0.001	0.050	0.0035	0.05	Standard
G216	UA	845	Sulfate, total	mg/L	01/21/2015 - 10/07/2015	CI around mean	196	400	387	400	Standard
G216	UA	845	Thallium, total	mg/L	04/10/2015 - 10/07/2015	Most recent sample	0.001	0.002	0.001	0.002	Standard
G216	UA	845	Total Dissolved Solids	mg/L	01/21/2015 - 10/07/2015	CI around mean	671	1200	975	1200	Standard
G217	UA	845	Antimony, total	mg/L	04/10/2015 - 10/07/2015	Most recent sample	0.003	0.006	0.003	0.006	Standard
G217	UA	845	Arsenic, total	mg/L	01/21/2015 - 10/07/2015	CI around mean	-0.00234	0.011	0.011	0.01	Background
G217	UA	845	Barium, total	mg/L	04/10/2015 - 10/07/2015	Most recent sample	0.11	2.0	0.13	2	Standard
G217	UA	845	Beryllium, total	mg/L	04/10/2015 - 10/07/2015	Most recent sample	0.001	0.004	0.001	0.004	Standard
G217	UA	845	Boron, total	mg/L	01/21/2015 - 10/07/2015	CI around median	0	2.0	0.11	2	Standard
G217	UA	845	Cadmium,total	mg/L	01/21/2015 - 10/07/2015	All ND - Last	0.001	0.005	0.001	0.005	Standard
G217	UA	845	Chloride, total	mg/L	01/21/2015 - 10/07/2015	CI around mean	72	200	95	200	Standard
G217	UA	845	Chromium, total	mg/L	04/10/2015 - 10/07/2015	Most recent sample	0.0086	0.10	0.0096	0.1	Standard
G217	UA	845	Cobalt, total	mg/L	04/10/2015 - 10/07/2015	Most recent sample	0.0032	0.006	0.0037	0.006	Standard
G217	UA	845	Fluoride, total	mg/L	04/10/2015 - 10/07/2015	Most recent sample	0.29	4.0	0.55	4	Standard



Sample Location	HSU	Program	Constituent	Result Unit	Sample Date Range	Statistical Calculation	Statistical Result	GWPS	Background	Part 845 Standard	GWPS Source
G217	UA	845	Lead, total	mg/L	01/21/2015 - 10/07/2015	CI around mean	-0.00356	0.0075	0.0059	0.0075	Standard
G217	UA	845	Mercury, total	mg/L	04/10/2015 - 10/07/2015	Most recent sample	0.0002	0.002	0.0011	0.002	Standard
G217	UA	845	Molybdenum, total	mg/L	07/22/2015 - 10/07/2015	Most recent sample	0.0013	0.10	0.044	0.1	Standard
G217	UA	845	pH (field)	SU	01/21/2015 - 10/07/2015	CI around mean	6.6	6.5/9.0	6.8/7.4	6.5/9	Standard/Standard
G217	UA	845	Selenium, total	mg/L	04/10/2015 - 10/07/2015	Most recent sample	0.001	0.050	0.0035	0.05	Standard
G217	UA	845	Sulfate, total	mg/L	01/21/2015 - 10/07/2015	CI around median	0	400	387	400	Standard
G217	UA	845	Thallium, total	mg/L	04/10/2015 - 10/07/2015	Most recent sample	0.001	0.002	0.001	0.002	Standard
G217	UA	845	Total Dissolved Solids	mg/L	01/21/2015 - 10/07/2015	CI around mean	603	1200	975	1200	Standard
G218	UA	257	Antimony, total	mg/L	04/10/2015 - 07/17/2017	All ND - Last	0.003	0.006	0.003	0.006	Standard
G218	UA	257	Arsenic, total	mg/L	01/21/2015 - 07/17/2017	CI around median	0.001	0.010	0.010	0.01	Standard
G218	UA	257	Barium, total	mg/L	04/10/2015 - 07/17/2017	CI around mean	0.14	2.0	0.27	2	Standard
G218	UA	257	Beryllium, total	mg/L	04/10/2015 - 07/17/2017	All ND - Last	0.001	0.0067	0.0067	0.004	Background
G218	UA	257	Boron, total	mg/L	01/21/2015 - 01/26/2021	CI around median	0.010	2.0	0.39	2	Standard
G218	UA	257	Cadmium,total	mg/L	01/21/2015 - 07/17/2017	All ND - Last	0.001	0.005	0.0012	0.005	Standard
G218	UA	257	Chloride, total	mg/L	01/21/2015 - 01/26/2021	CB around linear reg	72	200	96	200	Standard
G218	UA	257	Chromium, total	mg/L	04/10/2015 - 07/17/2017	CI around median	0.004	0.10	0.013	0.1	Standard
G218	UA	257	Cobalt, total	mg/L	04/10/2015 - 07/17/2017	All ND - Last	0.002	0.0074	0.0074	0.006	Background
G218	UA	257	Fluoride, total	mg/L	04/10/2015 - 01/26/2021	CI around mean	0.30	4.0	0.50	4	Standard
G218	UA	257	Lead, total	mg/L	01/21/2015 - 07/17/2017	All ND - Last	0.001	0.018	0.018	0.0075	Background
G218	UA	257	Lithium, total	mg/L	11/24/2015 - 07/17/2017	All ND - Last	0.010	0.040	0.021	0.04	Standard
G218	UA	257	Mercury, total	mg/L	04/10/2015 - 07/17/2017	All ND - Last	0.0002	0.002	0.0002	0.002	Standard
G218	UA	257	Molybdenum, total	mg/L	07/22/2015 - 07/17/2017	CI around median	0.001	0.10	0.0069	0.1	Standard
G218	UA	257	pH (field)	SU	01/21/2015 - 01/26/2021	CI around mean	7.0	6.5/9.0	6.9/7.3	6.5/9	Standard/Standard
G218	UA	257	Radium-226 + Radium 228, tot	pCi/L	11/24/2015 - 07/17/2017	CI around mean	0.58	9.8	9.8	5	Background
G218	UA	257	Selenium, total	mg/L	04/10/2015 - 07/17/2017	All ND - Last	0.001	0.050	0.0097	0.05	Standard
G218	UA	257	Sulfate, total	mg/L	01/21/2015 - 01/26/2021	CB around linear reg	174	400	300	400	Standard



Sample Location	HSU	Program	Constituent	Result Unit	Sample Date Range	Statistical Calculation	Statistical Result	GWPS	Background	Part 845 Standard	GWPS Source
G218	UA	257	Thallium, total	mg/L	04/10/2015 - 07/17/2017	All ND - Last	0.001	0.002	0.001	0.002	Standard
G218	UA	257	Total Dissolved Solids	mg/L	01/21/2015 - 01/26/2021	CI around median	600	1200	949	1200	Standard
MW16S	UA	845	pH (field)	SU	04/09/2015 - 04/09/2015	Most recent sample	7.2	6.5/9.0	6.8/7.4	6.5/9	Standard/Standard
MW16S	UA	845	Total Dissolved Solids	mg/L	04/09/2015 - 04/09/2015	Most recent sample	410	1200	975	1200	Standard
R104	UA	845	Antimony, total	mg/L	04/08/2015 - 08/03/2016	All ND - Last	0.003	0.006	0.003	0.006	Standard
R104	UA	845	Arsenic, total	mg/L	04/08/2015 - 08/03/2016	CI around median	0.001	0.011	0.011	0.01	Background
R104	UA	845	Barium, total	mg/L	04/08/2015 - 08/03/2016	CI around mean	0.061	2.0	0.13	2	Standard
R104	UA	845	Beryllium, total	mg/L	04/08/2015 - 08/03/2016	All ND - Last	0.001	0.004	0.001	0.004	Standard
R104	UA	845	Boron, total	mg/L	04/08/2015 - 08/03/2016	CI around median	0.010	2.0	0.11	2	Standard
R104	UA	845	Cadmium,total	mg/L	04/08/2015 - 08/03/2016	All ND - Last	0.001	0.005	0.001	0.005	Standard
R104	UA	845	Chloride, total	mg/L	04/08/2015 - 08/03/2016	CI around mean	45	200	95	200	Standard
R104	UA	845	Chromium, total	mg/L	04/08/2015 - 08/03/2016	CI around median	0.004	0.10	0.0096	0.1	Standard
R104	UA	845	Cobalt, total	mg/L	04/08/2015 - 08/03/2016	All ND - Last	0.002	0.006	0.0037	0.006	Standard
R104	UA	845	Fluoride, total	mg/L	04/08/2015 - 08/03/2016	CI around mean	0.30	4.0	0.55	4	Standard
R104	UA	845	Lead, total	mg/L	04/08/2015 - 08/03/2016	CI around median	0.001	0.0075	0.0059	0.0075	Standard
R104	UA	845	Lithium, total	mg/L	11/17/2015 - 08/03/2016	All ND - Last	0.010	0.040	0.020	0.04	Standard
R104	UA	845	Mercury, total	mg/L	04/08/2015 - 08/03/2016	All ND - Last	0.0002	0.002	0.0011	0.002	Standard
R104	UA	845	Molybdenum, total	mg/L	07/23/2015 - 08/03/2016	CI around mean	0.00386	0.10	0.044	0.1	Standard
R104	UA	845	pH (field)	SU	01/20/2015 - 08/03/2016	CI around mean	7.3	6.5/9.0	6.8/7.4	6.5/9	Standard/Standard
R104	UA	845	Radium-226 + Radium 228, tot	pCi/L	11/17/2015 - 08/03/2016	CI around mean	-0.166	5.0	1.5	5	Standard
R104	UA	845	Selenium, total	mg/L	04/08/2015 - 08/03/2016	CI around mean	0.00582	0.050	0.0035	0.05	Standard
R104	UA	845	Sulfate, total	mg/L	04/08/2015 - 08/03/2016	CI around mean	72	400	387	400	Standard
R104	UA	845	Thallium, total	mg/L	04/08/2015 - 08/03/2016	All ND - Last	0.001	0.002	0.001	0.002	Standard
R104	UA	845	Total Dissolved Solids	mg/L	01/20/2015 - 08/03/2016	CI around mean	424	1200	975	1200	Standard



HISTORY OF POTENTIAL EXCEEDANCES COFFEEN POWER PLANT GMF GYPSUM STACK POND COFFEEN, ILLINOIS

Notes:

Potential exceedance of GWPS (note: No potential exceedances were determined based on data collected from 2015 through 2021)

HSU = hydrostratigraphic unit:

DA = deep aquifer

UA = uppermost aquifer

Program = regulatory program data were collected under:

257 = 40 C.F.R. Part 257 Subpart D (Standards for the Disposal of Coal Combustion Residuals in Landfills and Surface Impoundments)

845 = 35 I.A.C. Part 845 (Sampling events completed to assess well locations for inclusion in the Part 845 monitoring well network)

mg/L = milligrams per liter

pCi/L = picocuries per liter

SU = standard units

Statistical Calculation = method used to calculate the statistical result:

All ND - Last = All results were below the reporting limit, and the last determined reporting limit is shown

CB around linear reg = Confidence band around linear regression

CB around T-S line = Confidence band around Thiel-Sen line

CI around geomean = Confidence interval around the geometric mean

CI around mean = Confidence interval around the mean

CI around median = Confidence interval around the median

Future median = Median of the three most recent samples

Most recent sample = Result for the most recently collected sample used due to insufficient data

Statistical Result = calculated in accordance with Statistical Analysis Plan using constituent concentrations observed at monitoring well during all sampling events within the specified date range

For pH, the values presented are the lower / upper limits GWPS = Groundwater Protection Standard

GWPS Source:

Standard = standard specified in 35 I.A.C. § 845.600(a)(1)

Background = background concentration (see cover page for additional information)



ATTACHMENT N

Certification of Financial Assurance Requirements

On June 17, 2021, Illinois Power Generating Company provided financial assurance in the form of performance bonds to the Illinois Environmental Protection Agency in the amount of \$27,884,983 for Ash Pond 1, Ash Pond 2, and the GMF Pond System at the Coffeen Power Plant.¹

I, Matthew A. Goering, Senior Vice President of Illinois Power Generating Company, do hereby certify to the best of my knowledge for the above referenced CCR Units that the financial assurance instruments satisfy the requirements of 35 I.A.C. Part 845, Subpart I.

Matthew A. Goering

Senior Vice President

Illinois Power Generating Company

¹In the operating permit applications, the GMF Pond System is referred to as the GMF Gypsum Stack Pond and GMF Recycle Pond.

ATTACHMENT O



Stantec Consulting Services Inc. 1859 Bowles Avenue Suite 250, Fenton MO 63026-1944

October 12, 2016
File: let_001_175666013_certification
Revision 0

Initial Hazard Potential Classification Assessment EPA Final CCR Rule GMF Pond Coffeen Power Station Montgomery County, Illinois

1.0 PURPOSE

This report documents Stantec's certification of the initial hazard potential classification assessment for the Coffeen Power Station GMF Pond.

40 CFR 257.73(a)(2) requires the owner or operator of an existing CCR surface impoundment to conduct an initial hazard potential classification assessment and document the hazard potential classification, and the basis for the classification, of the CCR unit as either a high hazard potential CCR surface impoundment, a significant hazard potential CCR surface impoundment, or a low hazard potential CCR surface impoundment.

2.0 FINDINGS

Stantec reviewed an existing breach analysis (dated 2007) to evaluate potential hazards associated with a failure of the GMF Pond's perimeter containment dike and to see if the existing analysis was suitable for the purpose of informing Stantec's initial hazard potential classification assessment. The existing breach analysis utilized an approximate method of computing the inundation limits of gypsum slurry by computing a runout distance on a constant slope. The breach analysis used parameters consistent with the final design gypsum stack height and volume within the GMF Pond containment dike. The analysis also utilized topography and imagery of the surrounding area to estimate inundation limits. Breaches were simulated at multiple locations along the perimeter dike and the corresponding results were used to create inundation extents. The inundation extents are depicted for two inundations limits; a 10 foot inundation depth area and a 5 foot inundation depth area. There are a total of 12 potentially impacted structures (11 residential) within the 5 foot inundation depth area.

Based on its review, Stantec concluded that the existing analysis is suitable for the purpose of hazard potential classification and that there have not been any material changes to the GMF Pond or downstream areas that would significantly impact the previous analysis results. The existing breach analysis indicates that a breach failure near the northwest corner of the GMF Pond perimeter dike would inundate eight occupied structures, seven being residential structures and one industrial facility. The existing breach analysis also indicates that a breach to the east would inundate two residential structures along Red Ball Trail/County HWY 9 and a breach to the south would inundate frequently occupied Coffeen Power Station facilities. Based upon review of the existing breach analysis predicted depths and velocities at various frequently occupied Coffeen



Page 2 of 2

Power Station facilities and surrounding residential structures, it was concluded that a failure of the GMF Pond's perimeter dike will probably cause loss of human life.

40 CFR 257.53 defines a "high hazard potential CCR surface impoundment" as a diked surface impoundment where failure or mis-operation will probably cause loss of human life.

Based on the results of the analysis summarized above, the GMF Pond was assigned a High hazard potential classification per 40 CFR 257.53.

3.0 QUALIFIED PROFESSIONAL ENGINEER CERTIFICATION

I, Matthew Hoy, being a Professional Engineer in good standing in the State of Illinois, do hereby certify, to the best of my knowledge, information, and belief that;

- 1. the information contained in this report and the underlying data in the operating record was prepared in accordance with the accepted practice of engineering and is accurate as of the date of my signature below; and
- 2. the initial hazard potential classification assessment for the Coffeen Power Station GMF Pond was conducted in accordance with the requirements specified in 40 CFR 257.73.

SIGNATURE

ADDRESS:

Stantec Consulting Services Inc.

1859 Bowles Avenue Suite 250

Fenton MO 63026-1944

TELEPHONE: (636) 343-3880

MATTHEW A. HOY O62-063141

DATE 10/12/2016

Design with community in mind

ATTACHMENT P



Submitted to Illinois Power Generating Company 134 Cips Lane Coffeen, IL 62017 Submitted by AECOM 1001 Highlands Plaza Drive West Suite 300 St. Louis, MO 63110

October 2016

CCR Rule Report: Initial Structural Stability Assessment

For

GMF Pond

At Coffeen Power Station

1 Introduction

This Coal Combustion Residual (CCR) Rule Report documents that the Gypsum Management Facility (GMF) Pond at the Illinois Power Generating Company Coffeen Power Station meets the structural stability assessment requirements specified in 40 Code of Federal Regulations (CFR) §257.73(d). The GMF Pond is located near Coffeen, Illinois in Montgomery County, approximately 0.6 miles north of the Coffeen Power Station. The GMF Pond serves as the primary wet impoundment basin for gypsum produced by the wet scrubber system at the Coffeen Power Station.

The GMF Pond is an existing CCR surface impoundment as defined by 40 CFR §257.53. The CCR Rule requires that an initial structural stability assessment for an existing CCR surface impoundment be completed by October 17, 2016. In general, the initial structural stability assessment must document that the design, construction, operation, and maintenance of the CCR unit is consistent with recognized and generally accepted good engineering practices.

The owner or operator of the CCR unit must obtain a certification from a qualified professional engineer stating that the initial structural stability assessment was conducted in accordance with the requirements of 40 CFR § 257.73(d). The owner or operator must prepare a periodic structural stability assessment every five years.

2 Initial Structural Stability Assessment

40 CFR §257.73(d)(1)

The owner or operator of the CCR unit must conduct initial and periodic structural stability assessments and document whether the design, construction, operation, and maintenance of the CCR unit is consistent with recognized and generally accepted good engineering practices for the maximum volume of CCR and CCR wastewater which can be impounded therein. The assessment must, at a minimum, document whether the CCR unit has been designed, constructed, operated, and maintained with [the standards in (d)(1)(i)-(vii)].

An initial structural stability assessment has been performed to document that the design, construction, operation and maintenance of the GMF Pond is consistent with recognized and generally accepted good engineering practices and meets the standards in 257.73(d)(1)(i)-(vii). The results of the structural stability assessment are discussed in the following sections. Based on the assessment and its results, the design, construction, operation, and maintenance of the GMF Pond were found to be consistent with recognized and generally accepted good engineering practices.

2.1 Foundations and Abutments (§257.73(d)(1)(i))

CCR unit designed, constructed, operated, and maintained with stable foundations and abutments.

The stability of the foundations was evaluated using soil data from field investigations and reviewing design drawings, operational and maintenance procedures, and conditions observed in the field by AECOM. Additionally, slope stability analyses were performed to evaluate slip surfaces passing through the foundations. The GMF Pond is a ring dike structure and does not have abutments.

The foundation consists of medium stiff to stiff soil, overlying soft to very soft soil, which in turn overlies very stiff to hard glacial till. Slope stability analyses exceed the criteria listed in §257.73(e)(1)(i) through (iii) for slip surfaces passing through the foundation. The slope stability analyses are discussed in the *CCR Rule Report: Initial Safety Factor Assessment for GMF Pond at Coffeen Power Station* (October 2016). Additional slope stability analyses were performed to evaluate the effects of liquefaction and cyclic softening in the foundation, and were found to satisfy the criteria listed in §257.73(e)(1)(iv) applicable to dikes. A review of operational and maintenance procedures as well as current and past performance of the dikes has determined appropriate processes are in place for continued operational performance.

Based on the conditions observed by AECOM, the GMF Pond was designed and constructed with stable foundations. Operational and maintenance procedures are in place to address any issues related to the stability of foundations. Therefore, the GMF Pond meets the requirements in §257.73(d)(1)(i).

2.2 Slope Protection (§257.73(d)(1)(ii))

CCR unit designed, constructed, operated, and maintained with adequate slope protection to protect against surface erosion, wave action and adverse effects of sudden drawdown.

The adequacy of slope protection was evaluated by reviewing design drawings, operational and maintenance procedures, and conditions observed in the field by AECOM.

Based on this evaluation, adequate slope protection was designed and constructed at the GMF Pond. No evidence of significant areas of erosion or wave action was observed. The interior slopes are protected with a geomembrane liner that underlies the entire GMF Pond and extends up the interior slopes, and the exterior

slopes are protected with vegetation. The geomembrane liner on the interior slopes isolates the embankment soils from surface erosion or wave action. Operational and maintenance procedures to repair the vegetation (exterior slopes) and liner (interior slopes) as needed are appropriate to protect against surface erosion or wave action. Given the presence of a liner that serves to prevent saturation of the dike's soils below the normal pool, sudden drawdown, as well as the corresponding adverse effects, is not applicable to the GMF Pond. Therefore, the GMF Pond meets the requirements in §257.73(d)(1)(ii).

2.3 Dike Compaction (§257.73(d)(1)(iii))

CCR unit designed, constructed, operated, and maintained with dikes mechanically compacted to a density sufficient to withstand the range of loading conditions in the CCR unit.

The density of the dike materials was evaluated using soil data from field investigations and reviewing design drawings, operational and maintenance procedures, and conditions observed in the field by AECOM. Additionally, slope stability analyses were performed to evaluate slip surfaces passing through the dike over the range of expected loading conditions as defined within §257.73(e)(1).

Based on this evaluation, the dike consists of medium stiff to stiff material, which is indicative of mechanically compacted dikes. As discussed in the *CCR Rule Report: Initial Safety Factor Assessment for GMF Pond at Coffeen Power Station* (2016), slope stability analyses exceed the criteria listed in §257.73(e)(1) for slip surfaces passing through the dike. Therefore, the original design and construction of the GMF Pond included sufficient dike compaction. Operational and maintenance procedures are in place to identify and mitigate deficiencies in order to maintain sufficient compaction of the dikes to withstand the range of loading conditions. Therefore, the GMF Pond meets the requirements in §257.73(d)(1)(iii).

2.4 Vegetated Slopes (§257.73(d)(1)(iv))¹

CCR unit designed, constructed, operated, and maintained with vegetated slopes of dikes and surrounding areas, except for slopes which have an alternate form or forms of slope protection.

The adequacy of slope vegetation was evaluated by reviewing design drawings, operational and maintenance procedures, and conditions observed in the field by AECOM.

Based on this evaluation, the vegetation on the exterior slopes is adequate as no substantial bare or overgrown areas were observed. Exposed geomembrane liners on the interior slopes are used as an alternate form of slope protection, which is adequate as significant tears or defects were not observed. Therefore, the original design and construction of the GMF Pond included adequate vegetation of the dikes and surrounding areas. Adequate operational and maintenance procedures are in place to regularly manage vegetation growth, including mowing and seeding any bare areas, as evidenced by the conditions observed by AECOM. Therefore, the GMF Pond meets the requirements in §257.73(d)(1)(iv).

As modified by court order issued June 14, 2016, Utility Solid Waste Activities Group v. EPA, D.C. Cir. No. 15-1219 (order granting remand and vacatur of specific regulatory provisions).

2.5 Spillways (§257.73(d)(1)(v))

CCR unit designed, constructed, operated, and maintained with a single spillway or a combination of spillways configured as specified in [paragraph (A) and (B)]:

- (A) All spillways must be either:
 - (1) of non-erodible construction and designed to carry sustained flows; or
 - (2) earth- or grass-lined and designed to carry short-term, infrequent flows at non-erosive velocities where sustained flows are not expected.
- (B) The combined capacity of all spillways must adequately manage flow during and following the peak discharge from a:
 - (1) Probable maximum flood (PMF) for a high hazard potential CCR surface impoundment; or
 - (2) 1000-year flood for a significant hazard potential CCR surface impoundment; or
 - (3) 100-year flood for a low hazard potential CCR surface impoundment.

The spillway system was evaluated using design drawings, operational and maintenance procedures, and conditions observed in the field by AECOM. Additionally, hydrologic and hydraulic analyses were completed to evaluate the capacity of the spillway relative to inflow estimated for the probable maximum flood (PMF) inflow design flood (IDF) event for the high hazard potential GMF Pond. The hazard potential classification assessment was performed by Stantec in 2016 in accordance with §257.73(a)(2).

The spillway system at the GMF Pond includes a geomembrane-lined transfer channel and a high-density polyethylene low-flow pipe. Both the lined channel and the low-flow pipe are constructed from non-erodible materials that are designed to carry sustained flows. The capacity of the spillway system was evaluated using hydrologic and hydraulic analysis performed per §257.82(a). The analysis found that the spillway system can adequately manage flow during peak discharge resulting from the PMF IDF without overtopping of the embankments. The hydrologic and hydraulic analyses are discussed in the *CCR Rule Report: Initial Inflow Design Flood Control System Plan for GMF Pond at Coffeen Power Station* (October 2016). Operational and maintenance procedures are in place to repair any tears in the spillway liner and remove debris or other obstructions from the transfer channel and low-flow pipe, as evidenced by the conditions observed by AECOM. As a result, these procedures are appropriate for maintaining the spillway system. Therefore, the GMF Pond meets the requirements in §257.73(d)(1)(v).

2.6 Stability and Structural Integrity of Hydraulic Structures (§257.73(d)(1)(vi))

CCR unit designed, constructed, operated, and maintained with hydraulic structures underlying the base of the CCR unit or passing through the dike of the CCR unit that maintain structural integrity and are free of significant deterioration, deformation, distortion, bedding deficiencies, sedimentation, and debris which may negatively affect the operation of the hydraulic structure.

Based on an evaluation of design drawings, operational and maintenance procedures, and conditions observed in the field by AECOM, no hydraulic structures are present that underlie the base or pass through the dike of the GMF Pond. Therefore, the §257.73(d)(1)(vi) requirements are not applicable to the GMF Pond.

2.7 Downstream Slope Inundation/Stability (§257.73(d)(1)(vii))

CCR unit designed, constructed, operated, and maintained with, for CCR units with downstream slopes which can be inundated by the pool of an adjacent water body, such as a river, stream or lake, downstream slopes that maintain structural stability during low pool of the adjacent water body or sudden drawdown of the adjacent water body.

The structural stability of the downstream slopes of the GMF Pond was evaluated by comparing the location of the GMF Pond relative to adjacent water bodies using published Federal Emergency Management Agency (FEMA) Flood Insurance Rate Maps (FIRM), aerial imagery, and conditions observed in the field by AECOM.

Based on this evaluation, water bodies adjacent to the downstream slopes of the GMF Pond are not present. The nearest downstream water body is the GMF Recycle Pond, which is approximately 500 lateral feet beyond the

downstream slopes of the GMF Pond. The GMF Recycle Pond is a CCR unit, rather than a river, stream, or lake. Coffeen Lake is also located in the vicinity of the GMF Pond, but the GMF Pond is outside of the flood zone shown on the FEMA FIRM. Therefore, adjacent water bodies that can inundate the downstream slopes of the GMF Pond are not present.

Based on this evaluation, the requirements in §257.73(d)(1)(vii) are not applicable to the GMF Pond, as inundation of the downstream slopes is not expected to occur.

3 Certification Statement

CCR Unit: Illinois Power Generating Company; Coffeen Power Station; GMF Pond

I, Victor A. Modeer, being a Registered Professional Engineer in good standing in the State of Illinois, do hereby certify, to the best of my knowledge, information, and belief that the information contained in this CCR Rule Report, and the underlying data in the operating record, has been prepared in accordance with the accepted practice of engineering. I certify, for the above-referenced CCR Unit, that the initial structural stability assessment dated October 2016, 2016 was conducted in accordance with the requirements of 40 CFR § 257.73(d).

Printed Name

Date

October 2016

About AFCOM

AECOM (NYSE: ACM) is a global provider of professional technical and management support services to a broad range of markets, including transportation, facilities, environmental, energy, water and government. With nearly 100,000 employees around the world, AECOM is a leader in all of the key markets that it serves. AECOM provides a blend of global reach, local knowledge, innovation, and collaborative technical excellence in delivering solutions that enhance and sustain the world's built, natural, and social environments. A Fortune 500 company, AECOM serves clients in more than 100 countries and has annual revenue in excess of \$19 billion.

More information on AECOM and its services can be found at www.aecom.com.

ATTACHMENT Q



Submitted to Illinois Power Generating Company 134 Cips Lane Coffeen, IL 62017 Submitted by AECOM 1001 Highlands Plaza Drive West Suite 300 St. Louis, MO 63110

October 2016

CCR Rule Report: Initial Safety Factor Assessment

For

GMF Pond

At Coffeen Power Station

1 Introduction

This Coal Combustion Residual (CCR) Rule Report documents that the Gypsum Management Facility (GMF) Pond at the Illinois Power Generating Company Coffeen Power Station meets the safety factor assessment requirements specified in 40 Code of Federal Regulations (CFR) §257.73(e). The GMF Pond is located near Coffeen, Illinois in Montgomery County, approximately 0.6 miles north of the Coffeen Power Station. The GMF Pond serves as the primary wet impoundment basin for gypsum produced by the wet scrubber system at the Coffeen Power Station.

The GMF Pond is an existing CCR surface impoundment as defined by 40 CFR §257.53. The CCR Rule requires that the initial safety factor assessment for an existing CCR surface impoundment be completed by October 17, 2016.

The owner or operator of the CCR unit must obtain a certification from a qualified professional engineer stating that the initial safety factor assessment meets the requirements of 40 CFR § 257.73(e). The owner or operator must prepare a safety factor assessment every five years.

2 Initial Safety Factor Assessment

40 CFR §257.73(e)(1)

The owner or operator must conduct initial and periodic safety factor assessments for each CCR unit and document whether the calculated factors of safety for each CCR unit achieve the minimum safety factors specified in (e)(1)(i) through (iv) of this section for the critical cross section of the embankment. The critical cross section is the cross section anticipated to be the most susceptible of all cross sections to structural failure based on appropriate engineering considerations, including loading conditions. The safety factor assessments must be supported by appropriate engineering calculations.

- (i) The calculated static factor of safety under the long-term, maximum storage pool loading condition must equal or exceed 1.50.
- (ii) The calculated static factor of safety under the maximum surcharge pool loading condition must equal or exceed 1.40.
- (iii) The calculated seismic factor of safety must equal or exceed 1.00.
- (iv) For dikes constructed of soils that have susceptibility to liquefaction, the calculated liquefaction factor of safety must equal or exceed 1.20.

A geotechnical investigation program and stability analyses were performed to evaluate the design, performance, and condition of the earthen dikes of the GMF Pond. The exploration consisted of cone penetration test soundings. Data collected from the geotechnical investigation, available design drawings, construction records, inspection reports, previous engineering investigations, and other pertinent historic documents were utilized to perform the safety factor assessment and geotechnical analyses.

In general, the subsurface conditions at the GMF Pond consist of medium stiff to stiff embankment fill (clay) overlying medium stiff to stiff clay, overlying soft to very soft clay, with in turn overlies stiff to hard glacial till (clay) with dense to very dense sand and gravel. Phreatic water is typically near the embankment/foundation interface at the GMF Pond.

Four (4) representative cross sections were analyzed using limit equilibrium slope stability analysis software to evaluate stability of the perimeter dike system and foundations. The cross sections were located to represent critical surface geometry, subsurface stratigraphy, and phreatic conditions across the site. Each cross section was evaluated for each of the loading conditions stipulated in §257.73(e)(1).

The Soils Susceptible to Liquefaction loading condition, §257.73(e)(1)(iv), was not evaluated because a liquefaction susceptibly evaluation did not find soils susceptible to liquefaction within the GMF Pond dikes. As a result, this loading condition is not applicable to the GMF Pond.

Results of the Initial Safety Factor Assessments, for the critical cross-section for each loading condition, are listed in **Table 1** (i.e., the table identifies the lowest calculated factor of safety for any one of the four analyzed cross sections for each loading condition).

Table 1 – Summary of Initial Safety Factor Assessments

Loading Conditions	§257.73(e)(1) Subsection	Minimum Factor of Safety	Calculated Factor of Safety
Maximum Storage Pool Loading	(i)	1.50	3.45
Maximum Surcharge Pool Loading	(ii)	1.40	3.45
Seismic	(iii)	1.00	1.47
Soils Susceptible to Liquefaction	(iv)	1.20	Not Applicable

Based on this evaluation, the GMF Pond meets the requirements in §257.73(e)(1).

3 Certification Statement

CCR Unit: Illinois Power Generating Company; Coffeen Power Station; GMF Pond

A MODEER JR.

I, Victor A. Modeer, being a Registered Professional Engineer in good standing in the State of Illinois, do hereby certify, to the best of my knowledge, information, and belief that the information contained in this CCR Rule Report, and the underlying data in the operating record, has been prepared in accordance with the accepted practice of engineering. I certify, for the above-referenced CCR Unit, that the initial safety factor assessment dated October 13, 2016 meets the requirements of 40 CFR §257.73(e).

Printed Name

Date

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ATTACHMENT R



Submitted to Illinois Power Generating Company 134 Cips Lane Coffeen, IL 62017 Submitted by AECOM 1001 Highlands Plaza Drive West Suite 300 St. Louis, MO 63110

October 2016

CCR Rule Report: Initial Inflow Design Flood Control System Plan

For

GMF Pond

At Coffeen Power Station

1 Introduction

This Coal Combustion Residual (CCR) Rule Report documents that the initial inflow design flood control system plan for Gypsum Management Facility (GMF) Pond at the Illinois Power Generating Company Coffeen Power Station meets the requirements specified in 40 Code of Federal Regulations (CFR) §257.82. The GMF Pond is located near Coffeen, Illinois in Montgomery County, approximately 0.6 miles north of the Coffeen Power Station. The GMF Pond serves as the primary wet impoundment basin for gypsum produced by the wet scrubber system at the Coffeen Power Station.

The GMF Pond is an existing CCR surface impoundment as defined by 40 CFR §257.53. The CCR Rule requires that the initial inflow design flood control system plan for an existing CCR surface impoundment be prepared by October 17, 2016. The plan must document how the inflow design flood control system has been designed and constructed to meet the requirements of 40 CFR §257.82 and be supported by appropriate engineering calculations.

The owner or operator of the CCR unit must obtain a certification from a qualified professional engineer stating that the inflow design flood control system meets the requirements of 40 CFR §257.82. The owner or operator must prepare an inflow design flood control system plan every five years.

2 Initial Inflow Design Flood Control System Plan

40 CFR §257.82

- (a) The owner or operator of an existing ... CCR surface impoundment ... must design, construct, operate, and maintain an inflow design flood control system as specified in paragraphs (a)(1) and (2) of this section.
 - (1) The inflow design flood control system must adequately manage flow into the CCR unit during and following the peak discharge of the inflow design flood specified in paragraph (a)(3) of this section.
 - (2) The inflow design flood control system must adequately manage flow from the CCR unit to collect and control the peak discharge resulting from the inflow design flood specified in paragraph (a)(3) of this section.
 - (3) The inflow design flood is:
 - (i) For a high hazard potential CCR surface impoundment, ..., the probable maximum flood;
 - (ii) For a significant hazard potential CCR surface impoundment, ..., the 1,000-year flood;
 - (iii) For a low hazard potential CCR surface impoundment, ..., the 100-year flood; or
 - (iv) For an incised CCR surface impoundment, the 25-year flood.
- (b) Discharge from the CCR unit must be handled in accordance with the surface water requirements under §257.3-3.

Analyses completed for the initial inflow design flood control system plan of the GMF Pond are described in the following subsections. Data and analysis results in the following subsection are based on spillway design information shown on design drawings, construction information, topographic surveys, information about operational and maintenance procedures provided by Illinois Power Generating Company, and field measurements collected by AECOM. The analysis approach and results of the hydrologic and hydraulic analyses are presented in the following subsections.

The GMF Pond has a high hazard potential, based on the initial hazard potential classification assessment performed by Stantec in 2016 in accordance with §257.73(a)(2).

2.1 Initial Inflow Design Flood Control Systems (§257.82(a))

An initial inflow design flood control system plan, supported by a hydraulic and hydrologic analysis, was developed for the GMF Pond by evaluating the effects of a 24-hour duration design storm for the probable maximum flood (PMF) Inflow Design Flood (IDF) using a hydrologic HydroCAD (Version 10) computer model and a starting water surface elevation of 621.2 feet. The computer model evaluated the GMF Pond's ability to collect and control the PMF IDF under existing operational and maintenance procedures. Rainfall data for the PMF IDF, which corresponds to the probable maximum precipitation rainfall event, was obtained from the National Weather Service Hydrometeorological Report No. 51 (HMR 51). The HMR 51 rainfall depth is 34.25 inches.

The HydroCAD model results for the GMF Pond indicate that the CCR unit has sufficient storage capacity and spillway structures to adequately manage (1) flow into the CCR unit during and following the peak discharge of the PMF IDF and (2) flow from the CCR unit to collect and control the peak discharge resulting from the PMF IDF. The peak water surcharge elevation is 623.8 feet during the IDF, and the minimum crest elevation of the GMF Pond dike is 631.0 feet. Therefore, overtopping is not expected.

Based on this evaluation, the GMF Pond meets the requirements in §257.82(a).

2.2 Discharge from the CCR Unit (§257.82(b))

40 CFR §257.82(b) provides that the discharge from the CCR unit must be handled in accordance with the surface water requirements under 40 CFR §257.3-3, which states the following:

- (a) For purposes of section 4004(a) of the Act, a facility shall not cause a discharge of pollutants into waters of the United States that is in violation of the requirements of the National Pollutant Discharge Elimination System (NPDES) under section 402 of the Clean Water Act, as amended.
- (b) For purposes of section 4004(a) of the Act, a facility shall not cause a discharge of dredged material or fill material to waters of the United States that is in violation of the requirements under section 404 of the Clean Water Act, as amended. (c) A facility or practice shall not cause non-point source pollution of waters of the United States that violates applicable legal requirements implementing an areawide or Statewide water quality management plan that has been approved by the Administrator under section 208 of the Clean Water Act, as amended.
- (d) Definitions of the terms Discharge of dredged material, Point source, Pollutant, Waters of the United States, and Wetlands can be found in the Clean Water Act, as amended, 33 U.S.C. 1251 et seq., and implementing regulations, specifically 33 CFR part 323 (42 FR 37122, July 19, 1977).

The handling of discharge was evaluated by reviewing design drawings, operational and maintenance procedures, conditions observed in the field by AECOM, and the inflow design flood control system plan developed per §257.82(a).

Based on this evaluation, the GMF Pond does not discharge into waters of the United States. Clear water from the GMF Pond flows downstream into the GMF Recycle Pond. Hydraulic and hydrologic analyses performed as part of the initial inflow design flood control system plan found the GMF Pond adequately manages outflow during the PMF IDF, as overtopping of the GMF Pond embankments is not expected.

Therefore, discharge into waters of the United States is not expected during normal or PMF IDF conditions, and the GMF Pond meets the requirements in §257.82(b).

3 Certification Statement

CCR Unit: Illinois Power Resources Generating, LLC; Coffeen Power Station; GMF Pond

VICTOR A MODER SC.
Printed Name

18/13/16

Date

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ATTACHMENT S

PART 845 SAFETY AND HEALTH PLAN

COFFEEN POWER PLANT GMF GYPSUM STACK POND, GMF RECYCLE POND, AND ASH POND NO. 1

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PART 845 SAFETY AND HEALTH PLAN Coffeen Power Plant GMF Gypsum Stack Pond, GMF Recycle Pond, and Ash Pond No. 1

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ACRONYMS & ABBREVIATIONS

% Percent Section

35 I.A.C. Title 35 of the Illinois Administrative Code 29 C.F.R. Title 29 of the Code of Federal Regulations

ACGIH American Conference of Governmental Industrial Hygienists

AP1 Ash Pond No. 1

CCR Coal Combustion Residual
CPP Coffeen Power Plant

GMF Gypsum Management Facility GMF GSP GMF Gypsum Stack Pond

GMF RP GMF Recycle Pond

HAZWOPER Hazardous Waste Operations and Emergency Response

ID identification

IDLH Immediately Dangerous to Life and Health
IEPA Illinois Environmental Protection Agency
IPGC Illinois Power Generating Company

kV kilovolt

NID National Inventory of Dams

NIOSH National Institute for Occupational Safety and Health

No. number

IDLH Immediately Dangerous to Life and Health

kV kilovolt

NIOSH National Institute for Occupational Safety and Health OSHA Occupational Safety and Health Administration

Part 845 35 I.A.C. Part 845: Residuals in Surface Impoundments

PEL Permissible Exposure Level

PFAS Per- and polyfluoroalkyl substances

PFD Personal Flotation Device

PNOR particulates not otherwise recognized

POC Point of Contact

PPE personal protective equipment

ppm parts per million SDS Safety Data Sheet

Site GMF GSP, GMF RP, and AP1
STEL Short Term Exposure Limit
TLV Threshold Limit Value
TWA time-weighted averages
USCG United States Coast Guard

REVISION SUMMARY

Revision Date	Description of Changes (Section title or number – description)	Responsible Party (individual name or title, company / agency name, document reference and date)

PREFACE

Illinois Power Generating Company (IPGC) has prepared this Safety and Health Plan in accordance with requirements set forth in Title 35 of the Illinois Administrative Code (35 I.A.C.) Part 845: Residuals in Surface Impoundments (Part 845), Section (§) 845.530. IPGC assessed health and safety hazards of its coal combustion residual (CCR) surface impoundments to develop and update this Safety and Health Plan.

This document describes the minimum anticipated protective measures necessary for worker health and safety at the Coffeen Power Plant (CPP) Gypsum Management Facility (GMF) Gypsum Stack Pond (GMF GSP; Vistra identification [ID] number [No.] 103, Illinois Environmental Protection Agency [IEPA] ID No. W1350150004-03, National Inventory of Dams [NID] No. IL50579), GMF Recycle Pond (GMF RP; Vistra ID No. 104, IEPA ID No. W1350150004-04, NID No. IL50578), and Ash Pond No. 1 (AP1; Vistra ID No. 101, IEPA ID No. W13501050004-01, NID No. IL50722), collectively referred to as the Site. Employees of IPGC, contract workers, and third-party contractors must read and comply with the contents of this document. The contents of this document are not intended to cover all situations that may arise nor to waive any provisions specified in Federal, State, and local regulations or site owner / contractor health and safety requirements.

Third-party contractors are accountable for the health and safety of their employees. Third-party contractors are required to prepare a Safety and Health Plan that meets the minimum requirements herein. However, no requirements or provisions within this plan shall be construed as an assumption of IPGC of their legal responsibilities as an employer.

This Safety and Health Plan will be reviewed and updated annually, at a minimum. The Safety and Health Plan will also be updated if facility operations change, or a new hazard is identified.

1. INTRODUCTION

This Safety and Health Plan has been developed to outline the requirements to be met by employees of IPGC, contract workers, and third-party contractors while performing any activity to construct, operate, or close the CCR Units at the Site. This Safety and Health Plan has been developed to meet the requirements of 35 I.A.C. § 845.530 and describes the responsibilities, training requirements, protective equipment, and safety procedures necessary to minimize the risk of injury, fires, explosion, chemical spills, material damage incidents, and near misses related to CCR activities. This Safety and Health Plan incorporates by reference the Occupational Safety and Health Administration (OSHA) regulations contained in Title 29 of the Code of Federal Regulations (29 C.F.R.) § 1910 and 29 C.F.R. § 1926.

The requirements and guidelines in this Safety and Health Plan are based on a review of available information and data, and an evaluation of identified on-site hazards. This Safety and Health Plan will be reviewed with persons assigned to work at the Site and will be available on-site.

1.1 Site Description/History

The CPP is a retired coal-fired power plant located in Montgomery County, in central Illinois, within Section 11 Township 7 North and Range 7 East. The CPP is approximately 2 miles south of the city of Coffeen and about 8 miles southeast of the city of Hillsboro, Illinois. The Site is located between the two lobes of Coffeen Lake to the west, east, and south, and is bordered by agricultural land to the north. The approximately 1,100-acre Coffeen Lake was built by damming the McDavid Branch of the East Fork of Shoal Creek in 1963 for use as an artificial cooling lake for the CPP. Appendix A is a site map showing the location of the CCR Units at the Site.

1.2 Facility Personnel

The following table outlines key personnel with respect to facility operations and health and safety.

Name	Position	Phone Number
Scott Bell	Primary Point-of-Contact / Plant Manager	217-248-7720
John Romang	Secondary Point-of-Contact / Environmental Manager	217-341-7319
Gate House	Security Guard	217-534-2363
Matt Ballance	Engineering Manager	618-343-7739 (office)
		618-792-7274 (mobile)
Jason Campbell	Dam Safety Manager	271-753-8904 (Springfield)
		217-622-3491 (mobile)
Stu Cravens	Senior Technical Expert	217-390-1503 (mobile)
Vic Modeer	Engineering Manager	618-541-0878
Charles Koudelka	Plant Closure Director	903-235-8633

1.3 Responsibilities

The following persons have responsibilities associated with communicating and implementing the Safety and Health Plan for the CCR Units at the Site.

1.3.1 IPGC Point of Contact

The IPGC Point of Contact (POC) is a management-level person who is requiring employees, contract workers, or third-party contractors to enter the Site. The IPGC POC is responsible to communicate Safety and Health Plan information and requirements to employees, contract workers, and third-party contractors, and oversee work performed in the Site to the extent necessary to confirm implementation of Safety and Health Plan requirements.

1.3.2 IPGC Employees

IPGC employees are directly hired by IPGC. They are required to implement and/or follow Safety and Health Plan requirements as applicable to their work and exercise their "stop work authority" if safety requirements are unclear or unanticipated site conditions or hazards are observed.

1.3.3 Contract Workers

Contract workers are those hired by IPGC through an agency firm. Similar to IPGC employees, contract workers are required to implement and/or follow Safety and Health Plan requirements as applicable to their work and exercise their "stop work authority" if safety requirements are unclear or unanticipated site conditions or hazards are observed.

1.3.4 Third-Party Contractor Employees

Third-party contractor employees work for firms under contract to IPGC. Third-party contractors include prime contractors and all of their lower tier subcontractors. Similar to IPGC employees, third-party contractors are required to implement Safety and Health Plan requirements as applicable to their work and exercise their "stop work authority" if safety requirements are unclear or unanticipated site conditions or hazards are observed.

1.3.5 Third-Party Contractor Safety Competent Person

Third-party contractors will be required to designate a Safety Competent Person. The Safety Competent Person must be in a management position (*e.g.*, superintendent, foreman, etc.) with OSHA 30-hour construction safety certification who may perform other duties, unless IPGC requires a dedicated Safety Competent Person. A Safety Competent Person must be on site at all times when the subcontractor has employees performing work for IPGC and must possess a sound working knowledge of pertinent OSHA regulations, this Safety and Health Plan, and other applicable safety requirements related to the scope of work. Third-party contractors must also designate a backup Safety Competent Person that possesses the same authority and training. The competent person will ensure timely correction of safety deficiencies identified by IPGC. The Safety Competent Person is responsible to ensure Safety and Health Plan requirements have been communicated to lower-tier subcontractors and enforce Safety and Health Plan requirements.

2. SITE ACCESS & CONTROL

This section outlines requirements for ensuring that only authorized personnel and visitors are permitted at the Site.

2.1 Facility Security

Elements of site control include restricting access to the Site to persons until they have met the training requirements outlined in this Safety and Health Plan and have been authorized to do so by the CPP POC or their representative.

Upon arrival to the facility all IPGC employees, contract workers, and third-party contractors must sign in at the gate house. The security guard will call the POC to confirm the individual(s) is authorized to enter the facility.

Upon arrival to the Site, all IPGC employees, contract workers, and third-party contractors must check in/out at Security. A COVID-19 screening must also be completed per Section 3.8.

2.2 Third-Party Contractor Management

Prior to working at the Site, all third-party prime contractors must maintain an active registration with ISNetworld and maintain a grade of A or B. Lower tier subcontractors are currently not required to be registered in ISNetworld, but this requirement may change at the discretion of IPGC.

2.3 Third-Party Contractor Safety and Health Plan

Prior to being authorized to conduct work at the Site, third-party contractors must develop and submit a Safety and Health Plan. The third-party contractor's Safety and Health Plan must be specific to the scope of work that they will be performing at the Site. The third-party contractor's Safety and Health Plan must meet or exceed all the requirements in this Safety and Health Plan, other IPGC requirements, and applicable regulations. All lower tier subcontractors of third-party contractors must meet the requirements in this Safety and Health Plan as well as the requirements outlined in the Safety and Health Plan of the third-party with whom they are contracted.

2.4 Authorized Personnel

At a minimum, authorized personnel who will be granted unescorted access to the project include IPGC employees, contract workers, and third-party contractors that meet the following:

- Reviewed this Safety and Health Plan and other applicable safety planning documentation
- Have completed all the training, medical surveillance, and drug screen and background investigation requirements as outlined in Section 3 of this Safety and Health Plan.
- Have received the Site Orientation Training

2.5 Visitors

Visitors must be escorted by Authorized Personnel through the Site if they have not reviewed this Safety and Health Plan or completed the training requirements outlined in Section 3 of this Safety and Health Plan. Visitors may not undertake any activity to construct, operate, or close a CCR surface impoundment.

2.6 Communication

Communication between workers and emergency services must be maintained at all times. Cellular service is consistently available and can be relied upon to summon emergency services.

3. TRAINING & MEDICAL REQUIREMENTS

Project personnel must be properly trained for the type of work being performed and in accordance with 35 I.A.C. § 845.530, 29 C.F.R. § 1926 and 29 C.F.R. § 1910, and IPGC policies. Additionally, personnel working in areas regulated by the OSHA Hazardous Waste Operations and Emergency Response (HAZWOPER) standards (29 C.F.R. § 1910.120 and 29 C.F.R. § 1926.65) must have current medical surveillance. All employees, contractors, and third-party contractors must complete the following prior to beginning any activity to construct, operate, or close the CCR Units at the Site.

3.1 HAZWOPER Training

35 I.A.C. § 845.530(c)(2)(E) requires that all employees, contract workers, and third-party contractors be trained in accordance with 29 C.F.R. § 1910.120 and 29 C.F.R. § 1926.65. The following training will be completed as required by job function:

- **OSHA 40-Hour Training** per 29 C.F.R. § 1910.120 and 29 C.F.R. § 1926.65, for those personnel who are expected to have extensive contact with contaminated materials and/or may be required to wear a respirator.
- **OSHA 24-Hour Training** per 29 C.F.R. § 1910.120 and 29 C.F.R. § 1926.65, for those personnel who are expected to have minimal contact with contaminated materials and will NOT be required to wear a respirator.
- **OSHA 8-hour Supervisor Training** per 29 C.F.R. § 1910.120 and 29 C.F.R. § 1926.65, for Site Supervisors, Foremen, Superintendents, and others who will be directing and managing site activities.
- **OSHA 8-hour Refresher** per 29 C.F.R. § 1910.120 and 29 C.F.R. § 1926.65, completed within 12 months of initial 40-hour or 24-hour training and annually thereafter.

The following matrix outlines HAZWOPER training requirements based on typical job functions at the Site. It is not intended to be all inclusive, new job functions must be evaluated per 29 C.F.R. § 1910.120 and 29 C.F.R. § 1926.65.

Training	Job Function
OSHA 40-hour	Ash handlers
OSHA 24-hour	Personnel not required to handle CCR materials
OSHA 8-hour Supervisor Training	Third-Party Contractor Safety Competent Persons
OSHA 8-hour refresher	All personnel

3.2 OSHA Construction Outreach Training

35 I.A.C. § 845.530(c)(2)(E) requires that all employees, contract workers, and third-party contractors complete an OSHA 10-hour or 30-hour construction safety training. These trainings will be completed as follows:

- All employees, contract workers, and third-party contract employees: OSHA 10-hour or 30-hour construction outreach training.
- Supervisors, superintendents, foreman and safety professionals: OSHA 30-hour construction outreach training.

3.3 Site Safety and Health Plan Review

Pursuant to 35 I.A.C. § 845.530(d)(e), before beginning any activity at the Site, and annually thereafter, all IPGC employees, contract workers, and third-party contractors must review the content of this HASP. After reviewing this Safety and Health Plan all personnel will understand the following:

- Procedures for using, inspecting, repairing, and replacing facility emergency and monitoring equipment
- Communications or alarm systems outlined in Section 6
- Response to fires and explosions outlined in Section 6
- · Response to a spill or release of CCR
- Information about chemical hazards and hazardous materials outlined in Section 5
- The use of engineering controls, administrative controls, and personal protective equipment (PPE) outlined in Section 4

All personnel will acknowledge this HASP by signing the *Safety and Health Plan Acknowledgment Form (Appendix B)*.

3.4 Emergency and Monitoring Equipment Training

All IPGC employees, contract workers, and third-party contractors must be aware of how to respond to alarms and other emergencies as outlined in Section 6 of this plan. Individuals may only use facility emergency and monitoring equipment if they have been trained in their use and authorized to do so by the designated POC. Additionally, a written release may need to be completed as required by Vistra Corporate Procedure FFA-POL-0006.

Individual IPGC employees and contract workers may be responsible for using, inspecting, repairing and replacing facility emergency monitoring equipment. These individuals will be trained in accordance with procedures identified by IPGC. These individuals will review and adhere to the manufacturer's instructions, where applicable.

Third-party contractors are responsible for inspecting, repairing, and replacing any owned emergency (*i.e.*, fire extinguishers) and monitoring equipment (*i.e.*, air monitoring equipment). Third-party contractors will maintain procedures for using, inspecting, repairing, and replacing owned emergency and monitoring equipment that is consistent with the manufacturer's requirements. Third-party contractor employees who are responsible for this equipment will be trained in procedures for using, inspecting, and repairing owned equipment by their employer.

3.5 Hazard Communication

All employees, contract workers, and third-party contractors must be trained in chemical hazards (if any) associated with their work in accordance with 29 C.F.R. § 1910.1200. Work tasks performed on the Site may include exposure to compounds identified in the Hazard Communication section of this Safety and Health Plan and is included as part of the Safety and Health Plan Review outlined previously in Section 3.3.

3.6 Medical Surveillance

All employees, contract workers, and third-party contractors engaged in operations specified in 29 C.F.R. § 1910.120 and 29 C.F.R. § 1926.65 and meet one of the criteria outlined in 29 C.F.R. § 1910.120(f)(2) and 29 C.F.R. § 1926.65(f)(2) must participate in a medical surveillance program that is administered by their employer. The criteria for participating in a medical surveillance program are:

- All employees who are or may be exposed to hazardous substances at or above the
 established permissible exposure limit, without regard to the use of respirators, for 30 days or
 more a year;
- All employees who wear a respirator for 30 days or more a year; or
- All employees who are injured, become ill or develop signs or symptoms due to possible overexposure involving hazardous substances or health hazards from an emergency response or hazardous waste operation.

The medical surveillance program must result in documentation that an individual is cleared to work on sites covered by 29 C.F.R. § 1910.120 and 20 C.F.R. § 1926.65 and is medically fit to wear a respirator when applicable.

3.7 Drug Screen and Background Investigations

IPGC requires that contract worker agencies and third-party contractors are responsible for ensuring that all personnel have completed and passed a drug and alcohol test and background investigation prior to on-site work as described in Appendix C.

3.8 COVID-19 Site Entry Guidelines

All personnel entering Vistra work sites shall review and adhere to the site entry guidelines provided in Appendix D.

3.9 Document Management

IPGC will maintain employee and contract employee training and medical surveillance records in the main office. Third-party contractors are responsible for maintaining training and medical surveillance documentation for their employees. Third-party contractors will produce documentation upon IPGC request.

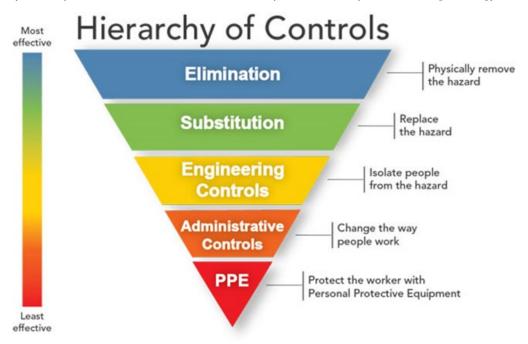
3.10 Industrial Hygiene Sampling Records

Upon receipt of exposure sampling results IPGC and third-party contractors must distribute exposure sampling results to employees within 15 business days unless otherwise required by applicable regulation. All personnel exposure sampling results and records must be maintained by the employee's company for at least 30 years following termination of employment.

4. HAZARD & CONTROLS

The following section outlines general controls for the hazards and controls. Third-party contractors are still responsible for developing a Safety and Health Plan that incorporates requirements of this Safety and Health Plan, other safety requirements for the CPP, as well as the third-party contractor's safety policies and procedures. Safety and Health Plans developed by third-party contractors must be specific to the site and the anticipated work means and methods. Safety and Health Plans that consist of only standard operating procedures or are not otherwise specific to the work performed at the Site will not be accepted by the IPGC.

IPGC requires that a hierarchy of controls be considered when performing work at the Site. Implement controls that favor elimination, substitution, and engineering over the use of administrative controls and PPE when feasible. See the figure below for additional guidance (courtesy of the National Institute for Occupational Safety and Health [NIOSH]).



4.1 Ash/Unstable Surfaces

At least 24 hours prior to working in or on an ash pond, third-party contractors must notify the facility POC. Work in or on an ash pond may not begin until the facility POC has approved the work. Upon completion of the work, third-party contractors must notify the POC that they have left the ash pond.

When working on ash ponds or unstable surfaces the following requirements must be implemented where applicable and feasible. The following table summarizes safety controls for work performed in ash ponds and on unstable surfaces and are aligned to the hierarchy of controls:

Elimination	Substitution	Engineering	Administrative	PPE
Change the work task or work methods so that work on ash ponds is no longer required	Use the lightest available tracked equipment to reduce ground pressure	Use crane mats or other cribbing to support heavy equipment on ash ponds	Traverse compacted paths that have previously been used by heavy equipment	Use a restraint (tethering) system to prevent falls or slips into unstable ash pond surfaces or surface water that represents a drowning hazard

Elimination	Substitution	Engineering	Administrative	PPE
			If an unstable condition exists, complete a Next Level Up Pre-Job Brief prior to accessing the ash pond.	
			Approach the ash pond from the most stable direction	
			Inspect travel paths for recent terrain shifts, particularly following heavy rains or rapid dewatering	
			Working alone on ash ponds is prohibited without pre-approval from the POC.	
			When a drowning hazard exists, implement requirements for working on/near water as outlined in Section 4.4.	
			Implement an emergency response plan with trained responders for falls into (or engulfment by) ash	

4.2 Ash Inhalation/Airborne Exposure

Ash that becomes airborne due to site activities or environmental conditions may result in an exposure to its components as outlined in Section 5.1. IPGC and third-party contractors are responsible for ensuring their respective employees' and contract workers' exposures are below occupational exposure limits. Upon request, third-party contractors must demonstrate to IPGC that exposure control methods are adequate. The following table summarizes airborne exposure controls and is aligned to the hierarchy of controls:

Elimination	Substitution	Engineering	Administrative	PPE
Change the work task or work methods so that work on ash ponds is no longer required	Substitute manual work methods for those that can be completed from the cab of a vehicle	Continually wet work areas to reduce the amount of ash that becomes airborne Equip vehicles and heavy equipment cabs with filters. Clean and change filters as required	Conduct air monitoring or exposure sampling to confirm that airborne exposure is below regulatory limits	If exposure levels are above the PEL, equip employees with respirators appropriate to the level of exposure

4.3 Stuck Vehicles/Equipment

If a vehicle or piece of equipment becomes stuck, a third-party towing or wrecking company who is trained in vehicle extraction must be retained and the POC will be notified. The POC will make recommendations on extraction companies depending on the type of vehicle/equipment that is stuck. Third-party contractors may extract their own vehicle if they have an approved extraction plan, and a competent person is on site to implement the extraction. The extraction plan shall be included as part of the third-party contractor's reviewed and approved Safety and Health Plan. The above notifications are still required.

The hazards presented by stuck vehicles/equipment must not be underestimated. While the weight of the stuck equipment can be calculated, it's impossible to precisely calculate the other forces that are pulling against the towing vehicle which requires special training and experience to properly size towing equipment and select towing techniques. This is especially true for "complex" or high-hazard extractions involving equipment stuck at axle depth (or beyond) or sloped surfaces or any area where extraction activities could trigger shifts in the ground surface. No chains shall be used to remove stuck vehicles/equipment.

The following table summarizes safety controls related to stuck vehicles and equipment and are aligned to the hierarchy of controls:

Elimination	Substitution	Engineering	Administrative	PPE
Change the work task or work methods so that work on ash ponds is no longer required	Use the lightest available tracked equipment to reduce ground pressure Substitute tracked equipment for wheeled equipment	Use crane mats or other cribbing to support heavy equipment on ash ponds Lighten the load – Remove materials from stuck vehicles or equipment prior to extraction if possible	Only persons trained in vehicle extraction are permitted to remove stuck vehicles/equipment A professional towing/wrecking service is required Prepare for spills (damage to fuel or hydraulic systems)	All persons involved in removing stuck equipment must wear PPE that includes hard hat, safety boots, safety glasses, high visibility vests, and cut resistant gloves

4.4 Working Near/Over Water

All employees, contract workers, and third-party contractors must wear a United States Coast Guard (USCG) approved personal floatation device (PFD), when within 6 feet of water, over water, and/or wading in water where the danger of drowning exists. The PFD must be properly

secured to the wearer, free of all defects including rips, tears, stress, and fading, and be kept clean and free of excessive dirt and oil.

If the possibility of falling into water has been eliminated through the use of guardrails, fall restraint, or other method, the use of a PFD is no longer required.

When performing work on water from a vessel, at least one lifesaving rescue vessel (e.g., a skiff) shall be immediately available at locations where employees are working over, in, on, or adjacent to water where the danger of drowning exists. However, if the water is so shallow that rescuers could simply walk/run into the water body without endangering themselves and/or others or the work was being conducted very close to shore (e.g., the length of the skiff from shore would be greater than the working distance from shore and/or the skiff would foul on the bottom), a skiff would not be required.

The following table summarizes the requirements for working over/near water where a drowning hazard exists and are aligned to the hierarchy of controls:

Elimination	Substitution	Engineering	Administrative	PPE
Change the work task or work methods so that work near a drowning hazard is no longer required		Install guardrails that separate work areas from the drowning hazard	All work to be performed by at least two people where each is equipped with proper safety gear and capable of summoning emergency rescue	All personnel are required to wear suitable PFDs
		Utilize equipment (crowd-control barricades, safety fence, etc.) that will keep personnel at least 6 feet from a drowning hazard	When working on water use of a rescue skiff as outlined above	
			Use of a ring buoy with 90 feet of braided polycarbonate (or equivalent) line	
			Ring buoys must be positioned within 100 feet of work (maximum of 200 feet spacing)	

4.5 Heavy Equipment

All heavy equipment operators must be competent and authorized to operate each piece of heavy equipment. Forklift and telehandler (e.g., Lull, JLG) operators must have a license or certificate that indicates they have passed a written test and "road" test for the equipment they will be operating within the last 3 years. Third-party contractors will provide proof of qualification upon request of IPGC.

Persons working around heavy equipment must implement the "25 Foot Rule." The 25 Foot Rule requires that persons get the operator's attention and permission prior to approaching closer than 25 feet to heavy equipment. Persons must walk quickly through blind spots. Loitering in heavy equipment blind spots (especially to the rear) must be avoided.

Temporary fuel storage tanks will be labelled as to their content and be protected from collision by Site vehicles using solid barricades including balusters, chain link fence, or equivalent. Spill kit (55-gallon sorbent capacity contained in an overpack) and one 20-pound Type ABC fire extinguisher will be located within 45 feet of fueling areas. Tanks will be rated for above ground use and will be double walled or have secondary containment in case of a leak. Tanks and dispensing hose will be bonded and grounded. On-site filling of fuel storage tanks will be completed with trucks that have automatic over-flow shutoffs. These trucks will be properly bonded to the storage tank and meet all of the other storage tank requirements. Temporary secondary containment must be provided in the refueling area that includes the storage tank and dispensing hoses.

Elimination	Substitution	Engineering	Administrative	PPE
		Heavy equipment (and vehicles) must be equipped with backup alarms, horns, roll- over protection (when feasible)	Operators must be competent and authorized	Operators must use seatbelts when equipped
		Vehicles and heavy equipment operated at night must have headlights, tail lamps, and reflectors	Forklift operators must have a current license or certificate (within 3 years)	High visibility vests are required when working around heavy equipment
			All vehicles and equipment must be turned off when not in use	
			Operators must inspect equipment daily prior to use	
			Persons working near heavy equipment must follow the "25 Foot Rule" and avoid lingering in blind spots as outlined above	
			Always obey site speed limits – 15 mph unless otherwise posted	

4.6 Overhead Powerlines

All overhead powerlines must be assumed to be energized until confirmed otherwise. The minimum clearance distance for equipment working near energized power lines must be in accordance with table found in 29 C.F.R. § 1926.1408(h).

The following table summarizes safety controls for work near energized power lines:

Elimination	Substitution	Engineering	Administrative	PPE
Plan to work away from powerlines	Use heavy equipment with shorter booms/attachments to avoid coming close to power lines	Contact the utility owner to deenergize the line	Install signs to warn personnel of overhead powerlines	
		Contact the utility owner to install insulated sleeves over energized lines	Install a non- conductive distance marker to delineate minimum clearance	
			Use a dedicated spotter to ensure equipment does not enter minimum clearance distances	

4.7 Severe Weather

Severe weather conditions include but are not limited to high winds, electrical storms, heavy rain, and tornados can cause hazardous conditions at CCR surface impoundments. The primary control for severe weather is monitoring weather reports prior to beginning work and as work occurs throughout the day.

Monitor lightning using a commercially available mobile application if cellular service is available. When lightning is observed within 10 miles of the CCR surface impoundment, or a storm is imminent, take shelter in the nearest solid structure or fully enclosed vehicle. If possible secure all tools, materials, and equipment prior to the storm arriving. Work may resume 30 minutes after the last lightning strike is observed within 10 miles. The severe weather shelter location is the storm shelter located next to the security office.

Do not conduct work on a CCR surface impoundment when there is a risk for tornados in the area. If on a CCR surface impoundment and a tornado forms, seek the nearest substantial shelter. The closest tornado shelter is the storm shelter located next to the security office; shelter locations will be reviewed during the Site Orientation Training. If no shelter is available, attempt to evacuate to a shelter using a vehicle. If a tornado forms and you are not in a shelter, take one of the following actions:

- Stay in a vehicle with the seat belt on, keep your head below the windows and cover it with your hands
- If there is an area which is noticeably lower than the work area, lie in that area and cover your head with your hands.

The following table summarizes safety controls related to severe weather:

Elimination	Substitution	Engineering	Administrative	PPE
Plan outdoor tasks			Prior to beginning	
on days with low			outdoor work	
potential for			monitor the day's	
severe weather.			weather.	

Elimination	Substitution	Engineering	Administrative	PPE
			Periodically monitor weather throughout the day. Use a weather app which issues alerts for severe weather and lightning, assuming cell service is available	
			Utilize a weather radio if cellular service is inconsistent	
			Stop all outdoor work and seek shelter when lightning is observed	

4.8 Heat Stress

Heat stress can be a significant hazard, especially for workers wearing protective clothing. Depending on the ambient conditions and the work being performed, heat stress can occur very rapidly, within as little as 15 minutes. Employees, contract workers, and third-party contractors will be instructed in the identification of a heat stress victim, the first-aid treatment procedures for the victim, and in the prevention of heat stress incidents.

Workers will be encouraged to immediately report any heat-related problems that they experience or observe in fellow workers. Any worker exhibiting signs of heat stress and exhaustion should be made to rest in a cool location and drink plenty of water. Emergency help by a medical professional is required immediately for anyone exhibiting symptoms of heat stroke, such as red, dry skin, confusion, delirium, or unconsciousness. Heat stroke is a life-threatening condition that must be treated immediately by competent medical authority.

4.8.1 Heat Stress Prevention

To prevent heat stress, IPGC employees, contract workers, and third-party contractors will implement heat stress prevention measures as outlined in OSHA's Heat Index (below). A summary of these precautions is described below.

Heat Index	Risk Level	Protective Measures
Less than 91°F	Lower (Caution)	Basic heat safety and planning
91°F to 103°F	Moderate	Implement precautions and heighten awareness
103°F to 115°F	High	Additional precautions to protect workers
Greater than 115°F	Very High to Extreme	Triggers even more aggressive protective measures

Know the Symptoms: Some symptoms associated with heat stress are: Employees should be aware of these symptoms with themselves and with their co-workers:

- Elevated heart rate, lack of concentration, difficulty focusing on a task, fatigue
- Irritability and/or sickness
- · Cramps, rash, headache
- · Loss of desire to drink water
- Fainting
- Skin clammy, moist, and pale (severe heat exhaustion)
- Skin extremely dry and red (heat stroke)

Acclimatize: When high heat stress conditions arise, employees should be exposed to the heat for short work periods followed by longer periods of work. Acclimatization usually takes five (5) days and should be provided for all new employees and employees returning from an absence of two (2) weeks or more. Contact Corporate Health and Safety for proper procedures.

Hydration & Pace of Work: Make sure all employees intake plenty of water throughout the work day (sometimes as much as a quart per worker per hour) and let employees know where the drinking water is located. Adjust your work pace and expectations on how much work can be done during periods of high heat stress. Workers cannot do as much during periods of high heat stress compared with similar periods of low heat stress. After acclimatization, workers may be able to resume a more "normal" work pace as long as fluid intake is adequate.

Work/Rest Periods: If possible, heavy work should be scheduled during the cooler parts of the day (*i.e.*, early morning) and rest periods should be taken in cool areas for longer periods.

Personal Protective Equipment (PPE): Employees using PPE (*i.e.*, Tyvek® suits or other equipment which may retain heat) can be more susceptible to heat stress due to the fact that heat/sweat often cannot escape the suits and/or the equipment. Persons wearing PPE that contributes to heat stress require more hydration, longer rest periods, or a reduced pace of work. Also, more careful monitoring of each person's health status is required by co-workers and management.

The following table summarizes safety controls for heat related illnesses:

Elimination	Substitution	Engineering	Administrative	PPE
Perform outdoor, strenuous, tasks at cooler times of day/year	Use mechanized equipment in place of manual labor	Install fans or air conditioning units in the work area	Train all personnel to know the signs of heat stress/stroke and how to prevent it	Implement the use of cooling vests or other similar PPE
		Install a canopy to provide shade to work areas	Allow workers to acclimatize to the work environment	
		Provide cool, shaded break areas	Adjust work pace to allow for the effects of heat	
			Implement work/rest periods	

4.9 Cold Stress

The four environmental conditions that cause cold-related stress are low temperatures, high/cool winds (wind chill), dampness, and cold water. One, or any combination of these factors, can cause cold-related hazards. Cold stress, including frostbite and hypothermia, can result in severe health effects. Employees, contract employees, and third-party contractors will be instructed in the identification of a cold stress victim, the first-aid treatment procedures for the victim and in the prevention of heat stress incidents.

A dangerous situation of rapid heat loss may arise for any individual exposed to high winds and cold temperatures. Major risk factors for cold-related stresses include:

- Wearing inadequate or wet clothing thus increasing the effects of cold on the body.
- Taking certain drugs or medications such as alcohol, nicotine, caffeine, and medication thus inhibiting the body's response to the cold and/or impairing judgment.
- Having a cold or certain disease, such as diabetes, heart, vascular and thyroid problems, and thereby increasing susceptibility to the winter elements.
- Lower body-fat composition or other physiological differences. Statistics show that men experience far greater death rates due to cold exposure than women, potentially attributable to participation in risk-taking activities, lower body-fat composition and/or other physiological differences.
- Becoming exhausted or immobilized, especially due to injury or entrapment, thus speeding up the effects of cold weather.

The following table provides the resulting equivalent chill temperature to exposed skin because of increasing wind speeds at decreasing actual temperatures. Personnel shall be aware of predicted weather conditions before beginning site work and stay apprised of changes.

	Actual	Tempe	erature	Readi	ing (°F)							
Estimated Wind Speed (in mph)	50	40	30	20	10	0	-10	-20	-30	-40	-50	-60
	10. 01			70	Equiva	lent Chi	ll Tempe	erature (°F)		10	
calm	50	40	30	20	10	0	-10	-20	-30	-40	-50	-60
5	48	37	27	16	6	-5	-15	-26	-36	-47	-57	-68
10	40	28	16	4	-9	-24	-33	-46	-58	-70	-83	-95
15	36	22	9	-5	-18	-32	-45	-58	-72	-85	-99	-112
20	32	18	4	-10	-25	-39	-53	-67	-82	-96	-110	-12
25	30	16	0	-15	-29	-44	-59	-74	-88	-104	-118	-133
30	28	13	-2	-18	-33	-48	-63	-79	-94	-109	-125	-140
35	27	11	-4	-20	-35	-51	-67	-82	-98	-113	-129	-145
40	26	10	-6	-21	-37	-53	-69	-85	-100	-116	-132	-148
(Wind speeds greater than 40 mph have little additional effect.)	LITTLE DANGER In < hr with dry skin. Maximum danger of false sense of security		INCREASING DANGER Danger from freezing of exposed flesh within one minute. GREAT DANGER Flesh may freeze within 30 seconds.					1 30				
		Tı	renchfo	oot and	immersi	ion foot	may occ	ur at an	y point or	n this cha	ırt.	

per cold stress TLV

The following table summarizes safety controls for preventing cold stress:

Elimination	Substitution	Engineering	Administrative	PPE
Perform work during warm parts of the day or warmer parts of the year		Install heaters in enclosed work areas	Train all personnel on the symptoms of cold stress and how to prevent it	All personnel must wear multiple layers of clothing
		Provide a warm break area	Implement work/rest schedule	Utilize hand/foot warmers when required

An additional hazard in cold weather conditions is the increased risk for slips from the accumulation of ice and snow in general work areas, ruts where water is accumulated, and heavy equipment. The following table outlines controls that may be used for preventing slips:

Elimination	Substitution	Engineering	Administrative	PPE
Perform work during warm parts of the day or in areas free of accumulated areas		Clear snow in work areas		Use traction control devices (i.e., YakTrax) on work boots to provide additional traction.
		Apply salt/sand to icy areas		
		Use equipment to access work areas		

4.10 Biological Hazards

The following are biological hazards that may be present at the Site.

4.10.1 Ticks (Lyme Disease) & Mites

Although Lyme disease has been detected throughout the continental United States, it is prevalent primarily in certain areas in New England, the Mid-Atlantic and the northern Midwest states.

Although Lyme disease is the most common tickborne illness, other tickborne illnesses include southern tick-associated rash illness, Rocky Mountain spotted fever, ehrlichiosis, and tularemia. More information on Lyme disease and other tickborne illnesses can be found from the CDC.

Prevention

- Standard field gear (work boots, socks, and light-colored coveralls) provides good protection against tick bites, particularly if the joints are taped. However, even when wearing field gear, the following precautions shall be taken when working in areas that might be infested with ticks:
 - Wear long pants and long-sleeved shirts that fit tightly at the ankles and wrists, tape cuffs
 if necessary
 - Wear light colored clothing so ticks can be easily spotted
 - o Per- and polyfluoroalkyl substances (PFAS)-free tick repellents (DEET and Permethrin) must be used when walking in all overgrown areas. DEET (≥25 percent [%]) must be applied to skin while permethrin must be applied to clothes and allowed to dry. Spray outer clothing, particularly your pant legs and socks, BUT NOT YOUR SKIN, with an insect repellent that contains permethrin. For heavily infested tick areas, wear spun polypropylene coveralls that have been sprayed with permethrin.
 - Inspect clothing frequently
 - Inspect head and body thoroughly when you return from the field, particularly on your lower legs and areas covered with hair
 - When walking in wooded areas, wear a hard hat, and avoid contact with bushes, tall grass, or brush as much as possible

Removal

- Remove any ticks by tugging with tweezers or special tick removal tools
- Do not squeeze or crush the tick
- DO NOT use matches, a lit cigarette, nail polish, or any other type of chemical to "coax" the tick out

Treatment

- Disinfect the area with alcohol or a similar antiseptic after removal
- Notify the Safety Competent Person of the embedded tick
- For several days to several weeks after removal of the tick, look for the signs of the onset of Lyme disease, such as a rash.
- No further treatment is necessary for ticks embedded <48 hours.
- If other signs or symptoms of Lyme are observed (fever/chills, aches, and pains), then notify the Safety Competent Person and seek medical attention

The following table summarizes safety controls to reduce the hazards associated with ticks and mites.

Elimination	Substitution	Engineering	Administrative	PPE
Use mechanical equipment to remove overgrown vegetation		Remove overgrowth and excessive vegetation from walkways and work areas (provide safe access)	Train personnel on tick and mite prevention. Areas of vegetation overgrowth and/or debris piles should be considered "high risk" areas	Wear light-colored long sleeved shirt tucked into pants. Tuck pant legs into socks

Elimination	Substitution	Engineering	Administrative	PPE
			Perform frequent tick checks in the field and a thorough tick check after completing work activities	Apply Permethrin to clothes and DEET (20% or more) to exposed skin
			Call licensed pesticide contractors to remove infestations of bees, wasps, fire ants, etc.	

4.10.2 Insect Bites/Stings

Stinging/biting insects at the Site include spiders, wasps, and bees. Contact with these insects may result in project personnel experiencing adverse health effects that range from being mildly uncomfortable to being life-threatening. Therefore, insects present a serious hazard to project personnel, and extreme caution must be exercised whenever Site and weather conditions increase the risk of encountering stinging insects. Some of the factors related to stinging insects that increase the degree of risk associated with accidental contact are as follows:

- The nests for these insects are frequently found in remote wooded or grassy areas or equipment staging areas where equipment has not been moved recently.
- Some people are hypersensitive to the toxins injected by a sting, and when stung, experience a violent and immediate allergic reaction resulting in a life-threatening condition known as anaphylactic shock. Anaphylactic shock manifests itself very rapidly and is characterized by extreme swelling of the body, eyes, face, mouth, and respiratory passages.
- The hypersensitivity needed to cause anaphylactic shock, can in some people accumulate over time and exposure, therefore even if someone has been stung previously and not experienced an allergic reaction, there is no guarantee that they will not have an allergic reaction if they are stung again
- Spider bites generally only cause localized reactions such as swelling, pain, and redness. However, bites from a Black Widow or Brown Recluse, or if you are allergic to spiders, can cause symptoms that are more serious.
- If a worker knows that they are hypersensitive to bee, wasp, or hornet stings, or other insects, they must inform the Safety Competent Person prior to site work. Persons who have been prescribed epi-pens by their physician must have an epi-pen on the Site.
- Inspect any clothing or PPE that has been left for a period of time prior to putting it on. Shake out the clothing and inspect the inside of safety shoes/boots prior to putting them on
- Nests in active work areas must be eradicated. Small nests may be handled by Site personnel
 using consumer-type insecticide. A pest control contractor should be hired to handle large or
 difficult to reach nests.

The following table outlines safety controls to reduce the risk of hazards associated with stinging/biting insects.

Elimination	Substitution	Engineering	Administrative	PPE
Use mechanical equipment to remove overgrown vegetation		Remove overgrowth and excessive vegetation from walkways and work areas (provide safe access)	Train personnel on stinging/biting insect prevention. Areas of vegetation overgrowth and/or debris piles should be considered "high risk" areas	Wear light-colored long sleeved shirt tucked into pants. Tuck pant legs into socks
		Eradicate nests in the work area as outlined above.	Instruct personnel to inspect/shake out clothing and work boots that have been left for a period of time.	Apply Permethrin to clothes and DEET (20% or more) to exposed skin – NOTE this will not repel bees/wasps
			Instruct employees who are hypersensitive to insect bites/stings to carry their epipen while on site	

4.10.3 Venomous Snakes

There are four species of venomous snakes in Illinois, they are:

- Copperhead
- Cottonmouth Water Moccasin
- Timber rattlesnake
- Eastern Massasauga

Generally, these snakes are found in the southern one-third of the state, with the Cottonmouth Water Moccasin found mostly in the southernmost portions of Illinois. Snakes are generally found in tall grass, wood piles, or other covered areas. Snakes are generally not aggressive towards humans, but if they are encountered avoid the snake and do not provoke it. If bitten by a snake that may be venomous seek medical treatment.

The following table outlines safety controls to reduce the hazard associated with venomous snakes.

Elimination	Substitution	Engineering	Administrative	PPE
Use mechanical equipment to remove overgrown vegetation		Remove debris piles, overgrowth and excessive vegetation from walkways and work areas (provide safe access)	Train personnel on the identification of venomous snakes. Areas of vegetation overgrowth and/or debris piles should be considered "high risk" areas	If working in area with snakes cannot be avoided, wear snake chaps
			Instruct personnel to not disturb snakes if they identify one in their work area	

Elimination	Substitution	Engineering	Administrative	PPE
			Use caution when	
			moving staged	
			tools or materials	
			into which snakes	
			may have moved	

4.10.4 Poisonous Plants and Plant Hazards

Poison ivy and poison oak may be present at the Site. Poison ivy thrives in all types of light and usually grows in the form of a trailing vine; however, it can also grow as a bush and can attain heights of 10 feet or more. Poison ivy has pointed leaves that grow in clusters of three. Poison oak resembles poison ivy except that the poison oak leaves are more rounded rather than jagged like poison ivy, and the underside of poison oak leaves are covered with hair.

The skin reaction associated with contacting these plants is caused by the body's allergic reaction to toxins contained in oils produced by the plant. Becoming contaminated with the oils does not require contact with just the leaves. Contamination can be achieved through contact with other parts of the plant such as the branches, stems or berries, or contact with contaminated items such as tools and clothing. The allergic reaction associated with exposure to these plants will generally cause the following signs and symptoms:

Symptoms

- Blistering at the site of contact, usually occurring within 12 to 48 hours after contact and in many cases, persons experience almost immediate irritation.
- Reddening, swelling, itching, and burning at the site of contact.
- Pain, if the reaction is severe.
- Conjunctivitis, asthma, and other allergic reactions if the person is extremely sensitive to the poisonous plant toxin.

Prevention

- The best treatment appears to be removal of the irritating oil before it has had time to cause inflammation by wiping exposed skin with rubbing alcohol followed by washing with soap and water.
- A visual Site inspection and identification of the plants should be completed prior to starting work so that all individuals are aware of the potential exposure. Avoid contact with any poisonous plants on the Site, and keep a steady watch to identify, report, and mark poisonous plants found on the Site.
- · Avoid contact with, and wash daily, contaminated tools, equipment, and clothing.
- Barrier creams (Ivy Block®) and orally administered desensitization may prove effective and should be tried to find the best preventive solution.
- Keeping the skin covered as much as possible (i.e., long pants and long-sleeved shirts) in areas where these plants are known to exist will limit much of the potential exposure.
 PFAS-free spun polypropylene coveralls or Tyvek® may be worn to prevent contact of skin and clothes with poison ivy.

The following table outlines safety controls to mitigate the hazards associated with poisonous plants.

Elimination	Substitution	Engineering	Administrative	PPE
Use mechanical equipment to remove overgrown vegetation		Remove overgrowth and excessive vegetation from walkways and work areas (provide safe access)	Train personnel on the identification of poisonous plants	Wear pants and long sleeves when working in overgrown areas
			Instruct personnel to avoid areas where poisonous plants have been identified	Consider the use of a coverall when working in areas where these plants are present, especially for hypersensitive employees.
			Provide isopropyl alcohol along with soap and water to remove oils from skin, tools, and equipment.	

4.11 Working Alone

As outlined in Section 4.1, working alone while on an ash pond must be pre-approved by the POC. Working alone is prohibited for tasks deemed to be high risk by IPGC including, but not limited to, handling highly hazardous chemicals (sulfuric acid), work over/near water, excavation and trenching, hot work (grinding, welding and torch cutting), and elevated work that requires personal fall arrest. Third-party contractors are responsible for identifying potential high-risk tasks in their Safety and Health Plan and requiring that a buddy system be implemented while high risk work is performed. The buddy must be located in a safe area but may perform other tasks that do not prevent observing the person performing high risk work. Working alone may occur on and around other parts of the Site when there is no drowning hazard or risk of severe injury due to high-risk work.

Elimination	Substitution	Engineering	Administrative	PPE
	Modify work methods by substituting lower hazard methods for high hazard methods	Varies depending on the hazard, but for example, could include installing guardrails (temporary or permanent) which mitigates a fall hazard reducing the risk to levels where working alone may be permitted	Prohibit working alone on ash ponds and for other high hazard tasks without prior approval from the POC	
			Implement a buddy system whenever feasible (required for high hazard work)	

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Elimination	Substitution	Engineering	Administrative	PPE
			Implement a	
			worker check-in,	
			emergency alerting,	
			and monitoring	
			system	

5. HAZARD COMMUNICATION

As required by 35 I.A.C. § 845.530, the OSHA HAZWOPER standards (29 C.F.R. § 1910.120 and 29 C.F.R. § 1926.65) and OSHA Hazard Communication Standard, site personnel, subcontractors, and visitors must be informed of chemical hazards associated with their work area. The information in this section is based on:

- Recommendations in the most recent "NIOSH Pocket Guide to Chemical Hazards" by the Department of Health and Human Services, Centers for Disease Control and Prevention, and the NIOSH Pocket Guide.
- Requirements set forth in the OSHA regulations from as defined in Chapter 17 of 29 C.F.R. § 1910.1200(c) for all hazards not otherwise classified.

5.1 Coal Combustion Residuals

Primary exposure to CCR is through inhalation and skin contact. CCR is typically a fine, black, grey, or tan particulate. CCR is comprised of several components. The following table outlines the components of the CCR. The exact percentage of each component will vary based on the type of ash and location at the surface impoundment.

Chemical	Percentage	PEL	IDLH	ACGIH TLV	Symptoms of Exposure & Health Effects
Crystalline Silica	20-60% (total)	0.05 mg/m ³ (respirable)	25 mg/m³ (respirable)	0.025 mg/m ³ (respirable)	Cough, dyspnoea (breathing difficulty), wheezing; decreased pulmonary function, progressive respiratory symptoms (silicosis); irritation eyes; [potential occupational carcinogen]
Iron oxide	1-10%	5 mg/m ³	2500 mg/m ³	5 mg/m ³	Benign pneumoconiosis with X-ray shadows indistinguishable from fibrotic pneumoconiosis (siderosis)
Calcium oxide	10-30%	5 mg/m ³	25 mg/m ³	2 mg/m ³	irritation eyes, skin, upper respiratory tract; ulcer, perforation nasal septum; pneumonitis; dermatitis
Titanium dioxide	<3%	15 mg/m³	ND	10 mg/m ³	Lung fibrosis; [potential occupational carcinogen]
Aluminosilicates	10-60%				irritation eyes, skin, throat, upper
Magnesium oxide	2-10%	15 mg/m³ (PNOR)	J. ND	<i>3.</i>	respiratory system
Magnesium dioxide	<2%	- (FNOK)		(PNOR)	
Phosphorous pentoxide	≤2%				
Sodium oxide	1-10%				
Potassium oxide	≤1%				
Bromide salt	<0.1%				

Footnotes:

All values are 8-hour time-weighted averages (TWAs) unless otherwise indicated.

- PEL: Permissible Exposure Limit, the concentration an employee may be exposed to for an 8-hour work day for a 40-hour work week for which nearly all employees may be repeatedly exposed without adverse health effects.
- IDLH: IMMEDIATELY Dangerous to Life and Health, contaminant concentration which present the possibility for severe
 health consequences if exposed to the IDLH concentration without the appropriate personal protective equipment (PPE).
- ACGIH TLV: American Conference of Governmental Industrial Hygienists Threshold Limit Value
- mg/m³ = milligrams per cubic meter of air
- PNOR: Particulates Not Otherwise Regulated
- ND: Not Determined

5.2 Safety Data Sheets

Pursuant to 35 I.A.C. § 845.530(b)(3), IPGC will provide Safety Data Sheets (SDSs) to all employees, contract workers, and third-party contractors for the CCR located at the Site. Third-party contractors will incorporate SDSs in their Safety and Health Plan and provide SDSs to IPGC prior to bringing a material on site. SDSs are provided in Appendix E.

5.3 Signage

The absence of any of the following signage does not mean that a potential hazard does not exist. Signage will be posted by IPGC, but employees, contract workers, and third-party contractors must remain vigilant for changing site conditions.

To aid in hazard communication and pursuant to 35 I.A.C. § 845.530(f), IPGC will post the following signs at the Site:

- Signs identifying the hazards of CCR, including dust inhalation when handling CCR.
- Signs identifying unstable CCR areas that make the operation of heavy equipment hazardous.
- Signs identifying the necessary safety measures and necessary precautions, including the proper use of PPE.

The following signs may also be posted at the CCR units to aid in hazard communication:

• Overhead electrical lines that may be struck by heavy equipment of vehicles will have signs warning drivers of their presence.

6. EMERGENCY RESPONSE PLAN

This emergency response section details actions to be taken in the event of site emergencies. This section is consistent with the CPP Emergency Action Plan. All personnel on site must be familiar with emergency signals and the content of this section.

6.1 Emergency Phone Numbers & Notifications

Emergency Number			
Site Address Emergency Phone Number			
134 Cips Lane	911		
Coffeen, IL			
	Security Guard: 217-534-2363		

Medical	I Treatment
Local Hospital	Phone Number
Hillsboro Area Hospital	217-532-6111
1200 East Tremont Street	
Hillsboro, IL 62049	

Inc	ident Notifications	
Title	Name	Contact Number
Primary POC / Plant Manager	Scott Bell	217-248-7720
Secondary POC / Environmental Manager	John Romang	217-341-7319

6.2 Evacuation Signal

Upon hearing verbal notification (cell phones and radios) to evacuate all personnel will leave the work area and proceed to the muster point.

6.3 Muster Point

The muster point for the Site is located at the Plant Closure/Environmental Office. The severe weather shelter location is the storm shelter located next to the Security Office. The muster point and severe weather locations will be reviewed during the Site Orientation Training.

6.4 Calls for Emergency Support

In the case of an emergency site personnel will call the facility emergency phone number. Security will coordinate the arrival of on-site emergency personnel. The individual calling for emergency support will briefly explain the nature of the emergency and site conditions as follows:

- Indicate his/her name
- Location of emergency
- Description of emergency conditions that may require special rescue equipment, such as confined spaces, excavations, and elevated work platforms
- · Potential chemical hazards and recommended PPE

6.5 Fire & Explosion Response Plan

Trained site personnel may respond to incipient stage fires using a 20-pound Type ABC dry chemical fire extinguisher or hose. An incipient stage fire is a fire which is in the initial or beginning stage and which can be controlled or extinguished by portable fire extinguishers, Class II standpipe or small hose systems without the need for protective clothing or breathing apparatus. Personnel shall only attempt to extinguish the fire if it is safe to do so.

A fire that CANNOT be readily extinguished with a fire extinguisher will require evacuation of the work area personnel to Muster Point areas per this Safety and Health Plan. If personal injuries result from any fire or explosion, the procedures outlined in the Personal Injury Response Plan will also be followed.

All fires or explosions must be reported to the contacts outlined in Section 6.1 of this Safety and Health Plan.

6.6 Injury Response Plan

Treatment for minor injuries will be provided on site using available first aid supplies and personnel trained in first aid. All third-party contractors must have at least one individual on site who is trained in first aid, CPR, and AED use. Third-party contractors must provide their own first aid kits and AED. For minor injuries that are not life-threatening but require further medical attention, employees should be treated by occupational physicians at occupational clinics whenever possible. Treatment of minor injuries by emergency room or personal physicians should be avoided. When injured workers are released back to work with restrictions, all subcontractors are expected to accommodate those restrictions.

Emergency medical incidents include puncture wounds to the head, chest, and abdomen, serious head and spinal cord injuries, and loss of consciousness must be treated at the hospital emergency room listed in Section 6.1 of this Safety and Health Plan.

All injuries must be reported to the contacts outlined in Section 6.1 of this Safety and Health Plan.

6.7 Spill Response Plan

In general, IPGC employees, contract workers, and third-party contractors are trained and equipped to handle small spills associated with their work. Third-party contractors must include an approved spill response plan in their Safety and Health Plan. Site personnel will generally respond to spills as follows:

- Stop the leak immediately if it can be done without directly contacting the leaking material.
- Remove or stop all ignition sources (hot work, generators, etc.) that are within 25 feet of any part of the spill.
- On-site personnel should immediately secure the area to prevent unauthorized entry into the spill area.
- Although not likely given the anticipated types of spills, site personnel must immediately initiate evacuation if a spill may cause an explosion, death, or serious injury.
- Site personnel may only respond to incipient stage fires regardless if such fires are associated with a spill.
- PPE for spills to open areas generally requires Modified Level D PPE (poly-coat Tyvek®, nitrile gloves, and boot covers or boot decontamination). Over-boots or boot covers may also be used if persons cleaning the spill would have to walk on spilled materials. Latex gloves are not acceptable and will degrade with exposure to petroleum products.

6.8 CCR Spill or Release Response Plan

Response to minor or incidental spills of CCR will be managed as outlined in the General Spill Response Plan. An incidental release is a release of a hazardous substance which does not pose a significant safety or health hazard to employees in the immediate vicinity or to the employee cleaning it up, nor does it have the potential to become an emergency within a short time frame. Incidental releases are limited in quantity, exposure potential, or toxicity and present minor safety or health hazards to employees in the immediate work area or those assigned to clean them up. An incidental spill may be safely cleaned up by employees who are familiar with CCR. Response to major releases of CCR will be in accordance with the Site Emergency Response Plan.

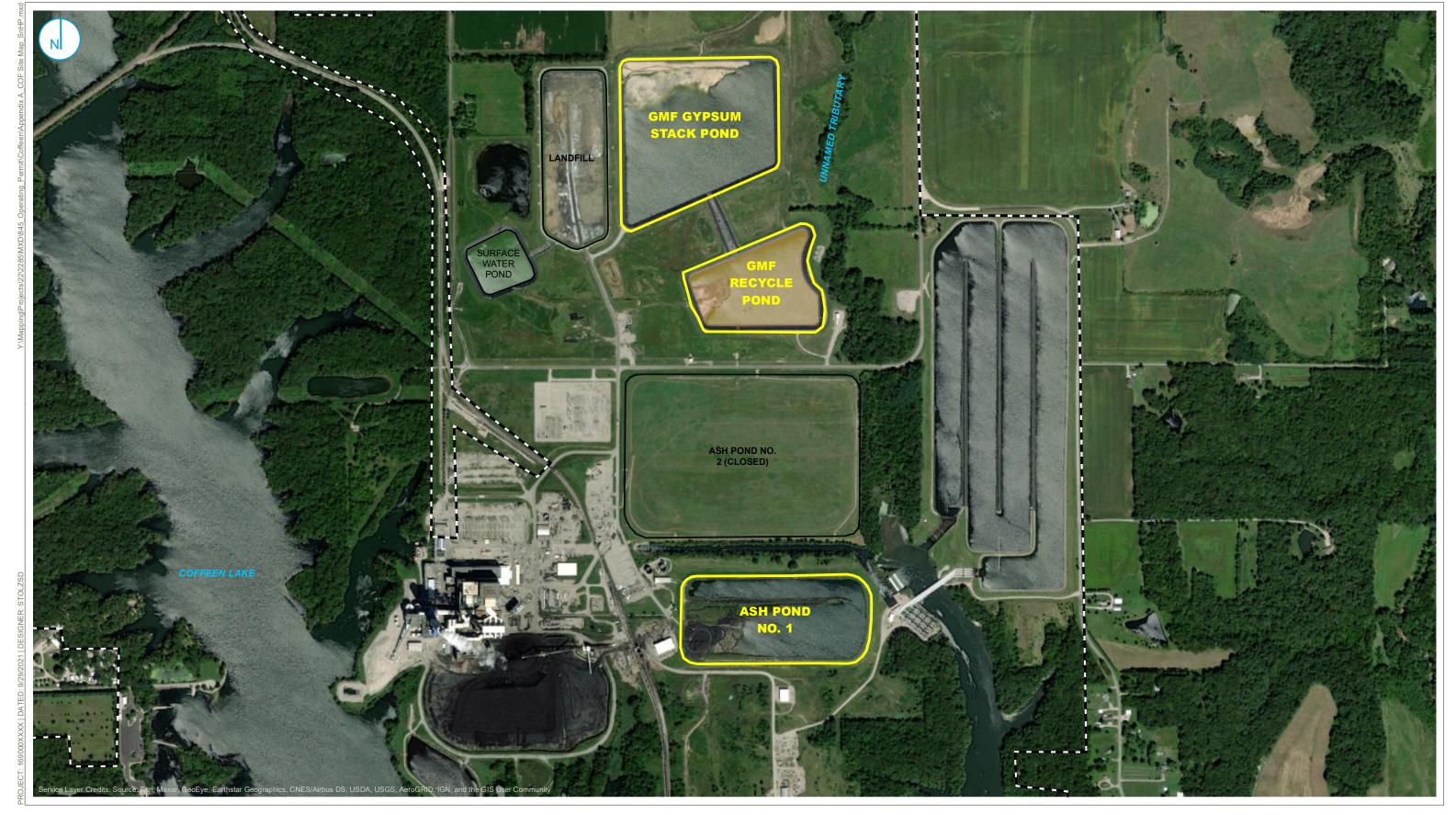
6.9 Ash Pond Rescue

Ash ponds may be unstable and represent an engulfment hazard if persons and equipment traverse the surface, berms, or other unstable areas. Special training is required on behalf of emergency responders to retrieve persons and equipment who become trapped in unstable ash. **Untrained persons must not enter unstable areas** in an attempt to conduct rescue because of the significant potential that they will also become victims. Call the CPP emergency number and state that an "ash pond rescue" is required. The CPP emergency contact will notify the designated service to perform the ash pond rescue. On-site personnel should remain on stand-by to support the ash pond rescue team as necessary.

6.10 Incident Reporting

All incidents must be reported to the contacts outlined in Section 6.1 of this Safety and Health Plan. An Incident Report must be completed for all injuries, illnesses, spills, fire, explosion, or property damage. The absence of an injury does not preclude the need to complete an Incident Report as such incidents will be classified as "near miss" or "other." It will include, but is not limited to, the nature of the problem, time, location, and corrective actions taken to prevent recurrence.

APPENDIX A SITE MAP



PART 845 REGULATED UNIT (SUBJECT UNIT)

SITE FEATURE PROPERTY BOUNDARY **SITE MAP**

APPENDIX A

RAMBOLL AMERICAS ENGINEERING SOLUTIONS, INC.

APPENDIX B SAFETY AND HEALTH PLAN ACKNOWLEDGMENT FORM

SAFETY AND HEALTH PLAN ACKNOWLEDGEMENT FORM

I HEREBY CERTIFY THAT I HAVE READ AND UNDERSTOOD ALL HEALTH AND SAFETY PROCEDURES AS STATED HEREIN:

Name and Affiliation (printed)		Signature	Date
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APPENDIX C DRUG SCREEN POLICIES AND SUPPLEMENTAL TERMS



Drug and Background Investigations

Contractor is solely responsible for ensuring that all members of Contractor Project Team have completed and passed all drug and alcohol tests and background investigations required under this Attachment and under Contractor's own programs before assigning such personnel to perform Work. Contractor is also solely responsible for ensuring that such testing and investigations are performed in accordance with all applicable laws.

- **1. Required Investigations.** Except as otherwise required by applicable law, Required Investigations shall consist of all of the following:
 - 1.1 a 7-panel drug screening;
 - **1.2** a background investigation that includes a criminal records check in all counties where the applicable person has resided for at least the last seven (7) years;
 - **1.3** a third-party verification of previous employment and the highest education level completed by the applicable person;
 - 1.4 a check of the National Sex Offender Registry and Terrorist Watch List (Denied Parties); and
 - **1.5** a check of Motor Vehicles Record (if work to be performed by the applicable person requires driving as part of the defined duties).
- 2. Notices to Tested Persons Regarding Background Checks. All background checks will be conducted in compliance with applicable provisions of the Fair Credit Reporting Act.
- 3. Forms and Testing Organization for Drug Tests. Except for those positions subject to Department of Transportation ("DOT") drug and alcohol testing regulations, all drug testing shall be performed using the Universal Toxicology four part "Non-DOT" Chain of Custody and Request Form with white and blue top page, and shall be conducted by an independent third-party organization.
- **4.** Pass/Fail Standards Background Checks. A person shall be deemed to have failed the applicable background check if:
 - **4.1** information is reported through the background check process indicating that such person has failed to disclose or misrepresented information requested at any time about such a person's criminal background history; or
 - **4.2** such person has ever committed any felony constituting a violent crime, crime against a person, sexual offense or fraud; or
 - **4.3** such person has committed any other felony, or has been incarcerated for a felony, within ten (10) years prior to the date of such background check (i.e., for these felonies there must be a ten (10) year lapse in time from the later of the commission and the end of any period of incarceration); or
 - **4.4** such person has committed any misdemeanor that:
 - **4.4.1** involves violence that is sexually related; or

- **4.4.2** consists of a DUI that is the second (or more) DUI in the last two (2) years prior to the date of the background check; or
- **4.4.3** consists of a theft-related offense; <u>provided</u> that there can be no more than one theft by check and it must have been for an amount less that \$100; or
- **4.4.4** consists of any drug-related misdemeanor committed at any time within forty-eight (48) months prior to the date of the background check.
- **4.4** For purposes of both felonies and misdemeanors, a person is deemed to have committed the applicable offense if he/she is convicted or enters a plea of guilty or nolo contendere for such offense (to include, without limitation, sentences of probation and deferred adjudication).
- 5. Pass/Fail Standards Drug Tests. A person shall be deemed to have failed the applicable drug test if any of the following maximum cut-off levels are exceeded, unless there is a legitimate medical explanation for the presence of a tested substance at or above the applicable cut-off level:

5.1 Amphetamines 500ng/mL

5.2 Barbiturates 150ng/mL

5.3 Benzodiazepines 150ng/mL

5.4 Cocaine 150ng/mL

5.5 Marijuana 150ng/mL

5.6 Opiates 2000ng/mL

5.7 Phencyclidine 25ng/mL

For any positions subject to DOT drug and alcohol testing requirements, testing shall be conducted according to the applicable DOT panel and cutoff levels.

- 6. Other Requirements.
 - **6.1** Background checks and drug tests will be paid for by Contractor without reimbursement by Company.
 - **6.2** Contractor will keep background checks and drug test records while the applicable persons are working pursuant to this Agreement and for three (3) years thereafter.
 - **6.3** Upon request, Contractor will provide a certification to Company that no person required hereunder to pass a background check or drug test has failed such investigation or test. Contractor will not provide the specific results of the background check or drug test of any individual to Company.
 - **6.4** If any person required under this Agreement to pass a background check or drug test fails such check or test, Contractor will not report the specific results of such check or test to Company and will not allow such individual to perform any Work for Company. Although such person may not be assigned to perform any Work for Company, nothing in this Attachment requires Contractor to take any other action with respect to such person's employment with Contractor.



Supplemental Terms for Onsite Services

1. SAFETY

- 1.1 Contractor agrees that any safety-related assistance or initiatives undertaken by Company will not relieve Contractor while on Company Property from responsibility for the implementation of, and compliance with, safe working practices, as developed from their own experience, or as imposed by law or regulation, and will not in any way, affect the responsibilities resting with Contractor under the provisions of any agreement to which these policies are attached and to meet all safety requirements as specified by the Occupational Safety & Health Administration (OSHA), the Mine Safety Health Administration (MSHA), including the "Mining Contractor Safety Reference Handbook" located at http://www.vistraenergy.com/wp-content/uploads/2016/12/Contractors-Safety-Handbook Final-MC-08262016.pdf, the Department of Transportation (DOT) and any other applicable state or federal safety and health laws or regulations.
- 1.2 In the event that a material safety data sheet, warning label, or other documentation concerning the use of hazardous chemicals at any property owned or controlled by Company or any of its affiliates (collectively, "Company Properties"), applies to any materials or equipment provided by Contractor as an aspect of the Work, such documentation will be provided by Contractor to Company prior to the commencement of any such Work.
- 1.3 Contractor will report to Company all accidents involving personal injuries (including death) and damage to property occurring directly or indirectly as a result of the Work performed by Contractor hereunder immediately, but in no event, no later than 24 hours after the occurrence of any such accident. Any accident or incident occurring directly or indirectly as a result of the Work which Contractor must report to a regulatory agency (e.g. OSHA, MSHA, TCEQ) must also be reported to Company immediately following notification to the regulatory agency.

2. SECURITY

- 2.1 It will be the affirmative duty of Contractor to ensure that Contractor Group assists in carrying out all security measures, to include reporting all information or knowledge of matters adversely affecting security to Company's designated security personnel.
- 2.2 Company reserves the right to exclude any of Contractor's employees from any Company Property by denial of access, suspension or revocation of access authorization, preemptory expulsion, or by any other means, without notice or cause. Former Company employees, and any of Contractor's employees who previously have been excluded from any Company Property, may be brought onto Company property or facilities only if prior approval from Company is obtained. If Contractor terminates a member of Contractor Group performing Work on Company's premises, Contractor shall inform Company immediately, but in no event, no later than twenty-four (24) hours after such employee is terminated in order for Company to remove access to Company Property for such employee.
- 2.3 Company measures may also include investigations, whether by Company or law enforcement officials. Contractor agrees to cooperate in such investigations and understands that Company

reserves the right to require anyone in Contractor Group to authorize appropriate agencies to release his or her criminal records to Contractor as a condition of either initial or continued permission for access to any Company Property. Investigations may include searches of Contractor Group. Such searches may include searches of facilities assigned to Contractor Group, search of all Company Property areas and property at such Company Property areas, searches of including, but not limited to, offices, lockers, desks, lunch boxes, packages and motor vehicles (regardless of ownership). Without limiting the foregoing, Contractor acknowledges and agrees that all members of Contractor Group, to the extent that Company reasonably determines that such members require security badge access prior to entering onto any Company Property, shall be required to comply with Company's standard security badge requirements, including without limitation a background check to be performed by Company.

3. ISNETWORLD

- 3.1 Contractor agrees to maintain at Contractor's expense a subscription with ISNetworld (www.ISNetworld.com), Company's safety compliance program or any replacement program therefor, as directed by Company, for the Term of the Agreement. Contractor shall also furnish ISNetworld with any information requested by ISNetworld relating to ISNetworld's evaluation of the Contractor's safety program and practices. As a minimum, requested documents will be related to safety, health, and insurance (i.e., regulatory required training, certifications, safety plans, safe and secure workplace practices, insurance certificates, etc.), OSHA and MSHA injury rates and Experience Modification Rate (EMR).
- 3.2 Contractor has and during the performance of this Agreement shall continue to report full, complete and accurate information to ISNetworld concerning Contractor's employees.
- 4. MATERIALS, EQUIPMENT AND LABOR. Contractor will be solely responsible for the proper storage, transportation and disposal of any product or waste, other than sandblasting waste, used or generated in connection with the Work in accordance with all applicable Environmental Laws. Contractor will dispose of all waste materials, other than sandblasting waste, at an off-site disposal facility approved for such waste materials pursuant to applicable Environmental Laws and will complete and sign all waste manifests as the generator of such waste. Company will be responsible for the storage, transportation and disposal of any sandblasting waste generated during the performance of the Work.

5. CONDITIONS AFFECTING WORK

- 5.1 Contractor will investigate and acquaint itself with the conditions affecting the Work, including but not limited to those related to the transportation, disposal, handling and storage of materials and waste; availability of labor, water, electric power and roads; the uncertainties of weather, river stages or similar physical conditions at the site; the conformation and condition of the ground; and the character of equipment and facilities needed preliminary to and during prosecution of the Work. Contractor has satisfied itself as to the character, quality and quantity of surface and subsurface materials or obstacles to be encountered. Contractor's failure to acquaint itself with any conditions affecting the Work or any available related information will not relieve it from responsibility for properly estimating the difficulty or cost of successfully performing the Work.
- 5.2 Contractor assumes full responsibility for investigating conditions and determining the existence and magnitude of any hazards to the physical well-being of property of Contractor, the employees, agents, and servants of Contractor, or any other person or entity who is or may become involved in

the performance of Work, and any and all other persons in the vicinity of the Work. Contractor will advise all of the above-specified persons or entities of any hazards relating to Work, and will ensure that those persons or entities are advised of and fully understand the nature of the hazards and safety precautions that can be taken to eliminate or minimize dangers relating to the hazards.

- 5.3 Contractor will provide information to Company regarding hazardous chemicals and/or consumable products that contain constituents listed in 40 CFR 372.65 used at any Company Property. Contractor will report the amount of such material carried on and off the site, the amount actually used and the manner of use. Contractor will provide the maximum quantity of the material stored on site at any one time and if a waste material was collected, where it was disposed of (location name and address). Contractor will provide information on the amount of material used for the previous calendar year by the first of February.
- 5.4 Contractor will use its best efforts to ensure that the Work is performed so as to minimize any adverse impact upon natural resources and the environment and will use best industry practices in this regard at all times.
- 5.5 Contractor acknowledges and agrees that all members of Contractor Group performing Work at any Company Generation or Mining Property are required to view Company's "Contractor/Visitor Safety Orientation" video (in the case of Company Generation property), when applicable, and to read and adhere to Company's "Contractor/Visitor Safety Booklet" (in the case of Company Mining property) prior to performing any Work at any Company Generation or Mining Property.
- 5.6 Contractor will immediately notify Company as soon as Contractor has reason to believe that Contactor, or any employee or other person performing the Work, is not or may not be performing the Work in compliance with applicable Environmental Laws. Contractor will provide Company with written notice to Company of such actual or potential non-compliance within three (3) days following the discovery thereof. Contractor will take immediate steps to ensure compliance with all applicable Environmental Laws and will, if directed by Company, cease all Work until authorized by Company to resume the Work.
- 5.7 Contractor will report to Company all accidents involving personal injuries (including death) and damage to property occurring directly or indirectly as a result of the Work performed by Contractor hereunder immediately, but in no event, no later than 24 hours after the occurrence of any such accident. Any accident or incident occurring directly or indirectly as a result of the Work which Contractor must report to a regulatory agency (e.g. OSHA, MSHA, TCEQ) must also be reported to Company immediately following notification to the regulatory agency.

6. WORK SITE PERMITS AND LICENSES

- 6.1 Subject to the following two paragraphs, Contractor will obtain, prior to the commencement of the Work, and provide to Company upon request, all permits, licenses and governmental authorizations, at its sole expense, required for the performance of the Work. Contractor will be solely responsible for maintaining compliance with such permits, licenses and governmental authorizations.
- 6.2 In the event that a storm water discharge permit is required for the performance of the Work, (i)

 Contractor will be responsible for filing a Notice of Intent with respect to the Work, in addition to any

 Notice of Intent that Company may be required to file, and (ii) Contractor will coordinate with

Company in the preparation and execution of a Storm Water Pollution Prevention Plan for the Work Site.

- 6.3 In the event that the performance of the Work involves the handling or abatement of asbestos-containing materials, Contractor will coordinate with Company in the preparation and filing of all required notification forms.
- 7. ACCESS. Should Contractor desire access to the Work Site over any land not controlled by Company, it will, at its sole expense, obtain all proper permits or written permission necessary for that access.
- 8. COMPANY FACILITIES. Contractor will not use Company's sanitary facilities, changehouses, shops, parks, storage buildings, tools, equipment or other facilities unless so directed by Company. Contractor will not discharge, without Company's prior written authorization, any product or waste used or generated in connection with the Work through any (i) Company-permitted outfall, (ii) Company-owned or operated pollution control equipment, or (iii) storm or sanitary sewer located at or in the vicinity of the Work Site. Any request for authorization to discharge will include, at a minimum, either a copy of the Material Safety Data Sheet for the product or a written description of the waste, including a list of the constituents of the waste and the relative concentrations thereof.

9. ENVIRONMENTAL

- 9.1 In the event that Contractor discovers during the performance of the Work any substance at the Work Site that is not the subject of the Work or has not otherwise been identified by Company for Contractor, which substance Contractor has reason to believe is or may be a Hazardous Substance that (i) has been or may be released or spilled into the soil, surface water, or groundwater or in a building or structure, or (ii) consists of asbestos-containing materials, lead-based paint, batteries, thermostats, lighting equipment, or equipment containing polychlorinated biphenyls, Contractor will immediately stop Work and notify Company of the discovery. Contractor will not resume the Work until receiving authorization from Company to do so.
- 9.2 The term "Hazardous Substance" means any product, waste, emission or substance defined, listed or designated as a hazardous or toxic substance, hazardous waste, hazardous material or pollutant by or pursuant to any Environmental Law and includes, but is not limited to, any petroleum-based product, substance or waste, including any additives associated therewith, pesticides, fertilizers, solvents, polychlorinated biphenyls, mercury, lead, lead-based paint, asbestos-containing material or explosives.
- 9.3 Contractor will immediately notify Company in the event of a spill or release of any material which Contractor knows or has reason to believe is a Hazardous Substance, whether onto the ground, into any body of water, a storm or sanitary sewer, or the air, or anywhere on property owned or controlled by Company, including within any building or structure. Contractor will be solely responsible, as may be required by applicable Environmental Laws, for, in consultation with Company, (i) notifying the appropriate governmental agencies of such spill or release caused or permitted by the acts or omissions of Contractor and (ii) for the cleanup and remediation of such spill or release.
- 10. PROTECTION OF HIGHWAYS AND RAILROADS. Contractor will make suitable arrangements with governmental authorities and railroads for the construction of all structures, whether underneath or over roads, railroads or rights-of-way to protect the public from accident or delay. Contractor will repair, at its

own expense, to the satisfaction of the governmental authorities or other owners, all roads, railroads and bridges that may be damaged by, or given undue wear due to the Work.

11. CLEANING UP

- 11.1 Contractor will at all times keep the Work Site free of waste materials or rubbish caused by the Work. After completing the Work, Contractor will remove all its waste materials, rubbish, tools, supplies, equipment and surplus materials from and about the Work Site.
- 11.2 If Contractor fails to keep the Work Site clean or to clean up after completing the Work, Company may do so and charge all costs of cleaning up to Contractor. Those costs may be deducted from the final payment to Contractor.
- 12. COLLATERAL WORK. Company and other contractors may be working at the Work Site. Company reserves the right to coordinate the performance of Contractor's Work with the work of others. Contractor will cooperate with and will not delay, impede or otherwise impair the work of others. Company does not guarantee Contractor continuous uninterrupted access to the Work Site, but will provide such access as good construction practices will allow, considering the other activities in the area.
- 13. ALCOHOLIC BEVERAGES, DRUGS AND WEAPONS. Contractor will inform all members of Contractor Group who may be involved in the performance of any Work of the following Company rules relating to alcoholic beverages, drugs and weapons, with which all personnel are expected to comply:
- Bringing, attempting to bring, possessing, using or being under the influence of intoxicants, drugs, or narcotics while on any Company Property, including but not limited to parking areas, is prohibited. Possessing alcoholic beverages in sealed containers is permitted, however, in designated parking areas.
- 13.2 Prescription or over-the-counter medications that could affect the performance of safety-sensitive work are allowed on Company Property only if they have been previously cleared by Contractor. Contractor must confirm that the medication and dosage do not impair an individual's ability to perform safety-sensitive work before clearing the individual to perform such work while under the influence of the medication.
- 13.3 Bringing, attempting to bring, possessing or using firearms, whether classified as legal or illegal, while on any Company Property, including but not limited to buildings, parking areas, recreation facilities, equipment and vehicles, is prohibited, unless otherwise required by applicable law. Use or possession of firearms for specific situations is permitted if approved by function or higher level management of Company.
- 13.4 Off-the-job involvement with intoxicants, illegal drugs, or illegal narcotics that adversely affects Company's business, to include impairing the individual's ability to perform his job or the public trust in the safe operation of Company, is prohibited.
- 13.5 Any conduct on any Company Property which is in violation of any state or federal law or regulation is considered a violation of these rules and a breach of any agreement to which these policies are attached.

- 13.6 In order to enforce these rules, all individuals with access to any Company Property as well as the vehicles, offices, lockers and any personal belongings of such individuals on any Company Property are subject to search by Company and its agents, to include security representatives appointed or employed by Company. Individuals may be required to take a blood, urinalysis or Breathalyzer test, or submit to other recognized investigatory tests or procedures as are deemed appropriate or necessary by Company in the investigation of a violation of these rules.
- 14. TITLE AND RIGHT. Nothing in the Agreement will vest Contractor with any right of property in materials used after they have been attached to or incorporated into the Work, nor materials for which Contractor has received full or partial payment. All those materials, upon being so attached, incorporated or paid for, will become the property of Company. Any gravel, sand, stone, minerals, timber or other materials excavated, uncovered, developed or obtained in the Work, or on any land belonging to Company may be used, in the performance of the Work, provided such materials meet the requirements of this Agreement. Any objects or natural materials or animals excavated or exposed that may have historical significance or constitute a threatened or endangered species must be brought to the attention of Company.

15. PROTECTION AGAINST LIENS AND ENCUMBRANCES

- 15.1 Contractor will not at any time permit any lien, attachment or other encumbrance ("Encumbrance") by any person or persons whosoever or by reason of any claim or demand against Contractor to be placed or remain on the property of Company, including, but not limited to, the Work Site upon which Work is being performed or equipment and materials that are being furnished. To prevent an Encumbrance from being placed on the property of Company, Contractor will furnish during the progress of any Work, as requested from time to time, verified statements showing Contractor's total outstanding indebtedness in connection with the Work.
- 15.2 If Contractor allows any indebtedness to accrue to subcontractors or others and fails to pay or discharge that indebtedness within five (5) days after demand, then Company may withhold any money due Contractor until that indebtedness is paid or pay the indebtedness and apply that amount against the money due Contractor.
- 15.3 If Contractor allows any Encumbrances, whether valid or invalid to be placed on the property of Company, any and all claims or demands for payment to Contractor will be denied by Company until the Encumbrance is removed. If the Encumbrance is not removed immediately, Company may pay that claim or demand and deduct the amount paid, together with all related expenses, including attorneys' fees, from any further payment due Contractor, or at Company's election, Contractor will, upon demand, reimburse Company for the amount paid and all related expenses. Any payment made in good faith by Company will be binding on Contractor.

16. TERMINATION FOR DEFAULT

assignment for the benefit of creditors, or if a receiver should be appointed due to the insolvency of Contractor, or if Contractor should refuse or fail to supply enough properly skilled workmen or proper equipment, materials or services or should fail to make prompt payment to subcontractors, or to pay promptly for materials or labor, or disregard laws, ordinances or the instruction of Company's Contract Coordinator, or if Contractor should refuse or fail to abide by the SOW Construction Schedule or otherwise violate any provisions of the Agreement or SOW, then Company, upon a

determination by Company's Contract Coordinator that sufficient cause exists to justify such action, may, without prejudice to any other right or remedy available to it after giving Contractor seven (7) days' written notice, terminate the Agreement or the SOW and take possession of the Work Site. In the event of such a termination, Company may use all or part of Contractor's equipment and materials and may finish the Work by whatever method Company may deem expedient. In such event, Contractor will not be entitled to receive any further payment hereunder until the Work is finished. If the unpaid balance of the SOW fees will exceed the expense of finishing the Work, including compensation of Company's Contract Coordinator, other Company personnel, third party engineering companies, or other contractors for additional services, such excess will be paid to Contractor. If the expense of finishing the Work will exceed such unpaid balance, Contractor will pay the difference to Company within fifteen (15) days of receiving an invoice for same. The expenses incurred by Company herein, and the damage incurred through Contractor's default, will be determined by Company's Contract Coordinator, in its sole discretion, and such determination will be binding as between the parties.

- 16.2 In the event of a termination under the provisions of this Section 3, Contractor will transfer and assign to Company, in accordance with Company's instructions, all Work, all construction records, reports, permits, data and information, other materials (including all Company-supplied materials), supplies, Work in progress and other goods for which Contractor is entitled to receive reimbursement hereunder, and any and all plans, drawings, sketches, specifications, and information in connection with the Work, and will take such action as may be necessary to secure Company, at Company's sole election, the rights of Contractor under any or all orders and subcontracts made in connection with the Work.
- 16.3 In the event that Company so directs or authorizes, Contractor will sell at a price approved by Company, or retain at a mutually agreeable price, any such materials, supplies, Work in progress, or other goods as referred to in the preceding paragraph. In any event, Company will receive any and all records, plans, drawings, data, permits, specifications, sketches, reports, or other information relating to the Work. The proceeds of any such sale or the agreed price will be paid or credited to Company in such manner as Company may direct so as to reduce the amount payable by Company under this Section 3.

APPENDIX D COVID-19 SITE ENTRY GUIDELINES



COVID-19 Vistra Site Entry Guidelines – *Effective: June 17, 2021* These guidelines are applicable to ALL PERSONNEL entering Vistra work sites.

To enter a Vistra work site, each person must answer the following three questions with a "no" answer and pass the required temperature testing unless they display their Vistra vaccination sticker on their employee badge or hardhat:

Site Entry Questions:

- 1. In the past 10 days, have you tested positive for COVID-19 or are you currently waiting on test results?
- 2. In the past 10 days, have you been within six feet of someone, where masks were not worn, who:
 - a. has tested positive for COVID-19,
 - b. is known to be waiting on test results for COVID-19, or
 - c. is under a quarantine order?
- 3. In the past 10 days, have you or someone who has been within six feet of you where masks were not worn had:
 - a. flu-like symptoms,
 - b. a deep, dry cough,
 - c. recent shortness of breath or difficulty breathing,
 - d. new loss of taste or smell, and/or
 - e. fever of 100 degrees or above?

Temperature Testing:

You must register a temperature between 96- and 100-degrees Fahrenheit as described in the temperature procedures. (see next page for testing procedures)

- If your temperature is below 96 degrees, retest with a different device.
- If your temperature is 100-degrees Fahrenheit or above, retest on another device preferably an ear thermometer, if your temperature still registers 100-degrees Fahrenheit or above you may not enter the site.

Clearance to enter the site:

- If you have answered "no" to all three questions and passed the temperature test, you may enter the site.
- If you have an approved Vistra vaccination sticker, you are cleared to enter the site without the temperature test or answering COVID screening questions.
- If you passed the temperature test **and** answered "Yes" to any of the questions, but have been cleared through VistraTravelerSafety (HR clearance) to enter the Vistra work site for that instance of exposure, testing, or symptoms, you may enter the site.

Anyone *not* cleared to enter the work site must immediately leave the work site and notify their supervisor who will notify HR at VistraTravelerSafety@vistracorp.com for next steps.

Required Temperature Testing Procedures:

All persons entering the site without a Vistra vaccination sticker, who have cleared all questions above, will also submit to temperature testing or self-administer a temperature test as required by the facility management. If a self-administered test is required, then a member of the management team or their designee will witness the testing; however, where that is not practicable, each person must attest that they are only entering the site premises because they have passed the screening questions and temperature test required for entry. Also:

- a. Hats may cause false high temperatures and should not be worn for five minutes immediately preceding a forehead temperature test.
- b. Each person is responsible for ensuring all self-testing materials and areas touched during testing are sanitized.
- c. All personnel should maintain a **distance of at least six feet** from other people during this process or wear required masks.

Temperature Testing Requirements:

- 1. All persons entering the site without a Vistra vaccination sticker must register a temperature between 96- and 100-degrees Fahrenheit. Any such person who has a temperature not within that range or who triggers an alarm on a thermal camera must retest with a different device, preferably an ear thermometer, if available. If the second test registers a temperature of 100 degrees or above:
 - a. That person **may not enter** the Vistra work site and must notify their supervisor, who will notify HR at <u>VistraTravelerSafety@vistracorp.com</u> for next steps.
 - b. If there is significant inconsistency between the two tests, repeat another temperature test and use the two closest readings.
- 2. Anyone who registers a temperature between 96- and 100-degrees Fahrenheit may proceed to their work site.
 - If temperature is below 96 degrees, wait a few minutes and retest with a different device.

Control rooms and communal areas:

All persons entering the site without a Vistra vaccination sticker should maintain at least six-feet distance from other people as much as possible and should wear face coverings when six-feet distance is not feasible. No one should gather in communal areas (including the temperature-testing area) without a Vistra vaccination sticker. Only operators are allowed in control rooms without plant manager approval.

Vistra Vaccination Sticker protocols:

All persons with a valid Vistra vaccination sticker do not have to socially distance or wear masks while at the site. They will also not be required to quarantine as a part of COVID-19 exposures unless exhibiting COVID-19 symptoms. To be eligible for these protocols, each person must have their approved Vistra vaccination sticker easily visible at all times while at work. If someone who has applied for a Vistra vaccination sticker believes they have specific health conditions that may affect the ability to have a full immune response to the vaccination, please consult your health provider prior to working without a mask.

APPENDIX E SAFETY DATA SHEETS



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Bottom Ash SDS Number: 0.0 Revision Date: 03/2018

Safety Data Sheet Section 1

1.1 Product Identifier

Product Name/Identification:	ASTM Bottom Ash
Synonyms:	Ash; Ashes; Ash residues; Ashes, residues, bottom; Bottom ash; Bottom ash residues; Coal Fly Ash; Pozzolan; Waste solids.
Formula:	UVCB Substance

Identification of the Substance and of the Supplier

1.2 Relevant Identified Uses of the Substance or Mixture and Uses Advices Against

Relevant Identified Uses:	Component of wallboard, concrete, roofing material, bricks, cement kiln feed.	
Uses Advised Against:	None known.	

1.3 Details of the Supplier of the SDS

Manufacturer/Supplier:	Dynegy, Inc.
Street Address:	601 Travis Street, Suite 1400
City, State and Zip Code:	Houston, TX 77002
Customer Service Telephone:	800-633-4704

Preparation Date: 02/23/2018



Revision Date: 03/2

Section 2 Hazards Identification

2.1 Classification of the Substance

GHS Classification(s) according to OSHA Hazard Communication Standard (29 CFR 1910.1200):

- Eye Irritant, Category 2A
- STOT-SE, Category 3 (Respiratory Irritation)
- Carcinogen, Category 1A
- STOT-RE, Category 1 (Lungs)
- Toxic to Reproduction, Category 2

2.2 Label Elements

Labelling according to 29 CFR 1910.1200 Appendices A, B and C*		
₹		
DANGER		
Causes serious eye irritation.		
May cause respiratory irritation.		
May cause damage to lungs after repeated/prolonged exposure via inhalation.		
May cause cancer of the lung.		
Suspected of damaging fertility or the unborn child.		
Obtain special instructions before use.		
Do not handle until all safety precautions have been read and understood. Avoid breathing dust.		
Wash thoroughly after handling.		
Do not eat drink or smoke when using this product.		
Wear protective gloves/protective clothing/eye protection/face protection. Use outdoors or in a well-ventilated area.		
If exposed or concerned: Get medical advice/attention.		
Store in a secure area.		
Dispose of product in accordance with local/national regulations.		

^{*} Fly ash and other coal combustion products (CCPs) are UVCB substances (unknown or variable composition or biological). Various CCPs, noted as ashes/ash residuals; Ashes, residues, bottom; Bottom ash; Bottom ash residues; Waste solids, ashes under TSCA are defined as: "The residuum from the burning of a combination of carbonaceous materials. The following elements may be present as oxides: aluminum, calcium, iron, magnesium, nickel, phosphorus, potassium, silicon, sulfur, titanium, and vanadium." Ashes including fly ash and fluidized bed combustion ash are identified by CAS number 68131-74-8. The exact composition of the ash is dependent on the fuel source and flue additives composed of many constituents. The classification of the final substance is dependent on the presence of specific identified oxides as well as other trace elements.



2.3 Other Hazards

Listed Carcinogens:

-Respirable Crystalline Silica

IARC: [Yes] NTP: [Yes] OSHA: [Yes] Other: (ACGIH) [Yes]

Section 3 Composition/Information on Ingredients

Substance	CAS No.	Percentage (%)	GHS Classification	
Crystalline Silica	14808-60-7	20 - 40%	Repeat Dose STOT, Category 1 Carcinogen, Category 1A	
Silica, crystalline respirable (RCS)	14808-60-7	See Footnote 1	Repeat Dose STOT, Category 1 Carcinogen. Category 1A	
Aluminosilicates ²	Various, see Footnote 2	10 - 60%	Single Exposure STOT, Category 3	
Calcium oxide (CaO)	1305-78-8	10 - 30%	Skin Irritant, Category 2 Eye Irritant, Category 1 Single Exposure STOT, Category 3	
Iron oxide	1309-37-1	1 - 10%	Not Classified	
Manganese dioxide (MnO ₂)	1313-13-9	<2%	Skin Irritant, Category 2 Eye Irritant, Category 2B	
Magnesium oxide	1309-48-4	2 - 10%	Not Classified	
Phosphorus pentoxide (P_2O_5)	1314-56-3	≤2%	Skin Irritant, Category 2 Eye Irritant, Category 2B	
Sodium oxide	1313-59-3		Not Classified	
Potassium oxide (K₂O)	12136-45-7	≤1%	Skin Irritant Category 2 Eye Irritant Category 2B	
Titanium dioxide (TiO ₂)	13463-67-7	<3%	Not Classified	
Bromide salt (calcium)	7789-41-5	See Footnote 3	Toxic to Reproduction Category 2	

¹The percentage of respirable crystalline silica has not been determined. Therefore, a GHS classification of Carcinogen 1A has been assigned.

²Aluminosilicates (CAS# 1327-36-2) may be in the form of mullite (CAS# 1302-93-8); aluminosilicate glass; pozzolans (CAS# 71243-67-9); or calcium aluminosilicates such as tricalcium aluminate (C3A), or calcium sulfoaluminate (C4A3S). The form is dependent on the source of the coal and or the process used to create the CCP. Pulverized coal combustion would be more likely to create high levels of pozzolans. Aluminosilicates may have inclusions of calcium, titanium, iron, potassium, phosphorus, magnesium and other metal oxides.

³ Analytical data are not available to demonstrate that the concentration of bromide salt is <0.1%; therefore, a GHS classification of Toxic to Reproduction Category 2 has been assigned.



Section 4
First Aid Measures

4.1 Description of First Aid Measures

Inhalation:	If product is inhaled and irritation of the nose or coughing occurs, remove person to fresh air. Get medical advice/attention if respiratory symptoms persist.
Skin Contact:	If skin exposure occurs, wash with soap and water.
Eye Contact:	If product gets into the eye, rinse copiously with water for several minutes. Remove contact lenses, if present and easy to do. Seek medical attention/advice if irritation occurs or persists.
Ingestion:	No specific first aid measures are required.

4.2 Most Important Health Effects, Both Acute and Delayed

Acute Effects: Direct exposure may cause respiratory irritation, eye irritation and skin irritation. The product dust can dry and irritate the skin and cause dermatitis and can irritate eyes and skin through mechanical abrasion.

Chronic Effects: Chronic exposure may cause lung damage from repeated exposure. Prolonged inhalation of respirable crystalline silica above certain concentrations may cause lung diseases, including silicosis and lung cancer. Repeated exposure to dusts containing inorganic bromide salts may affect fertility and/or result in effects to the unborn child.

4.3 Indication of Any Immediate Medical Attention and Special Treatment Needed

Seek first aid or call a doctor or Poison Control Center if contact with eyes occurs and irritation remains after rinsing. Get medical advice if inhalation occurs and respiratory symptoms persist.



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Section 5 Firefighting Measures

5.1 Extinguishing Media

Suitable Extinguishing Media:	Product is not flammable. Use extinguishing media appropriate for surrounding fire.	
Unsuitable Extinguishing Media:	Not applicable, the product is not flammable.	

5.2 Special Hazards Arising from the Substance or Mixture

Hazardous Combustion Products:	None known.
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5.3 Advice for Firefighters

Section 6 Accidental Release Measures

6.1 Personal Precautions, Protective Equipment and Emergency Procedures

Personal precautions/Protective Equipment:	See Section 8.2.2 Individual Protective Measures. For concentrations exceeding Occupational Exposure Levels (OELs), use a self-contained breathing apparatus (SCBA).
Emergency procedures:	Use scooping, water spraying/flushing/misting or ventilated vacuum cleaning systems to clean up spills. Do not use pressurized air.

6.2 Environmental Precautions

Environmental precautions:	Prevent contamination of drains or waterways and dispose according to local and national regulations.
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6.3 Methods and Material for Containment and Cleaning Up

Methods and materials for containment and cleaning up:

Do not use brooms or compressed air to clean surfaces. Use dust collection vacuum and extraction systems.

Large spills of dry product should be removed by a vacuum system. Dampened material should be removed by mechanical means and recycled or disposed of according to local and national regulations.

See Sections 8 and 13 for additional information on exposure controls and disposal.

Section 7 Handling and Storage

7.1 Precautions for Safe Handling

Practice good housekeeping. Use adequate exhaust ventilation, dust collection and/or water mist to maintain airborne dust concentrations below permissible exposure limits (note: respirable crystalline silica dust may be in the air without a visible dust cloud).

Do not permit dust to collect on walls, floors, sills, ledges, machinery, or equipment. Maintain and test ventilation and dust collection equipment. In cases of insufficient ventilation, wear a NIOSH approved respirator for silica dust when handling or disposing dust from this product. Avoid contact with skin and eyes. Wash or vacuum clothing that has become dusty. Avoid eating, smoking, or drinking while handling the material.

7.2 Conditions for Safe Storage, Including any Incompatibilities

Minimize dust produced during loading and unloading.



Section 8 Exposure Controls/Personal Protection

8.1 Control Parameters

OCCUPATIONAL EXPOSURE LIMITS					
SUBSTANCE		OSHA PEL TWA (mg/m³)	NIOSH REL TWA (mg/m³)	ACGIH TLV TWA (mg/m³)	CA - OSHA PEL (mg/m³)
Calcium oxide		5	2	2	2
Particulates Not Otherwise Regulated	Total	15	15	10	10
	Respirable	5	5	3	5
Respirable Crystalline Silica	Respirable	0.05	0.05	0.025	0.05
Manganese dioxide (as manganese compounds)	Total	5 (Ceiling)	1 3 (STEL)	0.1	0.2
	Respirable	-	-	0.02	-

8.2 Exposure Controls

8.2.1 Engineering Controls

Provide ventilation to maintain the ambient workplace atmosphere below the occupational exposure limit(s). Use general and local exhaust ventilation and dust collection systems as necessary to minimize exposure.

8.2.2 Personal Protective Equipment (PPE)

Respiratory protection:	Wear a NIOSH approved particulate respirator if exposure to airborne particulates is unavoidable and where occupational exposure limits may be exceeded. If airborne exposures are anticipated to exceed applicable PELs or TLVs, a self-contained breathing apparatus or airline respirator is recommended.
Eye and face protection:	If eye contact is possible, wear protective glasses with side shields. Avoid contact lenses.
Hand and skin protection:	Wear gloves and protective clothing. Wash hands with soap and water after contact with material.



Section 9
Physical and Chemical Properties

9.1 Information on Basic Physical and Chemical Properties

Property: Value	Property: Value
Appearance (physical state, color, etc.): Fine tan/ gray particulate	Upper/lower flammability or explosive limits: Not applicable
Odor: Odorless ¹	Vapor Pressure (Pa): Not applicable
Odor threshold: Not applicable	Vapor Density: Not applicable
pH (25 °C) (in water): 8 - 11	Specific gravity or relative density: 2.2 – 2.9
Melting point/freezing point (°C): Not applicable	Water Solubility: Slight
Initial boiling point and boiling range (°C): Not applicable	Partition coefficient: n-octane/water: Not determined
Flash point (°C): Not determined	Auto ignition temperature (°C): Not applicable
Evaporation rate: Not applicable	Decomposition temperature (°C): Not determined
Flammability (solid, gas): Not combustible	Viscosity: Not applicable

The use of urea or aqueous ammonia injected into the flue gas to reduce nitrogen oxides (NOx) emissions may result in the presence of ammonium sulfate or ammonium bisulfate in the ash at less than 0.1%. When ash containing these substances becomes wet under high pH (>9), free ammonia gas may be released resulting in objectionable/nuisance ammonia odor and potential exposure to ammonia gas especially in confined spaces.



Section 10 Stability and Reactivity

10.1 Reactivity:	The material is an inert, inorganic material primarily composed of elemental oxides.
10.2 Chemical stability:	The material is stable under normal use conditions.
10.3 Possibility of hazardous reactions:	The material is a relatively stable, inert material; however, when ash containing ammonia becomes wet under high pH (>9), free ammonia gas may be released resulting in an objectionable/nuisance ammonia odor and potential exposure to ammonia gas especially in confined spaces. Polymerization will not occur.
10.4 Conditions to avoid:	Product can become airborne in moderate winds. Dry material should be stored in silos. Materials stored out of doors should be covered or maintained in a damp condition.
10.5 Incompatible materials:	None known.
10. 6 Hazardous decomposition products:	None known.



Section 11 Toxicological Information

11.1 Information on Toxicological Effects

Endpoint	Data
Acute oral toxicity	LD50 > 2000 mg/kg
Acute dermal toxicity	LD50 > 2000 mg/kg
Acute inhalation toxicity	LD50 > 5.0 mg/L
Skin corrosion/irritation	Does not meet the classification criteria but may cause slight skin irritation. Product dust can dry the skin which can result in irritation.
Eye damage/irritation	Causes serious eye irritation. Positive scores for conjunctiva irritation and chemosis in 2/3 animals based on average of 24, 48 and 72-hour scores with irritation clearing within 21 days; no corneal or iritis effects observed.
Respiratory/skin sensitization	Not a respiratory or dermal sensitizer.
Germ cell mutagenicity	Not mutagenic in in-vitro and in-vivo assays with or without metabolic activation.
Carcinogenicity	Not available. Respirable crystalline silica has been identified as a carcinogen by OSHA, NTP, ACGIH and IARC.
Reproductive toxicity	No developmental toxicity was observed in available animal studies. Reproductive studies on CCPs showed either no reproductive effects, or some effects on male and female reproductive organs and parameters but without a clear dose response.
	Inorganic bromide salts have been shown to have adverse effects on reproductive parameters in some animal studies.
STOT-SE	CCPs when present as a nuisance dust may result in respiratory irritation.
STOT-RE	In a 180-day inhalation study with fly ash dust, no effects were observed at the highest dose tested. NOEC = 4.2 mg/m³; it is not possible to assess the level at which toxicologically significant effects may occur. Repeated inhalation exposures to high levels of respirable crystalline silica may result in lung damage (i.e., silicosis).
Aspiration Hazard	Not applicable based product form.
	The spiritual states because in

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Section 12 **Ecological Information**

Toxicity 12.1

Fly Ash (CAS# 68131-74-8)			
Toxicity to Fish	LC50 > 100 mg/L		
Toxicity to Aquatic Invertebrates	Data indicates that the test substance is not toxic to <i>Daphnia magna</i> (EC50 undetermined)		
Toxicity to Aquatic Algae and Plants	EC50 = 10 mg/L		
Calcium oxide CAS# 1305-78-8			
Toxicity to Fish	LC50 = 50.6 mg/L The findings were closely related to the pH of the test solutions; therefore, pH is considered to be the main reason for the effects.		
Toxicity to Aquatic Invertebrates	EC50 = 49.1 mg/L The findings were closely related to the pH of the test solutions; therefore, pH is considered to be the main reason for the effects.		
Toxicity to Aquatic Algae and Plants	NOEC =48 mg/L @ 72 hours based on Ca(OH) ₂ The initial pH of the test medium was not directly related to the biologically relevant effects. The formation of precipitates is likely the result of the reaction between CO ₂ dissolved in the medium.		

12.2 Persistence and Degradability

Not relevant for inorganic materials.

12.3 Bioaccumulative Potential

This material does not contain any compounds that would bioaccumulate up the food chain.

12.4 Mobility in Soil

No data available.

12.5 Results of PBT and vPvB Assessment

This material does not contain any compounds classified as "persistent, bioaccumulative or toxic" nor as "very persistent/very bioaccumulative".

12.6 Other Adverse Effects

Preparation Date: February 23, 2018

None known.



Section 13
Disposal Considerations

See Sections 7 and 8 above for safe handling and use, including appropriate industrial hygiene practices.

Dispose of all waste product and containers in accordance with federal, state and local regulations.

Section 14 Transport Information

	Shipping Name:	Not Regulated
Regulatory entity: U.S. DOT	Hazard Class:	Not Regulated
	ID Number:	Not Regulated
	Packing Group:	Not Regulated

Section 15 **Regulatory Information**

15.1 Safety, Health and Environmental Regulations/Legislation Specific for the Mixture

TSCA Inventory Status

All components are listed on the TSCA Inventory.

California Proposition 65

The following substances are known to the State of California to be carcinogens and/or reproductive toxicants:

- Respirable crystalline silica
- Titanium dioxide
- State Right-to-Know (RTK)

Component	CAS	MA ^{1,2}	NJ ^{3,4}	PA ⁵	RI⁵
Ammonium bisulfate	7803-63-6	No	Yes	No	No
Ammonium sulfate	7783-20-2	Yes	No	Yes	No
Calcium oxide	1305-78-8	Yes	Yes	Yes	No
Iron oxide	1309-37-1	Yes	Yes	Yes	No
Magnesium oxide	1309-48-4	No	Yes	No	No
Phosphorus pentoxide (or	1314-56-3	Yes	Yes	Yes	No
phosphorus oxide)					
Potassium oxide	12136-45-7	No	Yes	No	No
Silica-crystalline (SiO ₂), quartz	14808-60-7	Yes	Yes	Yes	No
Sodium oxide	1313-59-3	No	Yes	No	No
Titanium dioxide	13463-67-7	Yes	Yes	Yes	Yes

⁷ Massachusetts Department of Public Health, no date ² 189th General Court of The Commonwealth of Massachusetts, no date

New Jersey Department of Health and Senior Services, 2010a

⁴ New Jersey Department of Health, 2010b

⁵ Pennsylvania Code, 1986

⁶ Rhode Island Department of Labor and Training, no date



Section 16

Other Information, Including Date of Preparation or Last Revision

16.1 Indication of Changes

Date of preparation or last revision: February 23, 2018

16.2 Abbreviations and Acronyms

ACGIH: American Conference of Industrial Hygienists

CA: California

CAS: Chemical Abstract Services
 CCP: Coal Combustion Product
 CFR: Code of Federal Regulations
 EPA: Environmental Protection Agency

GHS: Globally Harmonized System of Classification and Labelling

IARC: International Agency for Research on Cancer

LC50: Concentration resulting in the mortality of 50 % of an animal population

LD50: Dose resulting in the mortality of 50 % of an animal population

MA: Massachusetts
NA: Not Applicable
NJ: New Jersey

NOEC: No observed effect concentration

NIOSH: National Institute of Occupational Safety and Health

NOx: Nitrogen oxides

NTP: US National Toxicology ProgramOEL: Occupational Exposure Limit

OSHA: Occupational Safety and Health Administration

PA: Pennsylvania

PBT: Persistent, Toxic and Bioaccumulative

PEL: Permissible exposure limit
 PPE: Personal Protective Equipment
 REL: Recommended exposure limit

RI: Rhode Island

RCS: Respirable Crystalline Silica

RTK: Right-to-Know

SCBA: Self-contained breathing apparatus

SDS: Safety Data SheetSTEL: Short-term exposure limit

STOT-RE: Specific target organ toxicity-repeated exposure
 STOT-SE: Specific target organ toxicity-single exposure

TLV: Threshold limit value

TSCA: Toxic Substances Control Act
 TWA: Time-weighted average
 UEL: Upper explosive limit

UVCB: Unknown or Variable Composition/Biological

U.S.: United States

Preparation Date: February 23, 2018

• U.S. DOT: United States of Department of Transportation



16.3 Other Hazards

Hazardous Mate	Hazardous Materials Identification System (HMIS)						
Degree of hazard	Degree of hazard (0= low, 4 = extreme)						
Health:	2*	Flammability:	0	Physical Hazards:	0	Personal protection:**	

DISCLAIMER:

This SDS has been prepared in accordance with the Hazard Communication Rule 29 CFR 1910.1200. Information herein is based on data considered to be accurate as of date prepared. No warranty or representation, express or implied, is made as to the accuracy or completeness of this data and safety information. No responsibility can be assumed for any damage or injury resulting from abnormal use, failure to adhere to recommended practices, or from any hazards inherent in the nature of the product.

^{*} Chronic Health Effects

^{**} Appropriate personal protection is defined by the activity to be performed. See Section 8 for additional information.





Section 1 Identification of the Substance and of the Supplier

1.1 Product Identifier

Product Name/Identification:	ASTM Class C Fly Ash
Synonyms:	Coal Fly Ash, Pozzolan
Formula:	UVCB Substance

1.2 Relevant Identified Uses of the Substance or Mixture and Uses Advices Against

Relevant Identified Uses:	Component of wallboard, concrete, roofing material, bricks, cement kiln feed.	
Uses Advised Against:	None known.	

1.3 Details of the Supplier of the SDS

Manufacturer/Supplier:	Dynegy, Inc.
Street Address:	601 Travis Street, Suite 1400
City, State and Zip Code:	Houston, TX 77002
Customer Service Telephone:	800-633-4704



Section 2
Hazards Identification

2.1 Classification of the Substance

GHS Classification(s) according to OSHA Hazard Communication Standard (29 CFR 1910.1200):

- Eye Irritant, Category 2A
- STOT-SE, Category 3 (Respiratory Irritation)
- Carcinogen, Category 1A
- STOT-RE, Category 1 (Lungs)
- Toxic to Reproduction, Category 2

2.2 Label Elements

Labelling according to 29 CFR 1910.1200 Appendices A, B and C*			
Hazard Pictogram(s):			
Signal word:	DANGER		
Hazard Statement(s):	Causes serious eye irritation. May cause damage to lungs after repeated/prolonged exposure via inhalation. May cause respiratory irritation. May cause cancer of the lung. Suspected of damaging fertility or the unborn child.		
Precautionary Statement(s):	Obtain special instructions before use. Do not handle until all safety precautions have been read and understood. Avoid breathing dust. Wear protective gloves/protective clothing/eye protection/face protection. Wash thoroughly after handling. Do not eat drink or smoke when using this product. Use outdoors or in a well-ventilated area. If exposed or concerned: Get medical advice/attention. Store in a secure area. Dispose of product in accordance with local/national regulations.		

^{*} Fly ash and other coal combustion products (CCPs) are UVCB substances (unknown or variable composition or biological). Various CCPs, noted as ashes/ash residuals; Ashes, residues, bottom; Bottom ash; Bottom ash residues; Waste solids, ashes under TSCA are defined as: "The residuum from the burning of a combination of carbonaceous materials. The following elements may be present as oxides: aluminum, calcium, iron, magnesium, nickel, phosphorus, potassium, silicon, sulfur, titanium, and vanadium." Ashes including fly ash and fluidized bed combustion ash are identified by CAS number 68131-74-8. The exact composition of the ash is dependent on the fuel source and flue additives composed of many constituents. The

Page 2 of 16



classification of the final substance is dependent on the presence of specific identified oxides as well as other trace elements.

2.3 Other Hazards

Listed Carcinogens:

-Respirable Crystalline Silica

IARC: [Yes] NTP: [Yes] OSHA: [Yes] Other: (ACGIH) [Yes]

Section 3 Composition/Information on Ingredients

Substance	CAS No.	Percentage (%)	GHS Classification
Crystalline Silica	14808-60-7	30 - 60%	Repeat Dose STOT, Category 1 Carcinogen, Category 1A
Silica, crystalline respirable (RCS)	14808-60-7	See Footnote 1	Repeat Dose STOT, Category 1 Carcinogen, Category 1A
Aluminosilicates	71243-67-9 1327-36-2	30 - 60%	Single Exposure STOT, Category 3
Iron oxide	1309-37-1	1 - 10%	Not Classified
Calcium oxide (CaO)	1305-78-8	20 - 30%	Skin Irritant, Category 2 Eye Irritant, Category 1 Single Exposure STOT, Category 3
Magnesium oxide	1309-48-4	2 - 10%	Not Classified
Phosphorus pentoxide (P ₂ O ₅)	1314-56-3	≤2%	Skin Irritant, Category 2 Eye Irritant, Category 2B
Sodium oxide	1313-59-3	1-8%	Not Classified
Potassium oxide (K ₂ O)	12136-45-7	≤1%	Skin Irritant, Category 2 Eye Irritant, Category 2B
Titanium dioxide (TiO ₂)	13463-67-7	<3%	Not Classified
Bromide salt (calcium)	<mark>7789-41-5</mark>	See Footnote 2	Toxic to Reproduction, Category 2

Footnote 1: The percentage of respirable crystalline silica has not been determined. Therefore, a GHS classification of Carcinogen, Category 1A has been assigned.

Footnote 2: Analytical data are not available to demonstrate that the concentration of bromide salt is <0.1%; therefore, a GHS classification of Toxic to Reproduction, Category 2 has been assigned.



Section 4 First Aid Measures

4.1 Description of First Aid Measures

Inhalation:	If product is inhaled and irritation of the nose or coughing occurs, remove person to fresh air. Get medical advice/attention if respiratory symptoms persist.
Skin Contact:	If skin exposure occurs, wash with soap and water.
Eye Contact:	If product gets into the eye, rinse copiously with water for several minutes. Remove contact lenses, if present and easy to do. Seek medical attention/advice if irritation occurs or persists.
Ingestion:	No specific first aid measures are required.

4.2 Most Important Health Effects, Both Acute and Delayed

Acute Effects: Direct exposure may cause respiratory irritation, eye irritation and skin irritation. The product dust can dry and irritate the skin and cause dermatitis and can irritate eyes and skin through mechanical abrasion.

Chronic Effects: Chronic exposure may cause lung damage from repeated exposure. Prolonged inhalation of respirable crystalline silica above certain concentrations may cause lung diseases, including silicosis and lung cancer. Repeated exposure to dusts containing inorganic bromide salts may affect fertility and/or result in effects to the unborn child.

4.3 Indication of Any Immediate Medical Attention and Special Treatment Needed

Seek first aid or call a doctor or Poison Control Center if contact with eyes occurs and irritation remains after rinsing. Get medical advice if inhalation occurs and respiratory symptoms persist.



Section 5 Firefighting Measures

5.1 Extinguishing Media

Suitable Extinguishing Media:	Product is not flammable. Use extinguishing media appropriate for surrounding fire.
Unsuitable Extinguishing Media:	Not applicable, the product is not flammable.

5.2 Special Hazards Arising from the Substance or Mixture

Hazardous Combustion Products:	None known.
--------------------------------	-------------

5.3 Advice for Firefighters

Special Protective Equipment As with any fire, wear self-contained breathing apparatus (NIOSH approved or equivalent) and full protective gear.



Section 6
Accidental Release Measures

6.1 Personal Precautions, Protective Equipment and Emergency Procedures

Personal precautions/Protective Equipment:	See Section 8.2.2 Individual Protective Measures. For concentrations exceeding Occupational Exposure Levels (OELs), use a self-contained breathing apparatus (SCBA).
Emergency procedures:	Use scooping, water spraying/flushing/misting or ventilated vacuum cleaning systems to clean up spills. Do not use pressurized air.

6.2 Environmental Precautions

Environmental precautions:	Prevent contamination of drains or waterways and dispose according to local and national regulations.
----------------------------	---

6.3 Methods and Material for Containment and Cleaning Up

Methods and materials for containment and cleaning up:	Do not use brooms or compressed air to clean surfaces. Use dust collection vacuum and extraction systems. Large spills of dry product should be removed by a vacuum system. Dampened material should be removed by mechanical means and recycled or disposed of according to local and national regulations.
--	--

See Sections 8 and 13 for additional information on exposure controls and disposal.



Section 7 Handling and Storage

7.1 Precautions for Safe Handling

Practice good housekeeping. Use adequate exhaust ventilation, dust collection and/or water mist to maintain airborne dust concentrations below permissible exposure limits (note: respirable crystalline silica dust may be in the air without a visible dust cloud).

Do not permit dust to collect on walls, floors, sills, ledges, machinery, or equipment. Maintain and test ventilation and dust collection equipment. In cases of insufficient ventilation, wear a NIOSH approved respirator for silica dust when handling or disposing dust from this product. Avoid contact with skin and eyes. Wash or vacuum clothing that has become dusty. Avoid eating, smoking, or drinking while handling the material.

7.2 Conditions for Safe Storage, Including any Incompatibilities

Minimize dust produced during loading and unloading.

Section 8 Exposure Controls/Personal Protection

8.1 Control Parameters

OCCUPATIONAL EXPOSURE LIMITS					
SUBSTANCE		OSHA PEL TWA (mg/m³)	NIOSH REL TWA (mg/m³)	ACGIH TLV TWA (mg/m³)	CA - OSHA PEL (mg/m³)
Calcium oxide		5	2	2	2
Particulates Not Otherwise Regulated	Total	15	15	10	10
	Respirable	5	5	3	5
Respirable Crystalline Silica	Respirable Crystalline Silica	0.05	0.05	0.025	0.05
Titanium dioxide	Total	15	2.4 (fine) 0.3 (ultrafine)	10	10
Manganese dioxide (as manganese compounds)	Total	5 (Ceiling)	1 3 (STEL)	0.1	0.2
	Respirable	-	-	0.02	-



8.2 Exposure Controls

8.2.1 Engineering Controls

Provide ventilation to maintain the ambient workplace atmosphere below the occupational exposure limit(s). Use general and local exhaust ventilation and dust collection systems as necessary to minimize exposure.

8.2.2 Personal Protective Equipment (PPE)

Respiratory protection:	Wear a NIOSH approved particulate respirator if exposure to airborne particulates is unavoidable and where occupational exposure limits may be exceeded. If airborne exposures are anticipated to exceed applicable PELs or TLVs, a self-contained breathing apparatus or airline respirator is recommended.	
Eye and face protection:	If eye contact is possible, wear protective glasses with side shields. Avoid contact lenses.	
Hand and skin protection:	Wear gloves and protective clothing. Wash hands with soap and water after contact with material.	



Section 9 Physical and Chemical Properties

9.1 Information on Basic Physical and Chemical Properties

Property: Value	Property: Value
Appearance (physical state, color, etc.): Fine tan/ gray particulate	Upper/lower flammability or explosive limits: Not applicable
Odor: Odorless ¹	Vapor Pressure (Pa): Not applicable
Odor threshold: Not applicable	Vapor Density: Not applicable
pH (25 °C) (in water): Not Determined	Specific gravity or relative density: 2.2 – 2.9
Melting point/freezing point (°C): Not applicable	Water Solubility: Slight
Initial boiling point/boiling range (°C): NA	Partition coefficient: n-octane/water: NA
Flash point (°C): Not determined	Auto ignition temperature (°C): Not applicable
Evaporation rate: Not applicable	Decomposition temperature (°C): Not determined
Flammability (solid, gas): Not combustible	Viscosity: Not applicable

The use of urea or aqueous ammonia injected into the flue gas to reduce nitrogen oxides (NOx) emissions may result in the presence of ammonium sulfate or ammonium bisulfate in the ash at less than 0.1%. When ash containing these substances becomes wet under high pH (>9), free ammonia gas may be released resulting in objectionable/nuisance ammonia odor and potential exposure to ammonia gas especially in confined spaces.



Section 10 Stability and Reactivity

10.1 Reactivity:	The material is an inert, inorganic material primarily composed of elemental oxides.
10.2 Chemical stability:	The material is stable under normal use conditions.
10.3 Possibility of hazardous reactions:	The material is a relatively stable, inert material; however, when ash containing ammonia becomes wet under high pH (>9), free ammonia gas may be released resulting in an objectionable/nuisance ammonia odor and potential exposure to ammonia gas especially in confined spaces. Polymerization will not occur.
10.4 Conditions to avoid:	Product can become airborne in moderate winds. Dry material should be stored in silos. Materials stored out of doors should be covered or maintained in a damp condition.
10.5 Incompatible materials:	None known.
10. 6 Hazardous decomposition products:	None known.



Section 11 Toxicological Information

11.1 Information on Toxicological Effects

Endpoint	Data
Acute oral toxicity	LD50 > 2000 mg/kg
Acute dermal toxicity	LD50 > 2000 mg/kg
Acute inhalation toxicity	LD50 > 5.0 mg/L
Skin corrosion/irritation	Does not meet the classification criteria but may cause slight skin irritation. Product dust can dry the skin which can result in irritation.
Eye damage/irritation	Causes serious eye irritation. Positive scores for conjunctiva irritation and chemosis in 2/3 animals based on average of 24, 48 and 72-hour scores with irritation clearing within 21 days; No corneal or iritis effects observed.
Respiratory/skin sensitization	Not a respiratory or dermal sensitizer.
Germ cell mutagenicity	Not mutagenic in in-vitro and in-vivo assays with or without metabolic activation.
Carcinogenicity	Not available. Respirable crystalline silica has been identified as a carcinogen by OSHA, NTP, ACGIH and IARC.
Reproductive toxicity	No developmental toxicity was observed in available animal studies. Reproductive studies on CCPs showed either no reproductive effects, or some effects on male and female reproductive organs and parameters but without a clear dose response. Inorganic bromide salts have been shown to have adverse effects
	on reproductive parameters in some animal studies.
STOT-SE	CCPs when present as a nuisance dust may result in respiratory irritation.
STOT-RE	In a 180-day inhalation study with fly ash dust, no effects were observed at the highest dose tested. NOEC = 4.2 mg/m³; it is not possible to assess the level at which toxicologically significant effects may occur. Repeated inhalation exposures to high levels of respirable crystalline silica may result in lung damage (i.e., silicosis).
Aspiration Hazard	Not applicable based product form.



Section 12 Ecological Information

12.1 Toxicity

Fly Ash C (CAS# 68131-74-8)	
Toxicity to Fish	LC50 > 100 mg/L
Toxicity to Aquatic Invertebrates	Data indicates that the test substance is not toxic to <i>Daphnia magna</i> (EC50 undetermined).
Toxicity to Aquatic Algae and Plants	EC50 = 10 mg/L

Calcium oxide CAS# 1305-78-8		
Toxicity to Fish	LC50 = 50.6 mg/L The findings were closely related to the pH of the test solutions; therefore, pH is considered to be the main reason for the effects.	
Toxicity to Aquatic Invertebrates	EC50 = 49.1 mg/L The findings were closely related to the pH of the test solutions; therefore, pH is considered to be the main reason for the effects.	
Toxicity to Aquatic Algae and Plants	NOEC =48 mg/L @ 72 hours based on Ca(OH) ₂ The initial pH of the test medium was not directly related to the biologically relevant effects. The formation of precipitates is likely the result of the reaction between CO ₂ dissolved in the medium.	

12.2 Persistence and Degradability

Not relevant for inorganic materials.

12.3 Bioaccumulative Potential

This material does not contain any compounds that would bioaccumulate up the food chain.

12.4 Mobility in Soil

No data available.

12.5 Results of PBT and vPvB Assessment

This material does not contain any compounds classified as "persistent, bioaccumulative or toxic" nor as "very persistent/very bioaccumulative".

12.6 Other Adverse Effects

None known.

Section 13



Disposal Considerations

See Sections 7 and 8 above for safe handling and use, including appropriate industrial hygiene practices.

Dispose of all waste product and containers in accordance with federal, state and local regulations.

Section 14 Transport Information

Regulatory entity: U.S. DOT	Shipping Name:	Not Regulated
	Hazard Class:	Not Regulated
	ID Number:	Not Regulated
	Packing Group:	Not Regulated

Section 15
Regulatory Information

15.1 Safety, Health and Environmental Regulations/Legislation Specific for the Mixture

TSCA Inventory Status

All components are listed on the TSCA Inventory.

California Proposition 65.

The following substances are known to the State of California to be carcinogens and/or reproductive toxicants:

- Respirable crystalline silica
- State Right-to-Know (RTK)

Component	CAS	MA ^{1,2}	NJ ^{3,4}	PA ⁵	RI⁵
Ammonium bisulfate	7803-63-6	No	Yes	No	No
Ammonium sulfate	7783-20-2	Yes	No	Yes	No
Calcium oxide	1305-78-8	Yes	Yes	Yes	No
Iron oxide	1309-37-1	Yes	Yes	Yes	No
Magnesium oxide	1309-48-4	No	Yes	No	No
Manganese oxide-as	1313-13-9;	No	No	Yes	Yes
manganese compounds	Various				
Phosphorus pentoxide (or	1314-56-3	Yes	Yes	Yes	No
phosphorus oxide)					
Potassium oxide	12136-45-7	No	Yes	No	No
Silica-crystalline (SiO2), quartz	14808-60-7	Yes	Yes	Yes	No
Sodium oxide	1313-59-3	No	Yes	No	No
Titanium dioxide	13463-67-7	Yes	Yes	Yes	Yes

¹ Massachusetts Department of Public Health, no date

Section 16

Other Information, Including Date of Preparation or Last Revision

16.1 Indication of Changes

Date of preparation or last revision: February 23, 2018

16.2 Abbreviations and Acronyms

ACGIH: American Conference of Industrial Hygienists

CA: California

CAS: Chemical Abstract Services
 CCP: Coal Combustion Product
 CFR: Code of Federal Regulations
 EPA: Environmental Protection Agency

² 189th General Court of The Commonwealth of Massachusetts, no date

³ New Jersey Department of Health and Senior Services, 2010a

⁴ New Jersey Department of Health, 2010b

⁵ Pennsylvania Code, 1986

⁶ Rhode Island Department of Labor and Training, no date



GHS: Globally Harmonized System of Classification and Labelling

IARC: International Agency for Research on Cancer

• LC50: Concentration resulting in the mortality of 50 % of an animal population

LD50: Dose resulting in the mortality of 50 % of an animal population

MA: MassachusettsNA: Not ApplicableNJ: New Jersey

NOEC: No observed effect concentration

NIOSH: National Institute of Occupational Safety and Health

NOx: Nitrogen oxides

NTP: US National Toxicology ProgramOEL: Occupational Exposure Limit

OSHA: Occupational Safety and Health Administration

PA: Pennsylvania

PBT: Persistent, Toxic and Bioaccumulative

PEL: Permissible exposure limit
 PPE: Personal Protective Equipment
 REL: Recommended exposure limit

RI: Rhode Island

RCS: Respirable Crystalline Silica

RTK: Right-to-Know

SCBA: Self-contained breathing apparatus

• SDS: Safety Data Sheet

• STEL: Short-term exposure limit

STOT-RE: Specific target organ toxicity-repeated exposure
 STOT-SE: Specific target organ toxicity-single exposure

TLV: Threshold limit value

TSCA: Toxic Substances Control Act
 TWA: Time-weighted average
 UEL: Upper explosive limit

UVCB: Unknown or Variable Composition/Biological

U.S.: United States

• U.S. DOT: United States of Department of Transportation

16.3 Other Hazards

Hazardous Materials Identification System (HMIS)							
Degree of hazard (0= low, 4 = extreme)							
Health:	2*	Flammability:	0	Physical Hazards:	0	Personal protection:**	

^{*} Chronic Health Effects

^{**} Appropriate personal protection is defined by the activity to be performed. See Section 8 for additional information.



DISCLAIMER:

This SDS has been prepared in accordance with the Hazard Communication Rule 29 CFR 1910.1200. Information herein is based on data considered to be accurate as of date prepared. No warranty or representation, express or implied, is made as to the accuracy or completeness of this data and safety information. No responsibility can be assumed for any damage or injury resulting from abnormal use, failure to adhere to recommended practices, or from any hazards inherent in the nature of the product.

ATTACHMENT T



Phil Morris
Illinois Power Generating Company
Luminant
1500 Eastport Plaza Drive
Collinsville, IL 62234

May 19, 2021

Mr. Darin LeCrone, P.E.
Manager, Industrial Unit
Bureau of Water, Division of Water Pollution Control, Permits Section
Illinois Environmental Protection Agency
1021 North Grand Avenue, East
Springfield, IL 62794-9276

Re: CCR Surface Impoundment Category Designation and Justification for Illinois Power Generating Company

Dear Mr. LeCrone:

Pursuant to 35 I.A.C. 845.700(c), Illinois Power Generating Company submits the information necessary to categorize the CCR surface impoundments located at the Newton Power Plant and the now retired Coffeen Power Plant. The following parameters were used in assessing and justifying each assigned category.

- Category 1 Impacts to existing potable water supply well or impacts to groundwater quality within the setback of an existing potable water supply well.
 - This review includes an assessment of potable water wells within 2,500 feet of CCR surface impoundments to determine whether any potential impacts are occurring within the setback zone of any community water supply well established under the Illinois Groundwater Protection Act.
 - o This information was developed during the Part 845 rulemaking and is summarized in Attachment 1, Table 2: Impacts to Potable Water Supply.
- Category 2 Imminent threat to human health or the environment or have been designated by IEPA under (g)(5)
 - o The surface impoundments at Newton and Coffeen Power Plants do not pose an imminent threat to human health or the environment. There are no known conditions at or around the facility where someone or something may be exposed to contaminant concentrations reasonably expected to cause harm
- Category 3 Located in areas of environmental justice ("EJ") concern
 - EJ areas were evaluated using the EJ mapping link from IEPA's webpage located at https://www2.illinois.gov/epa/topics/environmental-justice. Per the IEPA mapping tool, the EJ Status thresholds were determined as twice the state averages for Minority and Low Income consistent with 35 IAC 845.700(g)(6).
 - o An EJ map denoting the facilities with impoundments is located in Attachment 2.

Category 4-7

- o Category 4 Inactive CCR surface impoundments that have an exceedance of the groundwater protection standards in Section 845.600
- o Category 5 Existing CCR surface impoundments that have exceedances of the groundwater protection standards in Section 845.600
- o Category 6 Inactive CCR surface impoundments that are in compliance with the groundwater protection standards in Section 845.600.
- o Category 7 Existing CCR surface impoundments that are in compliance with the groundwater protection standards in Section 845.600

Based on the information above, category designations have been assigned. The category designations for each CCR impoundment are shown in Attachment 1, Table 1: Category Designations.

If you have any questions regarding this submittal, please contact Phil Morris at 618-343-7794 or phil.morris@vistracorp.com.

Sincerely,

Senior Environmental Director

Attachments

Table 1: Category Designation

Facility	Pond Description	Classifications	Potable Water Supply Impacts (Category 1)	Human Health or Environment Threat (Category 2)	Located within Environmental Justice Areas ¹ (Category 3)	Standards Exceedances ² (Categories 4,5,6,7)	Impoundment Category 845.700(g)
	Ash Pond 1	Inactive	No	No	No	Yes	5
Coffeen	GMF Pond	Inactive	No	No	No	Yes	5
	GMF Recycle Pond	Inactive	No	No	No	Yes	5
Newton	Primary Ash Pond	Existing	No	No	No	Yes	5

¹See Attachment 2 Environmental Justice Area Map

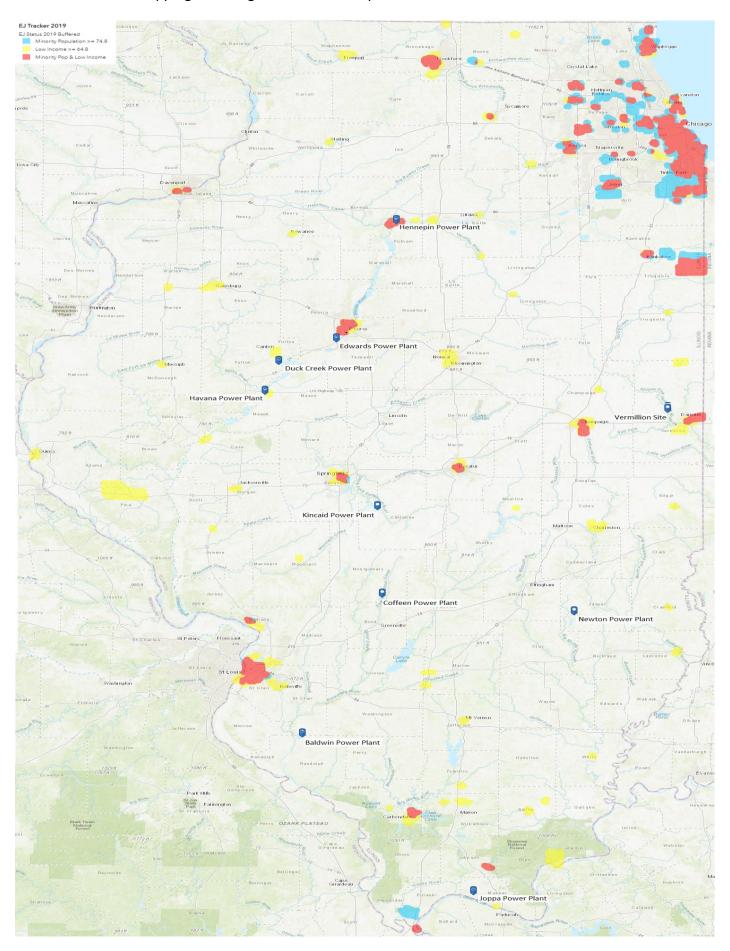
Table 2: Impacts to Potable Water Supply¹

Site Name	Private and Semi-Private Wells	Non-Community Water Supply (CWS) Wells	Non-CWS Surface Water Intakes	Community Water Supply Wells	CWS Surface Water Intakes
Coffeen	Present, but not at risk Thirty-four (34) water wells were identified; however, they are unlikely to be at risk because of their hydrogeologic location relative to the power plant, they are abandoned, or they do not appear to be used for potable purposes. None of the off-site wells are located in a downgradient direction.	Present, but not at risk Three (3) non-CWS wells were identified; however, they are unlikely to be at risk because of their hydrogeologic location relative to the power plant and/or their inactive status.	Absent	Absent	Absent
Newton	Present, but not at risk Twenty-four (24) water wells were identified; however, they are unlikely to be at risk because of their hydrogeologic location relative to the power plant, they are abandoned, and/or they are unlikely to be present based on the mapped location. None of the offsite wells are located in a downgradient direction.	Absent	Absent	Absent	Absent

¹ Ramboll, WELL/WATER SUPPLY SURVEY AND EVALUATION COAL-FIRED POWER PLANTS IN ILLINOIS (September 24, 2020), filed with the Illinois Pollution Control Board in R2020-019.

² Ground water analyses for purposes of categories 4-7, assumptions have been made based on current groundwater data. However, since sampling and analysis is ongoing and subject to IEPA review and approval, IPGC reserves the right to update its category designations for Categories 4-7.

Attachment 2: EJ Mapping Denoting Facilities with Impoundments



ATTACHMENT U



October 11, 2021

Illinois Power Generating Company 134 Cips Lane Coffeen, Illinois 62017

Subject: USEPA CCR Rule and IEPA Part 845 Rule Applicability Cross-Reference

2021 USEPA CCR Rule Periodic Certification Report

GMF Gypsum Stack Pond, Coffeen Power Plant, Coffeen, Illinois

At the request of Illinois Power Generating Company (IPGC), Geosyntec Consultants (Geosyntec) has prepared this letter to document how the attached 2021 United States Environmental Protection Agency (USEPA) CCR Rule Periodic Certification Report (Report) was prepared in accordance with both the Federal USEPA CCR Rule¹ and the state-specific Illinois Environmental Protection Agency (IEPA) Part 845 Rule². Specific sections of the report and the applicable sections of the USEPA CCR Rule and Illinois Part 845 Rule are cross-referenced in **Table 1**. A certification from a Qualified Professional Engineer for each of the CCR Rule sections listed in **Table 1** is provided in Section 9 of the attached Report. This certification statement is also applicable to each section of the Part 845 Rule listed in **Table 1**.

Table 1 – USEPA CCR Rule and Illinois Part 845 Rule Cross-Reference

Report Section	USEPA CCR Rule		Illinois Part 845 Rule		
3	§257.73 (a)(2)	§257.73 Hazard Potential		Hazard Potential Classification Assessment ³	
4	§257.73 (c)(1)	§257.73 History of Construction		Design and Construction Plans (Construction History)	
5	§257.73 (d)(1)	Structural Stability Assessment	845.450 (a) and (c)	Structural Stability Assessment	
6	§257.73 (e)(1)	Safety Factor Assessment	845.460 (a-b)	Safety Factor Assessment	
7	§257.82 (a)(1-3)	Adequacy of Inflow Design Control System Plan	845.510(a), (c)(1), (c)(3)	Hydrologic and Hydraulic Capacity Requirements / Inflow Design Flood Control System Plan	
	§257.82 (b)	Discharge from CCR Unit	845.510(b)	Discharge from CCR Surface Impoundment	

¹ United Stated Environmental Protection Agency, 2015. 40 CFR Parts 257 and 261, Hazardous and Solid Waste Management System, Disposal of Coal Combustion Residuals from Electric Utilities, Final Rule.

² State of Illinois, Joint Committee on Administrative Rule, Administrative Code (2021). *Title 35: Environmental Protection, Subtitle G: Waste Disposal, Chapter I: Pollution Control Board, Subchapter j: Coal Combustion Waste Surface Impoundment, Part 845 Standards for the Disposal of Coal Combustion Residuals in Surface Impoundments.*

³ "Significant" and "High" hazard, per the CCR Rule¹, are equivalent to Class II and Class I hazard potential, respectively, per Part 845².

Illinois Power Resources Generating Company October 11, 2021 Page 2

CLOSING

This letter has been prepared to demonstrate that the content and Qualified Professional Engineer Certification of the 2021 Periodic USEPA CCR Rule Certification Report fulfills the corresponding requirements of Part 845 of Illinois Administrative Code listed in **Table 1**.

Sincerely,

Lucas P. Carr, P.E.

Senior Engineer

John Seymour, P.E.

Senior Principal

2021 USEPA CCR RULE PERODIC CERTIFICATION REPORT §257.73(a)(2), (c), (d), (e) and §257.82 GMF GYPSUM STACK POND Coffeen Power Plant Coffeen, Illinois

Submitted to

Illinois Power Generating Company

134 Cips Lane Coffeen, Illinois 62017

Submitted by



engineers | scientists | innovators

1 McBride and Son Center Drive, Suite 202 Chesterfield, Missouri 63005

October 11, 2021

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Attachment E	Periodic Inflow Design Flood Control System Plan Analyses

EXECUTIVE SUMMARY

This Periodic United States Environmental Protection Agency (USEPA) Coal Combustion Residuals (CCR) Rule [1] certification report (Periodic Certification Report) for the Gypsum Management Facility (GMF) Gypsum Stack Pond (GMF GSP)¹ at the Coffeen Power Plant (CPP), also known as the Coffeen Power Station (COF), has been prepared in accordance with Rule 40, Code of Federal Regulations (CFR) §257. herein referred to as the "CCR Rule" [1]. The CCR Rule requires that initial certifications for existing CCR surface impoundment, completed in 2016 and subsequently posted on the Illinois Power Generating Company (IPGC) CCR Website ([2], [3], [4], [5], [6]) be updated on a five-year basis.

The initial certification reports developed in 2016 and 2017 were independently reviewed by Geosyntec ([2], [7], [3], [8], [4], [5], [6]). Additionally, field observations, interviews with plant staff, updated engineering analyses, and evaluations were performed to compare conditions in 2021 at the GMF GSP relative to the 2016 and 2017 initial certifications. These tasks determined that updates are not required for the Initial Hazard Potential Classification. However, due to changes at the site and technical review comments, updates were required and were performed for the:

- History of Construction Report,
- Structural Stability Assessment,
- Initial Safety Factor Assessment, and
- Initial Inflow Design Flood Control System Plan.

Geosyntec's evaluations of the initial certification reports and updated analyses identified that the GMF GSP meets all requirements for hazard potential classification, history of construction reporting, structural stability, safety factor assessment, and hydrologic and hydraulic control. **Table 1** provides a summary of the initial 2016 certifications and the updated 2021 periodic certifications.

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¹ The GMF GSP is also referred to as ID Number W13501250004-03, GMF GSP by the Illinois Environmental Protection Agency (IEPA); CCR unit ID 103 by IPGC; and IL50579 within the National Inventory of Dams (NID) maintained by the Illinois Department of Natural Resources (IDNR). Within this document it is referred to as the GMF GSP.

Table 1 – Periodic Certification Summary

			2016 Initial Certification		2021 Periodic Certification	
	CCR Rule Reference	Requirement Summary	Requirement Met?	Comments	Requirement Met?	Comments
Hazard	Potential Classification		WICC:	Comments	Wict:	Comments
3	§257.73(a)(2)	Document hazard potential classification	Yes	Impoundment was determined to have a High hazard potential classification [2].	Yes	The Initial Hazard Potential Classification (HPC) is conservative due to the consideration of ultimate buildout conditions relative to existing conditions. An update to the Initial HPC is not required at this time but could be performed to potentially reduce the HPC to Significant.
	of Construction		T			
4	\$257.73(c)(1)	Compile a history of construction	Yes	A History of Construction report was prepared for the GMF GSP, Ash Pond 1, Ash Pond 2, and the GMF Recycle Pond [3].	Yes	A letter listing updates to the History of Construction report is provided in Attachment C.
	ral Stability Assessmen		T			
5	§257.73(d)(1)(i)	Stable foundations and abutments	Yes	Foundations were found to be stable. Abutments were not present [8].	Yes	Foundations and abutments were found to be stable after performing updated slope stability analyses.
	§257.73(d)(1)(ii)	Adequate slope protection	Yes	Slope protection was adequate [8].	Yes	No changes were identified that may affect this requirement.
	§257.73(d)(1)(iii)	Sufficiency of dike compaction	Yes	Dike compaction was sufficient for expected ranges in loading conditions [8].	Yes	Dike compaction was found to be sufficient after performing updated slope stability analyses.
	\$257.73(d)(1)(iv)	Presence and condition of slope vegetation	Yes	Vegetation was present on exterior slopes and was maintained. Interior slopes had alternate protection (geomembrane liner) [8].	Yes	No changes were identified that may affect this requirement.
	\$257.73(d)(1)(v)(A) and (B)	Adequacy of spillway design and management	Yes	Spillways were adequately designed and constructed to adequately manage flow during the probable maximum flood (PMF) [8].	Yes	Spillways were found to be adequately designed and constructed and are expected to adequately manager flow during the PMF, after performing updated hydrologic and hydraulic analyses.
	\$257.73(d)(1)(vi)	Structural integrity of hydraulic structures	Not Applicable	Hydraulic structures penetrating the dikes or underlying the base of the GMF GSP were not present. This requirement was not applicable [8].	Not Applicable	No changes were identified that may affect this requirement.
	\$257.73(d)(1)(vii)	Stability of downstream slopes inundated by water body.	Not Applicable	Inundation of exterior slopes were not expected. This requirement was not applicable [8].	Not Applicable	No changes were identified that may affect this requirement.
Safety F	actor Assessment					
6	§257.73(e)(1)(i)	Maximum storage pool safety factor must be at least 1.50	Yes	Safety factors were calculated to be 3.45 and higher [8].	Yes	Safety factors from updated slope stability analyses were calculated to be 3.45 and higher.
	\$257.73(e)(1)(ii)	Maximum surcharge pool safety factor must be at least 1.40	Yes	Safety factors were calculated to be 3.45 and higher [8].	Yes	Safety factors from updated slope stability analyses were calculated to be 3.45and higher.
	§257.73(e)(1)(iii)	Seismic safety factor must be at least 1.00	Yes	Safety factors were calculated to be 1.47 and higher [8].	Yes	Safety factors from updated slope stability analyses were calculated to be 1.45 and higher.
	\$257.73(e)(1)(iv)	For dike construction of soils that have susceptible to liquefaction, safety factor must be at least 1.20	Not Applicable	Dike soils were not susceptible to liquefaction. This requirement was not applicable [8].	Not Applicable	No changes were identified that may affect this requirement.
	Design Flood Control S	<u>-</u>	T		1 37	
7	\$257.82(a)(1), (2), (3)	Adequacy of inflow design control system plan.	Yes	Flood control system adequately managed inflow and peak discharge during the Probable Maximum Precipitation, 24-hr Inflow Design Flood [8].	Yes	The flood control system was found to adequately manage inflow and peak discharge during the Probable Maximum Precipitation, 24-hour Inflow Design Flood, after performing updated hydrologic and hydraulic analyses.
	\$257.82(b)	Discharge from CCR Unit	Yes	Discharges into Waters of the United States were not expected to occur during normal and Probable Maximum Precipitation, 24-hr, Inflow Design Flood conditions [8].	Yes	Discharge into Waters of the United States were found to not be expected to occur during both normal and Probable Maximum Precipitation, 24-hour Inflow Design Flood conditions, after performing updated hydrologic and hydraulic analyses.

INTRODUCTION AND BACKGROUND

This Periodic United States Environmental Protection Agency (USPA) Coal Combustion Residual (CCR) Rule [1] Certification Report was prepared by Geosyntec Consultants (Geosyntec) for Illinois Power Generating Company (IPGC) to document the re-certification of the GMF GSP at the Coffeen Power Plant (CPP), also known as the Coffeen Power Station (COF), located at 134 Cips Lane in Coffeen, Illinois, 62017. The location of CPP is provided in **Figure 1**, and a site plan showing the location of the GMF GSP, among other closed and active CCR units and non-CCR surface impoundments, is provided in **Figure 2**.

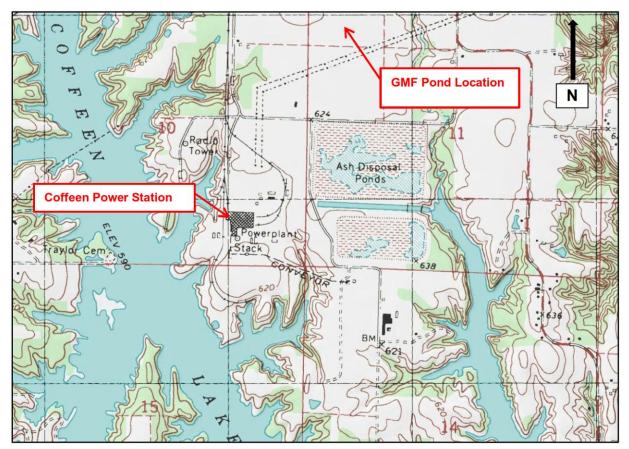


Figure 1 – Site Location Map (from AECOM, 2016)



Figure 2 – Site Plan (adapted from AECOM, 2016)

1.1 GMF GSP Description

CPP was retired in 2019. Prior to retirement, three active CCR surface impoundments – the GMF GSP, the GMF Recycle Pond, and AP1 – and one CCR landfill – were used for managing CCRs generated at CPP. This certification report only pertains to the GMF GSP. The GMF GSP has a High hazard potential, based on the initial hazard potential classification assessment performed by Stantec in 2016 in accordance with §257.73(a)(2) ([2], [7]).

The GMF GSP formerly served as the primary wet impoundment basin for gypsum produced by the wet scrubber system at CPP. The GMF GSP was constructed between 2008 and 2009 and received inflow from two pairs of high-density polyethylene (HDPE) gypsum slurry pipes. Clear water discharge from the GMF GSP flowed downstream into the GMF Recycle Pond via a lined channel (transfer channel) and a 14-in. diameter HDPE low-flow pipe buried beneath the transfer channel. The transfer channel effectively acts as the primary spillway for the GMF GSP, as the bottom elevation of the transfer channel is equal to the adjacent exterior toe elevation of the dike. The transfer channel is approximately 580 ft in length, trapezoidal in shape, lined with 60-mil HDPE, has three horizontal to one vertical (3H:1V) side slopes, and the bottom elevation² decreases from 624 ft at the upstream end to 622 ft at the downstream end.

The 14-in. diameter low-flow pipe has an invert elevation of 619.0 ft at the upstream end and 617.6 ft at the downstream end. A berm was constructed within the transfer channel in 2020 with a crest elevation of approximately elevation 627 ft [9] to retain additional water in the GMF GSP and reduce the pool level in the downstream GMF Recycle Pond. The GMF Recycle Pond formerly acted as a polishing pond, and outflow was pumped to the CPP to be recycled for use in the wet scrubber system [8].

The GMF GSP has a composite liner system that extends up to interior dike crests at elevation 630.5 ft and is present beneath the entire footprint of the pond. The liner system includes a 3-ft thick layer of compacted clay that is overlain by a 60-mil textured HDPE geomembrane. The geomembrane liner is exposed at the pond bottom and side slopes [8].

As formerly operated, the normal pool elevation of the GMF GSP was observed to be 621.2 ft in the 2015 Weaver Consultants survey of the site [10], as controlled by the 14-in. diameter low-level outlet pipe and recycle water inflow and outflow pumping rates [8]. The water elevation in the GMF GSP had increased to 625.2 ft by the time of the periodic survey in December of 2020 [9], due to the construction of the berm in the transfer channel and could rise as high as approximately El. 627 ft due to the berm that was constructed in the transfer channel.

The GMF GSP is approximately 36.2 acres in size and was formed with a continuous embankment, a ring dike, which has a total perimeter length of approximately 5,000 ft. The perimeter dike was constructed to include a crest width of between approximately 15 to 25 ft and a crest height of 5 ft at the north embankment and 9 ft at the east embankment. The interior of the GMF GSP extends deeper than the exterior natural grade, and the maximum interior slope height is approximately 25 ft in the southeast corner of the pond. The elevation of the embankment crest ranges from 631 to 632 ft. Both interior and exterior slopes have 3H:1V orientations [8].

Initial certifications for the GMF GSP for Hazard Potential Classification (§257.73(a)(2)), History of Construction (§257.73(c)), Structural Stability Assessment (§257.73(d)), Safety Factor Assessment (§257.73(e)(1)), and Inflow Design Flood Control System Plan (§257.82) were

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² All elevations in the report are in the North American Vertical Datum of 1988 (NAVD88), unless otherwise noted.

completed by Stantec and AECOM in 2016 and 2017 and subsequently posted to IPGC's CCR Website ([2], [3], [4], [5], [6]). Additional documentation for the initial certifications included detailed operating record reports containing calculations and other information prepared for the hazard potential classification by Stantec [7] and for the structural stability assessment, safety factor assessment, and inflow design flood control system plan by AECOM [8]. These operating record reports were not posted to IPGC's CCR Website.

1.2 Report Objectives

These following objectives are associated with this report:

- Compare site conditions from 2015/2016 to site conditions in 2020/2021, and evaluate if updates are required to the:
 - o §257.73(a)(2) Hazard Potential Classification [2];
 - o §257.73(c) History of Construction [3];
 - §257.73(d) Structural Stability Assessment [4];
 - o §257.73(e) Safety Factor Assessment [5], and/or
 - §257.82 Inflow Design Flood Control System Plan [6].
- Independently review the Hazard Potential Classification ([2], [7]), Structural Stability Assessment ([4], [8]), Safety Factor Assessment ([5], [8]), and Inflow Design Flood Control System Plan ([6], [8]) to determine if updates may be required based on technical considerations.
 - The History of Construction report [3] was not independent reviewed for technical consideration, as this report contained historical information primarily developed prior to promulgation of the CCR Rule [1] for the CCR units at CPP, and did not include calculations or other information used to certify performance and/or integrity of the impoundments under §257.73(a)(2)-(3), §257.73(c)-(e), or §257.82.
- Confirm that the GMF GSP meets all of the requirements associated with §257.73(a)(2)-(3), (c), (d), (e), and §257.82, or, if the GMF GSP does not meet any of the requirements, provide recommendations for compliance with these sections of the CCR Rule [1].

COMPARISION OF INITIAL AND PEROIODIC SITE CONDITIONS

2.1 Overview

This section describes the comparison of conditions at the GMF GSP between the start of the initial CCR certification program in 2015 and subsequent collection of periodic certification site data in 2020 and 2021.

2.2 Review of Annual Inspection Reports

Annual onsite inspections of the GMF GSP were performed from 2016 to 2020 ([11], [12], [13], [14], [15]) and were certified by a licensed professional engineer in accordance with §257.83(b). Each inspection report stated the following information, relative to the previous inspection:

- A statement that no changes in geometry of the impounding structure were observed since the previous inspection;
- A statement that no geotechnical instrumentation was present;
- Approximate volumes of impounded water and CCR at the time of inspection;
- A statement that no appearances of actual or potential structural weakness or other disruptive conditions were observed; and
- A statement that no other changes which may have affected the stability or operation of the impounding structure were observed.

In summary, the reports did not indicate any significant changes to the GMF GSP between 2015 and 2020. No signs of instability, structural weakness, or changes which may have affected the operation or stability of the GMF GSP were noted in the inspection reports.

2.3 Review of Instrumentation Data

Nineteen groundwater monitoring wells, (G102, G103, R104, G105, G106, G205, G206, G207, G208, G209, G210, G211, G212, G213, G214, G215, G216, G217, and G218), are present at the GMF GSP. Groundwater level readings were collected generally on a quarterly basis and provided between February 17, 2016 and January 27, 2021. Geosyntec reviewed the groundwater level data to evaluate if significant fluctuations, partially increases in phreatic levels, may have occurred after development of the initial structural stability and factor of safety certifications ([4], [5], [8]), which utilized phreatic conditions estimated from cone penetration testing (CPT) data. Available water

level readings are plotted in **Attachment A** and **Figure 3** provides approximate locations of the monitoring wells.

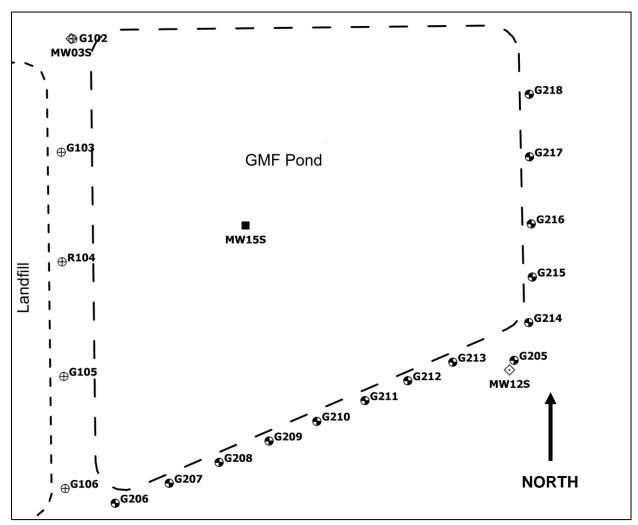


Figure 3 – GMF GSP Monitoring Well Locations (Not to Scale, adapted from Hanson, 2021)

In summary, groundwater levels in the monitoring well network were observed to be relatively consistent between individual wells. Water levels were typically no more than 1 to 4 ft different between individual wells and seasonal fluctuations were on the order of 1 to 4 ft. Water levels ranged from a low of El. 617 ft to a high of El. 627 ft, resulting in a total fluctuation of 10 ft. These water levels are approximately 1 to 3 ft higher than water levels utilized in the slope stability analyses prepared to support the initial structural stability and safety factor assessments ([4], [5], [8]).

The water levels in the initial assessments were based on cone penetration testing (CPT) pore pressure dissipation (PPD) testing collected at a discrete point in time (August 2015) and are

therefore less representative of long-term groundwater trends than the water level data collected from monitoring wells.

2.4 Comparison of Initial to Periodic Surveys

The initial survey of the GMF GSP, conducted at the site by Weaver Consultants (Weaver) in 2015 [10], was compared to the periodic survey of the GMF GSP, conducted by IngenAE, LLC (IngenAE) in 2020 [9], using AutoCAD Civil3D 2021 software. This comparison quantified changes in the volume of CCR placed within the GMF GSP and considered volumetric changes above and below the starting water surface elevation (SWSE) used for the 2016 §257.82 inflow design flood control plan hydraulic analysis [6]. Potential changes to embankment geometry were also evaluated. This comparison is presented in a side-by-side comparison of the surveys in **Drawing 1** and a plan view isopach map denoting changes in ground surface elevation in **Drawing 2**. A summary of the water elevations and changes in CCR volumes is provided in **Table 2**.

Table 2 – Initial to Periodic Survey Comparison

Initial Surveyed Pool Elevation (ft)	621.2
Periodic Surveyed Pool Elevation (ft)	625.2
Initial §257.82 Starting Water Surface Elevation (SWSE) (ft)	621.2
Total Change in CCR Volume (CY)	+74,294
Change in CCR Volume Above SWSE (CY)	+30,006
Change in CCR Volume Below SWSE (CY)	+44,288

The comparison indicated that approximately 74,000 CY of CCR was placed in the GMF GSP between 2015 and 2020, including approximately 30,000 CY above the SWSE, thereby leading to a potential for the peak water surface elevation (PWSE) to increase during the design 1,000-year flood event.

Furthermore, the surveyed pool elevation increased by approximately 4 ft, due to the construction of a berm in the transfer channel. A review of the 2020 survey data indicated the crest elevation of the new berm is approximately 628 ft; this is higher than the periodic surveyed pool level elevation of 625.2 ft. No other significant changes in embankment geometry or other features were noted in the comparison.

2.5 Comparison of Initial to Periodic Aerial Photography

Initial aerial photographs of the GMF GSP collected by Weaver 2015 [10] were compared to periodic aerial photographs collected by IngenAE in 2020 [10] to visually evaluate if potential site changes (i.e., changes to the embankment, outlet structures, limits of CCR, other appurtenances) may have occurred between. A comparison of these aerial photographs is provided in **Drawing 3**, and the following changes were identified:

• The berm in the transfer channel discussed in **Section 2.4** was identified in the channel.

Minor changes in site conditions outside of the GMF GSP were identified, including the
expansion of existing haul roads and the seeding of the GMF GSP exterior embankment
near the transfer channel. However, these minor changes are not expected to significantly
affect the design and/or operation of the GMF GSP.

2.6 Comparison of Initial to Periodic Site Visits

An initial site visit to the GMF GSP was conducted by AECOM in 2015 and documented with a Site Visit Summary and corresponding photographs [16]. A periodic site visit was conducted by Geosyntec on May 28, 2021, with Mr. Lucas P. Carr, P.E. conducting the site visit. The site visit was intended to evaluate potential changes at the site since 2015 (i.e., modification to the embankment, outlet structures or other appurtenances, limits of CCR, maintenance programs, and repairs), in addition to performing visual observations of the GMF GSP to evaluate if the structural stability requirements (§257.73(d)) were still met. The site visit included driving the perimeter of the GMF GSP, periodically stopping to exit the vehicle and visually observe conditions, recording filed notes, and collecting photographs. The site visit is documented in a photographic log provided in **Appendix B**. One significant finding was identified during the periodic site visit and is listed below:

• A berm was constructed in the transfer channel in 2020, as discussed in **Section 2.4**.

2.7 <u>Interview with Power Plant Staff</u>

An interview with Mr. John Romang of CPP was conducted by Mr. Lucas P. Carr, P.E. of Geosyntec on May 28, 2021. Mr. Romang had been, at the time of the interview, employed at CPP for approximately 20 years as the environmental and chemistry manager or supervisor and was responsible for general oversight and compliance for the GMF GSP, including weekly CCR inspections and identifying required repairs. The interview included a discussion of potential changes that may have occurred at the GMF GSP since the development of the initial certifications ([2], [7] [3], [8], [4], [5], [6]). A summary of the interview is provided below.

- Were any construction projects completed for the GMF GSP between 2015 and 2021, and, if so, are design drawings and/or details available?
 - A berm was constructed in the transfer channel between the GMF GSP and the GMF Recycle Pond in 2020 and excess water from the GMF Recycle Pond was pumped into the GMF GSP.
- Were there any changes to the purpose of the GMF GSP between 2015 and 2017?
 - o No, outside of plant retirement.

- Were there any changes to the to the instrumentation program and/or physical instruments for the GMF GSP between 2015 and 2021?
 - No instruments are present at the GMF GSP.
- Were there any changes to spillways and/or diversion features for the GMF GSP completed between 2015 and 2021?
 - Yes, the berm was constructed within the GMF GSP transfer channel.
- Have any area-capacity curves been developed for the GMF GSP since 2015?
 - o No known curves have been developed.
- Were there any changes to construction specifications, surveillance, maintenance, and repair procedures for the GMF GSP between 2015 and 2021?
 - o No.
- Were there any instances of dike and/or structural instability for the GMF GSP between 2015 and 2021?
 - o No known instances occurred.

HAZARD POTENTIAL CLASSIFICATION - §257.73(A)(2)

3.1 Overview of Initial HPC

The Initial Hazard Potential Classification (Initial HPC) was prepared by Stantec Consulting Services, Inc. (Stantec) in 2016 ([2], [7]), following the requirements of §257.73(a)(2). The Initial HPC included the following information:

- Reviewing a breach analysis prepared by Hanson Professional Services (Hanson) in 2007
 [17], as part of the permitting of obtaining a permit to construct the GMF GSP as a
 regulated dam though the Illinois Department of Natural Resources, Offices of Water
 Resources (IDNR-OWR).
 - The review indicated that 12 structures were located within an area where the inundation depth was estimated to be 5 ft, including:
 - Eight (8) occupied structures, including seven residential structures, for a breach at the northwest corner of the GMF GSP perimeter dike.
 - Two (2) residential structures for a breach at the east side of the GMF GSP perimeter dike.
 - The CPP plant building, which was frequently occupied, for a breach to at the south side of the GMF GSP perimeter dike.
 - The review also noted that the breach analyses considered the final buildout height of the GMF GSP as a gypsum stack extending approximately 100 ft above the surrounding grades, rather than the current configurations, where the level of CCR and water inf the GMF GSP is approximately equal to surrounding grades.
- While a breach map is not included within the Initial HPC, it included within the \$257.73(a)(3) Initial Emergency Action Plan (Initial EmAP) [18].

The breach analysis concluded that a breach of the GMF GSP, at its maximum height, would result in a probable threat to human life at multiple residential and other occupied structures. The Initial HPC therefore recommended a "High" hazard potential classification for the GMF GSP [7].

3.2 **Review of Initial HPC**

Geosyntec performed a review of the Initial HPC ([2], [7]), in terms of technical approach, input parameters, and assessment of results. The review included the following tasks:

- Reviewing the breach assessment inputs for appropriateness;
- Reviewing the selected HPC for appropriateness based on the results of the breach analysis, including flow velocities and depths;
- Reviewing the HPC vs. applicable requirements of the CCR Rule.

The review noted that the Initial HPC considered ultimate buildout conditions for the GMF GSP, where it extends approximately 100 ft above grade using the upstream method of construction and dikes comprised of CCR, relative to existing conditions where the GMF GSP is essentially atgrade, as discussed in **Section 3.1**. The GMF GSP is unlikely to reach ultimate buildout conditions due to closure of CPP and the cessation of CCR generation. Therefore, the Initial HPC includes a conservative volume of breach material relative to the amount of material than is currently in the pond.

No other significant technical issues were noted in the technical review, although a detailed review (e.g., check) of the calculations was not performed.

3.3 Summary of Site Changes Affecting the Initial HPC

The GMF GSP is currently considered a High hazard potential CCR surface impoundment [2]; this is the highest hazard classification within §257.53 of the CCR Rule [1]. Therefore, the hazard potential classification would not increase if new structures were to be constructed within the existing mapped breach areas, and a visual assessment of these areas was not performed.

3.4 <u>Periodic Hazard Potential Classification</u>

The current hazard potential classification for the GMF GSP, which is "High" per §257.73(a)(2), is considered conservative as the GMF GSP has not reached and is not expected to reach ultimate buildout conditions. The "High" hazard potential classification is conservative and could maintained or could potentially be revised to "Significant" if a revised breach analysis is performed. However, Geosyntec recommends retaining the current "High" hazard potential classification, unless a revised breach analysis is performed to justify a "Significant" hazard potential classification.

HISTORY OF CONSTRUCTION REPORT - §257.73(C)

4.1 Overview of Initial HoC

The Initial History of Construction report (Initial HoC) was prepared by AECOM in 2016 [3], following the requirements of §257.73(c), and included information on all CCR surface impoundments at CPP, including AP1, AP2, the GMF GSP, and the GMF Recycle Pond. The Initial HoC included the following information for each CCR surface impoundment:

- The name and address of the owner/operator,
- Location maps,
- Statements of purpose,
- The names and size of the surrounding watershed,
- A description of the foundation and abutment materials,
- A description of the dike materials,
- Approximate dates and stages of construction,
- Available design and engineering drawings,
- A summary of instrumentation,
- Area-capacity curves for the GMF GSP,
- Information on spillway structures,
- Construction specifications,
- Inspection and surveillance plans,
- Information on operational and maintenance procedures, and
- A statement that no known instability has occurred at the GMF GSP.

4.2 Summary of Site Changes Affecting the Initial HoC

Several significant changes at the site were identified since development of the Initial HOC and required updates to the HoC report. Each change is described below:

- A state identification number (ID) of W1350150004-03 was assigned to the GMF GSP by the Illinois Environmental Protection Agency (IEPA).
- Electricity generation at CPP ceased in 2019 and the GMF GSP is no longer being used to actively store CCR generated by CPP as CCR is no longer being generated. Additionally, the GMF GSP no longer received regular process water inflows our outflows.
- A berm was constructed within the transfer channel between the GMF GSP and GMF Recycle Pond in 2020, as discussed in **Section 2.4**.
- Revised area-capacity curves and spillway design calculations for the GMF GSP were prepared as part of the updated Periodic Inflow Design Flood Control System Plan, as described in **Section 6.3**.

A letter documenting changes to the HoC report is provided in **Attachment C**.

STRUCTURAL STABILITY ASSESSMENT - §257.73(D)

5.1 Overview of Initial SSA

The Initial Structural Stability Assessment (Initial SSA) was prepared by AECOM in 2016 [4], following the requirements of §257.73(d)(1), and included the following evaluations:

- Stability of dike foundations, dike abutments, slope protection, dike compaction, and slope vegetation;
- Spillway stability including capacity, structural stability and integrity;
- An evaluation of the effects of liquefaction in the foundation soils using a slope stability analysis considering post-cyclic softening in the foundation soils; and
- An evaluation to determine if downstream water bodies that could induce a sudden drawdown condition to the exterior slopes were present.

The Initial SSA concluded that the GMF GSP met all structural stability requirements for \$257.73(d)(1)(i)-(vii).

The Initial SSA referenced the results of the Initial Structural Factor Assessment (Initial SFA) ([5], [8]), to demonstrate stability of the stability of foundations and abutments (§257.73(d)(1)(i)) and sufficiency of dike compaction (§257.73(d)(1)(iii)) portions of the SSA criteria. This included stating that slope stability analyses for slip surfaces passing through the foundation met or exceeded the criteria listed in §257.73(e)(1), for the stability of foundations and abutments. For the sufficiency of dike compaction, this included stating that slope stability analyses for slip surfaces passing through the dike also met or exceeded the §257.73(e)(1) criteria.

5.2 Review of Initial SSA

Geosyntec performed a review of the Initial SSA ([4], [8]) in terms of technical approach, calculation input parameters and methodology, recommendations, and completeness. The review included the following tasks:

- Reviewing photographs collected in 2015 and used to demonstrate compliance with §257.73(d)(1)(i)-(vii).
- Reviewing geotechnical calculations used to demonstrate the stability of foundations, per \$257.73(d)(1)(i) and sufficiency of dike compaction, per \$257.73(d)(1)(iii), in terms of

supporting geotechnical investigation and testing data, input parameters, analysis methodology, selection of critical cross-sections, and loading conditions.

• Review of the methodology used to demonstrate that a downstream water body that could induce a sudden drawdown condition, per §257.73(d)(1)(vii), is not present.

No significant technical issues were noted within the technical review, although a detailed review (e.g., check) of the calculations was not performed.

5.3 Summary of Site Changes Affecting Initial SSA

Several changes at the site occurred after development of the Initial SSA were identified. These changes required updates to the Initial SSA. The changes and the recommend updates to the Initial SSA and are described below.

- The Initial SSA utilized the results of the Initial Inflow Design Flood Control System Plan (IDF) to demonstrate compliance with the adequacy of spillway design and management (§257.73(d)(1)(v)(A)-(B)). The Initial IDF was subsequently updated to develop a Periodic IDF, based on site changes, as discussed in **Section 7**.
- The Initial SSA utilized the slope stability analysis results of the Initial Safety Factor Assessment (SFA) as part of the compliance demonstration for the stability of foundations and abutments (§257.73(d)(1)(i)) and sufficiency of dike compaction (§257.73(d)(1)(iii)) as discussed in **Section 5.1**. The Initial SFA slope stability analyses, including the sudden drawdown analyses, were subsequently updated to develop a Periodic SFA, based on site changes, as discussed in **Section 6**.

5.4 Periodic SSA

The Periodic SFA (**Section 6**) indicated that foundations and abutments are stable and dike compaction is sufficient for expected ranges in loading conditions, as slope stability factors of safety were found to meet or exceed the requirements of §257.73(e)(1), including for post-earthquake (i.e., liquefaction) loading conditions considering seismically induced strength loss in the foundation soils. Therefore, the requirements of §257.73(d)(1)(i) and §257.73(d)(1)(iii) are still met for the Periodic SSA.

The updated Periodic IDF (**Section 7**) indicated that spillways are adequately designed and constructed to adequately manage flow during the PMF flood, as the spillway can adequately manage flow during peak discharge from the PMP storm event without overtopping of the embankments. Therefore, the requirements of §257.73(d)(1)(v)(A)-(B) are met for the Periodic SSA.

SAFETY FACTOR ASSESSMENT - §257.73(E)(1)

6.1 Overview of Initial SFA

The Initial Safety Factor Assessment (Initial SFA) was prepared by AECOM in 2016 ([5], [8]), following the requirements of §257.73(e)(1). The Initial SFA included the following information:

- A geotechnical investigation program with in-situ testing;
- An assessment of the potential for liquefaction in the dike and foundation soils;
- The development of four (4) slope stability cross-sections for limit equilibrium stability analysis utilizing GeoStudio SLOPE/W software; and
- The analysis of each cross-sections for maximum storage pool, maximum surcharge pool, and seismic loading conditions.
 - Liquefaction (i.e., post-earthquake) loading conditions were analyzed due to the
 presence of a soft layer in the foundation material that may be susceptible to cyclic
 softening and/or liquefaction. However, this assessment was utilized to support the
 Initial SSA rather than the Initial SFA, as liquefaction-susceptible soil layers were
 not identified in the embankment soils.

The Initial SFA concluded that the GMF GSP met all safety factor requirements, per §257.73(e), as all calculated safety factors were equal to or higher than the minimum required values.

6.2 Review of Initial SFA

Geosyntec performed a review of the Initial SFA ([5], [8]) in terms of technical approach, calculation input parameters and methodology, recommendations, and completeness. The review included the following tasks:

- Reviewing geotechnical calculations used to demonstrate the acceptable safety factors, per \$257.73(e)(1), in terms of:
 - Completeness and adequacy of supporting geotechnical investigation and testing data;
 - o Completeness and approach of liquefaction triggering assessments; and
 - Input parameters, analysis methodology, selection of critical cross-sections, and loading conditions utilized for slope stability analyses.

No significant technical issues were noted within the technical review, although a detailed review (e.g., check) of the calculations was not performed.

6.3 Summary of Site Changes Affecting the Initial SFA

Several changes at the site, occurred after development of the Initial SFA ([5], [8]), were identified. These changes required updates to the Initial SFA and are described below:

- The normal pool levels within the GMF GSP increased from 621.2 ft to 625.2 ft, due to the construction of a berm in the transfer channel (**Section 7**), resulting in 4.0 ft of additional water loading on the embankment dikes for the maximum storage pool and seismic loading conditions (§257.73(e)(1)(i) and (iii)), relative to the Initial SFA.
- Peak pool levels in the GMF GSP during the PMP design flood event increased from 623.8 ft to 626.7 ft, per the updated Periodic IDF (**Section 7**), resulting in 2.9 ft of additional water loading on the embankment dikes for the maximum surcharge pool loading conditions (§257.73(e)(1)(iv)), relative to the initial SFA.
- Groundwater levels in foundation soils around the GMF GSP, as measured from the monitoring well network over a multi-year period, were observed to be approximately 1 to 3 ft higher than groundwater levels utilized in the slope stability analyses supporting the Initial SFA (see **Section 2.3**). Therefore, the groundwater levels in the slope stability analysis do not represent long-term trends at the GMF GSP.

6.4 Periodic SFA

Geosyntec revised existing slope stability analyses associated with the Initial SFA ([5], [8]), for the four cross-sections (13+50, 22+50, 46+50, and 58+00) previously evaluated to account for site changes, as described in **Section 6.3**. The following approach and input data were used to revise the analyses:

- Water levels in the GMF GSP for the maximum storage pool, and seismic slope stability analysis loading conditions were increased to El. 625.2 ft in all the cross-sections, based on the Periodic IDF (Section 7.4).
- Water levels in the GMF GSP for the maximum surcharge pool slope stability analysis loading conditions were increased to El. 626.7 ft in all the cross-sections based on the Periodic IDF (Section 7.4).
- According to updated groundwater level monitoring plot (**Section 2.3**), the phreatic level in the location of related piezometers increased for all the loading conditions from El. 621.8 to El. 623.3 ft in cross-section 22+50, from El. 623.3 to El. 624.0 ft in cross-section 46+50, and from El. 620.0 to El. 623.0 ft in cross-section 58+00.

• All other analysis input data and settings from the Initial SFA ([5], [8]), were utilized, including, but not limited to, subsurface stratigraphy and soil strengths, phreatic conditions, ground surface geometry, software package and version, slip surface search routines and methods, and input data for the seismic analyses.

Factors of safety from the Periodic SFA are summarized in **Table 3** and confirm that the GMF GSP meets the requirements of §257.73(e)(1). Slope stability analysis output associated with the Initial SFA is provided in **Attachment D**.

Table 3 – Factors of Safety from Periodic SFA

	Struc	Structural Stability Assessment (§257.73(d))			
Cross- Section	Maximum Storage Pool §257.73(e)(1)(i) Minimum Required = 1.50	Maximum Surcharge Pool ¹ §257.73(e)(1)(ii) Minimum Required = 1.40	Seismic §257.73(e)(1)(iii) Minimum Required = 1.00	Dike Liquefaction §257.73(e)(1)(iv) Minimum Required = 1.20	Foundation Liquefaction §257.73(d)(1)(i) Minimum Required = 1.20
13+50	3.45*	3.45*	1.6	N/A	2.46
22+50	3.48	3.48	1.45*	N/A	2.39*
46+50	4.17	4.17	1.74	N/A	3.01
58+00	3.57	3.57	1.63	N/A	2.57

Notes:

^{*}Indicates critical cross-section (i.e., lowest calculated factor of safety out of the ten cross-sections analyzed)

N/A – Loading condition is not applicable.

INFLOW DESIGN FLOOD CONROL SYSTEM PLAN - §257.82

7.1 Overview of 2016 Inflow Design Flood Control System Plan

The Initial Inflow Design Flood Control System Plan (Initial IDF) was prepared by AECOM in 2016 ([6], [8]), following the requirements of §257.82. The Initial IDF included the following information:

- A hydraulic and hydrologic analysis, performed for the Probable Maximum Flood design flood event because of the hazard potential classification of "High", which corresponded to 34.25 inches of precipitation over a 24-hour period.
- The Initial IDF utilized a HydroCAD Version 10 [19] model to evaluate spillway flows and pool level increases during the design flood, with a SWSE of 621.2 ft.

The Initial IDF concluded that the GMF GSP met the requirements of §257.82, as the peak water surface estimated by the HydroCAD model was El. 623.8 ft, relative to the minimum GMF GSP dike crest elevation of 631.0 ft. Therefore, overtopping was not expected. The Initial IDF also evaluated the potential for discharge from the CCR unit and determined that discharge from the unit was not expected, as the GMF GSP does not discharge into waters of the United States and overtopping of the GMF GSP embankments was not expected during the PMF inflow design flood.

7.2 Review of Initial IDF

Geosyntec performed a review of the Initial IDF ([6], [8]) in terms of technical approach, calculation input parameters and methodology, recommendations, and completeness. The review included the following tasks:

- Reviewing the return interval used vs. the hazard potential classification.
- Reviewing the rainfall depth and distribution for appropriateness.
- Performing a high-level review of the inputs to the hydrological modeling.
- Reviewing the hydrologic model parameters for spillway parameters, starting pool elevation, and storage vs. the reference data.
- Reviewing the overall IDF vs. the applicable requirements of the CCR Rule [1].

One comment was identified during review of the Initial IDF. The comment is described below:

 The Initial IDF considered the GMF GSP, but the HydroCAD analysis supporting the Initial IDF did not explicitly consider the downstream GMF Recycle Pond (GMF RP) within the model.

7.3 Summary of Site Changes Affecting the Initial IDF

Two changes at the site that occurred after development of the Initial IDF were identified. These changes required updates to the Initial IDF and are described below:

- A berm was constructed in the transfer channel between the GMF GSP and the GMF RP, with a crest elevation of approximately 626 ft, thereby increasing the SWSE in the GMF GSP relative to the Initial IDF.
- Approximately 30,000 CY was placed above the SWSE in the GMF GSP, thereby altering the stage-storage curve relative to the Initial IDF.

7.4 Periodic IDF

Geosyntec revised the Initial IDF to account for the increase in SWSE and additional CCR placement, as described in **Section 7.2** and **7.3**. The following approach and input data were used for the revised analyses: The model was expanded to include the Gypsum Management Facility Recycle Pond (GMF RP) pond and its drainage area.

• The drainage area to the GMF RP was modeled as a subcatchment and assigned an area of 18.3 ac per the 2020 site survey [9]. It was assigned a Curve number (CN) of 98 and a time of concentration of 6 min (direct inflow).

Table 4 – GMF RP Culvert Attributes in Periodic IDF

Tuble 1 Givi in Curvett Humbatte in 1 chouse 121				
Parameter	Value			
Orifice/Grate				
Invert Elevation (ft)	624.0			
Discharge Coefficient	0.6			
Orifice Width (in)	60			
Orifice Length (in)	60			
Culvert				
Inlet Elevation (ft)	615.0			
Crest Breadth (ft)	1.0			
Outlet Elevation (ft)	613.0			
Length (ft)	92.0			
Diameter (in)	45			
Manning's n	0.013			
Entrance Loss Coefficient	0.5			
Contraction Coefficient	0.9			

- The GMFR Pond was modeled as a pond with three identical emergency spillway outlets.
 - o The outlets were modeled as horizontal orifices routed to culverts, with attributed listed in **Table 4**.
- The routing method for the model was updated to account for routing between the ponds. The Reach Routing Method was updated from "Storage Indication+ Translation" to "Dynamic Storage Indication". The Pond Routing Method was updated from "Storage Indication" to "Dynamic Storage Indication".
- The stage-storage curve was updated for both the GMF GSP and GMF RP Ponds based on the 2020 site survey [9].
 - o Revised stage-volume curves for the GMF RP and GMF GSP were prepared based on measuring the storage volume of the GMF RP and GMF GSP at every one-foot increment of depth from an elevation at the bottom of the ponds (621.1 ft for GMF GSP; 604.9 ft for GMF RP) to the approximate minimum perimeter dike embankment crest elevation (632 ft for GMF GSP; 629 ft for GMF RP). This analysis identified an overall decrease of 9.67 ac-ft of storage volume at the GMF GSP from the storage used in the 2016 Initial IDF Certification.
- The subcatchment area draining to the GMF GSP was updated from 33.8 ac to 36.2 ac to reflect the 2020 site survey [9].
- The time of concentration (ToC) for drainage areas to the GMF GSP was updated from 5 minutes to 6 minutes to reflect direct run-on inflow in accordance with TR-20 [20].
- The SWSE within the GMF GSP was updated from 621.2 ft to 625.2 ft to reflect the water surface elevation from 2020 site survey [9].
- The SWSE in the GMF RP was assumed to be El. 622.1 ft, based on the Updated IDF for the GMF RP [21].
- The GMF GSP and transfer channel geometry were updated to reflect the new berm at the inlet to the transfer channel.
 - The outlet invert from the GMF Pond to the transfer channel between the GMF Pond and the GMFR Pond was raised from 625 ft to 626 ft per the 2020 site survey [9]. The geometry of the outlet was updated based on the 2020 site survey, as listed in **Table 5**.

Table 5 – GMF GSP Outlet Geometry in Periodic IDF

Head (ft)	Channel Width (ft)
0	45
2	60
4	75

o The transfer channel geometry was updated based on the 2020 site survey, as listed in **Table 6**.

Table 6 - GMF GSP Transfer Channel Geometry in Periodic IDF

Parameter	Value
Bottom Width (ft)	32.7
Channel Depth (ft)	6
Left Side Slope	3
Right Side Slope	1.6
Channel Length (ft)	450

• The three outlet structures in the GMF RP were updated from 24 ft broad-crested weirs to horizontal, rectangular orifices with dimensions of 5 ft by 5 ft to reflect the riser structures existing on site. The inlet elevation of the orifices was set to 624 ft per the initial certification reports for the GMF RP ([22], [23]).

The results of the Periodic IDF are summarized in **Table 7** and confirm that the GMF GSP meets the requirements of §257.82(a)-(b), as the peak water surface elevation does not exceed the minimum perimeter dike crest elevations, as long as the SWSE in the GMF GSP is maintained at El. 625.2 ft or lower. Additionally, all discharge from the GMF GSP is routed through the existing spillway system to the GMF RP during both normal and IDF conditions. Updated area-capacity curves and HydroCAD model output are provided in **Attachment E**.

Table 7 – Water Levels from Updated Periodic IDF

<u> </u>					
	Starting Water	Peak Water Surface	Minimum Dike		
Analysis	Surface Elevation (ft)	Elevation (ft)	Crest Elevation (ft)		
Initial IDF	621.2	623.8	631.0		
Periodic IDF Update	625.2	626.7	632.0		
Initial to Periodic Change ¹	+4.0	+2.9			

Notes:

¹Postive change indicates increase in the WSE relative to the Initial IDF, negative change indicates decrease in the WSE, relative to the Initial IDF.

CONCLUSIONS

The GMF GSP at CPP was evaluated relative to the USPEPA CCR Rule periodic assessment requirements for:

- Hazard potential classification (§257.73(a)(2)),
- History of Construction reporting (§257.73(d)),
- Structural stability assessment (§257.73(d)),
- Safety factor assessment (§257.73(e)), and
- Inflow design flood control system planning (§257.82).

Based on the evaluations presented herein, the referenced requirements are satisfied.

CERTIFICATION STATEMENT

CCR Unit: Illinois Power Generating Company, Coffeen Power Plant, GMF Gypsum Stack Pond

I, Lucas P. Carr, being a Registered Professional Engineer in good standing in the State of Illinois, do hereby certify, to the best of my knowledge, information, and belief that the information contained in this 2021 USEPA CCR Rule Periodic Certification Report, has been prepared in accordance with the accepted practice of engineering. I certify, for the above-referenced CCR Unit, that the periodic assessment of the hazard potential classification, history of construction report, structural stability, safety factors, and inflow design flood control system planning, dated October 2021, were conducted in accordance with the requirements of 40 CFR §257.73(a)(2), (c), (d), (e), and §257.82.

Lucas P. Carr

10/11/2021

- P. L

Date

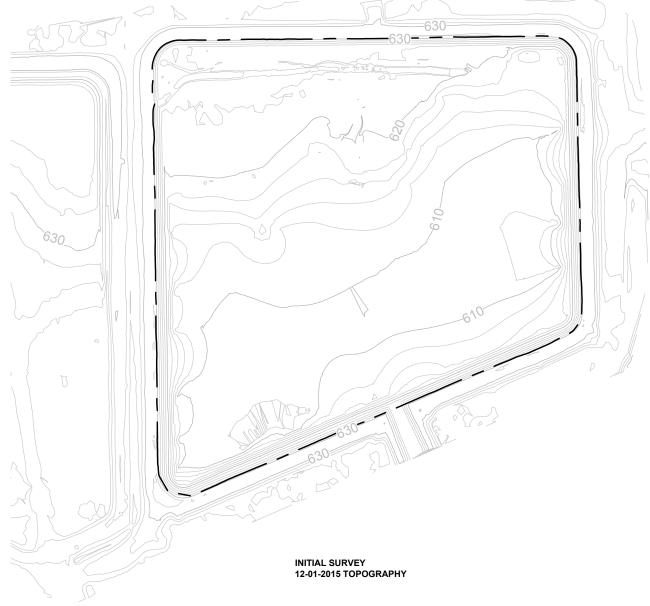
REFERENCES

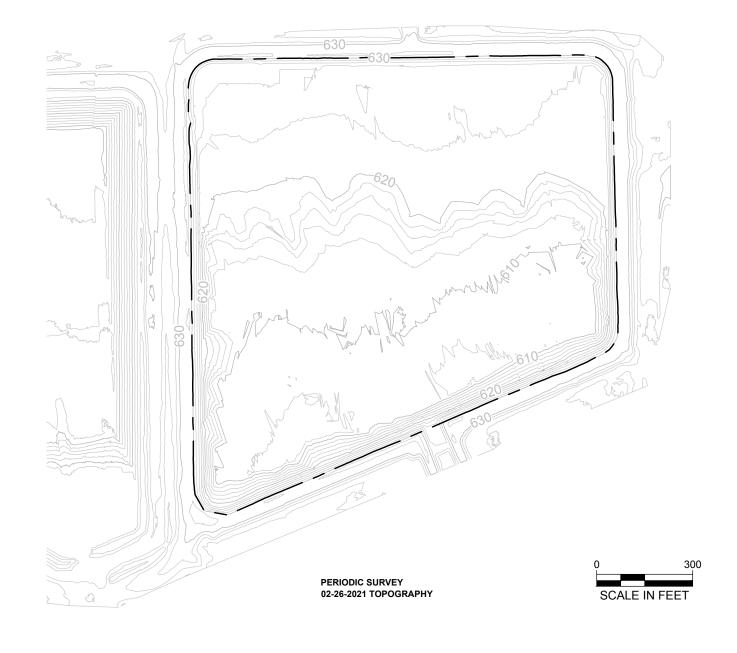
- [1] United States Environmental Protection Agency, 40 CFR Parts 257 and 261; Hazardous and Solid Waste Management System; Disposal of Coal Combustion Residuals from Electric Utilities; Final Rule, 2015.
- [2] Stantec Consulting Services, Inc., "Initial Hazard Potential Classification Assessment, EPA Final CCR Rule, GMF Pond, Coffeen Power Station, Montgomery County, Illinois," Fenton, MO, October 12, 2016.
- [3] AECOM, "History of Construction, USEPA Final CCR Rule, 40 CFR §257.73(c), Coffeen Power Station, Coffeen, Illinois," October 2016.
- [4] AECOM, "CCR Rule Report: Initial Structural Stability Assessment for GMF Pond At Coffeeen Power Station," St. Louis, MO, October 2015.
- [5] AECOM, "CCR Rule Report: Initial Safety Factor Assessment for GMF Pond at Coffeen Power Station," St. Louis, MO, October 2016.
- [6] AECOM, "CCR Rule Report: Initial Inflow Design Flood Control System Plan for GMF Pond at Coffeen Power Station," St. Louis, MO, October 2016.
- [7] Stantec Consulting Services, Inc., "Documentation of Initial Hazard Potential Classification Assessment, GMF Pond, Coffeen Power Station, Montgomery County, Illinois," October 12, 2016.
- [8] AECOM, "CCR Certification Report: Initial Structural Stability Assessment, Initial Safety Factor Assessment, and Initial Inflow Design Flood Control System Plan for GMF Pond at Coffeen Power Station," St. Louis, MO, October 2016.
- [9] IngenAE, "Luminant, Illinois Power Generating Company, Coffeen Power Station, December 2020 Topography," February 26, 2021.
- [10] Weaver Consultants Group, "Dynegy, Collinsville, IL, 2015 Coffeen Topography," December 1, 2015.
- [11] J. Knutelski and J. Campbell, Annual CCR Surface Impoundment Inspection Report (per 40 CFR 257.83(b)(2)), Coffeen Power Station, Gypsum Stack Pond, January 18, 2017.
- [12] J. Knutelski and J. Campbell, Annual CCR Surface Impoundment Inspection Report (per 40 CFR 257.83(b)(2)), Coffeen Power Station, Gypsum Stack Pond, February 7, 2018.
- [13] J. Knutelski, Annual Inspection by a Qualified Professional Engineer, Coffeen Power Station, Gypsum Stack Pond, November 30, 2019.
- [14] J. Knutelski, Annual Inspection by a Qualified Professional Engineer, 40 CFR §257.83(b), Coffeen Power Station, Gypsum Stack Pond, January 8, 2020.
- [15] J. Knutelski, Annual Inspection by a Qualified Professional Engineer, 40 CFR §257.83(b), Coffeen Power Station, Gypsum Stack Pond, January 6, 2021.
- [16] AECOM, "Initial Site Visit CCR Unit Summary, Dynegy CCR Compliance Program, Coffeen Power Station GMF Gypsum Stack Pond," September 28, 2015.

- [17] Hanson Professional Services, Inc., "Breach Analysis for Coffeen Gypsum Stack Dam," 2007.
- [18] Stantec Consulting Services, Inc., "Illinois Power Generating Company, Coffeen Power Station, Montgomery County, Illinois, Emergency Action Plan (EAP)," Fenton, MO, April 13, 2017.
- [19] HydroCADTM Software Solutions, LLC, "HydroCADTM Stormwater Modeling System, Version 10," Chocorua, New Hampshire, 2016.
- [20] National Resoruce Conservation Service, U.S. Department of Agriculture, "WinTR-20 Project Formulation Hydrology," 2021.
- [21] Geosyntec Consultants, "Final Draft 2021 USEPA CCR Rule Periodic Certification Report, §257.73(a)(2), (c), (d), (e), and §257.82, Ash Pond No. 1, Coffeen Power Plant, Coffeen Illinois," Chesterfield, Missouri, September, 2021.
- [22] Hanson Professional Services, Inc., "CCR Rule Report: Initial Inflow Design Flood Control System Plan, GMF Recycle Pond, Coffeen Power Station, Montgomery County, Illinois," October 2016.
- [23] Hanson Professional Services, Inc., "CCR Documentation Report: Initial Structural Stability Assessment, Initial Safety Factor Assessment, and Initial Inflow Design Flood Control System Plan, GMF Recycle Pond, Coffeen Power Station, Montgomery County, Illinois," October 2016.

DRAWINGS







NOTES:

- 1. THE INITIAL SURVEY WAS TAKEN FROM THE DRAWING PACKAGE TITLED "DYNEGY, COLLINSVILLE, ILLINOIS, 2015 - COFFEEN TOPOGRAPHY", PREPARED BY WEAVER CONSULTANTS GROUP, DATED DECEMBER 1, 2015.
- 2. THE PERIODIC SURVEY WAS TAKEN FROM THE DRAWING PACKAGE TITLED "LUMINANT, ILLINOIS POWER GENERATING COMPANY, COFFEEN POWER STATION, DECEMBER 2020 TOPOGRAPHY", PREPARED BY INGENAE, DATED FEBRUARY 26, 2021.
- 3. ALL SURVEY DATA WAS COLLECTED IN THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88) AND NORTH AMERICAN DATUM OF 1983 (NAD83) FOR VERTICAL AND HORIZONTAL COORDINATES, RESPECTIVELY.

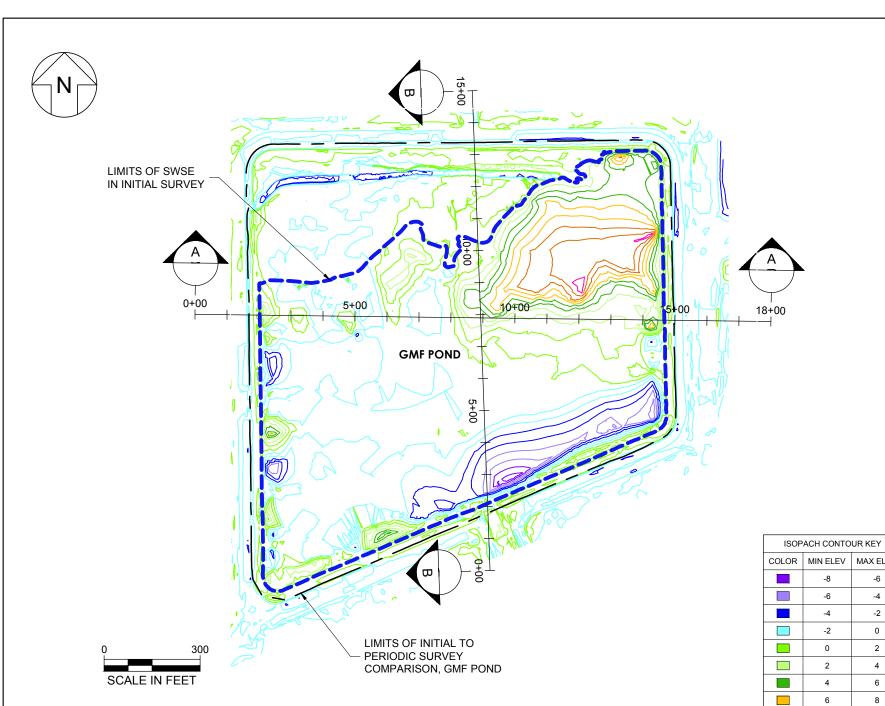
INITIAL TO PERIODIC SURVEY COMPARISON GMF GYPSUM STACK POND **COFFEEN POWER PLANT** COFFEEN, ILLINOIS

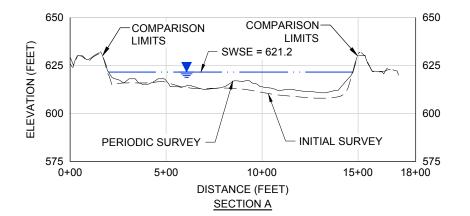
> Geosyntec^D consultants

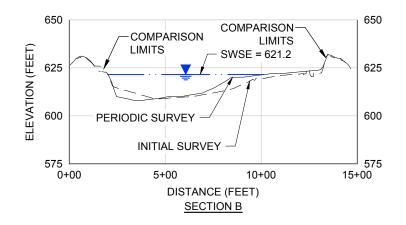
GLP8027.02

MAY 2021

DRAWING









NOTES:

- 1. THE INITIAL SURVEY WAS TAKEN FROM THE DRAWING PACKAGE TITLED "DYNEGY, COLLINSVILLE, ILLINOIS, 2015 - COFFEEN TOPOGRAPHY", PREPARED BY WEAVER CONSULTANTS GROUP, DATED DECEMBER 1, 2015.
- 2. THE PERIODIC SURVEY WAS TAKEN FROM THE DRAWING PACKAGE TITLED "LUMINANT, ILLINOIS POWER GENERATING COMPANY, COFFEEN POWER STATION, DECEMBER 2020 TOPOGRAPHY", PREPARED BY INGENAE, DATED FEBRUARY 26, 2021.
- 3. ALL SURVEY DATA WAS COLLECTED IN THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88) AND NORTH AMERICAN DATUM OF 1983 (NAD83) FOR VERTICAL AND HORIZONTAL COORDINATES, RESPECTIVELY.
- 4. THE STARTING WATER SURFACE ELEVATION (SWSE) OF THE GMF POND IS EL. 621.2 FT, AS NOTED IN THE REPORT TITLED "CCR CERTIFICATION REPORT: INITIAL STRUCTURAL STABILITY ASSESSMENT, INITIAL SAFETY FACTOR ASSESSMENT, AND INITIAL INFLOW DESIGN FLOOD CONTROL SYSTEM PLAN FOR GMF POND AT COFFEEN POWER STATION", PREPARED BY AECOM, DATED OCTOBER, 2016.

INITIAL TO PERIODIC SURVEY COMPARISON SUMMARY				
CUT FILL NET (CU. YD.)				
GMF POND	20,320	94,614	74,294 (FILL)	
ABOVE SWSE	2,168	32,174	30,006 (FILL)	
BELOW SWSE	18,152	62,440	44,288 (FILL)	

MAX ELEV

-2

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SURVEY COMPARISION ISOPACH GMF GYPSUM STACK POND **COFFEEN POWER PLANT** COFFEEN, ILLINOIS

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INITIAL AERIAL 12-01-2015 IMAGERY

PERIODIC AERIAL 02-26-2021 IMAGERY SCALE IN FEET

NOTES:

- 1. THE INITIAL IMAGERY WAS TAKEN FROM THE DRAWING PACKAGE TITLED "DYNEGY, COLLINSVILLE, ILLINOIS, 2015 - COFFEEN TOPOGRAPHY", PREPARED BY WEAVER CONSULTANTS GROUP, DATED DECEMBER 1, 2015.
- 2. THE PERIODIC IMAGERY WAS TAKEN FROM THE DRAWING PACKAGE TITLED "LUMINANT, ILLINOIS POWER GENERATING COMPANY, COFFEEN POWER STATION, DECEMBER 2020 TOPOGRAPHY", PREPARED BY INGENAE, DATED FEBRUARY 26, 2021.

INITIAL TO PERIODIC AERIAL IMAGERY COMPARISON GMF GYPSUM STACK POND **COFFEEN POWER STATION** COFFEEN, ILLINOIS

Geosyntec consultants

GLP8027.02

MAY 2021

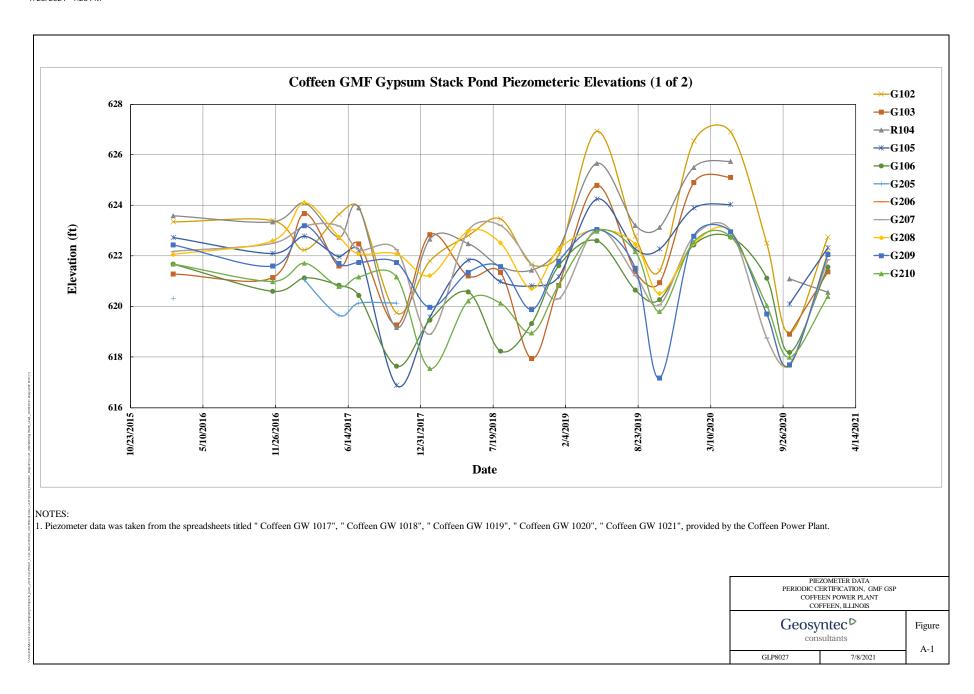
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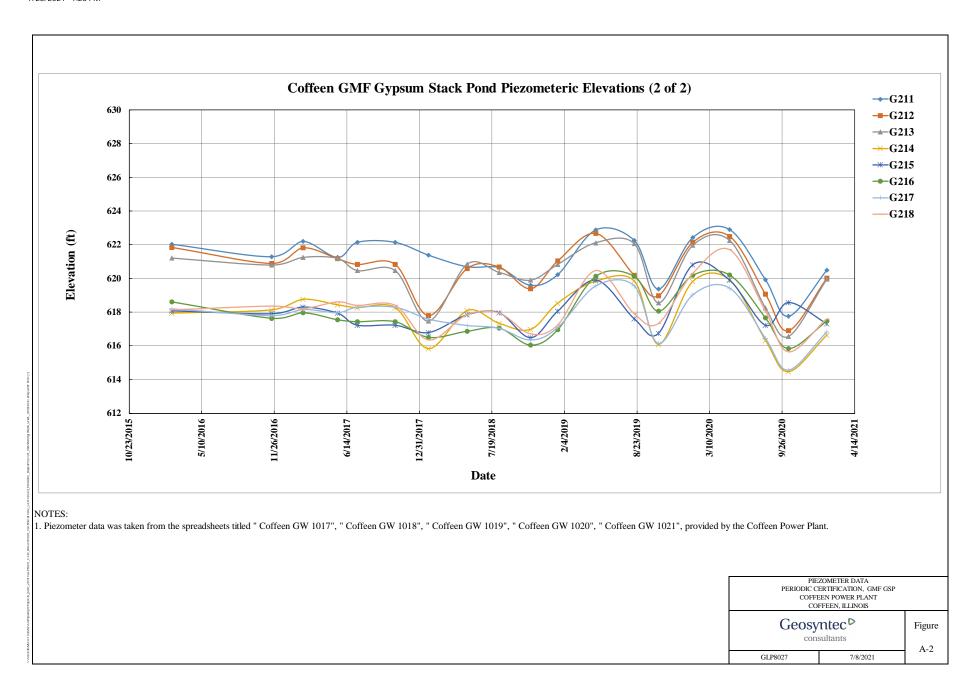
Periodic USEPA CCR Rule Certification Report GMF Gypsum Stack Pond – Coffeen Power Plant October 11, 2021

ATTACHMENTS

Attachment A

GMF GSP Phreatic Data Plots





Attachment B

GMF Gypsum Stack Pond Site Visit Photolog

GEOSYNTEC CONSULTANTS Photographic Record

Geosyntec consultants

Site Owner: Illinois Power Generating Company **Project Number:** GLP8027

CCR Unit: GMF Gypsum Stack Pond (GMF GSP) Site: Coffeen Power Plant

Photo: 01

Date: 05/28/2021

Direction Facing:

SE

Comments:

Berm installed in the transfer channel to reduce the water level in the downstream GMF Recycle Pond.



Photo: 02

Date: 05/28/2021

Direction Facing:

W

Comments:

Southwestern interior slopes.



GEOSYNTEC CONSULTANTS Photographic Record

Geosyntec consultants

Site Owner: Illinois Power Generating Company **Project Number:** GLP8027

CCR Unit: GMF Gypsum Stack Pond (GMF GSP) Site: Coffeen Power Plant

Photo: 07

Date: 05/28/2021

Direction Facing:

N

Comments:

West dike exterior slope. Note leachate valve sump.



Photo: 08

Date: 05/28/2021

Direction Facing:

NE

Comments:

Gypsum discharge

pipes.



GEOSYNTEC CONSULTANTS

Geosyntec consultants

Site Owner: Illinois Power Generating Company **Project Number:** GLP8027

CCR Unit: GMF Gypsum Stack Pond (GMF GSP) Site: Coffeen Power Plant

Photo: 09

Date: 05/28/2021

Direction Facing:

Ε

Comments:

North dike exterior

overview



Photo: 10

Date: 05/28/2021

Direction Facing:

E

Comments:

North dike interior

overview



GEOSYNTEC CONSULTANTS Photographic Record

Geosyntec consultants

Site Owner: Illinois Power Generating Company Project Number: GLP8027

CCR Unit: GMF Gypsum Stack Pond (GMF GSP) Site: Coffeen Power Plant

Photo: 11

Date: 05/28/2021

Direction Facing:

NE

Comments:

North dike exterior

overview



Photo: 12

Date: 05/28/2021

Direction Facing:

S

Comments:

East dike interior

overview



GEOSYNTEC CONSULTANTS

Geosyntec consultants

Site Owner: Illinois Power Generating Company Project Number: GLP8027

CCR Unit: GMF Gypsum Stack Pond (GMF GSP) Site: Coffeen Power Plant

Photo: 13

Date: 05/28/2021

Direction Facing:

S

Comments:

East dike exterior overview.



Photo: 14

Date: 05/28/2021

Direction Facing:

S

Comments:

Southeast corner interior overview



GEOSYNTEC CONSULTANTS Photographic Record

Geosyntec consultants

Site Owner: Illinois Power Generating Company Project Number: GLP8027

CCR Unit: GMF Gypsum Stack Pond (GMF GSP) Site: Coffeen Power Plant

Photo: 15

Date: 05/28/2021

Direction Facing:

SE

Comments:

Southeast corner exterior overview



Photo: 16

Date: 05/28/2021

Direction Facing:

N

Comments:

Area where geomembrane has bulged slightly due to

air.



GEOSYNTEC CONSULTANTS

Geosyntec consultants

Site Owner: Illinois Power Generating Company Project Number: GLP8027

CCR Unit: GMF Gypsum Stack Pond (GMF GSP) Site: Coffeen Power Plant

Photo: 17

Date: 05/28/2021

Direction Facing:

E

Comments:

Southeast embankment exterior overview



Photo: 18

Date: 05/28/2021

Direction Facing:

Е

Comments:

Southeast embankment interior overview



Periodic USEPA CCR Rule Certification Report GMF Gypsum Stack Pond – Coffeen Power Plant October 11, 2021

Attachment C

Periodic History of Construction Report Update Letter





October 11, 2021

Illinois Power Generating Company 134 Cips Lane Coffeen, Illinois 62017

Subject: Periodic History of Construction Report Update Letter

USEPA Final CCR Rule, 40 CFR §257.73(c)

Coffeen Power Plant Coffeen Illinois

At the request of Illinois Power Resources Generation Company (IPRG), Geosyntec Consultants (Geosyntec) has prepared this Letter to documents updates to the Initial History of Construction (HoC) report for the Coffeen Power Plant (CPP), also known as the Coffeen Power Station (COF). The Initial HoC report was prepared by AECOM in October of 2016 [1] in accordance with 40 Code of Federal Regulations (CFR) §257.73(c) of the United States Environmental Protection Agency (USEPA) Coal Combustion Residuals Rule, known as the CCR Rule [2]. This letter also includes information required by Section 845.220(a)(1)(B) (Design and Construction Plans) of the state-specific Illinois Environmental Protection Agency (IEPA) Part 845 CCR Rule [3] that is not expressly required by §257.73(c).

BACKGROUND

The CCR Rule required that, by October 17, 2016, Initial HoC reports to be compiled for existing CCR surface impoundments with: (1) a height of five feet or more and a storage volume of 20 acre-feet or more, or (2) a height of 20 feet or more. The Initial HoC report was required to contain, to the extent feasible, the information specified in 40 CFR §257.73(c)(1)(i)-(xii). The Initial HoC report for CPP, which included four existing CCR surface impoundments, Ash Pond No. 1 (AP1), Ash Pond No. 2 (AP2), the GMF Gypsum Stack Pond (GMF GSP, also known as the GMF Pond), and the GMF Recycle Pond (GMF RP), was prepared and subsequently posted to IPGC's CCR Website prior to October 17, 2016.

The CCR Rule requires that Initial HoC to be updated if there is a significant change to any information complied in the Initial HoC report, as listed below:

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§ 257.73(c)(2): If there is a significant change to any information complied under paragraph (c)(1) of this section, the owner or operator of the CCR unit must update the relevant information and place it in the facility's operating record as required by § 257.105(f)(9).

IPRG retained Geosyntec to review the Initial HoC report, review reasonably and readily available information for AP1, AP2, the GMF GSP, and the GMF RP generated since the Initial HoC report was prepared, and perform a site visit to CPP to evaluate if significant changes may have occurred since the Initial HoC report was prepared. This Letter contains the results of Geosyntec's evaluation and documents significant changes that have occurred at AP1, AP2, the GMF GSP, and the GMF RP, as they pertain the requirements of §257.73(c)(1)(i)-(xii).

UPDATES TO HISTORY OF CONSTRUCTION REPORT

Geosyntec's evaluation for the CPP AP1, AP2, GMF GSP, and GMF RP determined that no known significant changes requiring updates to the information in the Initial HoC report pertaining to §257.73(c)(1)(ii), (iv), (v), (vi), (vii), (xi), and (xii) of the CCR Rule had occurred since the Initial HoC report was developed.

However, Geosyntec's evaluation determined that significant changes at the CPP AP1, AP2, GMF GSP, and GMF RP, pertaining to §257.73(c)(1)(i), (iii), (viii), (ix), and (x) of the CCR Rule had occurred since the Initial HoC report had been developed. Additionally, information how long the CCR surface impoundments have been operating and the types of CCR in the surface impoundments, as required by Section 845.220(a)(1)(B) of the Part 845 Rule were not included in the Initial HoC report, as this information is not required by the CCR Rule. Each change and the subsequent updates to the Initial HoC report is described within this section.

Section 845.220(a)(1)(B): A statement of ... how long the CCR surface impoundment has been in operation, and the types of CCR that have been placed in the surface impoundment.

Ash Pond No. 1

The AP1 was in operation from 1964 until CPP was retired in 2019 and received CCR for approximately 55 years. As of the date of this report, the AP1 has been present for approximately 57 years [4].

CCR placed in the AP1 included bottom ash [4].

Ash Pond No. 2

The AP2 was in operation from 1971 to 1984, for a total of approximately 13 years. The AP2 was closed in 1984-1985 by installing a clay cover and has not since been active or

received CCR. As of the date of this report, AP2 has been present for approximately 50 years. [4].

CCR placed in the AP2 was used to store and dispose of fly ash and bottom ash [4].

GMF Gypsum Pond

The GMF GSP was in operation from 2010 until CPP was retired in 2019 and received CCR for approximately 9 years. As of the date of this report, the GMF GSP has been present for a total of approximately 11 years [4].

CCR placed in GMF GSP included gypsum [4].

GMF Recycle Pond

The GMF RP was in operation from 2010 until CPP was retired in 2019, for a total of 9 years [4]. As of the date of this report, the GMF RP has been present for approximately 11 years.

§ 257.73(c)(1)(i): The name and address of the person(s) owning or operating the CCR unit; the name associated with the CCR unit; and the identification number of the CCR unit if one has been assigned by the state.

State identification numbers (IDs) for AP1, AP2, the GMF GSP, and the GMF RP have been assigned by the Illinois Environmental Protection Agency (IEPA). Each ID is listed in **Table 1**.

 CCR Surface Impoundment
 State ID

 Ash Pond No. 1 (AP1)
 W1350150004-01

 Ash Pond No. 2 (AP2)
 W1350150004-02

 GMF Gypsum Stack Pond (GMF GSP)
 W1350150004-03

 GMF Recycle Pond (GMF RP)
 W1350150004-04

Table 1 – IEPA ID Numbers

§ 257.73(c)(1)(iii): A statement of the purpose for which the CCR unit is being used.

AP2 was closed in 2020, in substantial compliance with the written closure plan posted to IPRG's CCR Website [5], and as documented by a certified Notification of Completion of Closures posted to DMG's CCR Website [6].

The CPP was retired in December of 2019, with the generation of electricity ceased at that time. Therefore, AP1, the GMF GSP, and the GMF RP are no longer being used to store and dispose of new CCR that is actively generated by CPP, as CCR generation as ceased. All three impoundments still contain CCR and liquids that was present at the time of plant

retirement. The GMF RP also previously received dewatering discharge from AP2; this inflow was ceased after AP2 was closed in 202.

 $\S 257.73(c)(1)(viii)$: A description of the type, purpose, and location of existing instrumentation.

Instrumentation monitoring at AP2 is no longer required as the CCR surface impoundment was closed in accordance with §257.102 [6], and the instrumentation network was modified at that time. Therefore, the instrumentation locations shown in Appendix C of the Initial HoC report are no longer applicable to AP2.

 \S 257.73(c)(1)(ix): Area-capacity curves for the CCR unit.

Updated area-capacity curves were prepared for AP1, the GMF GSP, and the GMF RP in 2021 and are provided in **Figures 1**, **2**, and **3**, respectively.

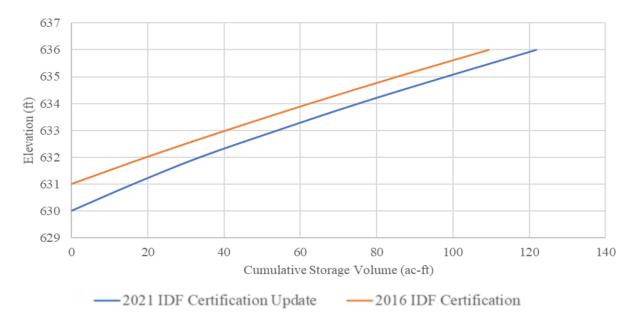
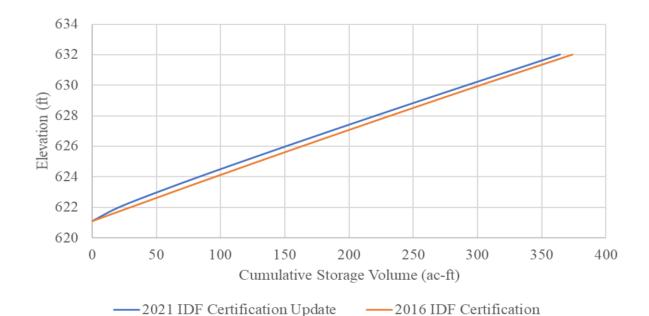


Figure 1 – Area-Capacity Curve for AP1



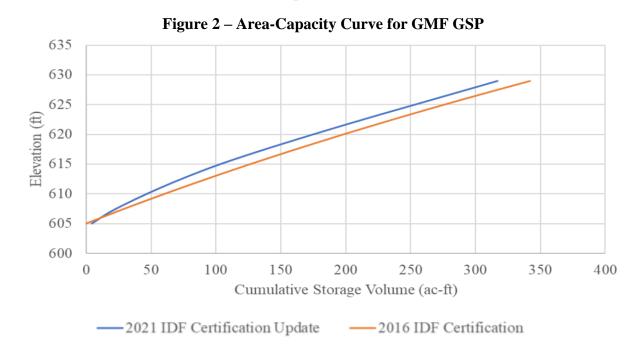


Figure 3 – Area-Capacity Curve for GMF RP

§ 257.73(c)(1)(x): A description of each spillway and diversion design features and capacities and calculations used in their determination.

The primary spillway structure for AP1 was modified in 2020 by constructing a berm of bottom ash around the entrance to the spillway, to reduce the potential for freezing around the spillway during post-CPP closure conditions, with a berm crest elevation of

approximately 630 ft. Design drawings for the bottom ash berm are not reasonably or readily available.

The transfer channel between the GMF GSP and the GMF RP was modified in 2020 by constructing a geomembrane-lined berm, in order to allow the normal pool level of the GMF GSP to be increased. Design drawings for the berm are not reasonably or readily available. However, survey data [3] indicates the berm has an elevation of approximately 628 ft, a top width (perpendicular to the flow direction) of approximately 75 ft, a total length (parallel to the flow direction) of 25 ft, and side slopes of approximately 4 horizontal to 1 vertical.

Valves were installed on the intake pipes for the GMF RP after the CPP was closed and plant process water intake pumping was ceased. Design drawings for these valves are not reasonably or readily available.

Updated discharge capacity calculations for the existing spillways of AP1, the GMF GSP, and the GMF RP were prepared in 2021 using HydroCAD 10 modeling software. The calculations indicate that the AP1 and the GMF RP have sufficient storage capacity and will not overtop the embankments during the 1,000-year, 24-hour, storm event. The calculations also indicate that the GMF GSP has sufficient storage capacity and will not overtop the embankments during the Probable Maximum Precipitation (PMP), 24-hour storm event. The results of the calculations are provided in **Table 2**.

Table 2 – Results of Updated Discharge Capacity Calculations

	AP1	GMF GSP	GMF RP
Approximate Berm Minimum Elevation ¹ , ft	636.0	632.0	629.0
Approximate Emergency Spillway Elevation ¹ , ft	Not Present	Not Present	624.0
Starting Water Surface Elevation ¹ (SWSE), ft	630.2	625.2	622.1
Peak Water Surface Elevation ¹ (PWSE), ft	631.4	626.7	623.9
Time to Peak, hr	No Discharge	10.6	No Discharge
Surface Area ² , ac	18.1	34.8	16.1
Storage ³ , ac-ft	19.5	52.9	29.0

Notes:

AP2 no longer retains free water as the CCR surface impoundments was closed in 2020 [6]. Therefore, the spillways are no longer present and the information regarding these structures, as presented in the Initial HoC report, is no longer applicable to AP2.

¹Elevations are based on the NAVD88 datum

²Surface area is defined as the water surface area at the PWSE

³Storage is defined as the volume between the SWSE and PWSE

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CLOSING

This letter has been prepared to document Geosyntec's evaluation of changes that have occurred at AP1, AP2, the GMF GSP, and the GMF RP since the Initial HoC was developed, based on reasonably and readily available information provided by IPRG, observed by Geosyntec during the site visit, or generated by Geosyntec as part of subsequent calculations.

Sincerely,

Lucas P. Carr, P.E.

2- 8/

Senior Engineer

John Seymour, P.E.

Senior Principal

REFERENCES

- [1] AECOM, "History of Construction, USEPA Final CCR Rule, 40 CFR § 257.73(c), Coffeen Power Station, Coffeen, Illinois," October 2016.
- [2] United Stated Environmental Protection Agency, "40 CFR Parts 257 and 261, Hazardous and Solid Waste Management System, Disposal of Coal Combustion Residuals from Electric Utilities, Final Rule, 2015," 2015.
- [3] Illinois Environmental Protection Agency, "35 Ill. Adm. Code Part 845, Standards for the Disposal of Coal Combustion Residuals in Surface Impoundments," Springfield, IL, 2021.
- [4] AECOM, "History of Construction, USEPA Final CCR Rule, 40 CFR § 257.73(c), Hennepin Power Station, Hennepin, Illinois," October 2016.
- [5] V. Modeer, "Closure Plan for Existing CCR Surface Impoundment, Coffeen Power Station, Illinois Power Generating Company, Ash Pond No. 2," October 17, 2016.
- [6] D. Tickner, "Coffeen Power Station; Ash Pond No. 2; Notification of Completion of Closure," December 17, 2020.

Periodic USEPA CCR Rule Certification Repor GMF Gypsum Stack Pond – Coffeen Power Plan October 11, 2021	t			
Attachment D				
Periodic Structural Stability and Safety Factor Assessment Analyses				

Coffeen Power Plant GMF Gypsum Stack Pond Station 13+50 Slope Stability - Static Drained Design by: Lucas Carr Checked by: Nick Sanna Modified by: Pourya Kargar Checked by: Zachary Fallert Date: 9/14/2021

Name: Embankment Fill Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion': 0 psf Phil': 31 ° Phi-B: 0 ° Piezometric Line: 1

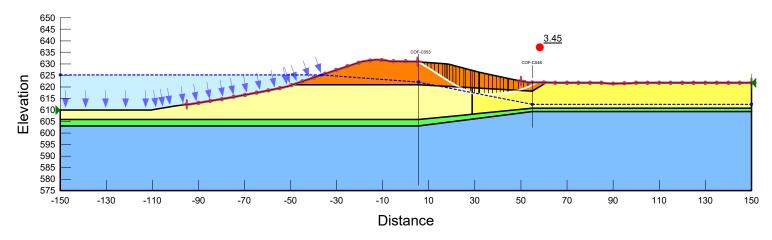
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Name: Soft Clay Foundation Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phil': 30 ° Phi-B: 0 ° Piezometric Line: 1

Name: Till Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion': 0 psf Phil': 40 ° Phi-B: 0 ° Piezometric Line: 1

Name: Foundation Clay (Below Embankment - CIU) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phil': 125 pcf Cohesion': 0 psf Phil': 32 ° Phi-B: 0 ° Piezometric Line: 1





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Coffeen Power Plant GMF Gypsum Stack Pond Station 13+50 Slope Stability - Static Drained Design by: Lucas Carr Checked by: Nick Sanna Modified by: Pourya Kargar Checked by: Zachary Fallert Date: 9/13/2021

Name: Embankment Fill Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion': 0 psf Phi': 31 ° Phi-B: 0 ° Piezometric Line: 1

Name: Foundation Clay (Free Field - DSS) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 30 ° Phi-B: 0 ° Piezometric Line: 1

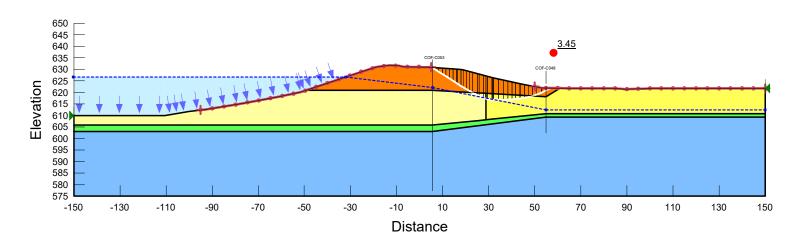
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Name: Till Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion': 0 psf Phi': 40 ° Phi-B: 0 ° Piezometric Line: 1

Name: Foundation Clay (Below Embankment - ClU) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 35 ° Phi-B: 0 ° Piezometric Line: 1

Name: Foundation Clay (Below Embankment - ClU) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 32 ° Phi-B: 0 ° Piezometric Line: 1





Dynegy Coffeen GMF Gypsum Stack Pond Station 13+50 Peak Undrained Soil Strengths Pseudostatic - Entry-Exit Design by: Lucas Carr Checked by: Nick Sanna Modified by: Pourya Kargar Checked by: Zachary Fallert Date: 9/14/2021

Name: Embankment Fill Model: S=f(overburden) Unit Weight: 135 pcf Vau/Sigma Ratio: 0.6 Minimum Strength: 450 psf Piezometric Line: 1

Name: Foundation Clay (Free Field - DSS) Model: S=f(overburden) Unit Weight: 125 pcf Vau/Sigma Ratio: 0.28 Minimum Strength: 450 psf Piezometric Line: 1

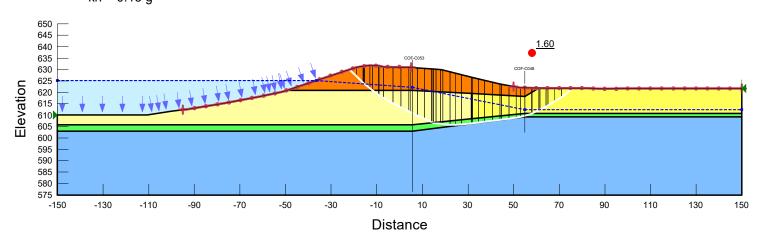
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Name: Till Model: S=f(overburden) Unit Weight: 135 pcf Vau/Sigma Ratio: 0.45 Minimum Strength: 700 psf Piezometric Line: 1

Name: Foundation Clay (Below Embankment - CIU) Model: S=f(overburden) Unit Weight: 125 pcf Vau/Sigma Ratio: 0.45 Minimum Strength: 20 psf Vau/Sigma Ratio: 0.28 Minimum Strength: 275 psf Vau/Sigma Ratio: 10 psf Vau/Sigma Ratio: 0.45 Minimum Strength: 700 psf Vau/Sigma Ratio: 0.45 Minimum Streng



Seismic Load: kh = 0.13 g



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Coffeen Power Plant GMF Gypsum Stack Pond Station 13+50 Slope Stability - Post Earthquake Design by: Lucas Carr Checked by: Nick Sanna Modified by: Pourya Kargar Checked by: Zachary Fallert Date: 9/14/2021

Name: Embankment Fill Model: S=f(overburden) Unit Weight: 135 pcf Tau/Sigma Ratio: 0.6 Minimum Strength: 450 psf Piezometric Line: 1

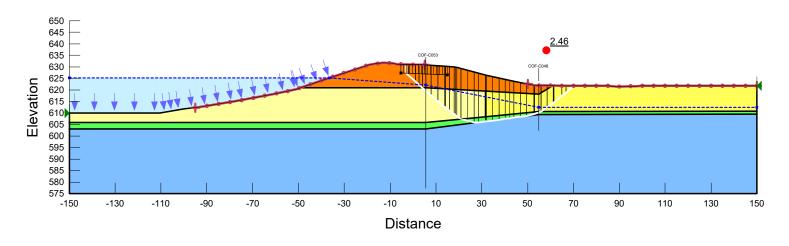
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Name: Till Model: S=f(overburden) Unit Weight: 135 pcf Tau/Sigma Ratio: 0.45 Minimum Strength: 700 psf Piezometric Line: 1

Name: Foundation Clay (Below Embankment - CIU) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.45 Minimum Strength: 700 psf Piezometric Line: 1

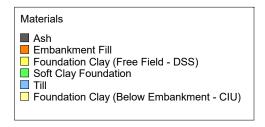


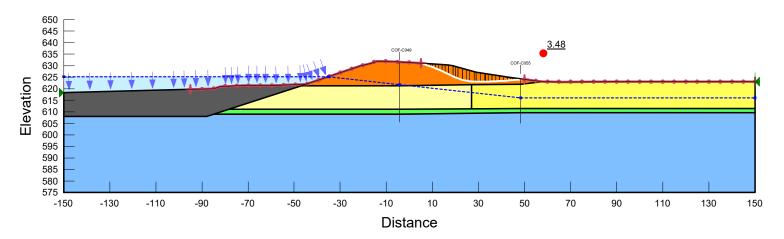


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Coffeen Power Plant GMF Gypsum Stack Pond Station 22+50 Slope Stability - Static Drained Design by: Lucas Carr Checked by: Nick Sanna Modified by: Pourya Kargar Checked by: Zachary Fallert Date: 9/14/2021

Name: Ash Model: Mohr-Coulomb Unit Weight: 112 pcf Cohesion': 0 psf Phi': 31 ° Phi: 31





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Coffeen Power Plant GMF Gypsum Stack Pond Station 22+50 Slope Stability - Static Drained

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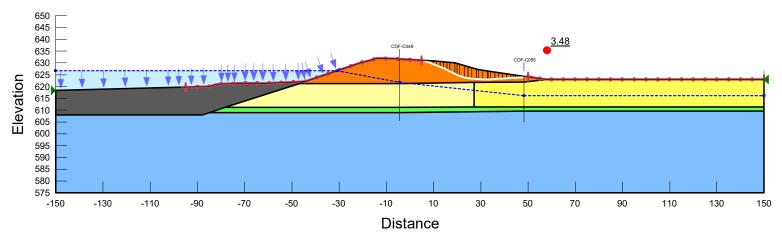
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Name: Till Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion': 0 psf Phi': 40 ° Phi-B: 0 ° Phi-B: 0 ° Piezometric Line: 1

Name: Foundation Clay (Below Embankment - ClU) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 40 ° Phi-B: 0 ° Phi-B:





Design by: Lucas Carr

Modidfied by: Betty Tesfu

Checked by: Nick Sanna

Date:

Modified by: Pourya Kargar Checked by: Zachary Fallert

9/13/2021

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Coffeen Power Plant GMF Gypsum Stack Pond Station 22+50 Peak Undrained Soil Strengths Pseudostatic - Entry & Exit Design by: Lucas Carr Checked by: Nick Sanna Modified by: Pourya Kargar Checked by: Zachary Fallert Date: 9/14/2021

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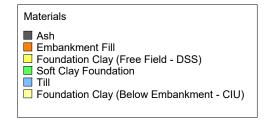
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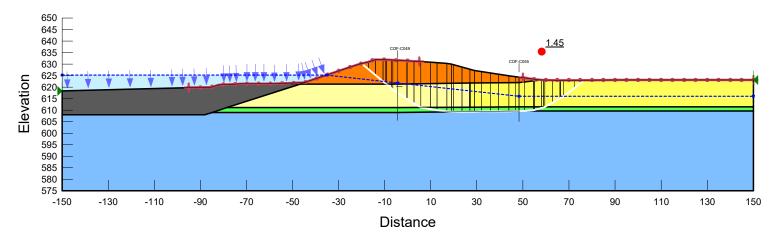
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Name: Till Model: S=f(overburden) Unit Weight: 135 pcf Tau/Sigma Ratio: 0.45 Minimum Strength: 700 psf Piezometric Line: 1

Name: Foundation Clay (Below Embankment - ClU) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.45 Minimum Strength: 125 pcf Tau/Sigma Ratio: 0.39 Minimum Strength: 700 psf Piezometric Line: 1



Seismic Load: kh = 0.13 g



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Coffeen Power PlantGMF Gypsum Stack Pond Station 22+50 Slope Stability - Post Earthquake

Design by: Lucas Carr Checked by: Nick Sanna Modified by: Pourya Kargar Checked by: Zachary Fallert Date: 9/14/2021

Name: Ash Model: S=f(overburden) Unit Weight: 112 pcf Tau/Sigma Ratio: 0.05 Minimum Strength: 0 psf Piezometric Line: 1

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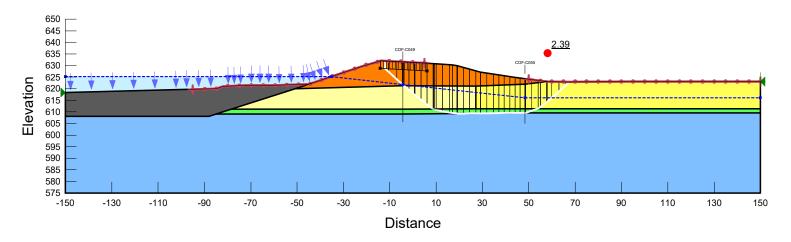
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Name: Till Model: S=f(overburden) Unit Weight: 135 pcf Tau/Sigma Ratio: 0.45 Minimum Strength: 700 psf Piezometric Line: 1

Name: Foundation Clay (Below Embankment - CIU) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.39 Minimum Strength: 700 psf Piezometric Line: 1





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Coffeen Power Plant GMF Gypsum Stack Pond Station 46+50 Static Drained - Entry & Exit Design by: Lucas Carr Checked by: Nick Sanna Modified by: Pourya Kargar Checked by: Zachary Fallert

Date: 9/8/2021

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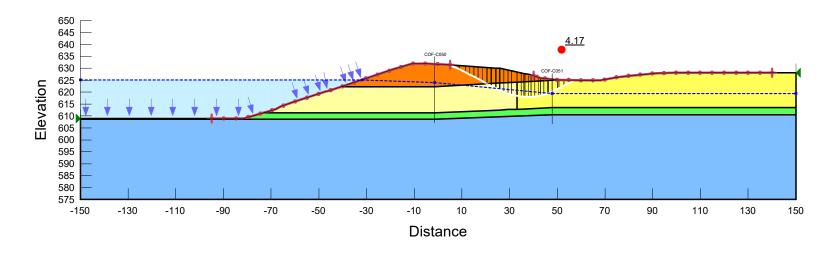
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Name: Soft Clay Foundation Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 30 ° Phi-B: 0 ° Piezometric Line: 1

Name: Till Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion': 0 psf Phil: 40 ° Phi-B: 0 ° Piezometric Line: 1

Name: Foundation Clay (Below Embankment - CIU) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 32 ° Phi-B: 0 ° Piezometric Line: 1





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Coffeen Power Plant GMF Gypsum Stack Pond Station 46+50 Surcharge Pool - Entry & Exit Design by: Lucas Carr Modidfied by: Betty Tesfu Checked by: Nick Sanna Modified by: Pourya Kargar Checked by: Zachary Fallert Date: 9/13/2021

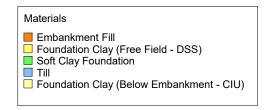
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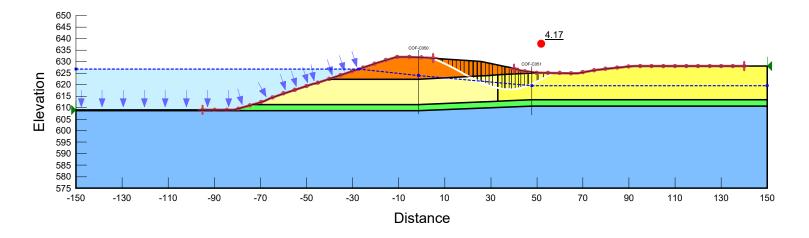
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Name: Till Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion': 0 psf Phi': 40 ° Phi-B: 0 ° Piezometric Line: 1

Name: Foundation Clay (Below Embankment - ClU) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 32 ° Phi-B: 0 ° Piezometric Line: 1





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Coffeen Power Plant GMF Gypsum Stack Pond Station 46+50 Peak Undrained Soil Strengths Name: Pseudostatic - Entry & Exit Design by: Lucas Carr Checked by: Nick Sanna Modified by: Pourya Kargar Checked by: Zachary Fallert Date: 9/14/2021

Name: Embankment Fill Model: S=f(overburden) Unit Weight: 135 pcf Tau/Sigma Ratio: 0.6 Minimum Strength: 450 psf Piezometric Line: 1

Name: Foundation Clay (Free Field - DSS) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.28 Minimum Strength: 450 psf Piezometric Line: 1

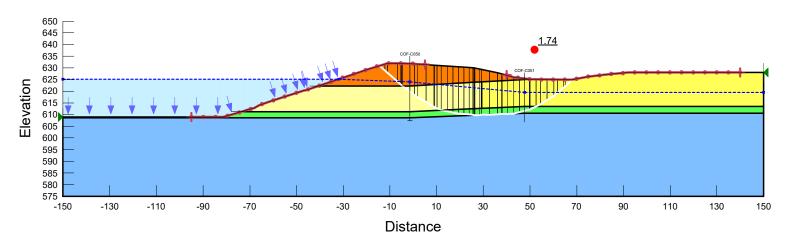
Name: Soft Clay Foundation Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.28 Minimum Strength: 275 psf Piezometric Line: 1

Name: Till Model: S=f(overburden) Unit Weight: 135 pcf Tau/Sigma Ratio: 0.45 Minimum Strength: 700 psf Piezometric Line: 1

Name: Foundation Clay (Below Embankment - ClU) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.45 Minimum Strength: 700 psf Piezometric Line: 1



Seismic Load kh = 0.13 g



\\STLOUISMO-01\Data\Company\Projects_post_2014\GLP8027_CCR_ReCert\500_Technical\502_COF\502d_Periodic_Report\GMF\Revised SFA\46+50\ Coffeen GMF 46+50 Peak Undrained LPC 20160902v1.gsz

Coffeen Power Plant GMF Gypsum Stack Pond Station 46+50 Slope Stability - Post Earthquake Design by: Lucas Carr Checked by: Nick Sanna Modified by: Pourya Kargar Checked by: Zachary Fallert Date: 9/14/2021

Name: Embankment Fill Model: S=f(overburden) Unit Weight: 135 pcf Tau/Sigma Ratio: 0.6 Minimum Strength: 450 psf Piezometric Line: 1

Name: Foundation Clay (Free Field - DSS) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.16 Minimum Strength: 200 psf Piezometric Line: 1

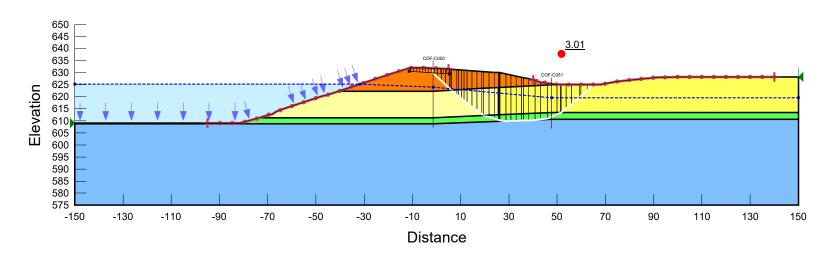
Name: Soft Clay Foundation Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.16 Minimum Strength: 200 psf Piezometric Line: 1

Name: Till Model: S=f(overburden) Unit Weight: 135 pcf Tau/Sigma Ratio: 0.45 Minimum Strength: 700 psf Piezometric Line: 1

Name: Foundation Clay (Below Embankment - CIU) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.45 Minimum Strength: 700 psf Piezometric Line: 1

Name: Foundation Clay (Below Embankment - CIU) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.45 Minimum Strength: 700 psf Piezometric Line: 1





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Coffeen Power Plant GMF Gypsum Stack Pond Station 58+00 Slope Stability - Static Drained Design by: Lucas Carr Checked by: Nick Sanna Modified by: Pourya Kargar Checked by: Zachary Fallert

Date: 9/8/2021

Name: Embankment Fill Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion': 0 psf Phi": 31 ° Phi-B: 0 ° Piezometric Line: 1

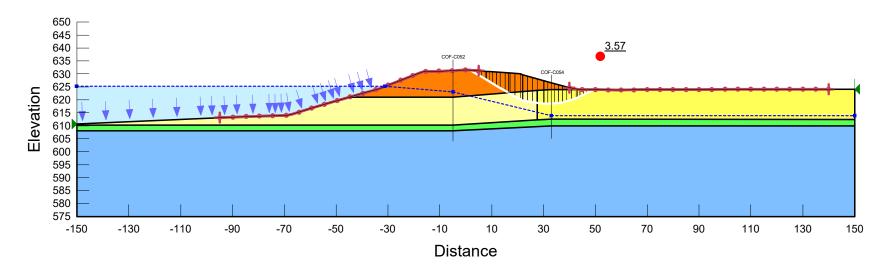
Name: Foundation Clay (Free Field - DSS) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 30 ° Phi-B: 0 ° Piezometric Line: 1

Name: Soft Clay Foundation Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 30 ° Phi-B: 0 ° Piezometric Line: 1

Name: Till Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion': 0 psf Phi': 40 ° Phi-B: 0 ° Piezometric Line: 1

Name: Foundation Clay (Below Embankment - CIU) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 32 ° Phi-B: 0 ° Piezometric Line: 1





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Coffeen Power Plant GMF Gypsum Stack Pond Station 58+00 Slope Stability - Static Drained

Design by: Lucas Carr Modidfied by: Betty Tesfu Checked by: Nick Sanna Modified by: Pourya Kargar Checked by: Zachary Fallert

Date: 9/13/2021

Name: Embankment Fill Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion': 0 psf Phi': 31 ° Phi-B: 0 ° Piezometric Line: 1

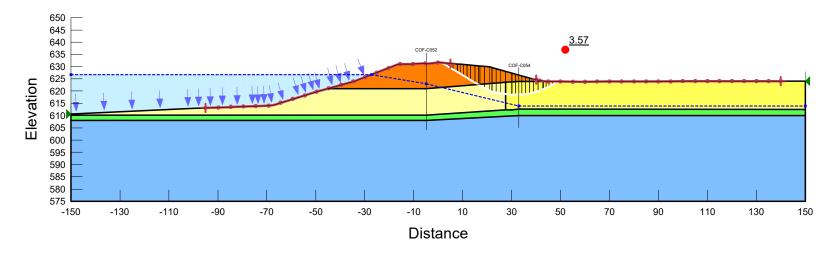
Name: Foundation Clay (Free Field - DSS) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 30 ° Phi-B: 0 ° Piezometric Line: 1

Name: Soft Clay Foundation Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 30 ° Phi-B: 0 ° Piezometric Line: 1

Name: Till Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion': 0 psf Phi': 40 ° Phi-B: 0 ° Piezometric Line: 1

Name: Foundation Clay (Below Embankment - CIU) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 32 ° Phi-B: 0 ° Piezometric Line: 1





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Coffeen Power Plant GMF Gypsum Stack Pond Station 58+00 Peak Undrained Soil Strengths Pseudostatic - Entry-Exit Design by: Lucas Carr Checked by: Nick Sanna Modified by: Pourya Kargar Checked by: Zachary Fallert Date: 9/14/2021

Name: Embankment Fill Model: S=f(overburden) Unit Weight: 135 pcf Tau/Sigma Ratio: 0.6 Minimum Strength: 450 psf Piezometric Line: 1

Name: Foundation Clay (Free Field - DSS) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.24 Minimum Strength: 450 psf Piezometric Line: 1

Name: Soft Clay Foundation Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.23 Minimum Strength: 275 psf Piezometric Line: 1

Name: Till Model: S=f(overburden) Unit Weight: 135 pcf Tau/Sigma Ratio: 0.45 Minimum Strength: 700 psf Piezometric Line: 1

Name: Foundation Clay (Below Embankment - ClU) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.39 Minimum Strength: 700 psf Piezometric Line: 1

Materials

Embankment Fill

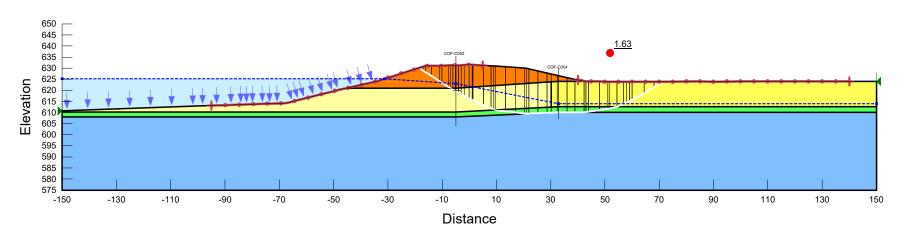
Foundation Clay (Free Field - DSS)

Soft Clay Foundation

Till

Foundation Clay (Below Embankment - CIU)

Seismic Load kh = 0.13 g



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Coffeen Power Plant GMF Gypsum Stack Pond Station 58+00 Slope Stability - Static Drained

Design by: Lucas Carr Modidfied by: Betty Tesfu Checked by: Nick Sanna Modified by: Pourya Kargar Checked by: Zachary Fallert Date: 9/13/2021

Name: Embankment Fill Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion': 0 psf Phi': 31 ° Phi-B: 0 ° Piezometric Line: 1

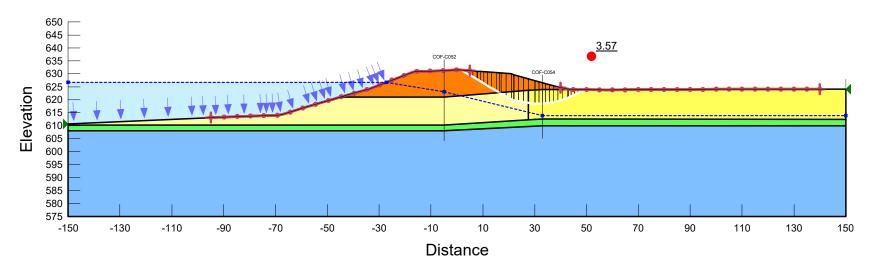
Name: Foundation Clay (Free Field - DSS) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 30 ° Phi-B: 0 ° Piezometric Line: 1

Name: Soft Clay Foundation Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 30 ° Phi-B: 0 ° Piezometric Line: 1

Name: Till Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion': 0 psf Phi': 40 ° Phi-B: 0 ° Piezometric Line: 1

Name: Foundation Clay (Below Embankment - CIU) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 32 ° Phi-B: 0 ° Piezometric Line: 1

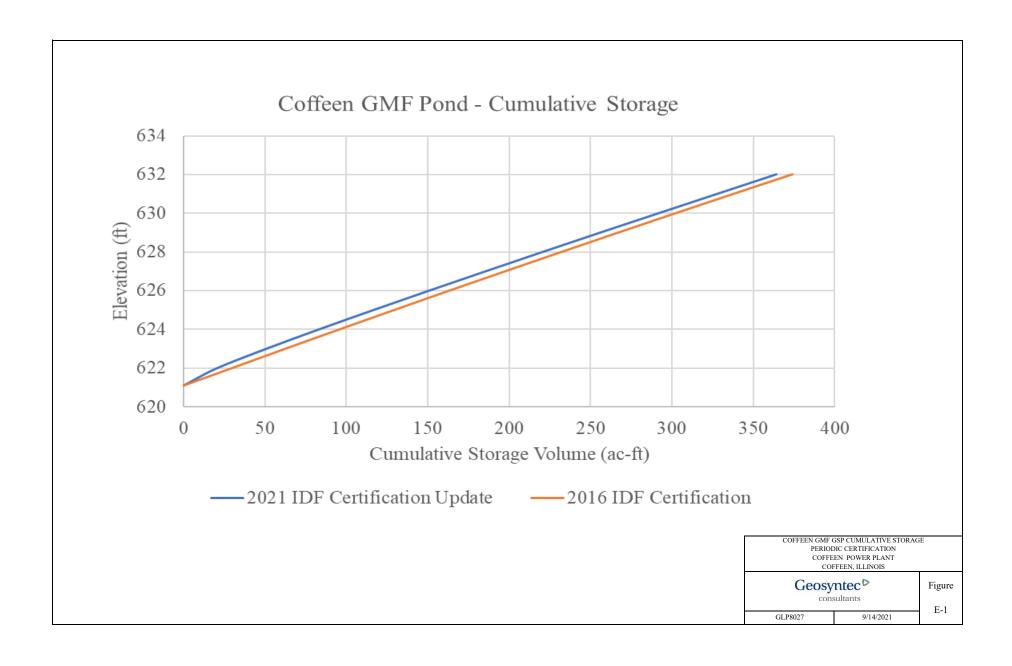


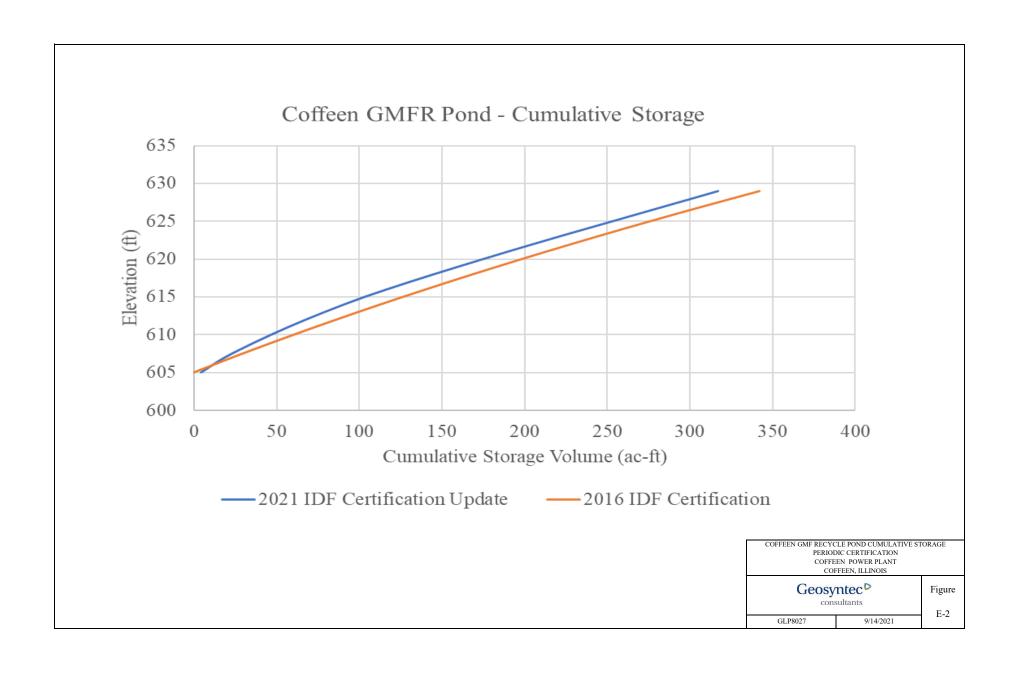


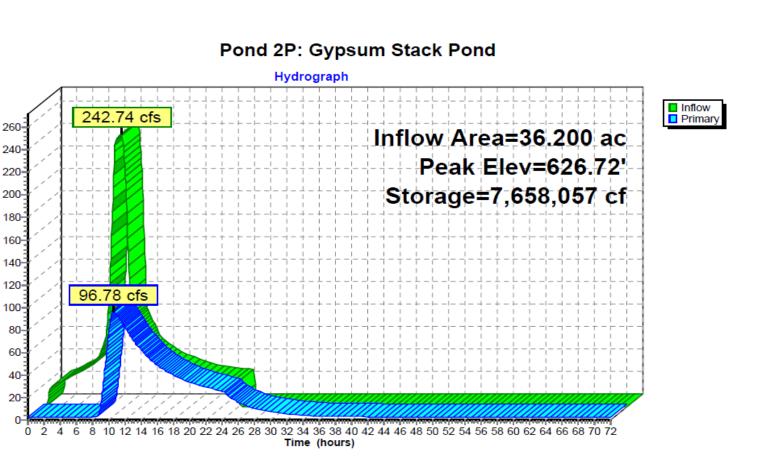
Periodic USEPA CCR Rule Certification Report GMF Gypsum Stack Pond – Coffeen Power Plant October 11, 2021

Attachment E

Periodic Inflow Design Flood Control System Plan Analyses

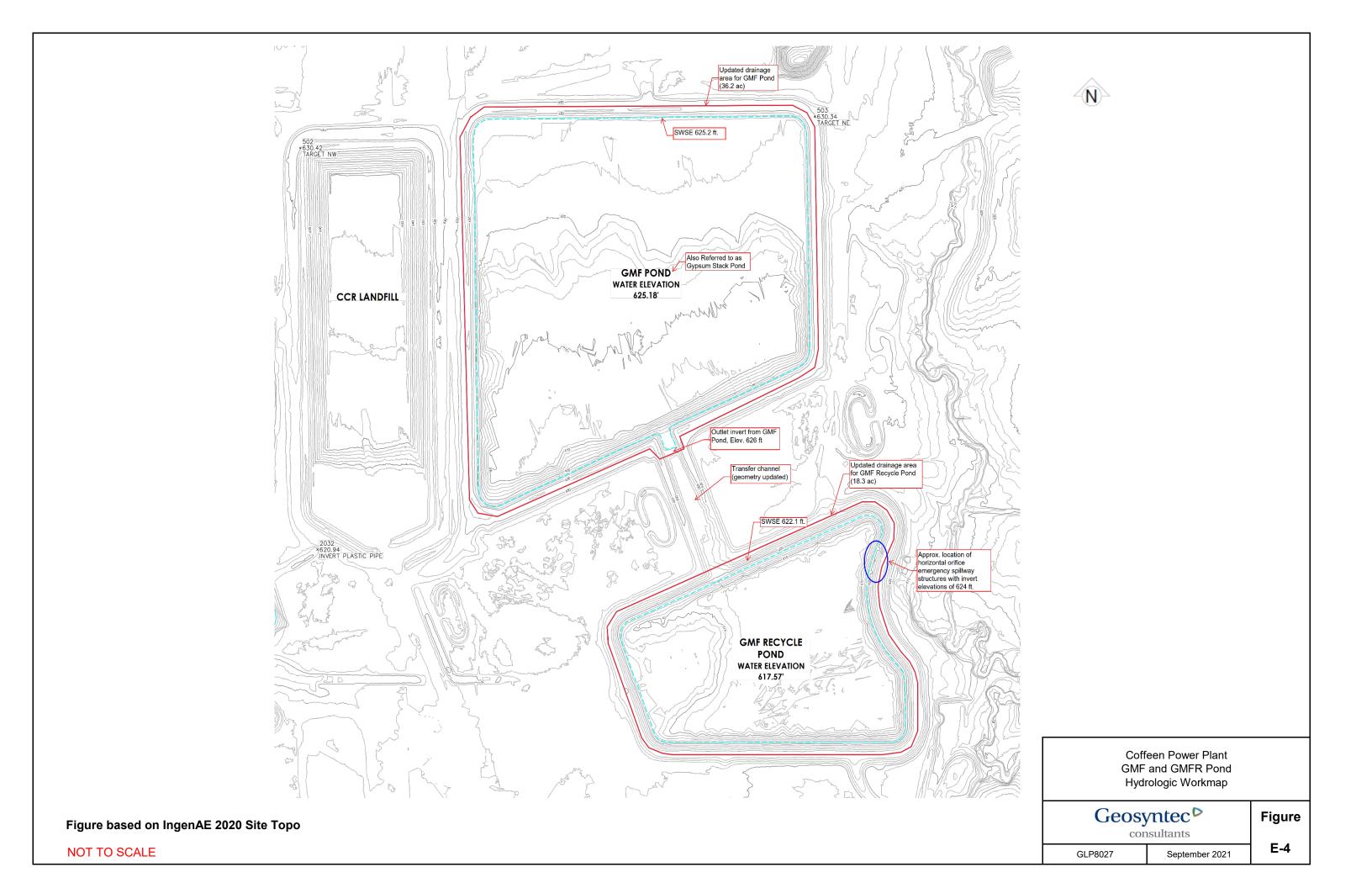


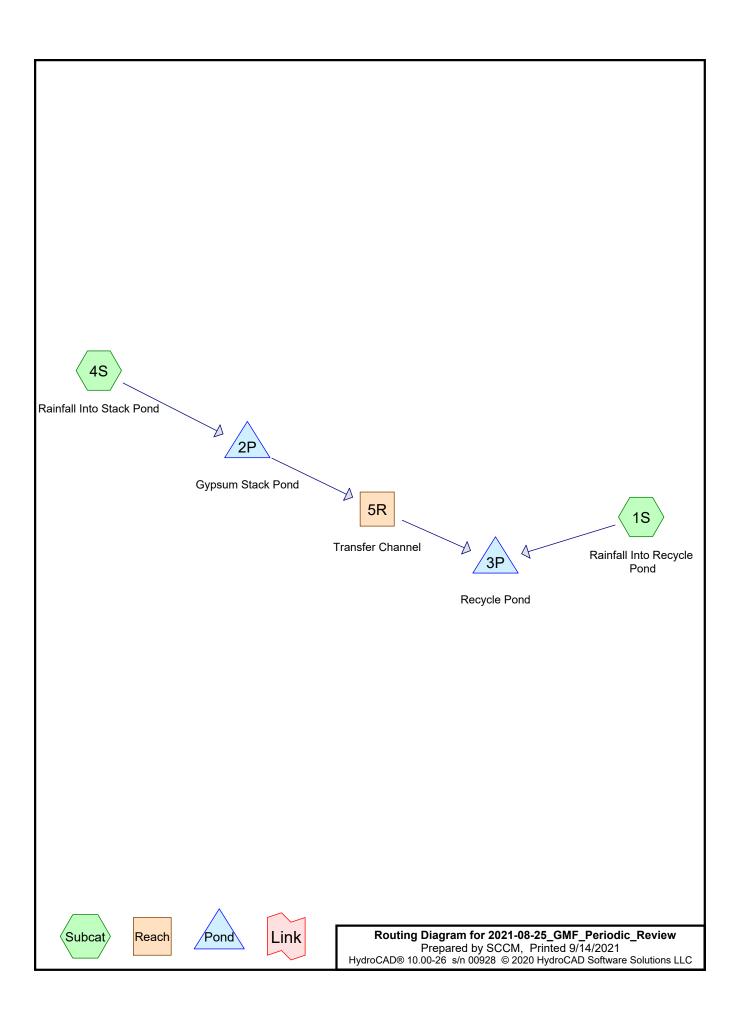




Flow (cfs)

GMF GS PERIOE COFFE COI		
Geosy	Figure	
COUR	E-3	
GLP8027	9/14/2021	LJ





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Area Listing (all nodes)

54.5	00	98	TOTAL AREA
54.5	00	98	Water Surface, HSG C (1S, 4S)
(acre	es)		(subcatchment-numbers)
Ar	ea	CN	Description

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Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
0.000	HSG B	
54.500	HSG C	1S, 4S
0.000	HSG D	
0.000	Other	
54.500		TOTAL AREA

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Ground Covers (all nodes)

HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
(acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover	Numbers
 0.000	0.000	54.500	0.000	0.000	54.500	Water Surface	1S, 4S
0.000	0.000	54.500	0.000	0.000	54.500	TOTAL AREA	

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Pipe Listing (all nodes)

Line#	Node	In-Invert	Out-Invert	Length	Slope	n	Diam/Width	Height	Inside-Fill
	Number	(feet)	(feet)	(feet)	(ft/ft)		(inches)	(inches)	(inches)
1	2P	619.00	617.60	580.0	0.0024	0.013	14.0	0.0	0.0
2	3P	615.00	613.00	92.0	0.0217	0.013	45.0	0.0	0.0
3	3P	615.00	613.00	92.0	0.0217	0.013	45.0	0.0	0.0
4	3P	615.00	613.00	92.0	0.0217	0.013	45.0	0.0	0.0

2021-08-25_GMF_P Spillway Emergency 24.00 hrs PMP - Emergency Spillway Rainfall=34.25" Prepared by SCCM Printed 9/14/2021

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Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: Rainfall Into Recycle Runoff Area=18.300 ac 100.00% Impervious Runoff Depth=34.01" Tc=6.0 min CN=98 Runoff=122.71 cfs 51.860 af

Subcatchment 4S: Rainfall Into Stack Runoff Area=36.200 ac 100.00% Impervious Runoff Depth=34.01"

Tc=6.0 min CN=98 Runoff=242.74 cfs 102.586 af

Reach 5R: Transfer ChannelAvg. Flow Depth=0.48' Max Vel=5.94 fps Inflow=96.78 cfs 82.274 af n=0.010 L=450.0' S=0.0044 '/' Capacity=7,454.18 cfs Outflow=96.77 cfs 82.255 af

Pond 2P: Gypsum Stack Pond Peak Elev=626.72' Storage=7,658,057 cf Inflow=242.74 cfs 102.586 af Outflow=96.78 cfs 82.274 af

Pond 3P: Recycle PondPeak Elev=624.70' Storage=10,813,783 cf Inflow=183.51 cfs 134.115 af Primary=38.24 cfs 34.428 af Secondary=38.24 cfs 34.428 af Outflow=114.71 cfs 103.284 af

Total Runoff Area = 54.500 ac Runoff Volume = 154.445 af Average Runoff Depth = 34.01" 0.00% Pervious = 0.000 ac 100.00% Impervious = 54.500 ac

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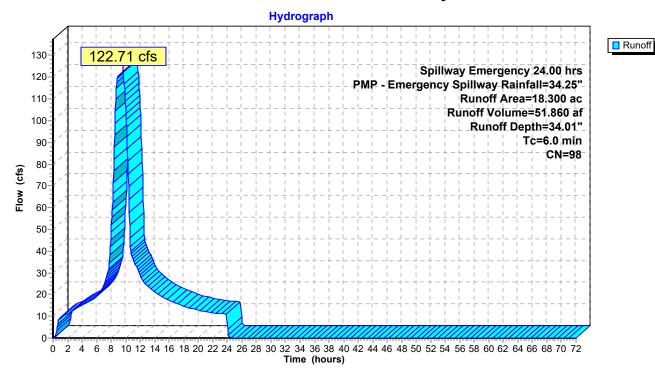
Summary for Subcatchment 1S: Rainfall Into Recycle Pond

Runoff = 122.71 cfs @ 9.62 hrs, Volume= 51.860 af, Depth=34.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Spillway Emergency 24.00 hrs PMP - Emergency Spillway Rainfall=34.25"

	Area	(ac)	CN	Desc	cription		
*	18.	300	98	Wate	er Surface,	HSG C	
	18.	300		100.	00% Impe	rvious Area	
	Тс	Lengt	:h	Slope	Velocity	Capacity	Description
	(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
	6.0						Direct Entry, Direct Fall

Subcatchment 1S: Rainfall Into Recycle Pond



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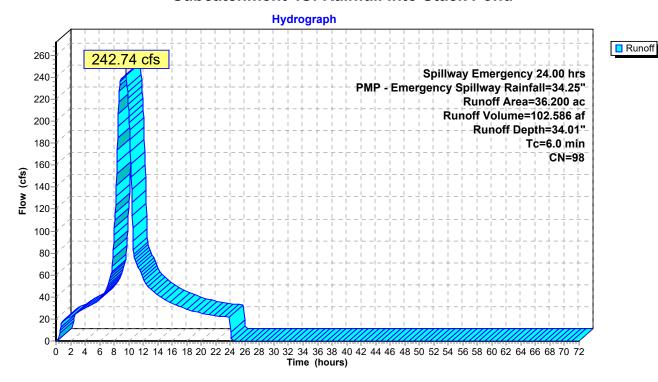
Summary for Subcatchment 4S: Rainfall Into Stack Pond

Runoff = 242.74 cfs @ 9.62 hrs, Volume= 102.586 af, Depth=34.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Spillway Emergency 24.00 hrs PMP - Emergency Spillway Rainfall=34.25"

_	Area	(ac)	CN	Desc	cription			
	36.	200	0 98 Water Surface, HSG C					
	36.200 100.00% Impervious Area				00% Impe	rvious Area		
	Tc (min)	Lengi (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	6.0						Direct Entry, Direct Fall	

Subcatchment 4S: Rainfall Into Stack Pond



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Summary for Reach 5R: Transfer Channel

Inflow Area = 36.200 ac,100.00% Impervious, Inflow Depth > 27.27" for PMP - Emergency Spillway event

Inflow = 96.78 cfs @ 10.58 hrs, Volume= 82.274 af

Outflow = 96.77 cfs (a) 10.59 hrs, Volume= 82.255 af, Atten= 0%, Lag= 0.8 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Max. Velocity= 5.94 fps, Min. Travel Time= 1.3 min Avg. Velocity = 2.28 fps, Avg. Travel Time= 3.3 min

Peak Storage= 7,334 cf @ 10.59 hrs Average Depth at Peak Storage= 0.48'

Bank-Full Depth= 6.00' Flow Area= 279.0 sf, Capacity= 7,454.18 cfs

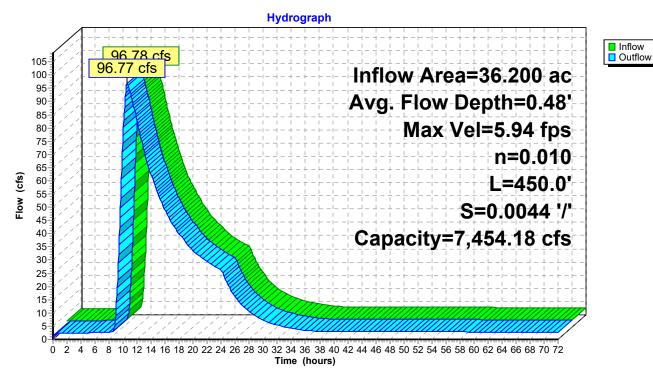
32.70' x 6.00' deep channel, n= 0.010 PVC, smooth interior Side Slope Z-value= $3.0 \, 1.6 \, \text{l}'$ Top Width= 60.30'

Length= 450.0' Slope= 0.0044 '/'

Inlet Invert= 624.00', Outlet Invert= 622.00'



Reach 5R: Transfer Channel



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Summary for Pond 2P: Gypsum Stack Pond

[44] Hint: Outlet device #2 is below defined storage

Inflow Area = 36.200 ac,100.00% Impervious, Inflow Depth = 34.01" for PMP - Emergency Spillway event

Inflow = 242.74 cfs @ 9.62 hrs, Volume= 102.586 af

Outflow = 96.78 cfs @ 10.58 hrs, Volume= 82.274 af, Atten= 60%, Lag= 57.2 min

Primary = 96.78 cfs @ 10.58 hrs, Volume= 82.274 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Starting Elev= 625.18' Surf.Area= 0 sf Storage= 5,353,910 cf

Peak Elev= 626.72' @ 10.58 hrs Surf.Area= 0 sf Storage= 7,658,057 cf (2,304,147 cf above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= 554.6 min (1,215.4 - 660.8)

Volume	Invert	Avail.Storage	Storage Description
#1	621.10'	15,871,813 cf	Custom Stage DataListed below

Elevation (feet)	Cum.Store (cubic-feet)
621.10	0
622.00	898,355
623.00	2,215,071
624.00	3,622,761
625.00	5,085,824
626.00	6,575,189
627.00	8,086,603
628.00	9,615,334
629.00	11,161,695
630.00	12,725,625
631.00	14,298,658
632.00	15,871,813

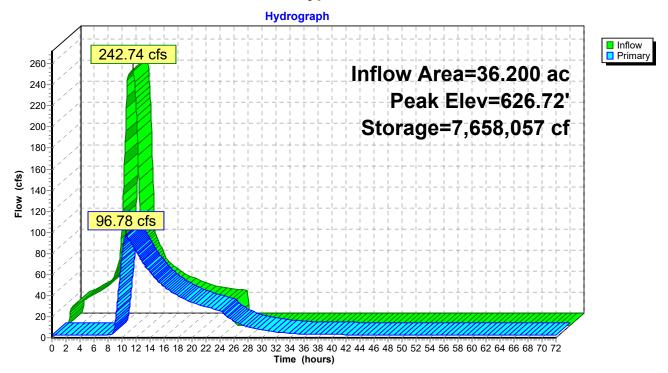
Device	Routing	Invert	Outlet Devices
#1	Primary	626.00'	Custom Weir/Orifice, Cv= 2.62 (C= 3.28)
	•		Head (feet) 0.00 2.00 4.00
			Width (feet) 45.00 60.00 75.00
#2	Primary	619.00'	14.0" Round Culvert
			L= 580.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 619.00' / 617.60' S= 0.0024 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.07 sf

Primary OutFlow Max=96.75 cfs @ 10.58 hrs HW=626.72' TW=624.48' (Dynamic Tailwater)

1=Custom Weir/Orifice (Weir Controls 93.61 cfs @ 2.74 fps)

—2=Culvert (Outlet Controls 3.14 cfs @ 2.93 fps)

Pond 2P: Gypsum Stack Pond



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Summary for Pond 3P: Recycle Pond

[63] Warning: Exceeded Reach 5R INLET depth by 0.26' @ 12.35 hrs

Inflow Area = 54.500 ac,100.00% Impervious, Inflow Depth > 29.53" for PMP - Emergency Spillway event Inflow 183.51 cfs @ 10.06 hrs, Volume= 134.115 af 103.284 af, Atten= 37%, Lag= 105.4 min Outflow = 114.71 cfs @ 11.82 hrs, Volume= 38.24 cfs @ 11.82 hrs, Volume= Primary 34.428 af 38.24 cfs @ 11.82 hrs, Volume= 38.24 cfs @ 11.82 hrs, Volume= Secondary = 34.428 af Tertiary 34.428 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Starting Elev= 622.10' Surf.Area= 0 sf Storage= 9,024,347 cf

Peak Elev= 624.70' @ 11.82 hrs Surf.Area= 0 sf Storage= 10,813,783 cf (1,789,436 cf above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= 234.9 min (1,236.7 - 1,001.8)

Volume Invert Avail.Storage Storage Description

#1	604.90'	13,809,827 cf	Custom Stage DataListed below
Elevation	Cum.		
(feet)	(cubic-	<u>-feet)</u>	
604.90		0	
605.00	193	3,406	
607.00	824	4,155	
609.00	1,613	3,462	
611.00	2,487	7,712	
613.00	3,446	3,903	
615.00	4,502	2,797	
617.00	5,698	3,519	
619.00	,	6,115	
621.00	8,279	9,014	
623.00	,	4,165	
624.00	10,326	•	
625.00	11,023	,	
626.00	11,719	·	
627.00	12,416	•	
628.00	13,112	,	
629.00	13,809	9,827	

Device	Routing	Invert	Outlet Devices
#1	Primary	615.00'	45.0" Round Culvert
			L= 92.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 615.00' / 613.00' S= 0.0217 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 11.04 sf
#2	Secondary	615.00'	45.0" Round Culvert
			L= 92.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 615.00' / 613.00' S= 0.0217 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 11.04 sf
#3	Tertiary	615.00'	45.0" Round Culvert
	,		L= 92.0' CPP, square edge headwall, Ke= 0.500

2021-08-25_GMF_P Spillway Emergency 24.00 hrs PMP - Emergency Spillway Rainfall=34.25" Prepared by SCCM Printed 9/14/2021

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			Inlet / Outlet Invert= 615.00' / 613.00' S= 0.0217 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 11.04 sf
#4	Device 1	624.00'	60.0" x 60.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#5	Device 2	624.00'	60.0" x 60.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#6	Device 3	624.00'	60.0" x 60.0" Horiz. Orifice/Grate C= 0.600
			I imited to weir flow at low heads

Primary OutFlow Max=38.23 cfs @ 11.82 hrs HW=624.70' (Free Discharge)
1=Culvert (Passes 38.23 cfs of 148.75 cfs potential flow)
4=Orifice/Grate (Weir Controls 38.23 cfs @ 2.73 fps)

Secondary OutFlow Max=38.23 cfs @ 11.82 hrs HW=624.70' (Free Discharge)
2=Culvert (Passes 38.23 cfs of 148.75 cfs potential flow)
5=Orifice/Grate (Weir Controls 38.23 cfs @ 2.73 fps)

Tertiary OutFlow Max=38.23 cfs @ 11.82 hrs HW=624.70' (Free Discharge)

3=Culvert (Passes 38.23 cfs of 148.75 cfs potential flow)

6=Orifice/Grate (Weir Controls 38.23 cfs @ 2.73 fps)

Pond 3P: Recycle Pond

